

Civil Engineers Structural Engineers Traffic Engineers Land Surveyors Landscape Architects Scientists



September 20, 2022

Beverly Mesa-Zendt, Planning Director City of Portsmouth Planning Department 1 Junkins Avenue, 3rd Floor Portsmouth, NH 03801

via email: View Point

RE: LU-22-61 – Response to TAC Comments 77 Meredith Way – Randi & Jeff Collins – Tax Map 162 Lot 16 TFM Project #47442-00

Dear Ms. Mesa-Zendt:

On behalf of our clients, Randi & Jeff Collins, TFMoran, Inc. (TFM) respectfully submits the following letter in response to the comments made by the City of Portsmouth Technical Advisory Committee (TAC), via email dated September 6, 2022. The following materials are included in this revised submission:

- Drainage Analysis Summary;
- Site Development Plan set entitled "Proposed 2 Lot Subdivision Plan, 77 Meredith Way, Portsmouth, New Hampshire", prepared by TFMoran, Inc., dated July 1, 2022, revised September 20, 2022 (1 copy at 22"x34).

To facilitate your review, we have provided your comments along with our responses, which are shown in *bold italics*.

TAC REVIEW COMMENTS:

September 6, 2022 Comments

1. The existing sewer line for #77 is to be cut and capped at the main in Pine St.

Revised, see inset on Sheet C-02.

2. The existing water line is to be cut and capped at the main in Meredith Way, the rest of the service should be abandoned, not removed.

Revised, see label on Sheet C-02.

3. The driveway to lot 'B' should be at least a foot off of the side property line.

Revised, see Sheet C-03.





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4. The roadway needs to be extended at least to the center of the garage proposed for Lot 'B' as previously stated. As shown that road is not long enough for our truck to actually turn around as the truck is almost 28' long.

Revised, see Sheet C-03.

5. The patios in the rear yards are to be porous so they do not encourage runoff.

Revised, see Sheet C-03 & detail on Sheet C-08.

6. Provide a test pit for Lot 'A' rain garden.

Test Pit #4 has been added, see Sheet C-04.

7. Both houses are to have gutters or French drains that divert the rain water into the associated rain gardens.

Added Note 10 on Sheet C-04.

8. Please remove the soil legend. This lot does not have hydrologic soil type 'A', it's more like a 'C'.

Soil legend removed, see Sheet C-04.

9. Use 2 -22 degree bends spaced 2' apart for the sewer lateral corners, not one 45 degree bend.

Revised, see Sheet C-05.

10. Sewer cleanouts will not be allowed in the City park.

Revised, see Sheet C-05.

11. Follow State standards for water service bury depth. Detail says insulation 'optional'. It may not be.

Detail updated, see Sheet C-07.

12. Warning tape does not need to be the metallic type. Specify sand for 6" over the pipe, crushed stone pipe bed to the haunch line

Detail updated, see Sheet C-07.

13. Specify that the road gets the heavy duty pavement section. 18" of 304.4 is acceptable for under the pavement.



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Detail updated, see Sheet C-07.

14. Specify a riser in each rain garden so that water can get down through the frozen media in winter to the stone below.

Notation added, see Sheet C-04.

15. Provide calculations for the rain garden footprint sizing based on the perc rate of the existing soil. Provide enough open voids in the stone above the ESHWT for at least 1" of rain. Make sure to specify removal of any clay or restrictive soils down to the bottom of the pond.

Enough room is provided within stone for 1" rain, see Sheet C-04. 1" rain event printouts are in the provided Drainage Summary.

16. Recommendation for approval will be contingent on successfully acquiring permission from the City Council for the sewer laterals.

Noted.

We trust that the above responses satisfy the concerns expressed in the City of Portsmouth's TAC comments. Should you wish to further discuss any of the above please contact us so that we may meet and resolve any outstanding concerns.

Respectfully, **TFMoran, Inc.**

Bund Kollow

Brenda Kolbow, LLS *Survey Department Manager*

BMK/bmk

cc: Randi & Jeff Collins Christopher Mulligan, Esquire September 20, 2022

GENERAL INFORMATION

OWNER MAP 162 LOT 16 RANDI & JEFF COLLINS 77 MEREDITH WAY PORTSMOUTH, NH 03801 774-278-8676

APPLICANT RANDI & JEFF COLLINS 77 MEREDITH WAY PORTSMOUTH, NH 03801 774-278-8676

RESOURCE LIST

PLANNING/ZONING DEPARTMENT I JUNKINS AVÉNUE PORTSMOUTH, NH 03801 603-610-7216 NICK CRACKNELL, PRINCIPAL PLANNER

PUBLIC WORKS 600 PEVERLY HILL ROAD PORTSMOUTH, STATE 03801 603-472-1530 DAVE DEFOSSES, CONSTRUCTION TECHNICAL SUPERVISOR

POLICE DEPARTMENT 3 JUNKINS AVENUE PORTSMOUTH, NH 03801 603-427-1510

FIRE DEPARTMENT 170 COURT STREET PORTSMOUTH, NH 03801 603-427-1515

ASSOCIATED PROFESSIONALS

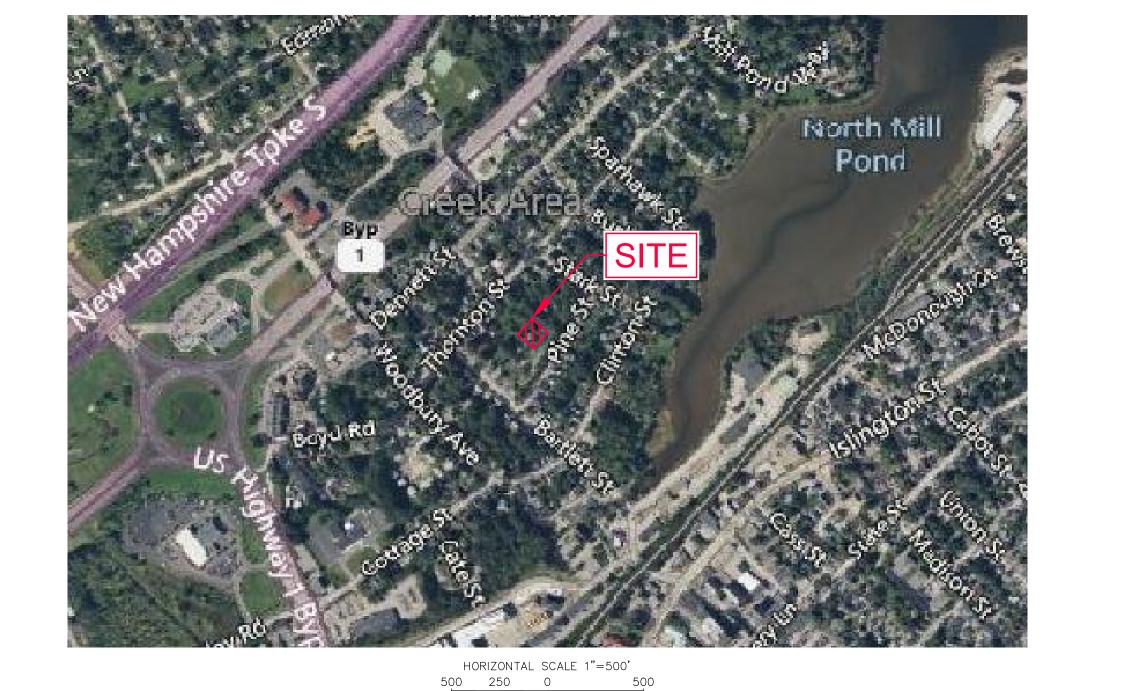
ATTORNEY BOSEN & ASSOCIATES 266 MIDDLE STREET PORTSMOUTH, NH 03801 603-427-5500 CHRISTOPHER P. MULLIGAN, ESQUIRE

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This plan is not effective unless signed by a duly authorized officer of FMoran, Inc.





PROPOSED 2 LOT SUBDIVISION

77 MEREDITH WAY **PORTSMOUTH, NEW HAMPSHIRE**

JULY 1, 2022 LAST REVISED SEPTEMBER 20, 2022

VICINITY PLAN

THESE PLANS ARE PERMIT DRAWINGS ONLY AND HAVE NOT BEEN DETAILED FOR CONSTRUCTION OR BIDDING.

4	9/20/2022	REVISED PER TAC COMMEN
3	8/31/2022	REVISED PER TAC COMMEN
2	8/23/2022	REVISED PER TAC COMMEN
1	7/21/2022	REVISED PER TAC COMMEN
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	INDEX OF SHEETS
SHEET	SHEET TITLE
C-00	COVER
C-01	NOTES & LEGEND
S-01	EXISTING CONDITIONS PLAN
S-02	SUBDIVISION PLAN
C-02	SITE PREPARATION & DEMOLITION PLAN
C-03	SITE LAYOUT PLAN
C-04	GRADING & DRAINAGE PLAN
C-05	UTILITY PLAN
C-06	ROAD PLAN AND PROFILE
C-07 THRU C-09	DETAILS

PERMITS/AI	DDDOVALC

NUMBER APPROVED EXPIRES

CITY PLANNING BOARD SUBDIVISION APPROVAL CITY ZONING BOARD VARIANCE REQUEST (ARTICLE 5 - SECTION 10.521)

LU-22-61 2022/06/22

2024/06/22

VARIANCE GRANTED

ON JUNE 22, 2022 THE CITY OF PORTSMOUTH ZONING BOARD OF ADJUSTMENT GRANTED RELIEF FROM THE FOLLOWING SECTION OF THE CITY OF PORTSMOUTH ZONING ORDINANCE:

ARTICLE 5 SECTION 10.521 - MINIMUM CONTINUOUS LOT FRONTAGE: TO ALLOW THE CONTINUOUS STREET FRONTAGE TO BE 73.99' FOR PROPOSED LOT A & 31.61' FOR PROPOSED LOT B, WHERE 100' IS REQUIRED AND 31.7' EXISTS.

OWNER'S SIGNATURE

THE PROPERTY WILL BE DEVELOPED IN ACCORDANCE WITH THIS PLAN AND THE ORDINANCES OF THE CITY OF PORTSMOUTH, NEW HAMPSHIRE.

OWNER OR AUTHORIZED AGENT

DA TE

AND

APPROVED BY THE CITY OF PORTSMOUTH PLANNING BOARD

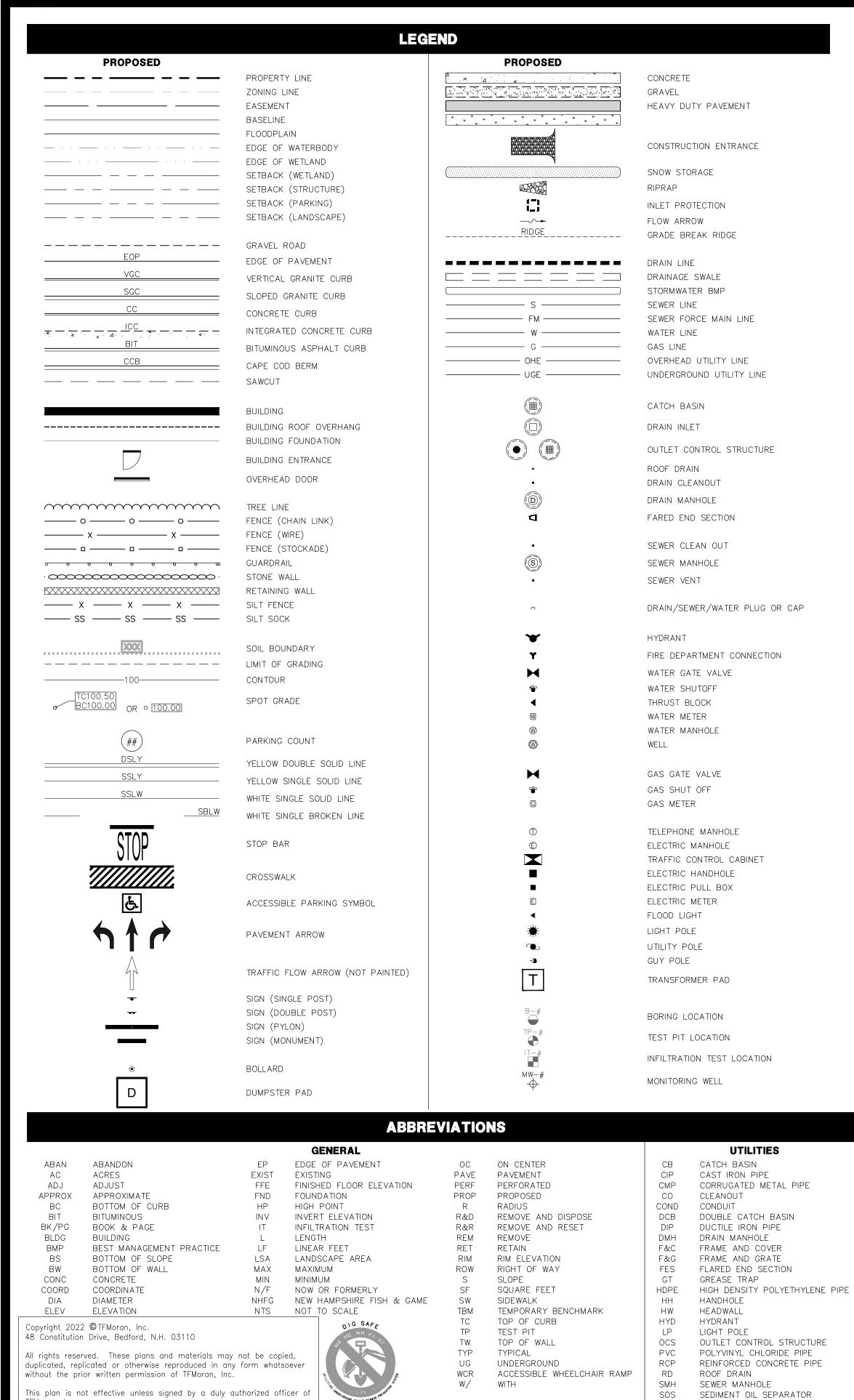
ON

BOARD MEMBER BOARD MEMBER



SITE DEVELOPMENT PLANS TAX MAP 162 LOT 16 COVER PROPOSED 2 LOT SUBDIVISION 77 MEREDITH WAY OWNED BY **RANDI & JEFF COLLINS** PREPARED FOR **RANDI & JEFF COLLINS** JULY 1, 2022 SCALE: AS SHOWN Seacoast Division | 170 Commerce Way, Suite 102 Civil Engineers Structural Engineers Portsmouth, NH 03801 Traffic Engineers Phone (603) 431-2222 _and Surveyors Fax (603) 431-0910 JKC JCC Landscape Architects www.tfmoran.com cientists JKC JCC 47442-00 JKC JCC DR CK C - 00

47442-00_COVER



CONTACT DIG SAFE 72 BUSINESS HOURS PRIOR TO CONSTRUCTION

FMoran, Inc.

