II. OLD BUSINESS

C. The request of Cynthia Austin Smith and Peter (Owners), for property located at 9 Kent Street whereas relief is needed to demolish the existing two-family and construct a single-family dwelling which requires the following: 1) Variances from Section 10.521 to allow a) a lot area and lot area per dwelling of 5,000 square feet where 7,500 square feet is required for each; b) 53% building coverage where 25% is the maximum allowed; c) a 4.5 foot rear yard where 20' is required; d) a 0.5 foot side yard where 10 feet is required; e) a 0 foot front yard where 11 feet is allowed under Section 10.516.10; and f) a 9.5 foot secondary front yard where 13 feet is allowed under Section 10.516.10. 2) A Variance from Section 10.515.14 to allow a 1.5 foot setback for a mechanical unit where 10 feet is required. Said property is located on Assessor Map 113 Lot 42 and lies within the General Residence A (GRA) District. (LU-23-28)

	Existing	Proposed	Permitted / Required	
Land Use:	Two- family	Demo structure and construct new single unit	Primarily residential	
<u>Lot area (sq. ft.)</u> :	5,000	5,000	7,500	min.
Lot Area per Dwelling Unit (sq. ft.):	5,000	5,000	7,500	min.
Street Frontage (ft.):	50'+	50' +	100	min.
Lot depth (ft.)	100	100	70	min.
Primary Front Yard (ft.):	7	0	11 (using front yard averaging in 10.516.10)	min.
Secondary Front Yard (ft.):	16	9.5	13 (using front yard averaging in 10.516.10)	min.
Right Yard (ft.):	0.5	0.5	10	min.
Rear Yard (ft.):	6	4.5	20	min.
<u>Height (ft.):</u>	<35	35	35	max.
Building Coverage (%):	35	53	25	max.
Open Space Coverage (%):	63.5	33	30	min.
<u>Parking</u>	0	2 (2 car garage)	2	
Estimated Age of Structure:	1900	Variance request(s) shown in red.	

Existing & Proposed Conditions

Other Permits/Approvals Required

Building Permit

Neighborhood Context



Previous Board of Adjustment Actions

July 19, 1988 – Relief from Zoning Ordinance including:

Variance from Article III, Section 10-302 are requested: a) construction of 4' x 20' rear egress