GENERAL NOTES

- 1. THESE PLANS ARE PERMIT DRAWINGS ONLY AND HAVE NOT BEEN DETAILED FOR CONSTRUCTION OR BIDDING.
- 2. THESE PLANS WERE PREPARED UNDER THE SUPERVISION OF A LICENSED PROFESSIONAL ENGINEER. TFMORAN, INC. ASSUMES NO LIABILITY AS A RESULT OF ANY CHANGES OR NON-CONFORMANCE WITH THESE PLANS EXCEPT UPON THE WRITTEN APPROVAL OF THE ENGINEER OF RECORD
- 3. THE SITE LAYOUT PLAN SHALL BE RECORDED IN THE ROCKINGHAM COUNTY REGISTRY OF DEEDS.
- 4. ALL IMPROVEMENTS SHOWN ON THE SITE PLAN SHALL BE CONSTRUCTED AND MAINTAINED IN ACCORDANCE WITH THE PLAN BY THE PROPERTY OWNER AND ALL FUTURE PROPERTY OWNERS. NO CHANGES SHALL BE MADE TO THIS SITE PLAN WITHOUT THE EXPRESS APPROVAL OF THE CITY PLANNING BOARD.
- 5. ALL WORK SHALL CONFORM TO THE APPLICABLE REGULATIONS AND STANDARDS OF THE CITY OF PORTSMOUTH, AND SHALL BE BUILT IN A WORKMANLIKE MANNER IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS, ALL WORK TO CONFORM TO CITY OF PORTSMOUTH DEPARTMENT OF PUBLIC WORKS STANDARD SPECIFICATIONS. ALL WORK WITHIN THE RIGHT-OF-WAY OF THE CITY AND/OR STATE SHALL COMPLY WITH APPLICABLE STANDARDS. COORDINATE ALL WORK WITHIN THE RIGHT-OF-WAY WITH APPROPRIATE CITY, COUNTY, AND/OR STATE AGENCY.
- 6. THE SITE CONTRACTOR SHALL ENSURE THAT ALL WORK IS PERFORMED IN ACCORDANCE WITH APPLICABLE SECTIONS OF ENV-WQ 1500. THE SITE CONTRACTOR SHALL NOTIFY THE ENGINEER IN ADVANCE OF CONSTRUCTION OF EACH STORMWATER FACILITY TO COORDINATE REQUIRED INSPECTIONS. THE CONTRACTOR SHALL TAKE PROGRESS PHOTOS DURING CONSTRUCTION OF ALL STORMWATER DRAINAGE COMPONENTS AND SEND TO THE ENGINEER.
- 7. SEE EXISTING CONDITIONS PLAN FOR THE HORIZONTAL AND VERTICAL DATUM.
- 8. SEE EXISTING CONDITIONS PLAN FOR BENCHMARK INFORMATION. VERIFY TBM ELEVATIONS PRIOR TO CONSTRUCTION.
- 9. CONTACT EASEMENT OWNERS PRIOR TO COMMENCING ANY WORK WITHIN THE EASEMENTS.
- 10. PRIOR TO COMMENCING ANY SITE WORK, ALL LIMITS OF WORK SHALL BE CLEARLY MARKED IN THE FIELD.
- 11. SITE WORK SHALL BE CONSTRUCTED FROM A COMPLETE SET OF PLANS, NOT ALL FEATURES ARE DETAILED ON EVERY PLAN. THE ENGINEER IS TO BE NOTIFIED OF ANY CONFLICT WITHIN THIS PLAN SET.
- 12. TFMORAN, INC. ASSUMES NO LIABILITY FOR WORK PERFORMED WITHOUT AN ACCEPTABLE PROGRAM OF TESTING AND INSPECTION AS APPROVED BY THE ENGINEER OF RECORD.
- 13. TEMPORARY FENCING SHALL BE PROVIDED AND COVERED WITH A FABRIC MATERIAL TO CONTROL DUST MITIGATION.
- 14. ALL DEMOLITION SHALL INSURE MINIMUM INTERFERENCE WITH ROADS, STREETS, WALKWAYS, AND ANY OTHER ADJACENT OPERATING FACILITIES. PRIOR WRITTEN PERMISSION FROM THE OWNER/DEVELOPER AND LOCAL PERMITTING AUTHORITY IS REQUIRED IF CLOSURE/OBSTRUCTIONS TO ROADS, STREET, WALKWAYS, AND OTHERS IS DEEMED NECESSARY. CONTRACTOR TO PROVIDE ALTERNATE ROUTES AROUND CLOSURES/OBSTRUCTIONS PER LOCAL/STATE/FEDERAL REGULATIONS.
- 15. REFER TO ARCHITECTURAL PLANS FOR LAYOUT OF BUILDING FOUNDATIONS AND CONCRETE ELEMENTS WHICH ABUT THE BUILDING SUCH AS STAIRS, SIDEWALKS, LOADING DOCK RAMPS, PADS, AND COMPACTOR PADS. DO NOT USE SITE PLANS FOR LAYOUT OF FOUNDATIONS.
- 16. IN THE EVENT OF A CONFLICT BETWEEN PLANS, SPECIFICATIONS, AND DETAILS, THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY FOR CLARIFICATION.
- 17. IF CONDITIONS AT THE SITE ARE DIFFERENT THAN SHOWN ON THE PLANS, THE ENGINEER SHALL BE NOTIFIED PRIOR TO PROCEEDING WITH THE AFFECTED WORK.
- 18. CONTRACTOR'S GENERAL RESPONSIBILITIES:
- A. BID AND PERFORM THE WORK IN ACCORDANCE WITH ALL LOCAL, STATE, AND NATIONAL CODES, SPECIFICATIONS, REGULATIONS, AND STANDARDS AND CONDITIONS OF ALL PROJECT-SPECIFIC PERMITS AND APPROVALS AS LISTED ON THE COVER SHEET TO THESE PLANS OR OTHERWISE REQUIRED.
- B. NOTIFY ENGINEER IN WRITING OF ANY DISCREPANCIES OF PROPOSED LAYOUT AND/OR EXISTING FEATURES.
- C. EMPLOY A LICENSED SURVEYOR TO DETERMINE ALL LINES AND GRADES AND LAYOUT OF SITE ELEMENTS AND BUILDINGS.
- D. THE CONTRACTOR SHALL BE RESPONSIBLE TO BECOME FAMILIAR WITH THE SITE AND ALL SURROUNDING CONDITIONS. THE CONTRACTOR SHALL ADVISE THE APPROPRIATE AUTHORITY OF INTENTIONS AT LEAST 48 HOURS IN ADVANCE.
- E. TAKE APPROPRIATE MEASURES TO REDUCE, TO THE FULLEST EXTENT POSSIBLE, NOISE, DUST, AND UNSIGHTLY DEBRIS. CONSTRUCTION ACTIVITIES SHALL BE CARRIED OUT BETWEEN THE HOURS OF 7:00 AM AND 9:00 PM MONDAY THROUGH ERIDAY IN ACCORDANCE WITH THE APPLICABLE MUNICIPAL ORDINANCES AND REGULATIONS OF THE CITY OF PORTSMOUTH, NEW HAMPSHIRE
- F. MAINTAIN EMERGENCY ACCESS TO ALL AREAS AFFECTED BY WORK AT ALL TIMES.
- G. IN ACCORDANCE WITH RSA 430:53 AND AGR 3800, THE CONTRACTOR SHALL NOT TRANSPORT INVASIVE SPECIES OFF THE PROPERTY, AND SHALL DISPOSE OF INVASIVE SPECIES ON-SITE IN A LEGAL MANNER.
- H. COORDINATE WITH ALL UTILITY COMPANIES AND CONTACT DIGSAFE (811 OR 888-344-7233) AT LEAST 72 HOURS PRIOR TO ANY EXCAVATION.
- I. PROTECT NEW AND EXISTING BURIED UTILITIES DURING INSTALLATION OF ALL SITE ELEMENTS. DAMAGED UTILITIES SHALL BE REPAIRED OR REPLACED AT NO ADDITIONAL COST TO THE OWNER.
- J. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION AND FOR CONDITIONS AT THE SITE. THESE PLANS, PREPARED BY TFMORAN. INC., DO NOT EXTEND TO OR INCLUDE SYSTEMS PERTAINING TO THE SAFETY OF THE CONSTRUCTION CONTRACTOR OR THEIR EMPLOYEES, AGENTS, OR REPRESENTATIVES IN THE PERFORMANCE OF THE WORK. THE SEAL OF THE SURVEYOR OR ENGINEER HEREON DOES NOT EXTEND TO ANY SUCH SAFETY SYSTEMS THAT MAY NOW OR HEREAFTER BE INCORPORATED INTO THESE PLANS. THE CONSTRUCTION CONTRACTOR SHALL PREPARE OR OBTAIN THE APPROPRIATE SAFETY SYSTEMS WHICH MAY BE REQUIRED BY THE US OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION (OSHA) AND/OR LOCAL REGULATIONS.
- K. WRITTEN DIMENSIONS HAVE PRECEDENCE OVER SCALED DIMENSIONS. THE CONTRACTOR SHALL USE CAUTION WHEN SCALING REPRODUCED PLANS. IN CASE OF CONFLICT BETWEEN THIS PLAN SET AND ANY OTHER DRAWING AND/OR SPECIFICATION, THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY FOR CLARIFICATIONS.
- L. VERIFY LAYOUT OF PROPOSED BUILDING FOUNDATIONS WITH ARCHITECT AND THAT PROPOSED FOUNDATION MEETS PROPERTY LINE AND/OR WETLAND SETBACKS PRIOR TO COMMENCING ANY FOUNDATION CONSTRUCTION.
- M. PROVIDE AN AS-BUILT PLAN AT THE COMPLETION OF THE PROJECT TO THE PLANNING DIRECTOR AND PER CITY REGULATIONS.
- N. IF ANY DEVIATIONS FROM THE APPROVED PLANS AND SPECIFICATIONS HAVE BEEN MADE, THE SITE CONTRACTOR SHALL PROVIDE AS-BUILT DRAWINGS STAMPED BY A LICENSED SURVEYOR OR QUALIFIED ENGINEER ALONG WITH A LETTER STAMPED BY A QUALIFIED ENGINEER DESCRIBING ALL SUCH DEVIATIONS, AND BEAR ALL COSTS FOR PREPARING AND FILING ANY NEW PERMITS OR PERMIT AMENDMENTS THAT MAY BE REQUIRED.
- O. AT COMPLETION OF CONSTRUCTION, THE SITE CONTRACTOR SHALL PROVIDE A LETTER CERTIFYING THAT THE PROJECT WAS COMPLETED IN ACCORDANCE WITH THE APPROVED PLANS AND SPECIFICATIONS, AND A LETTER STAMPED BY A QUALIFIED ENGINEER THAT THEY HAVE OBSERVED ALL UNDERGROUND DETENTION SYSTEMS, INFILTRATION SYSTEMS, OR FILTERING SYSTEMS PRIOR TO BACKFILL, AND THAT SUCH SYSTEMS CONFORM TO THE APPROVED PLANS AND SPECIFICATIONS.

GRADING & DRAINAGE NOTES

- 1. THE CONTRACTOR SHALL ENSURE THAT ALL WORK IS PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF NHDES ENV-WQ 1500 AS APPLICABLE.
- 2. THE CONTRACTOR SHALL COORDINATE WITH THE OWNER TO SUBMIT AN ENOI AT LEAST 14 DAYS IN ADVANCE OF ANY EARTHWORK ACTIVITIES AT THE SITE.
- 3. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO CHECK THE ACCURACY OF THE TOPOGRAPHY AND REPORT ANY DISCREPANCIES TO THE ENGINEER PRIOR TO ANY EARTHWORK BEING PERFORMED ON THE SITE. NO CLAIM FOR EXTRA WORK WILL BE CONSIDERED FOR PAYMENT AFTER EARTHWORK HAS COMMENCED.
- 4. THE CONTRACTOR SHALL REFER TO THE GEOTECHNICAL REPORT FOR INFORMATION ABOUT SOIL AND GROUNDWATER CONDITIONS. THE CONTRACTOR SHALL FOLLOW THE GEOTECHNICAL ENGINEER'S RECOMMENDED METHODS TO ADDRESS ANY SOIL AND GROUNDWATER ISSUES THAT ARE FOUND ON SITE, INCLUDING AND NOT LIMITED TO DEWATERING METHODS, PERIMETER DRAINS AND TIE INTO STORMWATER MANAGEMENT SYSTEM, ETC.
- 5. COORDINATE WITH GEOTECHNICAL/STRUCTURAL PLANS FOR SITE PREPARATION AND OTHER BUILDING INFORMATION.
- COORDINATE WITH ARCHITECTURAL PLANS FOR DETAILED GRADING AT BUILDING, AND SIZE AND LOCATION OF ALL BUILDING SERVICES.
- 7. COORDINATE WITH MECHANICAL AND PLUMBING PLANS FOR ROOF DRAIN INFORMATION.
- 8. I MITS OF WORK ARE SHOWN AS APPROXIMATE. THE CONTRACTOR SHALL COORDINATE ALL
- 9. THE CONTRACTOR SHALL PROVIDE A FINISH PAVEMENT SURFACE FREE OF LOW SPOTS AND PONDING AREAS.
- 10. THE SITE SHALL BE GRADED SO ALL FINISHED PAVEMENT HAS POSITIVE DRAINAGE AND SHALL NOT POND WATER DEEPER THAN 1/4" FOR A PERIOD OF MORE THAN 15 MINUTES AFTER FLOODING.
- 11. ROAD AND DRAINAGE CONSTRUCTION SHALL CONFORM TO THE TYPICAL SECTIONS AND DETAILS SHOWN ON THE PLANS AND SHALL MEET LOCAL STANDARDS AND THE REQUIREMENTS OF THE LATEST NHOOT STANDARD SPECIFICATIONS FOR ROADS AND BRIDGE CONSTRUCTION AND THE NHDOT STANDARD STRUCTURE DRAWINGS UNLESS OTHERWISE NOTED.
- 12. STORMWATER DRAINAGE SYSTEM SHALL BE CONSTRUCTED TO LINE AND GRADE AS SHOWN ON THE PLANS. CONSTRUCTION METHODS SHALL CONFORM TO NHDOT STANDARD SPECIFICATIONS. SECTION 603.
- 13. NO FILL SHALL BE PLACED IN ANY WETLAND AREA.
- 14. ALL EXCAVATIONS SHALL BE THOROUGHLY SECURED ON A DAILY BASIS BY THE CONTRACTOR AT THE COMPLETION OF CONSTRUCTION OPERATIONS IN THE IMMEDIATE AREA.
- 15. ALL DISTURBED AREAS NOT TO BE PAVED OR OTHERWISE TREATED SHALL RECEIVE 6" LOAM, SEED, FERTILIZER, AND MULCH.
- 16. DENSITY REQUIREMENTS: MINIMUM DENSITY*

95%

95%

90%

ASTM D-1556 OR ASTM D-6938.

LOCATION BELOW PAVED OR CONCRETE AREAS TRENCH BEDDING MATERIAL AND SAND BLANKET BACKFILL BELOW LOAM AND SEED AREAS *ALL PERCENTAGES OF COMPACTION SHALL BE OF THE MAXIMUM DRY DENSITY AT THE OPTIMUM MOISTURE CONTENT AS DETERMINED AND CONTROLLED IN ACCORDANCE WITH

> 4 9/20/2022 REVISED PER TAC COMMENTS 3 8/31/2022 REVISED PER TAC COMMENTS 2 8/23/2022 REVISED PER TAC COMMENTS 1 7/21/2022 REVISED PER TAC COMMENTS REV DATE DESCRIPTION

TAPPING SLEEVE, VALVE, AND BOX

TSV

UTILITY POLE

UTILITY NOTES

WORK TO PROVIDE SMOOTH TRANSITIONS. THIS INCLUDES GRADING AND PAVEMENT.

ASTM D-1557, METHOD C. FIELD DENSITY TESTS SHALL BE MADE IN ACCORDANCE WITH

1. LENGTH OF PIPE IS FOR CONVENIENCE ONLY. ACTUAL PIPE LENGTH SHALL BE DETERMINED IN THE FIELD.

2. ALL PROPOSED UTILITY WORK, INCLUDING MATERIAL, INSTALLATION, TERMINATION, EXCAVATION, BEDDING, BACKFILL, COMPACTION, TESTING, CONNECTIONS, AND CONSTRUCTION SHALL BE COORDINATED WITH AND COMPLETED IN ACCORDANCE WITH THE APPROPRIATE REQUIREMENTS, CODES, AND STANDARDS OF ALL CORRESPONDING UTILITY ENTITIES AND SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING AND DETERMINING THE LOCATION, SIZE, AND ELEVATION OF ALL EXISTING UTILITIES, SHOWN OR NOT SHOWN ON THESE PLANS, PRIOR TO THE START OF ANY CONSTRUCTION. THE ENGINEER SHALL BE NOTIFIED IN WRITING OF ANY UTILITIES FOUND INTERFERING WITH THE PROPOSED CONSTRUCTION AND APPROPRIATE REMEDIAL ACTION BE AGREED TO BY THE ENGINEER BEFORE PROCEEDING WITH THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE TO CONTACT "DIGSAFE" (811) AT LEAST 72 HOURS BEFORE DIGGING.

4. COORDINATE ALL WORK ADJACENT TO PROPOSED BUILDINGS WITH ARCHITECTURAL BUILDING DRAWINGS. CONFIRM UTILITY PENETRATIONS AND INVERT ELEVATIONS ARE COORDINATED PRIOR TO INSTALLATION.

5. THE CONTRACTOR SHALL CONTACT ALL UTILITY COMPANIES OWNING UTILITIES, EITHER OVERHEAD OR UNDERGROUND, WITHIN THE CONSTRUCTION AREA AND SHALL COORDINATE AS NECESSARY WITH THE UTILITY COMPANIES OF SAID UTILITIES. THE PROTECTION OR RELOCATION OF UTILITIES IS ULTIMATELY THE RESPONSIBILITY OF THE CONTRACTOR.

6. THE EXACT LOCATION OF NEW UTILITY CONNECTIONS SHALL BE DETERMINED BY THE CONTRACTOR IN COORDINATION WITH UTILITY COMPANY, COUNTY AGENCY, AND/OR PRIVATE UTILITY COMPANY

7. THE CONTRACTOR SHALL PROVIDE AND INSTALL ALL MANHOLES. BOXES, FITTINGS, CONNECTORS, COVER PLATES, AND OTHER MISCELLANEOUS ITEMS NOT NECESSARILY DETAILED ON THESE DRAWINGS TO RENDER THE UTILITY INSTALLATION COMPLETE AND OPERATIONAL

8. ALL UTILITY COMPANIES REQUIRE INDIVIDUAL CONDUITS. CONTRACTOR TO COORDINATE WITH TELEPHONE, CABLE, AND ELECTRIC COMPANIES REGARDING NUMBER, SIZE, AND TYPE OF CONDUITS REQUIRED PRIOR TO INSTALLATION OF ANY CONDUIT.

9. SANITARY SEWER SHALL BE CONSTRUCTED TO THE STANDARDS AND SPECIFICATIONS AS SHOWN ON THESE PLANS. ALL SEWER MAINS AND FITTINGS SHALL BE PVC AND SHALL CONFORM TO ASTM F 679 (SDR 35 MINIMUM). FORCE MAINS AND FITTINGS SHALL CONFORM TO NH CODE OF ADMINISTRATIVE RULES ENV-WQ 700. ALL SEWER CONSTRUCTION SHALL BE IN ACCORDANCE WITH NH CODE OF ADMINISTRATIVE RULES ENV-WQ 700. SANITARY MANHOLES SHALL CONFORM TO NHDES WATER DIVISION WASTEWATER ENGINEERING BUREAU STANDARDS AND SPECIFICATIONS SHOWN HEREON.

10. ON-SITE WATER DISTRIBUTION SHALL BE TO CITY OF PORTSMOUTH STANDARDS AND SPECIFICATIONS. WATER MAINS SHALL HAVE A MINIMUM OF 5.5' COVER. WHERE WATER PIPES CROSS SEWER LINES A MINIMUM OF 18" VERTICAL SEPARATION BETWEEN THE TWO OUTSIDE PIPE WALLS SHALL BE OBSERVED. HORIZONTAL SEPARATION BETWEEN WATER AND SEWER SHALL BE 10' MINIMUM. WHERE A SANITARY LINE CROSSES A WATER LINE, SEWER LINE MUST BE CONSTRUCTED OF FORCE MAIN MATERIALS (PER ENV-WQ 704.08) FROM BUILDING OR MANHOLE TO MANHOLE, OR SUBSTITUTE RUBBER-GASKETED PRESSURE PIPE FOR THE SAME DISTANCE. WHEN SANITARY LINES PASS BELOW WATER LINES, LAY PIPE SO THAT NO JOINT IN THE SANITARY LINE WILL BE CLOSER THAN 6' HORIZONTALLY TO THE WATER LINE

11. THRUST BLOCKS SHALL BE PROVIDED AT ALL LOCATIONS WHERE WATER LINE CHANGES DIRECTIONS OR CONNECTS TO ANOTHER WATER LINE.

12. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR CONDUIT AND WIRING TO ALL SIGNS AND LIGHTS. CONDUIT TO BE A MINIMUM OF 24" BELOW FINISH GRADE.

13. ALL PROPOSED UTILITIES SHALL BE UNDERGROUND. ALL UNDERGROUND CONDUITS SHALL HAVE NYLON PULL ROPES.

14. THE CONTRACTOR SHALL ARRANGE AND PAY FOR ALL INSPECTIONS. TESTING, AND RELATED SERVICES AND SUBMIT COPIES OF ACCEPTANCE TO THE OWNER, UNLESS OTHERWISE INDICATED.

15. PROVIDE PERMANENT PAVEMENT REPAIR FOR ALL UTILITY TRENCHES IN EXISTING ROAD OR PAVEMENT TO REMAIN. SAW CUT TRENCH, PAVEMENT, AND GRANULAR BASE THICKNESS TO MATCH EXISTING PAVEMENT. OBTAIN ALL PERMITS REQUIRED FOR TRENCHING.

16. UNLESS OTHERWISE SPECIFIED, ALL UNDERGROUND STRUCTURES, PIPES, CHAMBERS, ETC. SHALL BE COVERED WITH A MINIMUM OF 18" OF COMPACTED SOIL BEFORE EXPOSURE TO VEHICLE LOADS.

17. THE PROPERTY WILL BE SERVICED BY THE FOLLOWING:

DRAINAGE PRIVATE SEWER WATER GAS ELECTRIC **TELEPHONE** CABLE

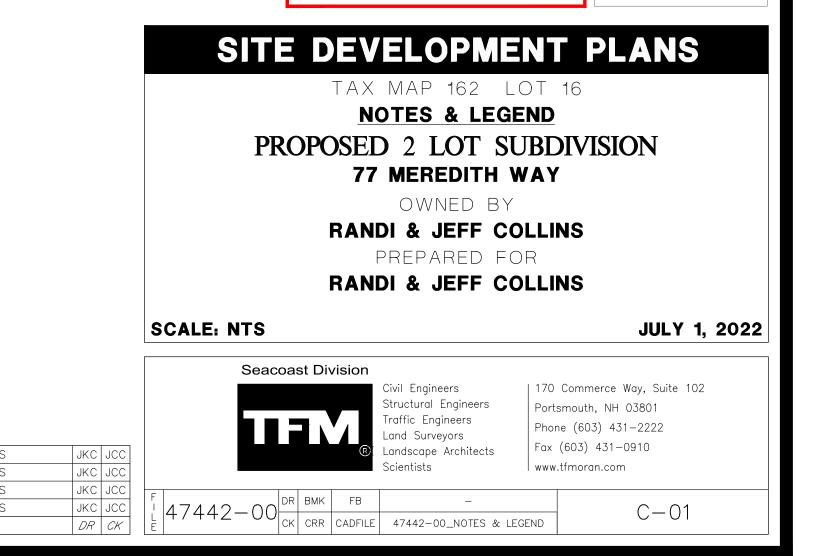
MUNICIPA MUNICIPAL NOT AVAILABL

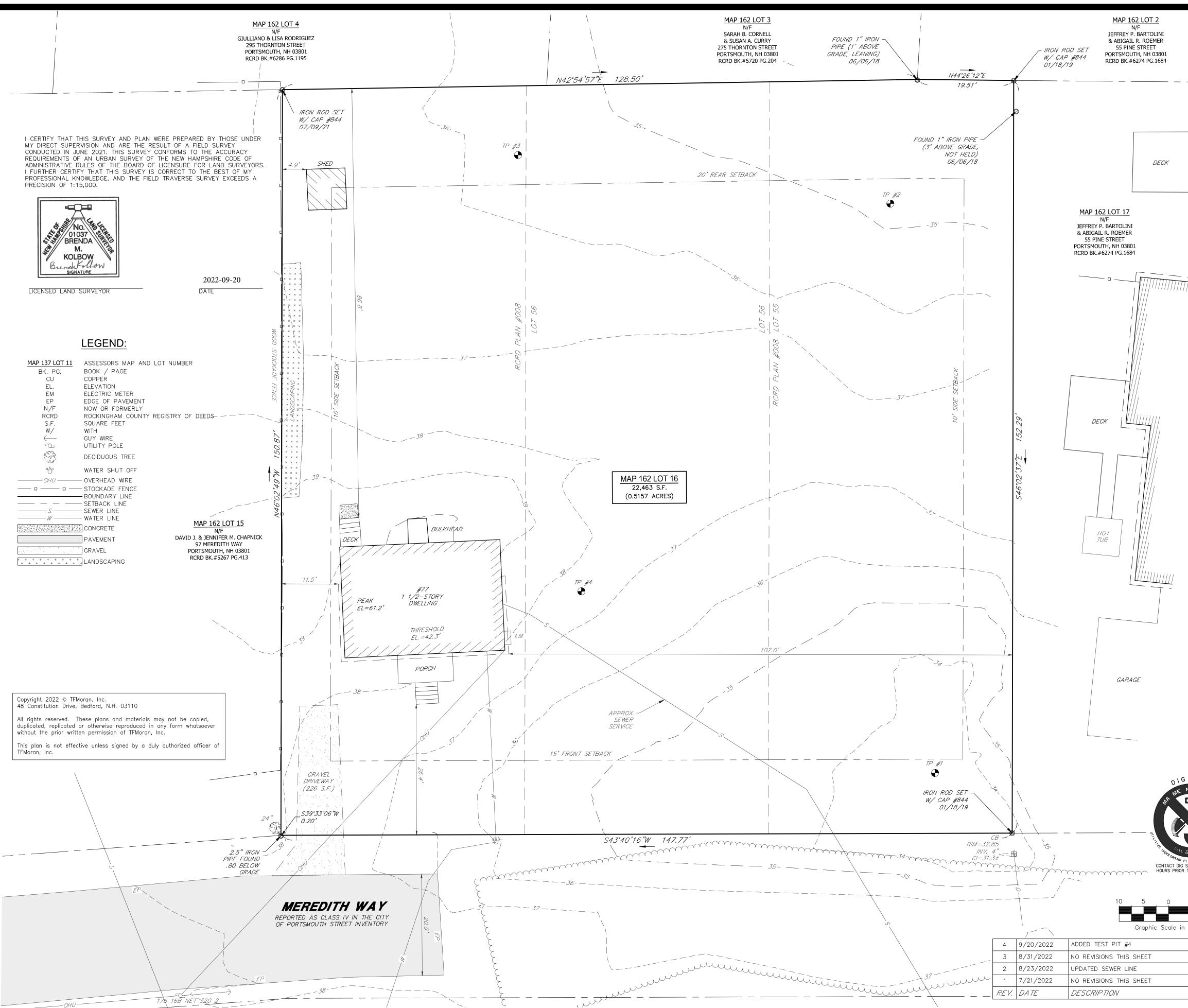
EVERSOURCE

CONSOLIDATED COMMUNICATIONS FKA FAIRPOINT COMMUNICATIONS COMCAST

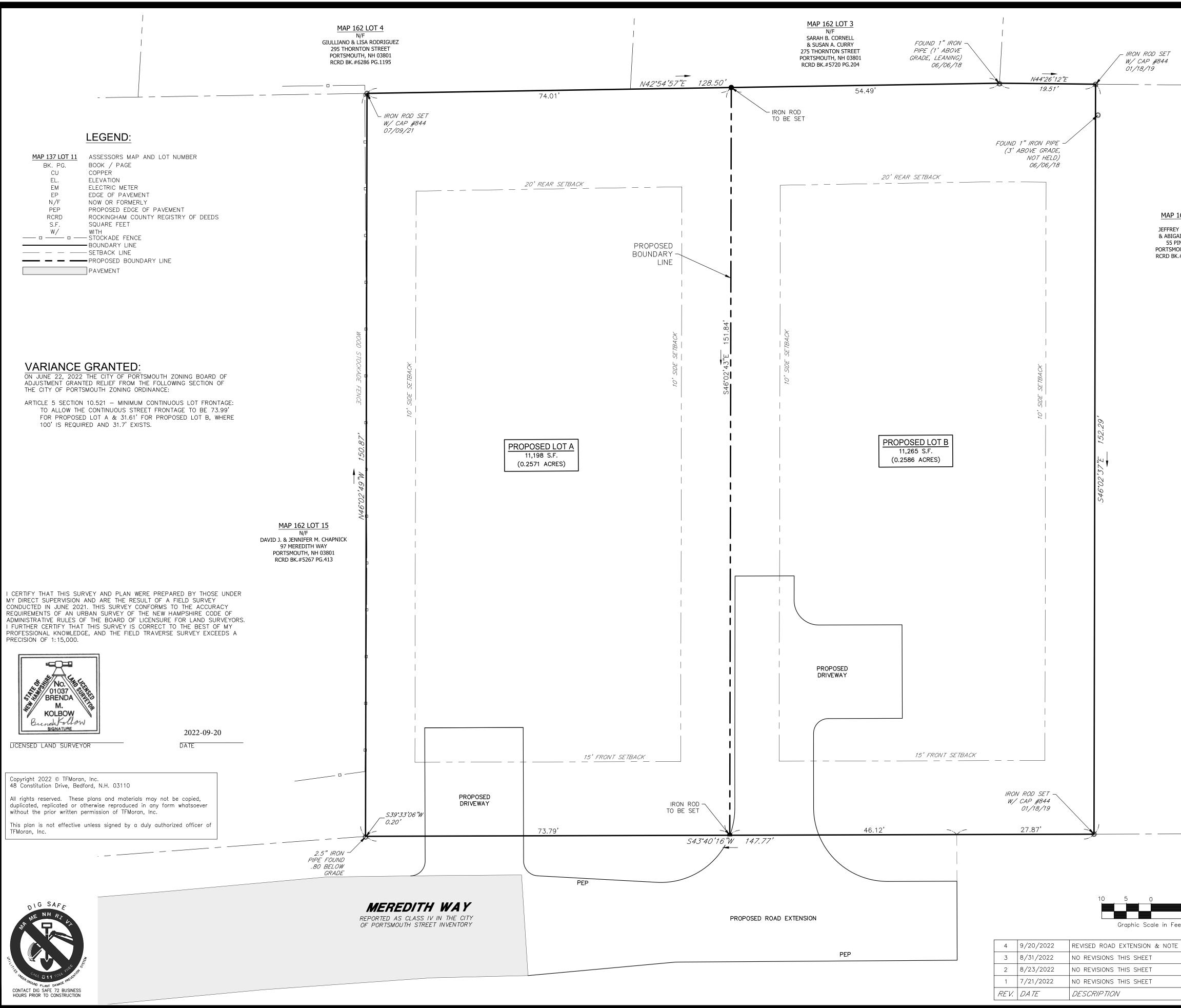


date: 09/20/2022

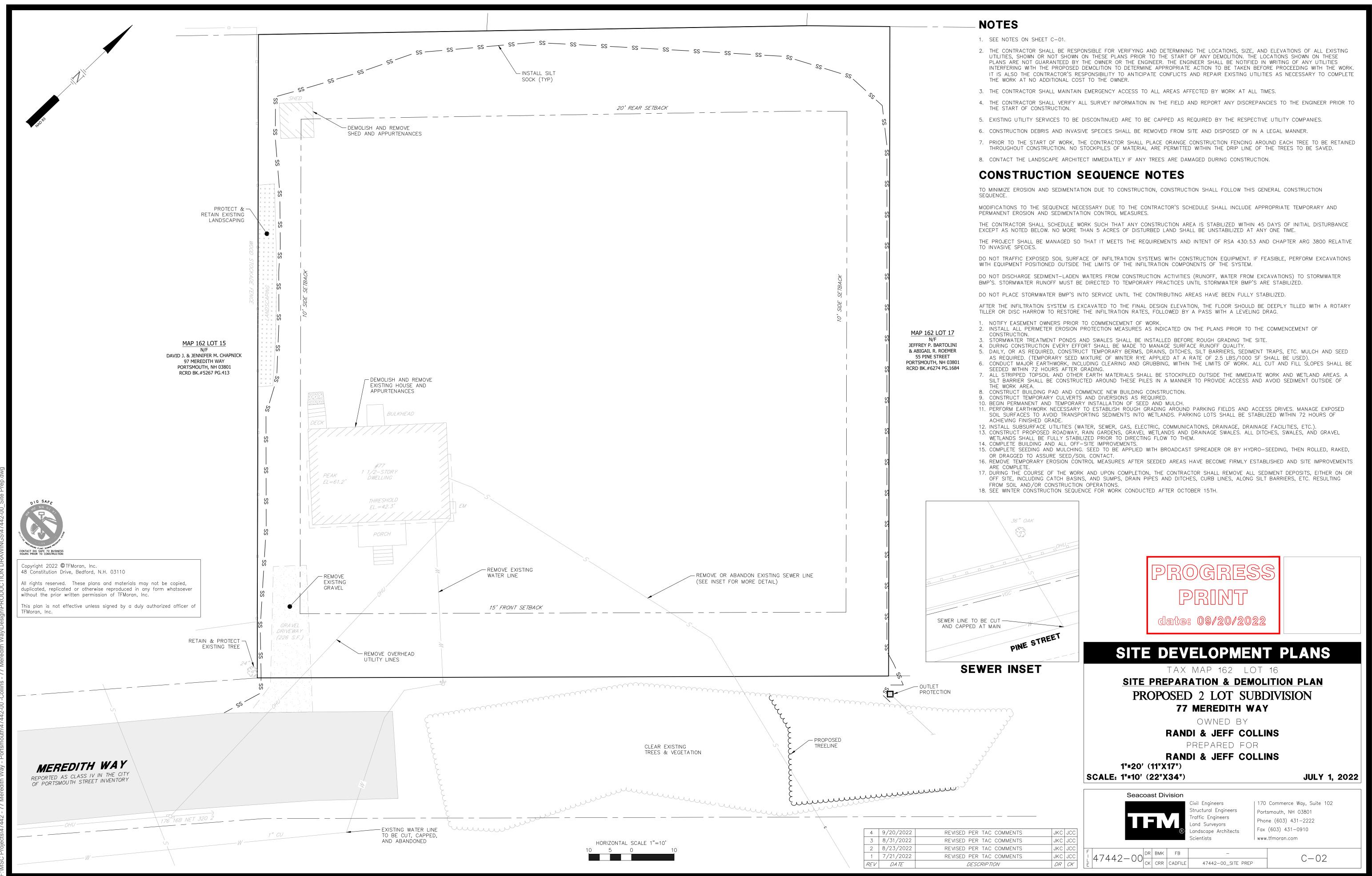


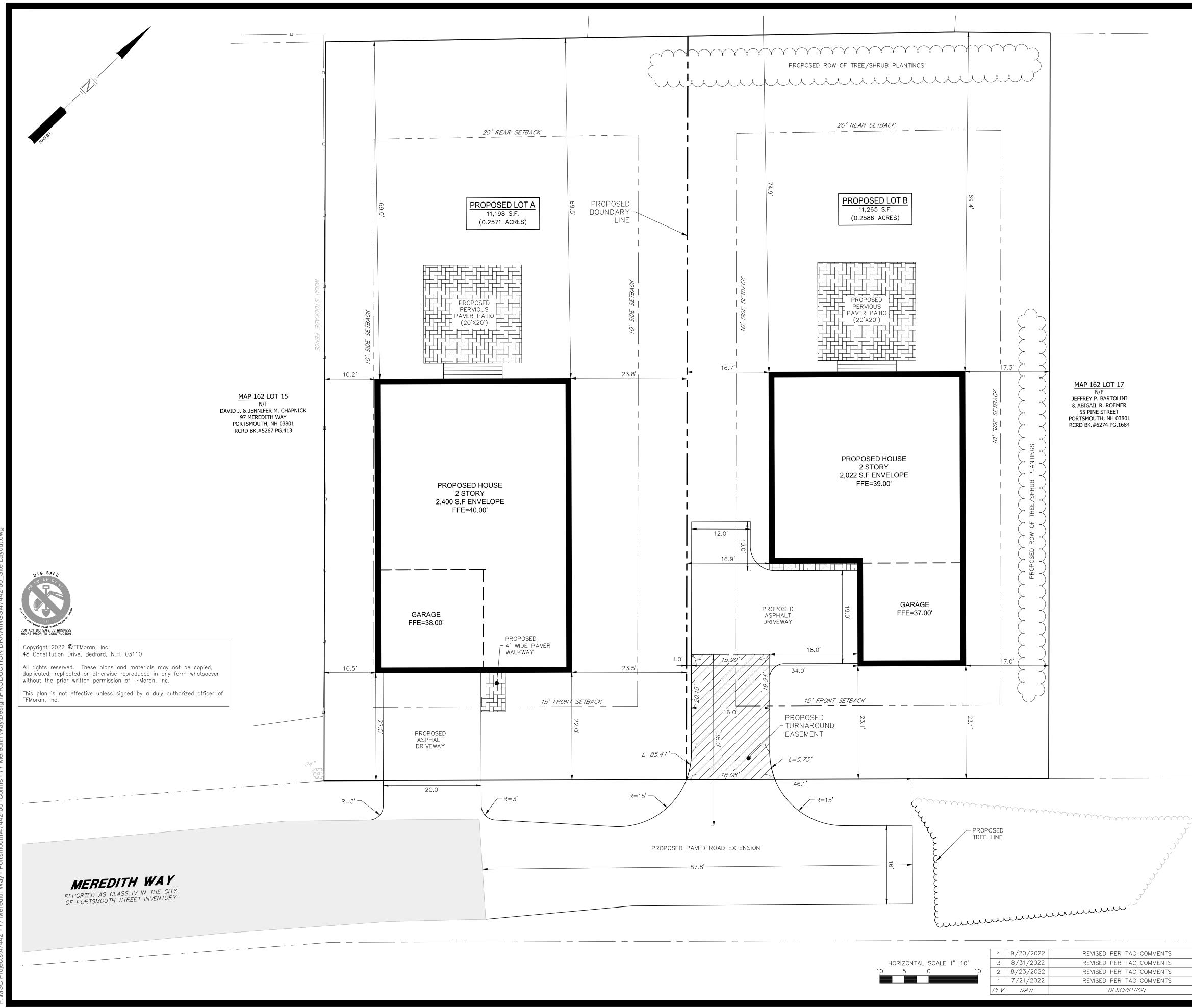


		MANOR DR.	JS J	RUNG X
	NOTES:	LOC	CATIOI	N PLAN
	 THE PARCEL IS LOCATED THE PARCEL IS SHOWN OF 			
	LOT 16. 3. THE PARCEL IS LOCATED PROGRAM (NFIP), FLOOD I HAMPSHIRE, PANEL 259 C JANUARY 29, 2021.	NSURANCE RATE MAP	(FIRM) ROCKING	GHAM COUNTY, NEW
	4. <u>MINIMUM LOT DIMENSIONS:</u> LOT AREA: LOT AREA PER DWELLING CONTINUOUS STREET FROM DEPTH <u>MINIMUM YARD DIMENSIONS</u> FRONT SIDE REAR <u>MAXIMUM STRUCTURE DIME</u> STRUCTURE HEIGHT: SLOPED ROOF:	UNIT: ITAGE: <u>S:</u>	REQUIRED: 7,500 S.F. 7,500 S.F. 100' 70' 15' 10' 20' 35'	EXISTING: 22,463 S.F. 22,463 S.F. 31.7' 151.6' 26.4' 11.5'/4.9' SHED 86.8' <35'
<i>#55</i>	FLAT ROOF: ROOF APPURTENANCE BUILDING COVERAGE: MINIMUM OPEN SPACE: 5. OWNER OF RECORD: <u>MAP 162 LOT 16:</u> RANDI & JEFF COLLINS 77 MEREDITH WAY PORTSMOUTH, NH 03801 RCRD BK.#6274 PG.#1666	E HEIGHT:	30' 8' 25% 30%	3.5% 85.3%
	6. PARCEL AREA: <u>MAP 162 LOT 16</u> : 22,463 S.F. (0.5157 ACRES)			
	7. THE INTENT OF THIS PLAN ACCORDANCE WITH THE C DEFINE THE EXTENT OF O'	URRENT LEGAL DESCRI	PTIONS. IT IS N	OT AN ATTEMPT TO
	8. THE PURPOSE OF THIS PL AND CURRENT SITE FEATU			S, TOPOGRAPHY
	9. FIELD SURVEY COMPLETED TOPCON FC-5000 DATA C	COLLECTOR.		
	10. HORIZONTAL DATUM IS NA VERTICAL DATUM IS NAVD INTERVAL IS 1 FOOT.			
	11. EASEMENTS, RIGHTS, AND WERE FOUND DURING RESI REGISTRY OF DEEDS. OTH WHICH A TITLE EXAMINATION	EARCH PERFORMED AT HER RIGHTS, EASEMEN	THE ROCKINGH	AM COUNTY TIONS MAY EXIST
	12. THE LOCATION OF ANY UN IS APPROXIMATE. TFMORAL COMPLETENESS OF UNDER ON SITE THE CONTRACTOR	N, INC. MAKES NO CLA GROUND UTILITIES SHO R SHALL CONTACT DIG	AIM TO THE ACC DWN. PRIOR TO SAFE.	CURACY OR ANY EXCAVATION
	13. THE EXISTING USE OF THIS		-AMILY RESIDEN	IIAL.
	1. "PLAN OF ELM PLACE, SIT #008.	UATED IN PORTSMOUT	H, N.H." DATED	1856. RCRD PLAN
	 "LOT LINE REVISION, PINE MAYO & CITY OF PORTSM INC., DATED 6/9/93 WITH 	OUTH" PREPARED BY	DURGIN, VERRA	AND ASSOCIATES,
G SAFE NH RELATION FILE BILL BILL TOLL FILE BO FLANT DAMOGE PRODUCTION	EXISTI 2 LO 7 PORTSM COUN	MAP 162 L ING CONDITION OT SUBDIVI 7 MEREDITH W OUTH, NEW HA OUTH, NEW HA NTY OF ROCKIN OWNED BY NDI & JEFF COL	NS PLAN ISION VAY AMPSHIRE NGHAM	
10	SCALE: 1" = 10' (22x34) 1" = 20' (11x17)			JULY 1, 2022
in Feet BMK JCC BMK JCC BMK JCC BMK JCC BMK JCC	F L L L L L L L L L L L L L	Civil Engineers Structural Engineers Traffic Engineers Land Surveyors Landscape Architects Scientists	Portsmout Phone (60	merce Way, Suite 102 th, NH 03801 03) 431-2222 0 431-0910 ran.com



MAP 162 LOT 2 N/F JEFFREY P. BARTOL & ABIGAIL R. ROEM 55 PINE STREET PORTSMOUTH, NH 02 RCRD BK.#6274 PG.1	INI JER 3801		
	NAD83	LOCUS ANDORURAL RA	
. <u>62 LOT 17</u> N/F	NOTEO	LOC	ATION PLAN
P. BARTOLINI IL R. ROEMER NE STREET	NOTES:		A (GRA) ZONING DISTRICT.
OUTH, NH 03801 #6274 PG.1684			H ASSESSOR'S MAP 162 AS LOT 16.
	(NFIP), FLOOD INSURANCE	RATE MAP (FIRM) ROCKING IUMBER 33015C0259F, MAP	NATIONAL FLOOD INSURANCE PROGRAM HAM COUNTY, NEW HAMPSHIRE, P REVISED JANUARY 29, 2021. QUIRED: <u>PROPOSED:</u>
	LOT AREA: LOT AREA PER DWELLING U CONTINUOUS STREET FRONT DEPTH	INIT: 7, TAGE: 10 70	/
	MINIMUM YARD DIMENSIONS: FRONT SIDE REAR MAXIMUM STRUCTURE DIMEN	- 15 10 20	' 10.2'/16.7' ' 69.0'/69.4'
	STRUCTURE HEIGHT: SLOPED ROOF: FLAT ROOF:	35 30	,
	ROOF APPURTENANCE BUILDING COVERAGE: MINIMUM OPEN SPACE:		% 21.4%/18.0%
	5. OWNER OF RECORD: <u>MAP 162 LOT 16:</u> RANDI & JEFF COLLINS 77 MEREDITH WAY PORTSMOUTH, NH 03801		,
	22,463 S.F. 11	1,198 S.F. 11	20POSED LOT B: 265 S.F. .2586 ACRES)
	7. THE INTENT OF THIS PLAN WITH THE CURRENT LEGAL OF OWNERSHIP OR DEFINE	IS TO SHOW THE LOCATIO DESCRIPTIONS. IT IS NOT THE LIMITS OF TITLE.	N OF BOUNDARIES IN ACCORDANCE AN ATTEMPT TO DEFINE THE EXTENT
	 THE PURPOSE OF THIS PLA FIELD SURVEY COMPLETED A TOPCON FC-5000 DATA HORIZONTAL DATUM IS NAC 	BY TCE JUNE 2021 & JUN COLLECTOR.	E 2022 USING A TOPCON DS103 AND
	11. EASEMENTS, RIGHTS, AND F FOUND DURING RESEARCH	RESTRICTIONS SHOWN OR I PERFORMED AT THE ROCKI , OR RESTRICTIONS MAY E	DENTIFIED ARE THOSE WHICH WERE NGHAM COUNTY REGISTRY OF DEEDS. XIST WHICH A TITLE EXAMINATION OF
		IC. MAKES NO CLAIM TO T	MATION SHOWN ON THIS PLAN IS THE ACCURACY OR COMPLETENESS OF AVATION ON SITE THE CONTRACTOR
	13. THE PROPOSED USE OF TH		-FAMILY RESIDENTIAL.
	1. "PLAN OF ELM PLACE, SITU	JATED IN PORTSMOUTH, N.I	H." DATED 1856. RCRD PLAN #008.
	& CITY OF PORTSMOUTH" F 6/9/93 WITH REVISION 1 D 3. "SITE DEVELOPMENT PLANS OWNED BY RANDI & JEFF (PREPARED BY DURGIN, VER DATED 10/4/93. RCRD PLA , TAX MAP LOT 16, TWO L COLLINS, PREPARED FOR R	OT ["] SUBDIVISION, 77 MEREDITH WAY,
		X MAP 162 LC SUBDIVISION PL	AN
	2	LOT SUBDIVIS	
		MOUTH, NEW HA	
	CO	UNTY OF ROCKING Owned by	GHAM
	R	ANDI & JEFF COLI	INS
	SCALE: 1" = 10' (22x34) 1" = 20' (11x17)		JULY 1, 2022
10	Seacoast Divis	sion	UVL 1 1, 2022
4 BMK JCC		Civil Engineers Structural Engineers Traffic Engineers Land Surveyors	170 Commerce Way, Suite 102 Portsmouth, NH 03801 Phone (603) 431-2222 Fax (603) 431-0910
BMK JCC BMK JCC BMK JCC	Г. DB ВМК	Landscape Architects Scientists	Fax (603) 431-0910 www.tfmoran.com
DR CK	47442-00	DFILE	S-2





Sep 20, 2022 - 1:58pm F:\MSC Projects\47442 - 77 Meredith Way - Portsmouth\47442-00 -Collins - 77 Meredith Way\Design\PRODUCTION DRAWINGS\47442-00 Site Layout.

SITE DATA

OWNER OF RECORD OF MAP 162 LOT 16: RANDI & JEFF COLLINS

77 MEREDITH WAY, PORTSMOUTH, NH 03801

DEED REFERENCE TO PARCEL IS BK 6274 PG 1666

AREA OF PARCEL = $22,463 \pm$ SF OR 0.5157 \pm ACRES

ZONED:GENERAL RESIDENCE A (GRA)EXISTING USE:1 LOT, SINGLE FAMILY DWELLING UNITPROPOSED USE:2 LOTS, 2 SINGLE FAMILY DWELLING UNITS

THE PURPOSE OF THIS PLAN IS TO DEPICT TWO PROPOSED SING FAMILY DWELLING UNITS WITH ACCESS ALONG MEREDITH WAY. ASSOCIATED IMPROVEMENTS INCLUDE AND ARE NOT LIMITED TO ACCESS, GRADING, STORMWATER MANAGEMENT SYSTEMS, UTILITIES.

DIMENSIONAL REQUIREMENTS (CURRENT ZONING)

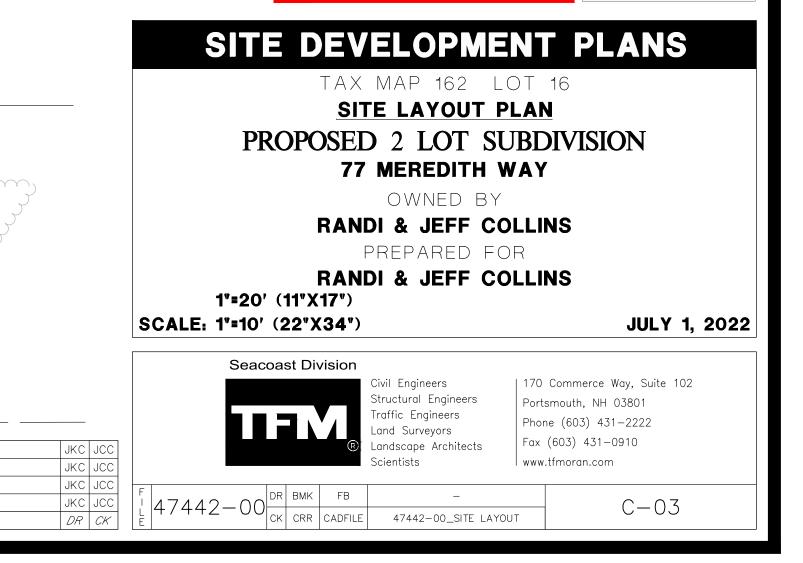
	REQUIRED:	PROVIDED: Lot A:	LOT B:
MINIMUM LOT DIMENSIONS: LOT AREA LOT FRONTAGE DEPTH	7,500 SF 100 FT 70 FT	11,198 SF 73.99 FT 151.4 FT	11,265 SF 31.61 FT 152.1 FT
MINIMUM YARD DIMENSIONS: FRONT SIDE REAR	15 FT 10 FT 20 FT	22.0 FT 10.2 FT 69.0 FT	23.1 FT 16.7 FT 69.4 FT
MAXIMUM STRUCTURE DIMENSIONS: SLOPED ROOF FLAT ROOF ROOF APPURTENANCE HEIGHT BUILDING LOT COVERAGE	35 FT 30 FT 8 FT 25% (MAX)	<35 FT NA >8 FT 21.4%	<35 FT NA >8 FT 18.0%
MINIMUM SETBACKS/BUFFER: BUILDING FRONT BUILDING SIDE BUILDING REAR	15 FT 10 FT 20 FT	15 FT 10 FT 20 FT	15 FT 10 FT 20 FT
MINIMUM OPEN SPACE	30%	70.4%	67.8%
PARKING REQUIREMENTS			
PARKING SPACES 1.3 SPACES/UNIT	2 SPACES	2 SPACES	2 SPACES

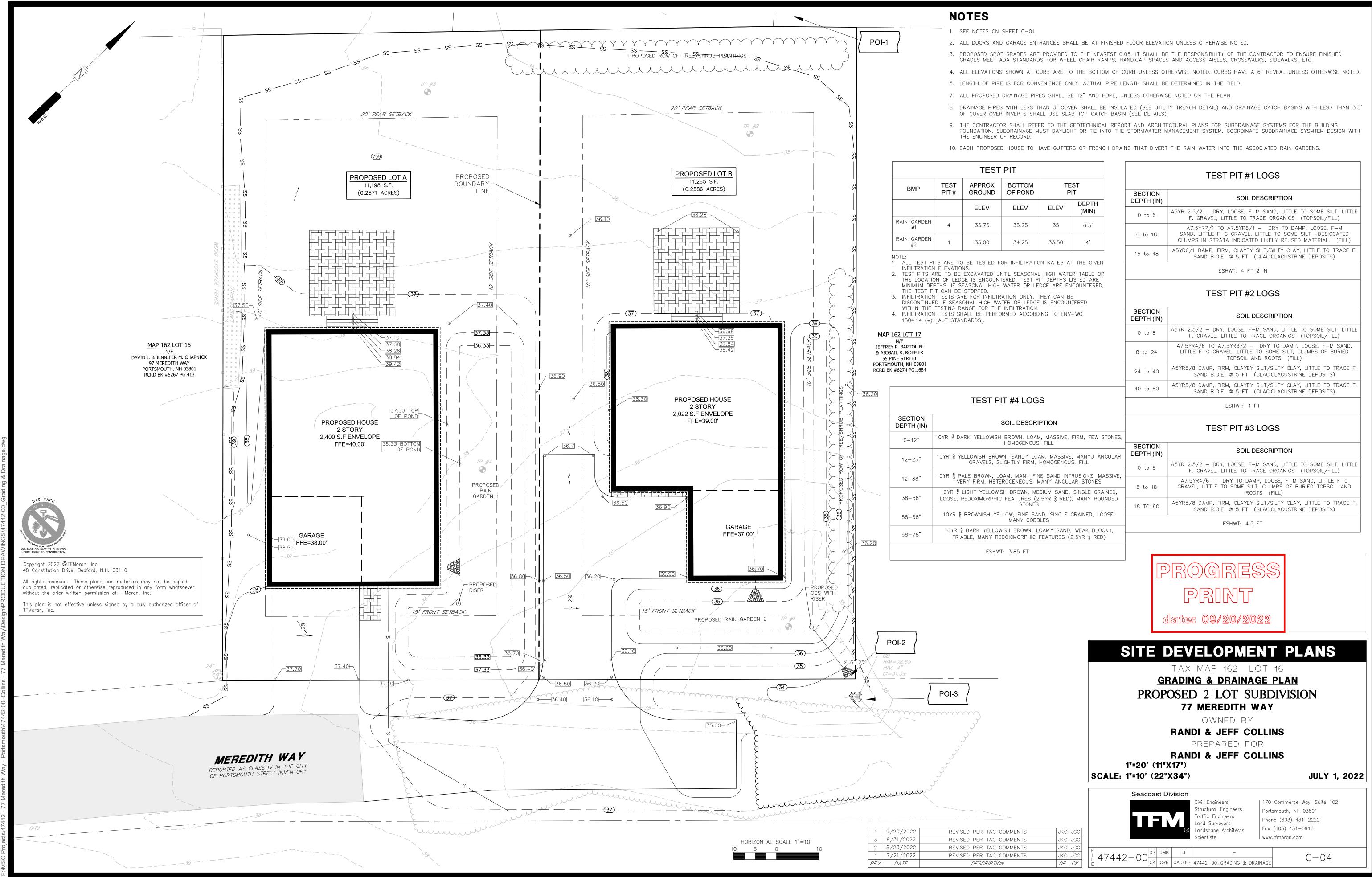
NOTES

- 1. SEE NOTES ON SHEET C-01.
- 2. ALL DIMENSIONS ARE TO THE FACE OF CURB UNLESS NOTED OTHERWISE.
- 3. LIGHTING, SIGNAGE, LANDSCAPING, AND SCREENING SHALL MEET THE REQUIREMENTS OF THE PORTSMOUTH ZONING ORDINANCE AND SITE PLAN REGULATIONS.
- 4. SNOW SHALL NOT BE STOCKPILED IN STORMWATER BMP'S, WETLAND BUFFERS, OR WETLANDS. SEE SNOW STORAGE LOCATIONS. IN THE EVENT THAT THE SNOW STORAGE AREAS PROVIDED ON THE SITE ARE COMPLETELY UTILIZED, EXCESS SNOW SHALL BE TRANSPORTED OFF SITE FOR DISPOSAL IN ACCORDANCE WITH NHDES REGULATION. IF SNOW IS STORED WITHIN PARKING AREA, KEEP CATCH BASINS CLEAR.
- 5. ALL CONDITIONS ON THIS PLAN SHALL REMAIN IN EFFECT IN PERPETUITY PURSUANT TO REQUIREMENTS OF THE SITE PLAN REVIEW REGULATIONS.
- THIS SITE PLAN SHALL BE RECORDED IN THE ROCKINGHAM COUNTY REGISTRY OF DEEDS.
 ALL IMPROVEMENTS SHOWN ON THIS SITE PLAN SHALL BE CONSTRUCTED AND MAINTAINED IN ACCORDANCE WITH THE PLAN BY THE PROPERTY OWNER AND ALL FUTURE PROPERTY OWNERS. NO CHANGES SHALL BE MADE TO THIS SITE PLAN WITHOUT THE EXPRESS APPROVAL OF THE PORTSMOUTH PLANNING DIRECTOR.



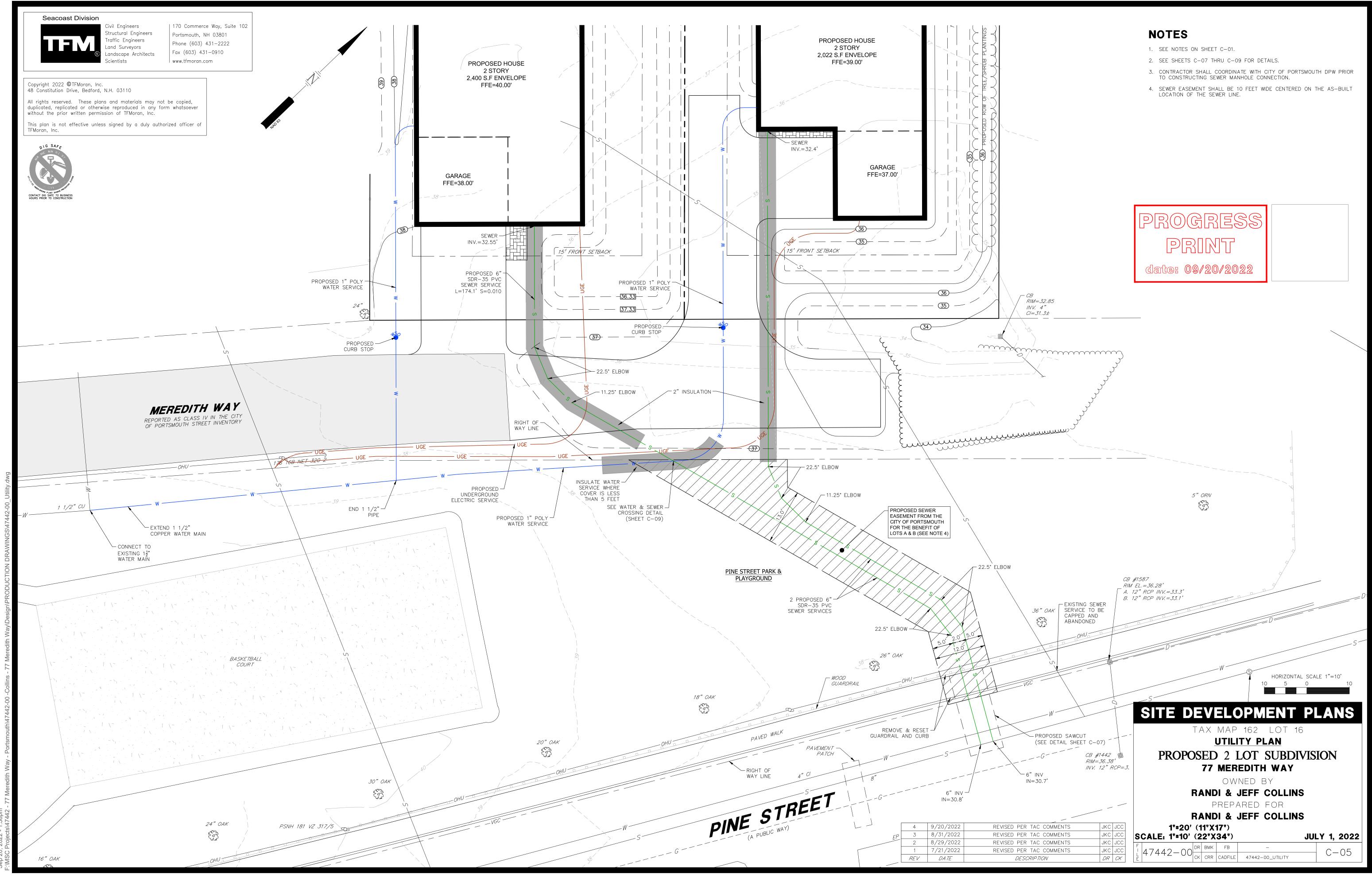
date: 09/20/2022



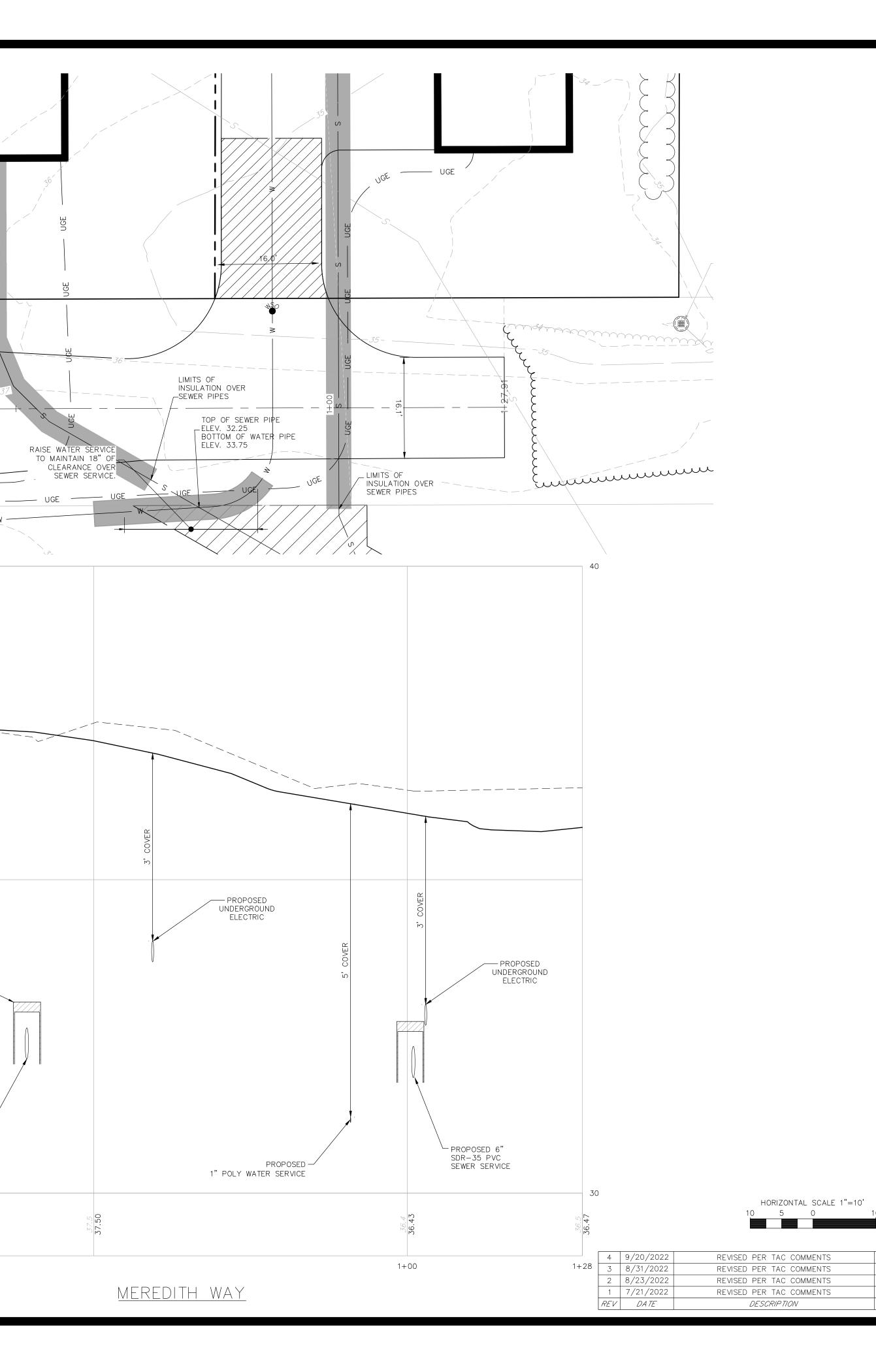


TOM TEST		TEST PIT #1 LOGS			
	SECTION DEPTH (IN)	SOIL DESCRIPTION			
EV ELEV (MIN)	0 to 6	A5YR 2.5/2 - DRY, LOOSE, F-M SAND, LITTLE TO SOME SILT, LITTLE F. GRAVEL, LITTLE TO TRACE ORGANICS (TOPSOIL/FILL)			
25 35 6.5' 25 33.50 4'	6 to 18	A7.5YR7/1 TO A7.5YR8/1 – DRY TO DAMP, LOOSE, F-M SAND, LITTLE F-C GRAVEL, LITTLE TO SOME SILT -DESICCATED CLUMPS IN STRATA INDICATED LIKELY REUSED MATERIAL. (FILL)			
25 33.30 4	15 to 48	A5YR6/1 DAMP, FIRM, CLAYEY SILT/SILTY CLAY, LITTLE TO TRACE F. SAND B.O.E. @ 5 FT (GLACIOLACUSTRINE DEPOSITS)			
ASONAL HIGH WATER TABLE OR		ESHWT: 4 FT 2 IN			
TEST PIT DEPTHS LISTED ARE OR LEDGE ARE ENCOUNTERED, DNLY. THEY CAN BE LEDGE IS ENCOUNTERED		TEST PIT #2 LOGS			
ACTION. According to env-wq	SECTION DEPTH (IN)	SOIL DESCRIPTION			
	0 to 8	A5YR 2.5/2 – DRY, LOOSE, F–M SAND, LITTLE TO SOME SILT, LITTLE F. GRAVEL, LITTLE TO TRACE ORGANICS (TOPSOIL/FILL)			
	8 to 24	A7.5YR4/6 TO A7.5YR3/2 – DRY TO DAMP, LOOSE, F-M SAND, LITTLE F-C GRAVEL, LITTLE TO SOME SILT, CLUMPS OF BURIED TOPSOIL AND ROOTS (FILL)			
	24 to 40	A5YR5/8 DAMP, FIRM, CLAYEY SILT/SILTY CLAY, LITTLE TO TRACE F. SAND B.O.E. @ 5 FT (GLACIOLACUSTRINE DEPOSITS)			
	40 to 60	A5YR5/8 DAMP, FIRM, CLAYEY SILT/SILTY CLAY, LITTLE TO TRACE F. SAND B.O.E. @ 5 FT (GLACIOLACUSTRINE DEPOSITS)			
LOGS		ESHWT: 4 FT			
ESCRIPTION		TEST PIT #3 LOGS			
I, LOAM, MASSIVE, FIRM, FEW STONES ENOUS, FILL	S, SECTION				
DY LOAM, MASSIVE, MANYU ANGULAR		SOIL DESCRIPTION			
FIRM, HOMOGENOUS, FILL	0 to 8	A5YR 2.5/2 – DRY, LOOSE, F–M SAND, LITTLE TO SOME SILT, LITTLE F. GRAVEL, LITTLE TO TRACE ORGANICS (TOPSOIL/FILL)			
ANY FINE SAND INTRUSIONS, MASSIVE, COUS, MANY ANGULAR STONES 	., 8 to 18	A7.5YR4/6 – DRY TO DAMP, LOOSE, F-M SAND, LITTLE F-C GRAVEL, LITTLE TO SOME SILT, CLUMPS OF BURIED TOPSOIL AND			
RES (2.5YR § RED), MANY ROUNDED Stones	18 TO 60	ROOTS (FILL) A5YR5/8 DAMP, FIRM, CLAYEY SILT/SILTY CLAY, LITTLE TO TRACE F. SAND B.O.E. © 5 FT (GLACIOLACUSTRINE DEPOSITS)			
NE SAND, SINGLE GRAINED, LOOSE, Y COBBLES		ESHWT: 4.5 FT			
OWN, LOAMY SAND, WEAK BLOCKY, RPHIC FEATURES (2.5YR 氰 RED)]				
FT	│				







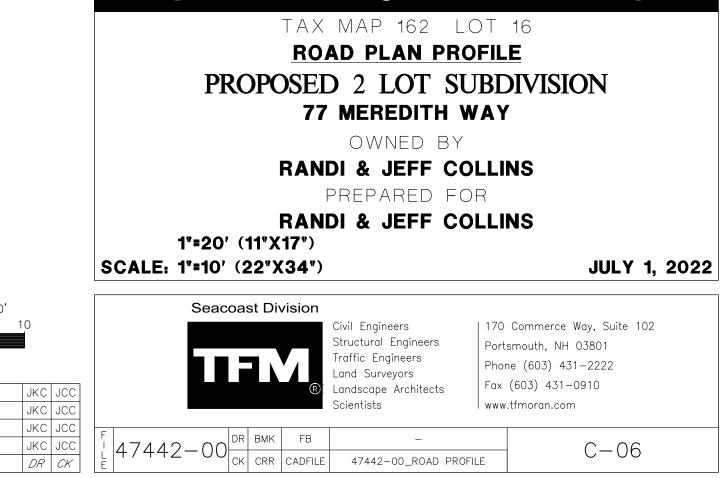


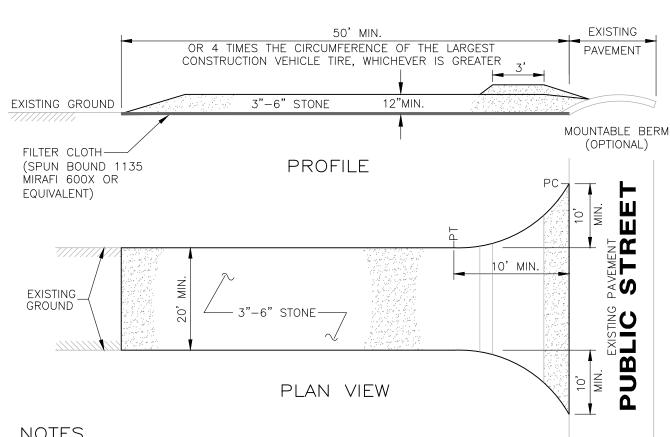
NOTES

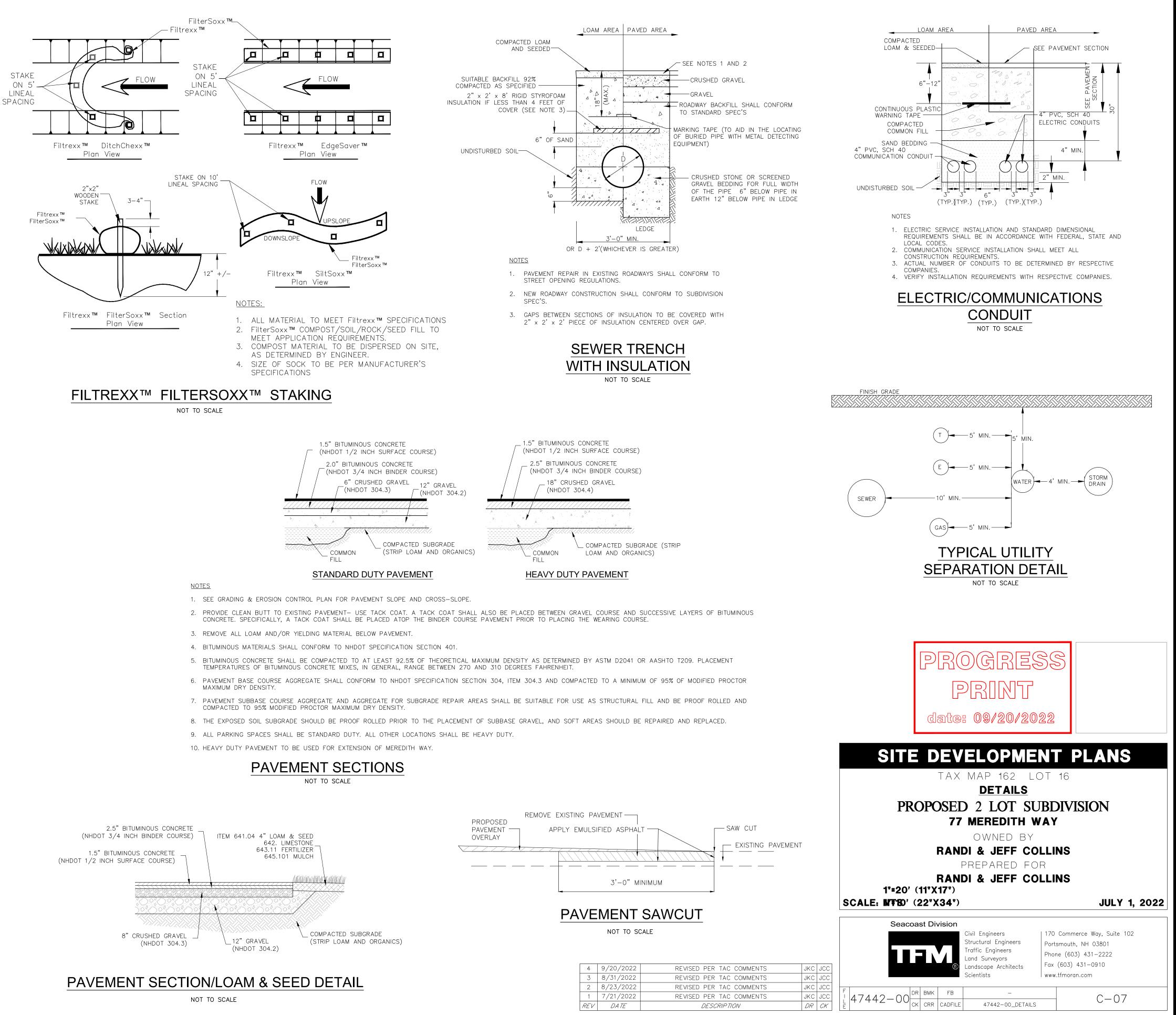
- 1. SEE NOTES ON SHEET C-01.
- 2. SEE UTILITY PLAN ON SHEET C-05 FOR MORE INFORMATION.

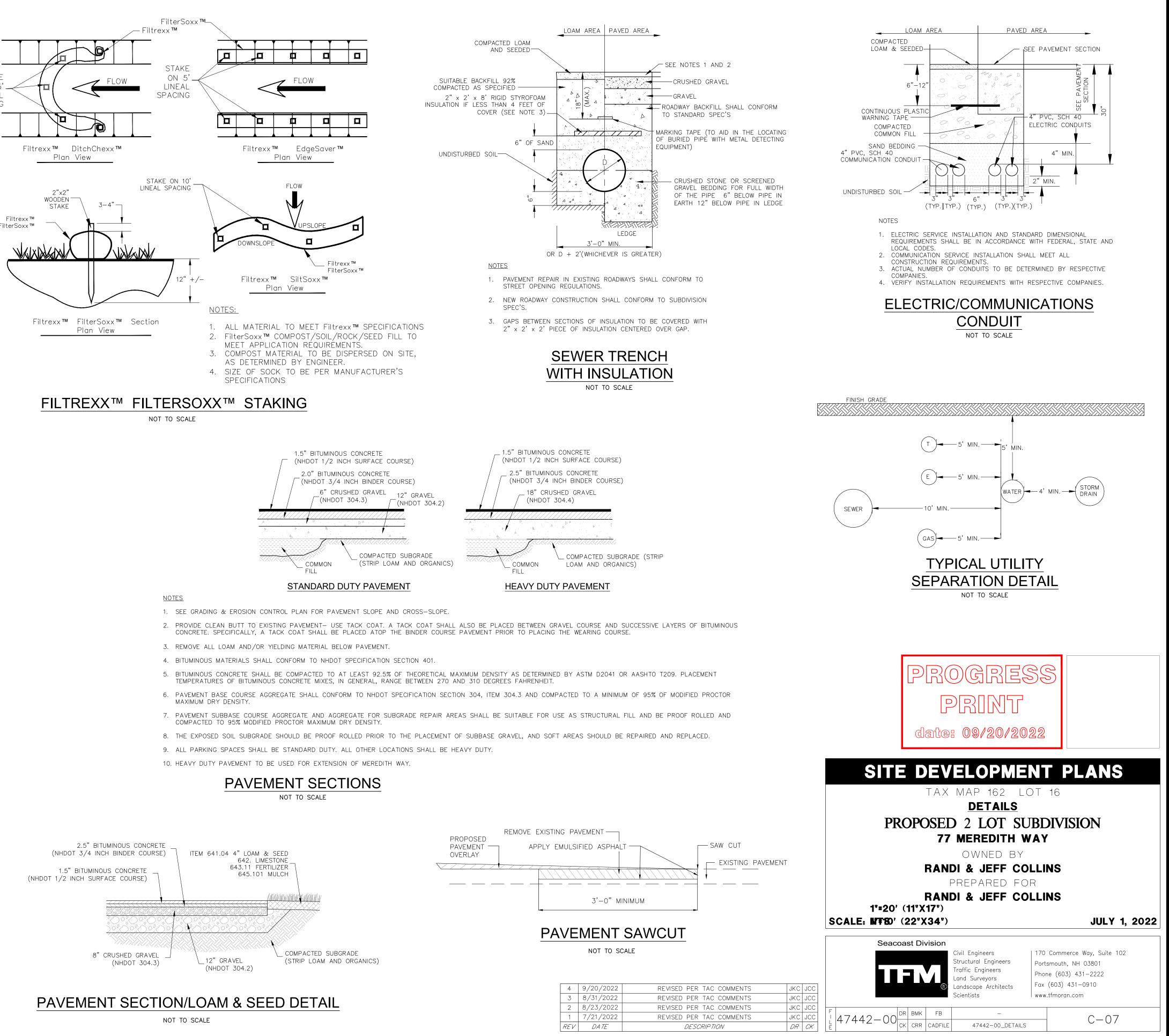


SITE DEVELOPMENT PLANS





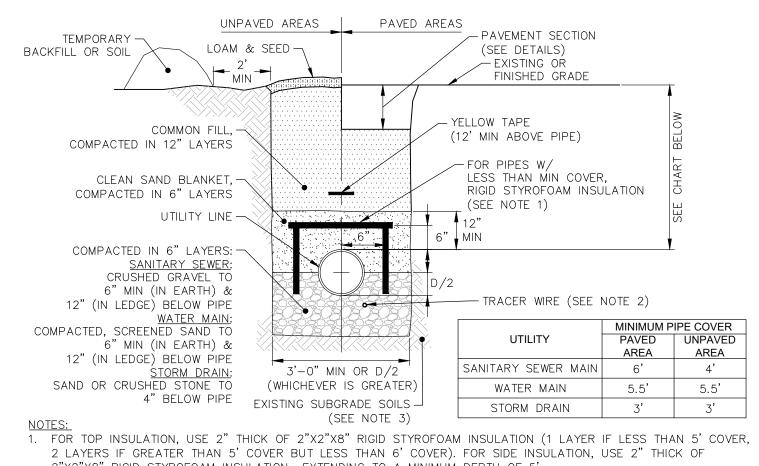






- 1. FILTER CLOTH WILL BE PLACED OVER THE ENTIRE AREA PRIOR TO PLACING OF STONE SURFACE.
- 2. WATER ALL SURFACE WATER FLOWING OR DIVERTED TOWARD CONSTRUCTION ENTRANCES SHALL BE PIPED ACROSS THE ENTRANCE. IF PIPING IS IMPRACTICAL, A MOUNTABLE BERM WITH 5:1 SLOPES WILL BE PERMITTED.
- 3. MAINTENANCE THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL STONE AS CONDITIONS DEMAND AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC RIGHTS-OF-WAY MUST BE REMOVED IMMEDIATELY.
- 4. WASHING WHEELS SHALL BE CLEANED TO REMOVE SEDIMENT PRIOR TO ENTRANCE ONTO PUBLIC RIGHTS-OF-WAY. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH STONE AND WHICH DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE.
- 5. PERIODIC INSPECTION AND NEEDED MAINTENANCE SHALL BE PROVIDED AFTER EACH RAIN STORM EVENT.





- 2"X2"X8" RIGID STYROFOAM INSULATION EXTENDING TO A MINIMUM DEPTH OF 5' 2. TRACER WIRE SPECIFIED FOR NON-METALLIC WATER LINES SHALL BE INSTALLED BELOW AND TO THE SIDE OF THE PIPE AND PER THE MANUFACTURER REQUIREMENTS. TRACER WIRE PRODUCT SHALL BE SELECTED FOR OPEN CUT
- INSTALLATION TECHNIQUE 3. IN LOCATIONS WITH EXISTING FILL SOILS, THE EXISTING SUBGRADE SOILS AT THE BOTTOM OF THE TRENCH SHALL BE OVER-EXCAVATED 2' DEEP AND RECOMPACTED IN 12" LIFTS TO 95% MAXIMUM DENSITY.

UTILITY TRENCH FOR SEWER, WATER, AND STORM DRAIN LINES

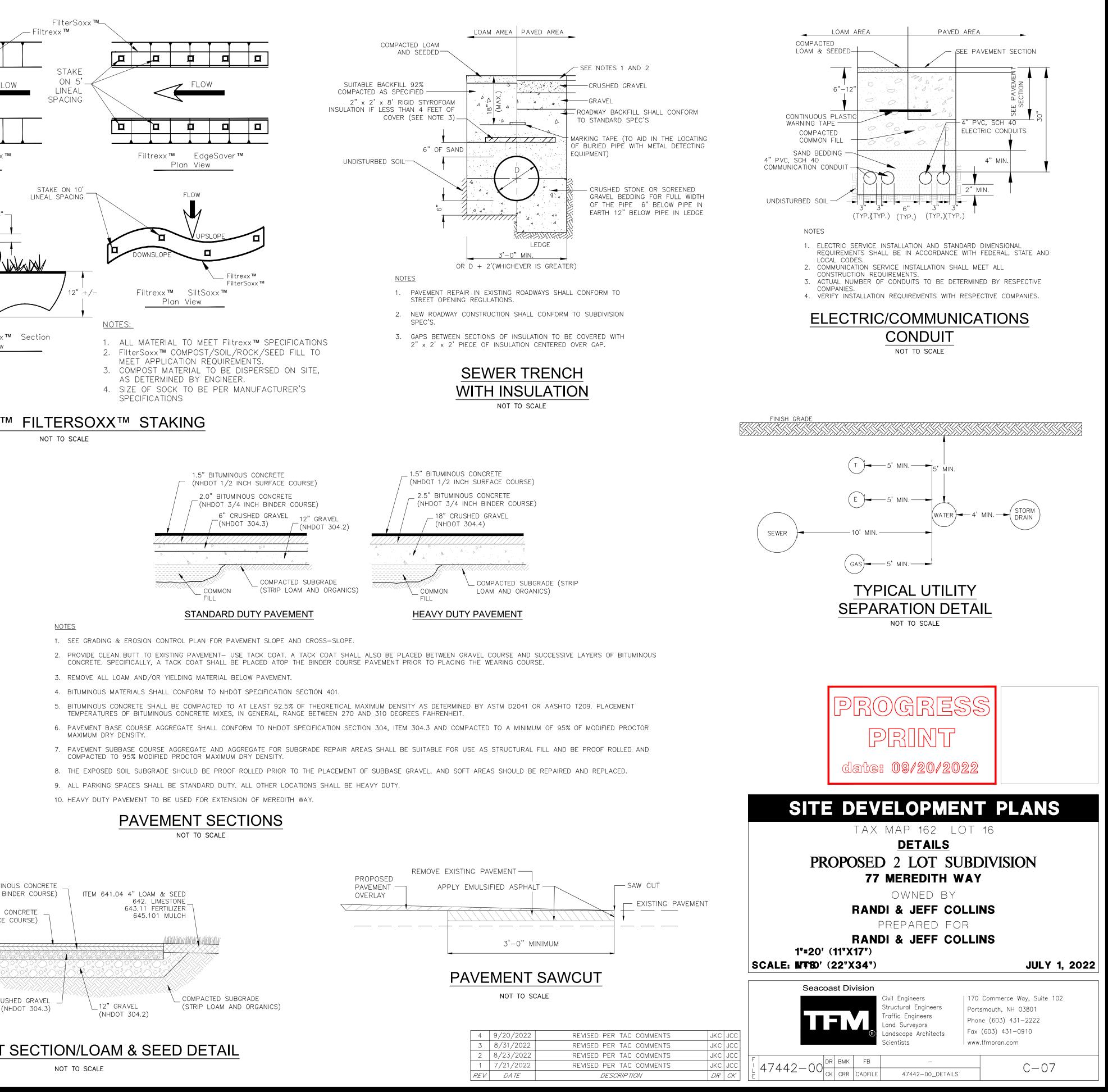
CONTACT DIG SAFE 72 BUSINESS HOURS PRIOR TO CONSTRUCTION

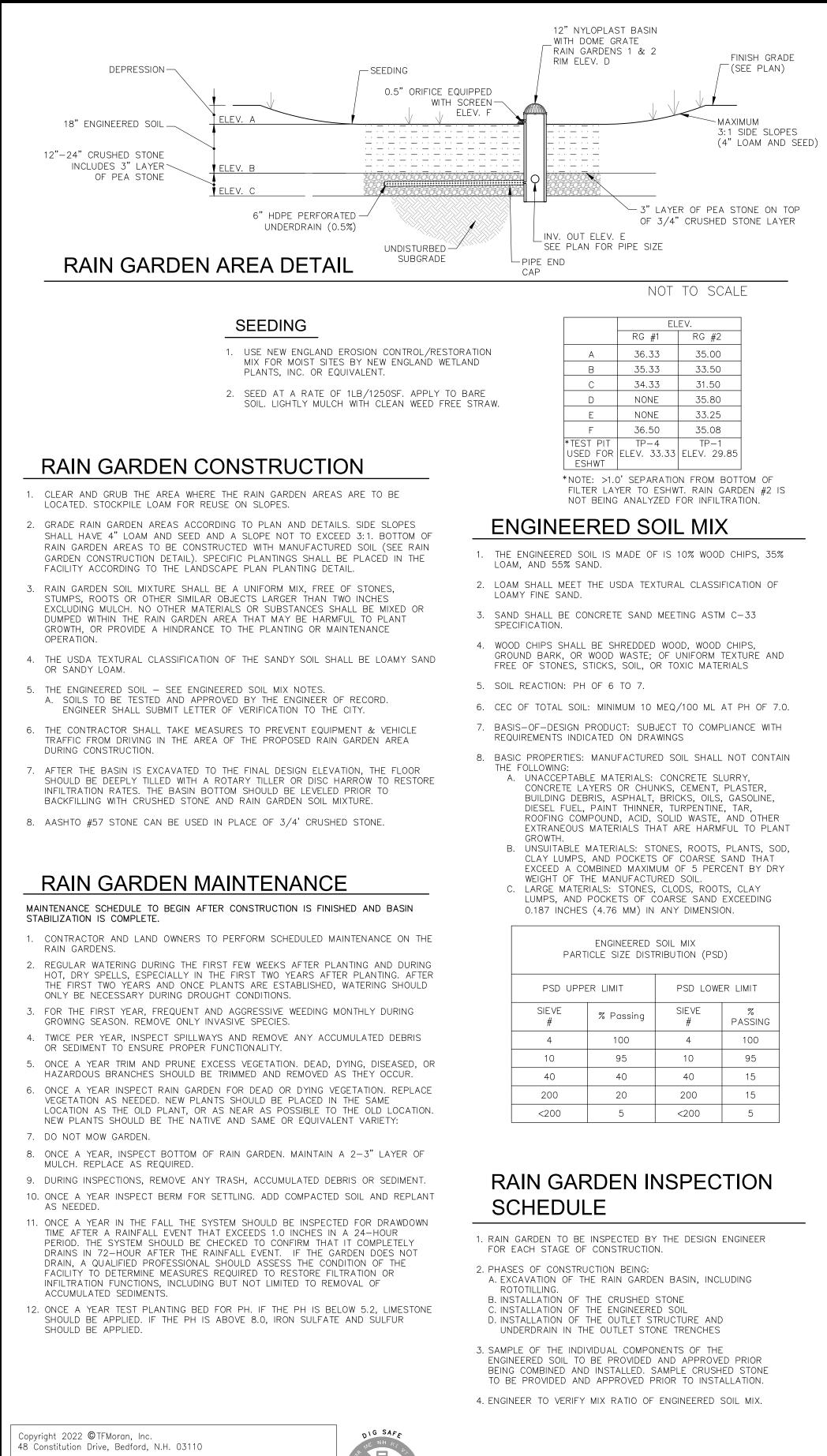
NOT TO SCALE

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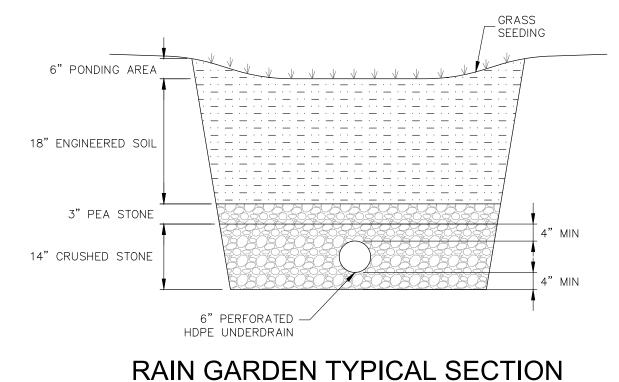


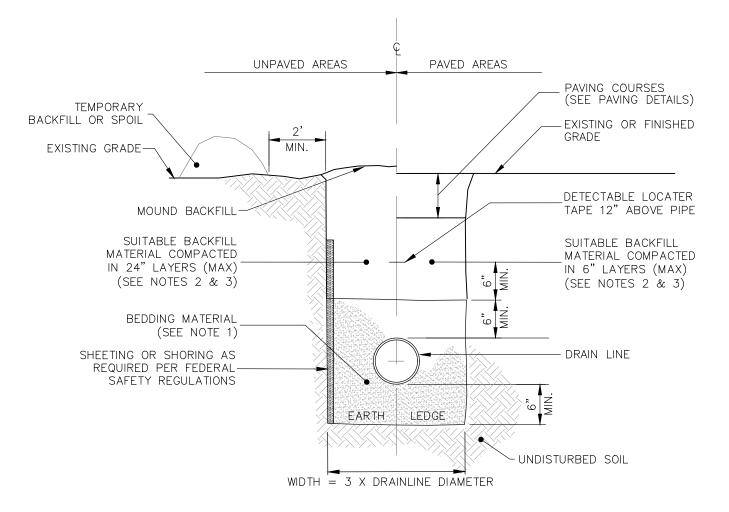
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FMoran, Inc.

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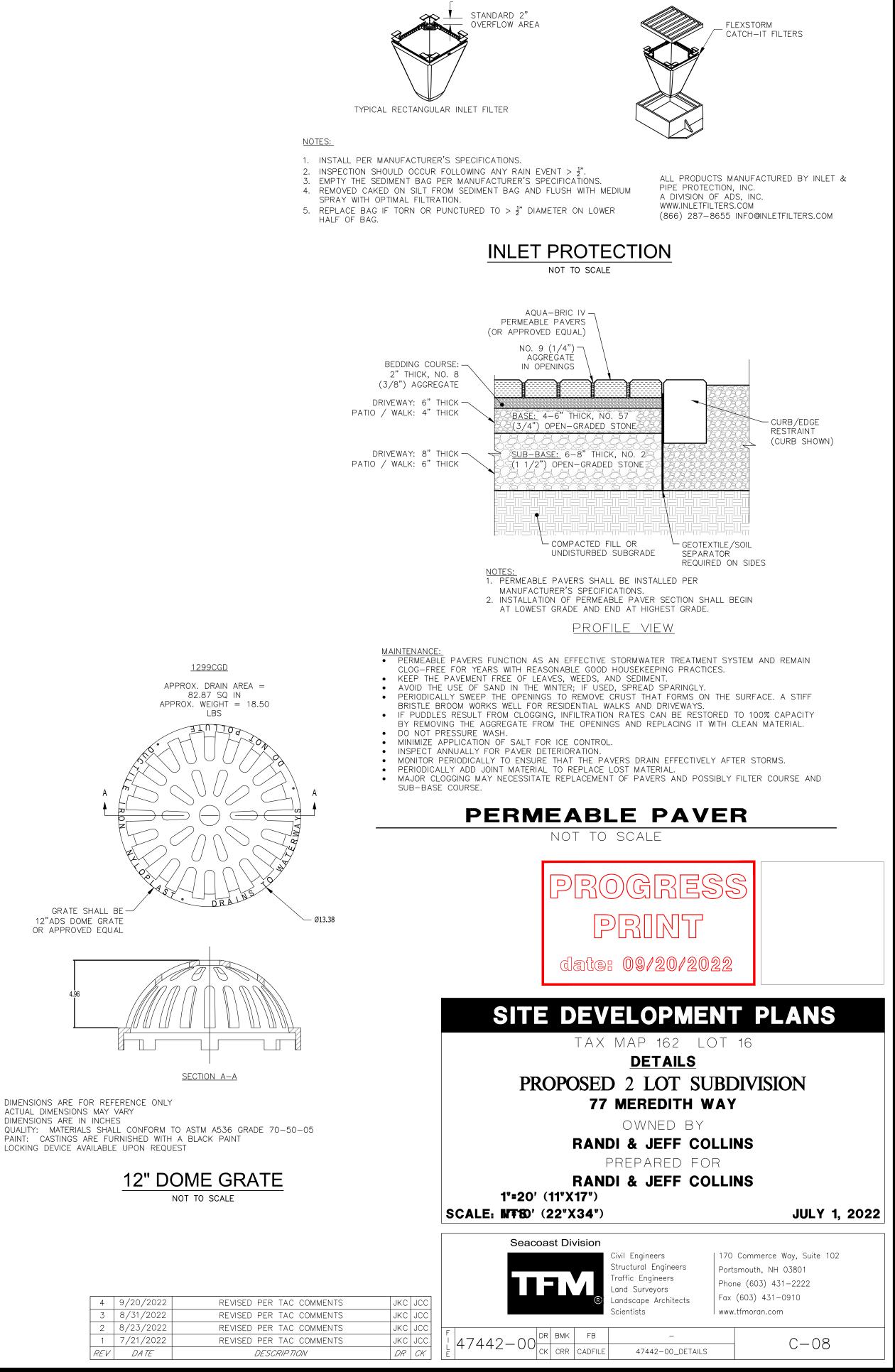


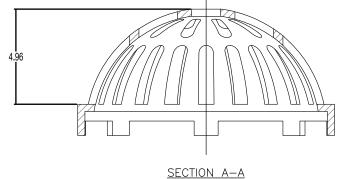
NOTES

- 1. BEDDING BEDDING FOR PIPES SHALL CONSIST OF PREPARING THE BOTTOM OF THE TRENCH TO SUPPORT THE ENTIRE LENGTH OF THE PIPE AT A UNIFORM SLOPE AND ALIGNMENT. CRUSHED STONE SHALL BE USED TO BED THE PIPE TO THE ELEVATION SHOWN ON THE DRAWINGS. NORMAL PIPE BEDDING IS CRUSHED STONE TO THE HAUNCH OF THE PIPE AND SAND BEDDING 6" ABOVE THE CROWN. IF THE TOP OF THE PIPE IS LESS THAN 30" FROM FINISH GRADE, BED PIPE COMPLETELY IN STONE UP TO 6" ABOVE PIPE CROWN. UNDERDRAIN TO HAVE 4" MINIMUM OF STONE OVER PIPE OR AS NECESSARY TO BE IN CONTACT WITH GRAVEL LAYER OF SELECTS ABOVE.
- 2. COMPACTION ALL BACKFILL SHALL BE COMPACTED AT OR NEAR OPTIMUM MOISTURE CONTENT BY PNEUMATIC TAMPERS, VIBRATORY COMPACTORS OR OTHER APPROVED MEANS. BACKFILL BENEATH PAVED SURFACES SHALL BE COMPACTED TO NOT LESS THAN 95% OF AASHTO T99, METHOD C.
- 3. SUITABLE MATERIAL IN ROADS, ROAD SHOULDERS, WALKWAYS AND TRAVELED WAYS, SUITABLE MATERIAL FOR TRENCH BACKFILL SHALL BE THE NATURAL MATERIAL EXCAVATED DURING THE COURSE OF CONSTRUCTION, BUT SHALL EXCLUDE DEBRIS; PIECES OF PAVEMENT; ORGANIC MATTER; TOP SOIL; ALL WET OR SOFT MUCK, PEAT, OR CLAY; ALL EXCAVATED LEDGE MATERIAL; ROCKS OVER 6" IN LARGEST DIMENSION; FROZEN EARTH AND ANY MATERIAL WHICH, AS DETERMINED BY THE ENGINEER, WILL NOT PROVIDE SUFFICIENT SUPPORT OR MAINTAIN THE COMPLETED CONSTRUCTION IN A STABLE CONDITION.
- 4. BASE COURSE AND PAVEMENT SHALL MEET THE REQUIREMENT OF THE NHOOT LATEST EDITION OF THE STANDARD SPECIFICATIONS FOR HIGHWAYS AND BRIDGES DIVISION 300 AND 400 RESPECTIVELY.

TRENCH FOR DRAIN LINE

NOT TO SCALE





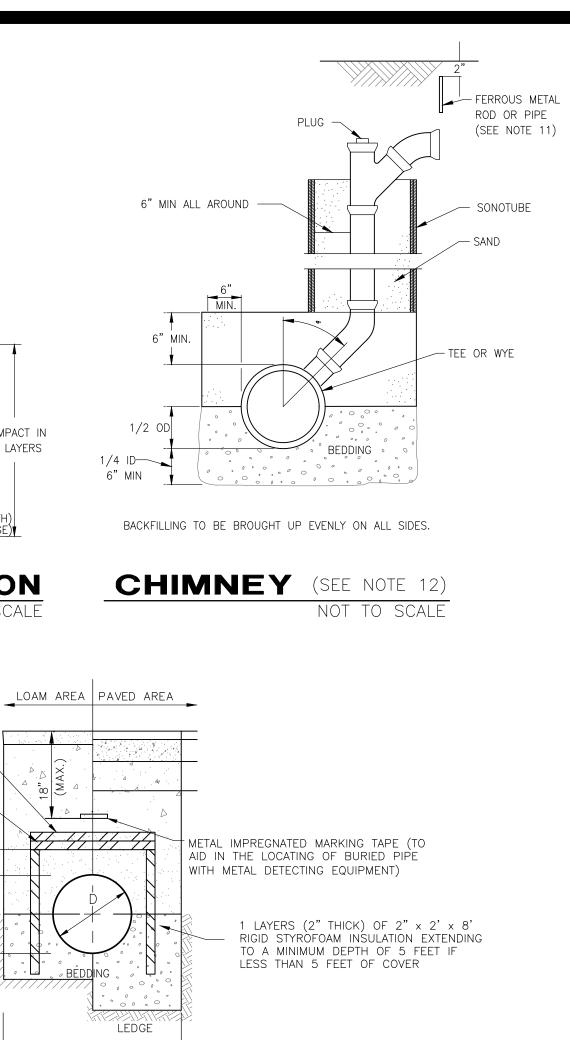
DIMENSIONS ARE IN INCHES

PAINT: CASTINGS ARE FURNISHED WITH A BLACK PAINT LOCKING DEVICE AVAILABLE UPON REQUEST



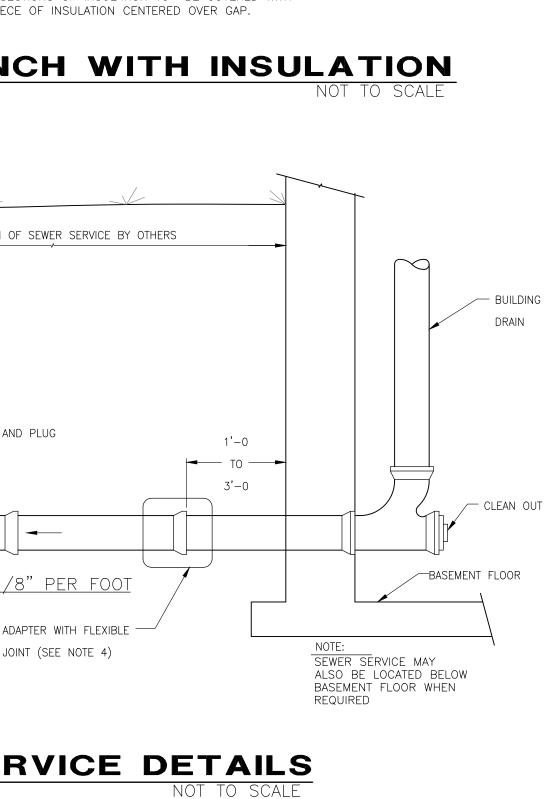
4	9/20/2022	REVISED PER TAC COMMENT
3	8/31/2022	REVISED PER TAC COMMENT
2	8/23/2022	REVISED PER TAC COMMENT
1	7/21/2022	REVISED PER TAC COMMENT
REV	DA TE	DESCRIP TION

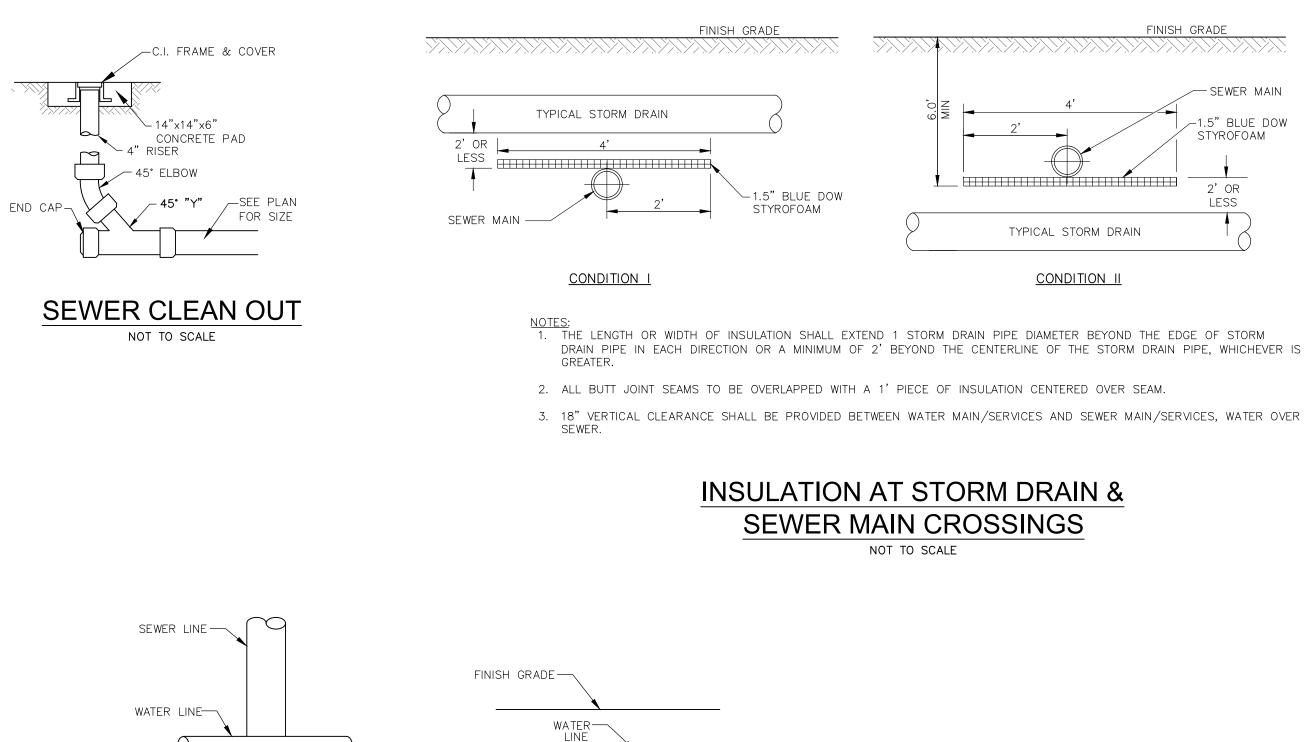
1. MINIMUM SIZE PIPE FOR S	SERVICE ewer service shall be four			
2. PIPE AND JOINT MATERIALS A. PLASTIC SEWER PIPE 1. PIPE AND FITTIN		FOLLOWING ASTM STANDARDS:		
ASTM STANDARDS	GENERIC PIPE MATERIAL	SIZES APPROVED		
D3034 F679 F789	*PVC (SOLID WALL) PVC (SOLID WALL) PVC (SOLID WALL)	8" THROUGH 15" (SDR 35) 18" THROUGH 27" (T-1 & T-2) 4" THROUGH 18" (T-1 TO T-3)	SUITAE	
F794 D2680	PVC (RIBBED WALL) *ABS (COMPOSITES WALL)	8" THROUGH 36" ` 8" THROUGH 15"	,	1' LAYERS
*ABS: ACR	Y VINYL CHLORIDE YLONITRILE-BUTADIENE-STYRENE			
	IATERIAL CONFORMING TO ASTM	ESISTANT COMPRESSION RINGS OF I D-3212 AND SHALL BE PUSH-ON,		
	PE AND FITTINGS SHALL CONFO SHALL BE TO ASTM D—1788 (RM TO ASTM D-2680, POLYMER CLASS 322).	SAND BL	ANKET 12" MIN.
ACCORDANCE V	VITH ASTM D-2680, FORMING A	MICAL WELDED COUPLINGS TYPE SC IN CHEMICAL WELDED JOINT.		сом
	FITTINGS AND JOINTS. PE AND FITTINGS SHALL CONFC THE UNITED STATES OF AMER			6"
A21.50 THI DI		RON PIPE AND WITH ASTM A-536		NG 6, MIN (EART
SA	AND-LINED MOLDS FOR WATER E OF THE MECHANICAL OR PUS		BEDDING TO BE THOROUGHLY	<u>○ , ○ , ○ , ○ , ○ , ○</u> <u>12" MIN∳(LEDG</u> COMPACTED (SEE NOTE 10)
A21.11 RU		ST IRON PRESSURE PIPE & FITTINGS THE JOB SITE.	TRENCH CR	
TIGHTNESS. ALL JOINTS WHERE DIFFERING MATERIA	SHALL BE PROPERLY MATCHED	ASTOMERIC GASKET FOR WATER— WITH THE PIPE MATERIALS USED. AT THE STREET SEWER WYE OR AT APTERS SHALL BE USED.		NOT TO S
APPROPRIATE CONNECTION	SHALL BE MADE, FOLLOWING I	LE IN THE EXISTING STREET SEWER, AN MANUFACTURERS' INSTRUCTIONS USING ED INTO A SMOOTHLY DRILLED OR		
SAWN OPENING IN THE SE HAMMER, STUFFING CLOTH	WER. THE PRACTICE OF BREAKI OR OTHER SUCH MATERIAL AR	NG AN OPENING WITH A SLEDGE OUND THE JOINT, OR APPLYING MORTAR JDE PRACTICES OR INEPT OR HASTY		(4" THICK) OF 2" x 2' x 8' YROFOAM INSULATION IF LESS
SHOWN IN THE DETAIL UP	TO AND INCLUDING 15" DIAME		1 LAYERS RIGID STYRO	(2" 刊枪水) ⁵ 0年57" 名 2 ^C 2V 68 — FOAM INSULATION IF GREATER FEET BUT LESS THAN 6 FEET
ACCORDANCE WITH INSTALL CAREFULLY BEDDED ON A NOTE 10. BEDDING AND R	6 INCH LAYER OF CRUSHED S E-FILL FOR DEPTH OF 12 INCI	LED, PLACED AND JOINTED IN RIATE MANUFACTURER. IT SHALL BE STONE AND/OR GRAVEL AS SPECIFIED IN HES ABOVE THE TOP OF THE PIPE D OR WITH APPROPRIATE MECHANICAL		OF COVER — تو
THE PIPE SHALL BE LAID CONNECTION TO THE FOUN	NDATION AT A GRADE OF NOT L IDER DRY CONDITIONS. IF WATE	ANT GRADE FROM THE STREET SEWER LESS THAN 1/4" INCH PER FOOT. PIPE R IS PRESENT, ALL NECESSARY STEPS	UN	DISTURBED SOIL
TESTING: THE COMPLETED		BJECTED TO A THIRD PARTY LEAKAGE BACKFILLING)		°9
INFLATABLE BLADDER OF THE TEE. AFTER INFLATI	R PLUG SHALL BE INSERTED JU	WN AND WHEN READY FOR TESTING, AN IST UPSTREAM FROM THE OPENING IN CED INTO THE SYSTEM ABOVE THE THE PLUG.		
NEARLY AS POSSIBLE, V Shall be permitted to	VET TRENCH CONDITIONS OR, IF	HOSED WITH WATER, TO SIMULATE, AS F TRENCH IS WET, THE GROUND WATER THE PIPE. INSPECTIONS FOR LEAKS SHLIGHT.		OR <u>NOTES</u> 1. GAPS BETWEEN S
TRENCH IS DRY, THE PI WET, GROUND WATER SI	PE SHALL BE LIBERALLY HOSE	THE TRENCH OVER THE PIPE. IF THE D WITH WATER, OR IF THE TRENCH IS I THE TRENCH OVER THE PIPE. RST DOWN-STREAM MANHOLE.	C E I	2" × 2' × 2' PI
	E PIPE SHALL BE DUG-UP IF	ATE TESTS SHALL BE CAUSE FOR NECESSARY AND RE-LAID SO AS TO	<u>3</u> E	WER TREN
ETC. SHALL BE PERMITTED CONNECTIONS CARRYING R	. ROOF LEADERS, FOOTING DRA	OW FROM TOILETS, SINKS, LAUNDRY AINS, SUMP PUMPS OR OTHER SIMILAR JND WATER SHALL NOT BE PERMITTED.		
	EL AND/OR CRUSHED STONE F	REE FROM CLAY, LOAM, ORGANIC		
100% PASSING 90%-100% PASSING	1 INCH SCREEN		STREET	THIS PORTION
20%-55% PASSING 0%-10% PASSING 0%-5% PASSING	\$		COUNTRY	
		RENCH BASE, SCREENED GRAVEL OR ISED.	CROSS C	- PAVEMENT
MUNICIPAL RECORDS. IN A	CRIBED IN THE TYPICAL "CHIMN	RECORDED AND FILED IN THE D OR PIPE SHALL BE PLACED OVER EY" DETAIL, TO AID IN LOCATING THE	VER -	UM COVER -
CONSTRUCTED FOR THE S		HAN 4 FEET, A CHIMNEY SHALL BE ISTALLATION AS RECOMMENDED BY THE E ENGINEER.	7	OBSERVATION TEE
			YE OR TEE → SEE NOTES 4 & 5)	
				1-95
			SFWFR SFRV	ICE: MINIMUM SLOPE 1
			<u>JLYVLIN JLINV</u>	
			STREET SEWER	
Copyright 2022 ©TFMoran, Inc. 48 Constitution Drive, Bedford,		DIG SAFE		
	s and materials may not be copi- se reproduced in any form whats sion of TFMoran, Inc.		S	SEWER SE
This plan is not effective unless TFMoran, Inc.	s signed by a duly authorized off	icer of		

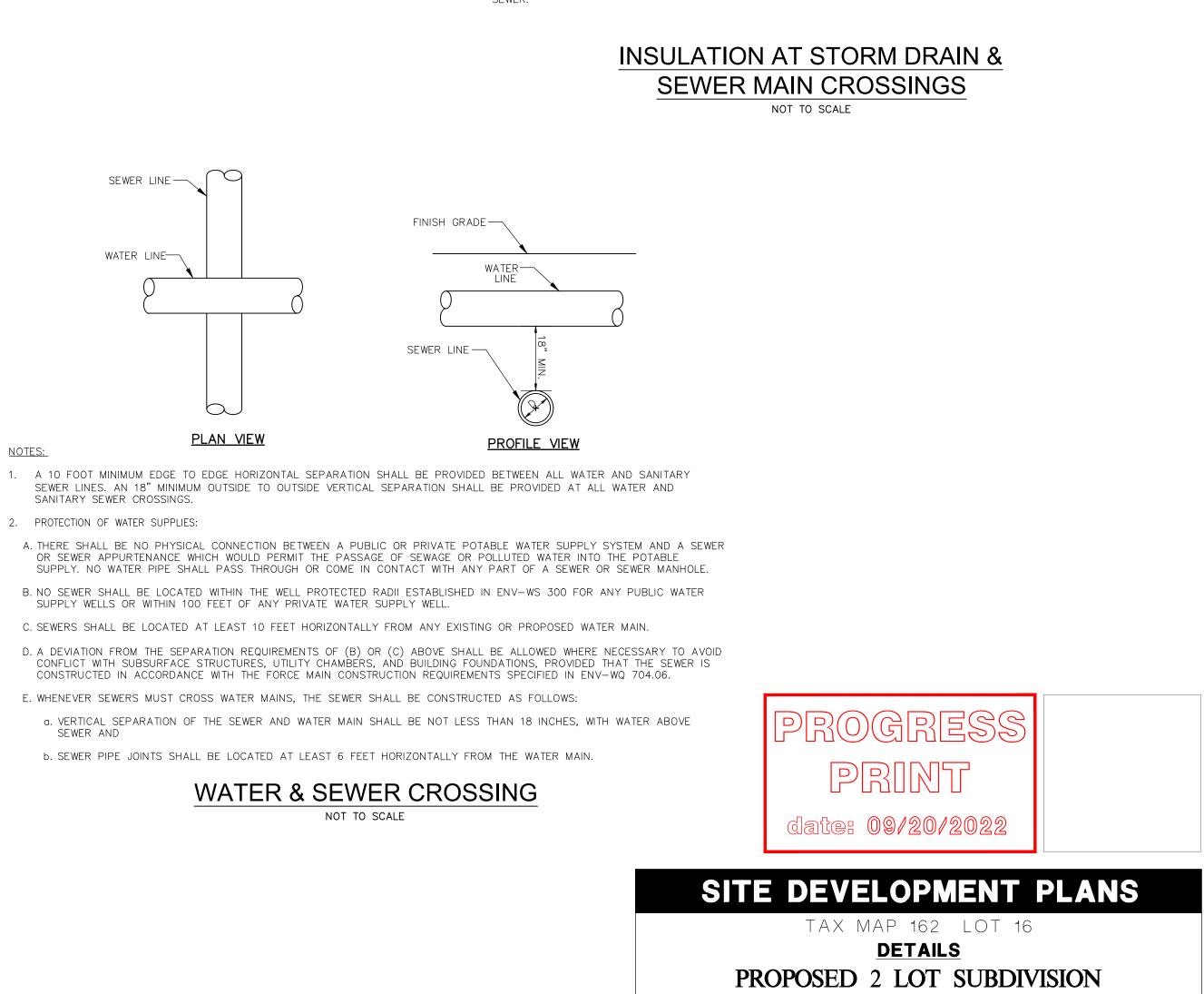


3'-0" MIN. D + 2'(WHICHEVER IS GREATER)

SECTIONS OF INSULATION TO BE COVERED WITH







<u>NOTES:</u>

SANITARY SEWER CROSSINGS.

2. PROTECTION OF WATER SUPPLIES:

SUPPLY WELLS OR WITHIN 100 FEET OF ANY PRIVATE WATER SUPPLY WELL.

4	9/20/2022	REVISED PER TAC COMMENT
3	8/31/2022	REVISED PER TAC COMMENT
2	8/23/2022	REVISED PER TAC COMMENT
1	7/21/2022	REVISED PER TAC COMMENT
REV	DA TE	DESCRIPTION

C-09

| 170 Commerce Way, Suite 102

Portsmouth, NH 03801

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Phone (603) 431-2222

JULY 1, 2022

77 MEREDITH WAY

OWNED BY

RANDI & JEFF COLLINS

PREPARED FOR

RANDI & JEFF COLLINS

Civil Engineers

Traffic Engineers

Land Surveyors

icientists

Structural Engineers

Landscape Architects

47442-00_DETAILS

1"=20' (11"X17")

Seacoast Division

47442-00 dr BMK FB

SCALE: NTTBD' (22"X34")

JKC JCC

JKC JCC

JKC JCC DR CK

DRAINAGE ANALYSIS SUMMARY

FOR

Proposed 2-Lot Subdivision

77 Meredith Way Portsmouth, New Hampshire Rockingham County

Tax Map 162, Lot 16

Owned by Randi & Jeff Collins Prepared for Randi & Jeff Collins

September 20, 2022

Prepared By:



Civil Engineers Structural Engineers Traffic Engineers Land Surveyors Landscape Architects Scientists

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3.0 – EXISTING SITE CONDITIONS	2
4.0 - PRE-DEVELOPMENT CONDITIONS	2
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1.0 - SUMMARY & PROJECT DESCRIPTION

The project includes a subdivision and development of two single family residences on 77 Meredith Way in Portsmouth, NH. The existing Tax Map 162 Lot 16 is approximately .5157 acres and currently contains a single family residence. The site is within the General Residence A Zone and is adjacent to Pine Street Playground.

The proposed project is to construct two 2-story dwellings. Associated improvements include and are not limited to access, grading, utilities, stormwater management system, and landscaping. The project proposes a 2,400 SF and 2,022 SF building footprint and total 6,079 SF of impervious area within the property lines and approximately 26,535 SF of disturbance to facilitate the development.

This analysis has been completed to verify the project will not pose adverse stormwater effects on-site and off-site. Compared to the pre-development conditions, the post-development stormwater management system has been designed to reduce runoff volume, reduces the risk of erosion and sedimentation, and improves stormwater runoff quality. In addition, Best Management Practices are employed to formulate a plan that assures stormwater quality both during and after construction. The following summarizes the findings from the study.

2.0 - CALCULATION METHODS

The design storms analyzed in this study are the 2-year, 10-year, 25year and 50-year 24-hour storm events. The software program, HydroCAD version 10.00¹ was utilized to calculate the peak runoff rates from these storm events. The program estimates the peak rates using the TR-20 method. A Type III storm pattern was used in the model. Rainfall frequencies for the analyzed region were also incorporated into the model. Rainfall frequencies from the higher of the Extreme Precipitation Rates from Cornell University's Northeast Regional Climate Center and Portsmouth Site Plan Review Regulations were used to determine the storm-event intensities, see Table 1. Due to the project's location within the Coastal/Great Bay Region community, the design rainfall increases the Cornell rates by 15% to address projected storm surge, sea level rise, and precipitation events per Env-Wq 1503.08(I). Design standards were taken from the New Hampshire Stormwater Manual, December 2008².

	24-HOUR RAINFALL RATES					
Storm-Event	Northeast Regional Climate Center	Design				
(year)	Extreme Precipitation Rainfall					
	(in)	(in)				
2	3.21	3.70				
10	4.87	5.60				
25	6.17	7.10				
50	7.39 8.50					
Table 1 – 24-Hour Rainfall Rates						

Table 1 – 24-Hour Rainfall Rates

Time of Concentration is the time it takes for water to flow from the hydraulically most remote point in the watershed (with the longest travel time) to the watershed outlet. This time is

¹ HydroCAD version 10.00, HydroCAD Software Solutions LLC, Chocorua, NH, 2013.

² New Hampshire Stormwater Manual: Volume One - Stormwater and Antidegradation, December 2008; Volume Two - Post-Construction Best Management Practices Selection and Design, December 2008; Volume Three Erosion and Sediment Controls During Construction, December 2008.

⁴⁷⁴⁴²⁻⁰⁰ Drainage Analysis Report.docx

determined by calculating the time it takes runoff to travel this route under one of three hydrologic conditions: sheet flow, shallow concentrated flow, or channel flow. Because the Intensity-Duration-Frequency (IDF) curve is steep with short TC's, estimating the actual intensity is subject to error and overestimates actual runoff. Due to this, the TC's are adjusted to a minimum of 6 minutes.

3.0 – EXISTING SITE CONDITIONS

Per NRCS, soils on-site are Group A soils. Based on City comments, as well as test pits & infiltration testing the soils more closely resemble a Group C soil, which is what the drainage analysis is based on.

Four test pits and infiltration tests were conducted. In nearly all test pit locations, fill material was discovered. Infiltration tests were determined per Ksat testing using a Compact Constant Head Permeameter (Amoozemeter) per Env-Wq 1504.14(d). The highest Estimated Seasonal High-Water Table (ESWT) observed were: elevation 32.15' at Proposed Rain Garden #1, and elevation 29.85 at Proposed Rain Garden #2.

4.0 - PRE-DEVELOPMENT CONDITIONS

The pre-development condition is characterized by two subcatchments composing one watershed, which flows towards an existing catch basin, which ultimately discharges to the Piscataqua River. Pre-development subcatchment areas are depicted on the attached plan entitled "Pre-Development Drainage Map," Sheet HSG-01 in Appendix A.

Stormwater runoff from the site primarily infiltrates into the well-drained soils on-site. The remaining stormwater runoff discharges towards a localized pond area to the north of the site (POI-1), and the existing municipal stormwater drainage system (POI-3).

In the pre-development condition, the total impervious area is 20,504 SF over a total drainage analysis area of 91,950 SF.

5.0 - POST-DEVELOPMENT CONDITIONS

The post-development condition is characterized by one watershed divided into three subcatchment areas. Post-development subcatchment areas are depicted on the attached plan entitled "Post-Development Drainage Map," sheet HSG-02 in Appendix B.

In the post-development condition, the total impervious area is 28,118 SF over a total drainage analysis area of 91,950 SF. Impervious area from the project consists of a 7,613 SF footprint across two residences and associated improvements. Two rain gardens are proposed to treat and mitigate the stormwater runoff from the impact of the new impervious area from the proposed development.

Four test pits and infiltration tests, at least one in each basin area, were conducted. In nearly all test pit locations, fill material was discovered. Infiltration tests were determined per default published Ksat values for the design infiltration rates per Env-Wq 1504.14(c) and/or Ksat testing using a Compact Constant Head Permeameter (Amoozemeter) per Env-Wq 1504.14(d).

Table <u>2</u> summarizes the pre- and post-development peak runoff rates for the 2-year, 10-year, 25-year and 50-year 24-hour Type III storm events for all discharge.

Table <u>3</u> summarizes the pre- and post-development peak runoff volumes for the 2-year, 10-year, 25-year, and 50-year 24-hour Type III storm events for all discharge.

TABLE 2 – SURFACE WATER PEAK RUNOFF RATE COMPARISON (CF)							
POINT OF			DESIG	IN STORM			
INTEREST		2-year	10-year	25-year	50-year		
	Pre	1.4	3.5	4.9	6.3		
POI-1	Post	1.2	2.5	3.5	4.5		
	Pre	0.3	0.7	1.0	1.3		
POI-2	Post	0.0	0.0	0.1	0.4		
	Pre	0.8	0.8	0.9	0.9		
POI-3	Post	0.9	0.9	0.9	1.0		

|--|

TABLE 3 – SURFACE WATER PEAK RUNOFF VOLUME COMPARISON (CF)									
POINT OF			DESIGN STORM						
INTEREST		2-year	10-year	25-year	50-year				
	Pre	4,661	9,322	13,242	17,076				
POI-1	Post	4,487	8,930	12,676	16,335				
POI-2	Pre	1,220	2,526	3,615	4,704				
FUI-2	Post	44	435	1,045	1,655				
POI-3	Pre	8,843	16,291	22,346	27,835				
P01-3	Post	7,754	14,375	19,646	24,306				

Table 3 - Pre and Post- Development Peak Runoff Volume Comparison

The proposed project reduces peak rates of runoff compared to existing conditions for all storm events resulting from on-site runoff (POI-1 & POI-2) and Portsmouth stormwater regulations. Additionally, per NHDES, the 2-year 24-hour storm does not result in an increased peak flow rate and reduces or increases volume within the limits of Env-Wq 1507.05(b)(1) from the pre-development to post-development condition. There will be no adverse effects on the abutting properties from the proposed stormwater management system.

6.0 - CONCLUSION

There are three analysis points modeled in the drainage analysis for this project - POI (point on interest) 1 thru 3:

- POI 1 represents the northern portion of the property which discharges to the north. Comparing pre-development to post-development conditions shows that there is a decrease in the peak rate of runoff and volume for all storm events (2 through 50-year storms).

POI 2 represents the southern portion of the property and the associated to discharge off-site (to Meredith Way). Comparing pre-development to post-development conditions shows that there is a decrease in the peak rate of runoff and volume for all storm events (2) through 50-year storms) from the site to Meredith Way.

POI 3 represents the small impoundment/low lying depression area located at the north end of Meredith Way. The outlet from this area is an existing 4" diameter pipe, which DPW has noted may be disconnected. Comparing pre-development to post-development conditions shows that the peak rate of runoff matches for the 25-year storm event, with a minor increase (0.1 cfs) in the 2, 10, and 50-year storm events. The peak elevation of the pond area increases by 0.83 feet in the 2-year storm event down to 0.43 feet in the 50-year storm event. The drain-down time for this impoundment is approximately 15 hours, and the runoff volume is reduced during all storm events (2 through 50-year storms). The increase in peak elevation is due to the extension of the roadway into a portion of the existing impoundment storage. To replace this storage would require removal of the existing mature wooded buffer east of Meredith Way. Since the water elevation increase is minor, of short duration, and there is less runoff volume in the post-development condition, preserving the wooded buffer appears to be the better solution.

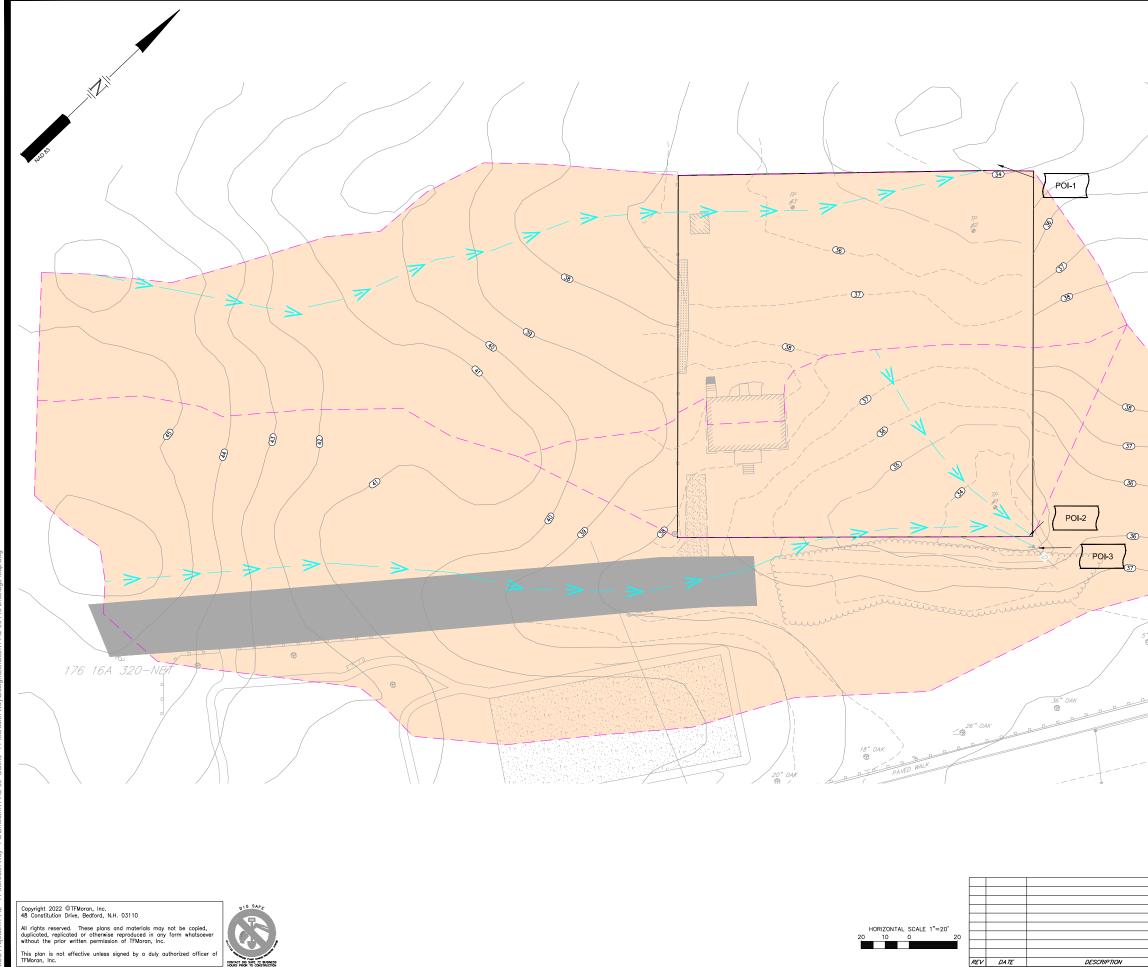
Respectfully, **TFMoran, Inc. Seacoast Division**

Jack McTigue, PE

Project Manager

JJM/crr

<u>APPENDIX A – PRE-DEVELOPMENT DRAINAGE</u> <u>MAP</u>



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00 0000 4.E4 mm

LEGEND _ PROPERTY LINE LIMITS OF DRAINAGE SUBCATCHMENT SOIL GROUP BREAKLINE FLOW PATH (Tc LINE) $\ge - \ge -$ REACH POI-1 POINT OF INTEREST ES-1 SUBCATCHMENT AREA POND, CULVERT, OR CATCH BASIN EP-1 ER-1 REACH LEGEND

HYDROLOGIC SOIL GROUP A

HYDROLOGIC SOIL GROUP B HYDROLOGIC SOIL GROUP C HYDROLOGIC SOIL GROUP D IMPERVIOUS COVER

						OPEN WATER FEATURE
SOIL PHASE LEGEND (PERCENT)						
А	В	С	D	E	F	
0-3	3-8	8-15	15-25	25-50	50+	

SOIL LEGEND (PER USDA NRCS WEB SOIL SURVEY)				
SYMBOL	DESCRIPTION	HYDROLOGIC SOIL GROUP	DRAINAGE CLASS	
799	URBAN LAND-CANTON COMPLEX, 3 TO 15% SLOPES	A	WELL DRAINED	



PRE-DEVELOPMENT HYDROLOGIC SOIL GROUP PLAN PROPOSED 2 LOT SUBDIVISION 77 MEREDITH WAY OWNED BY RANDI & JEFF COLLINS

PREPARED FOR RANDI & JEFF COLLINS 1"=40' (11'X17')

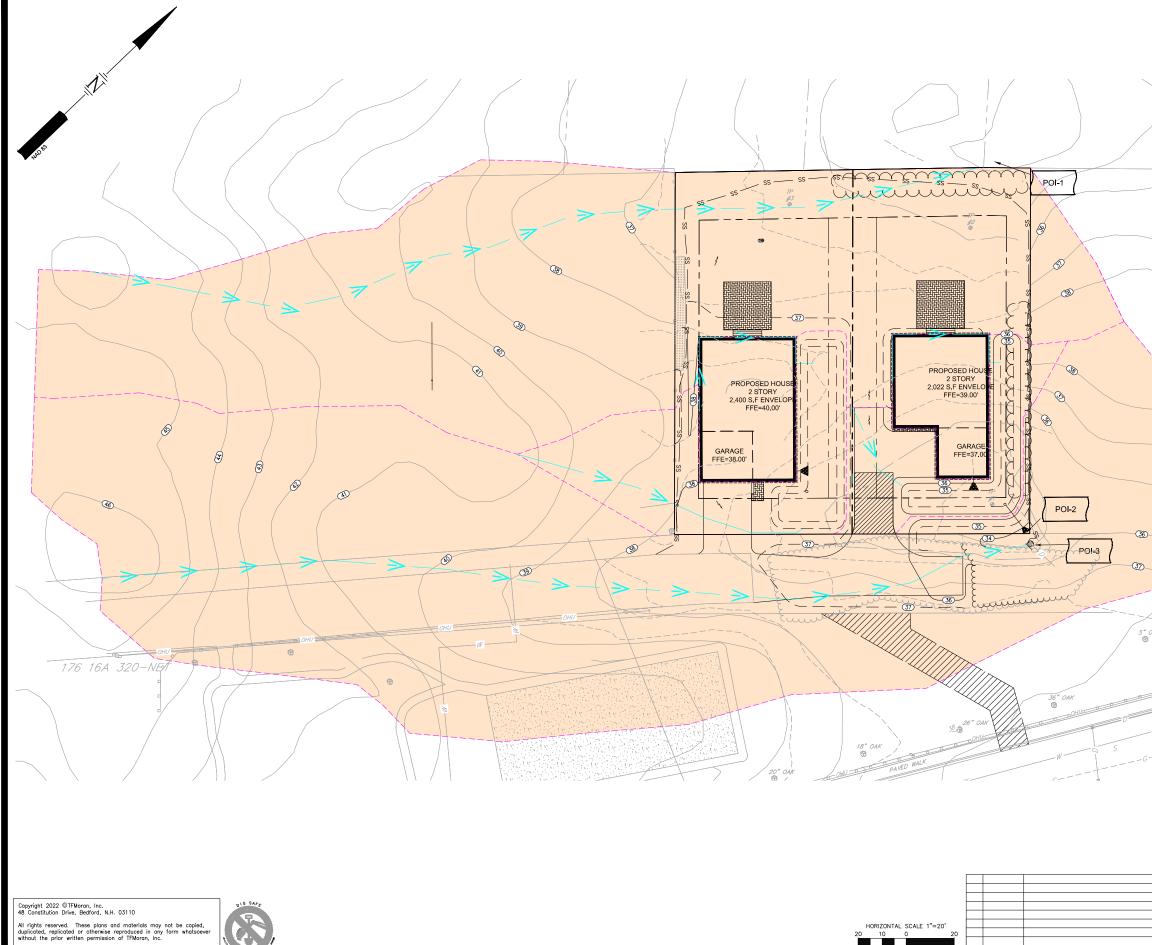
SCALE: 1'=20' (22'X34')





•••••••••						••
Seac			1	Civil Engineers Structural Engineers Traffic Engineers Land Surveyors Landscape Architects Scientists	Port Phor Fax	Commerce Way, Suite 102 smouth, NH 03801 ne (603) 431–2222 (603) 431–0910 .tfmoran.com
47442-00	DR	BMK	FB	-		HSG-01
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<u>APPENDIX B – POST DEVELOPMENT DRAINAGE</u> <u>MAP</u>



his plan is not effective unless signed by a duly authorized

CONTACT DIG SAFE 72 BUSINES

LEGEND _ _ _ PROPERTY LINE LIMITS OF DRAINAGE SUBCATCHMENT SOIL GROUP BREAKLINE > - > - > - > - > -FLOW PATH (Tc LINE) $\rightarrow - \rightarrow -$ REACH

POI-1

PS-1

PP-1 PR-1

POINT OF INTEREST

SUBCATCHMENT AREA

POND, CULVERT, OR CATCH BASIN

REACH

LEGEND

HYDROLOGIC SOIL GROUP A
HYDROLOGIC SOIL GROUP B
HYDROLOGIC SOIL GROUP C
HYDROLOGIC SOIL GROUP D
IMPERVIOUS COVER
OPEN WATER FEATURE

			E LEO	GEND)	1
А	в	E	F		
0-3	3-8	8-15	15-25	25-50	50+

		SOIL LEGE (PER USDA NRCS WEB		Y)
_	SYMBOL	DESCRIPTION	HYDROLOGIC SOIL GROUP	DRAINAGE CLASS
	799	URBAN LAND-CANTON COMPLEX, 3 TO 15% SLOPES	A	WELL DRAINED

SITE DEVELOPMENT PLANS

TAX MAP 162 LOT 16 POST-DEVELOPMENT HYDROLOGIC SOIL GROUP PLAN PROPOSED 2 LOT SUBDIVISION 77 MEREDITH WAY OWNED BY

RANDI & JEFF COLLINS PREPARED FOR RANDI & JEFF COLLINS

1"=40' (11"X17") SCALE: 1'=20' (22'X34')

JULY 1, 2022



DESCRIPTION

REV DATE

5" ORN

	00ALE: 1-20 (22 X04 /		
	Seacoast Division Civil Engineers Structural Engineers Traffic Engineers Land Surveyors Landscope Architects Scientists	Port Phor Fax	Commerce Way, Suite 102 smouth, NH 03801 ne (603) 431-2222 (603) 431-0910 .tfmoran.com
	F 47442-00 DR 80MK FB -		HSG-02
r CK	L 47442-00 CK BRR CADFILE 47442-00 POST DRAINAG	GE MAP	ПЗС-О2

APPENDIX C – HYDROCAD ANALYSIS FOR 1" STORM EVENT

Prepared by {enter your company name here} HydroCAD® 10.10-6a s/n 00866 © 2020 HydroCAD Software Solutions LLC

Summary for Pond RG-01: Rain Garden 1

Inflow Area = 0.180 ac, 38.46% Impervious, Inflow Depth = 0.26" for 1-inch event Inflow = 0.0 cfs @ 12.09 hrs, Volume= 0.004 af Outflow = 0.0 cfs @ 12.09 hrs, Volume= 0.004 af, Atten= 0%, Lag= 0.0 min Discarded = 0.0 cfs @ 12.09 hrs, Volume= 0.004 af Secondary = 0.0 cfs @ 0.00 hrs, Volume= 0.004 af Routed to Reach 5R : Front Lawn 0.000 hrs, Volume= 0.000 af					
			Span= 0.00-100.00 rea= 972 sf Stora	0 hrs, dt= 0.05 hrs ge= 0 cf	
	letention time= (n lass det. time= 0.		ed: outflow precede 7.7 - 807.7)	es inflow)	
Volume	Invert Ava	il.Storage	Storage Descripti	on	
#1	36.33'	1,388 cf			ow (Recalc) -Impervious
#2	35.33'	194 cf		gular)Listed below	v (Recalc) -Impervious
#3	34.33'	389 cf		avel (Irregular)Lis	ted below (Recalc)
		1,971 cf	Total Available S		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
36.33	972	177.0	0	0	972
37.33	1,566	201.0	1,257	1,257	1,718
37.41	1,700	210.0	131	1,388	2,013
Elevation	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>
35.33	972	177.0	0	0	972
36.33	972	177.0	972	972	1,149
Elevation	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>
34.33	972	177.0	0	0	972
35.33	972	177.0	972	972	1,149
	0		et Devices		
		7.40' 80.0 Hear 2.50 Coe	' long x 2.0' brea d (feet) 0.20 0.40 3.00 3.50	0.60 0.80 1.00	ea d Rectangular Weir 1.20 1.40 1.60 1.80 2.00 66 2.70 2.77 2.89 2.88

Discarded OutFlow Max=0.1 cfs @ 12.09 hrs HW=34.33' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.1 cfs)

Secondary OutFlow Max=0.0 cfs @ 0.00 hrs HW=34.33' TW=37.40' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir(Controls 0.0 cfs)

Summary for Pond RG-02: Rain Garden 2

0.306 ac, 46.57% Impervious, Inflow Depth = 0.15" for 1-inch event Inflow Area = 0.0 cfs @ 12.10 hrs, Volume= Inflow = 0.004 af 0.0 cfs @ 12.05 hrs, Volume= Outflow = 0.004 af, Atten= 87%, Lag= 0.0 min Discarded = 0.0 cfs @ 12.05 hrs, Volume= 0.004 af 0.0 cfs @ 0.00 hrs, Volume= 0.000 af Primary = Routed to Link PPOI2 : PPOI2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-100.00 hrs, dt= 0.05 hrs Peak Elev= 31.70' @ 12.88 hrs Surf.Area= 620 sf Storage= 49 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 61.5 min (877.4 - 815.8)

Volume	Inv	ert Ava	il.Storage	Storage Description	on		
#1	35.0	00'	1,245 cf	Pond Storage (In	regular)Listed bel	ow (Recalc) -Impervic	ous
#2	33.	50'	186 cf	•		v (Recalc) - Impervious	S
#3	31.	50'	496 cf	930 cf Overall x 2 Pea Stone (Irregu	ular)Listed below	(Recalc)	
			1 007 of	1,240 cf Overall x			
			1,927 cf	Total Available St	orage		
Elevatio	n	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(fee	t)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
35.0	0	620	219.9	0	0	620	
36.00		1,308	238.7	943	943	1,343	
36.2	0	1,728	270.0	303	1,245	2,611	
Elevatio	n	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(fee		(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
33.5	/	620	219.9	0	0	620	
35.0		620	219.9	930	930	950	
Elevatio	n	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(fee		(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
31.5	1	620	219.9	0	0	620	
33.5		620	219.9	1,240	1,240	1,060	
Device	Routing	In	vert Outl	et Devices			
#1	Primary			Round Culvert			
L= 15.0' CPP, square edge headwall, Ke= 0.500							
Inlet / Outlet Invert= 33.25' / 33.00' S= 0.0167 '/' Cc= 0.900							
n= 0.013 Corrupted PE smooth interior Elow Area= 0.09 sf							

n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.09 sf

47442-00_Drainage-C SOILS-Small Pond C

Prepared by {enter your company name here} HydroCAD® 10.10-6a s/n 00866 © 2020 HydroCAD Software Solutions LLC

#2	Discarded	31.50'	0.400 in/hr Exfiltration over Horizontal area

35.80' **12.0" Horiz. Grate** C= 0.600 Limited to weir flow at low heads #3 Device 1

#4 Device 1 35.08' **0.5" Vert. Orifice 2** C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.0 cfs @ 12.05 hrs HW=31.56' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=31.50' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Controls 0.0 cfs)

3=Grate (Controls 0.0 cfs) **4=Orifice 2** (Controls 0.0 cfs)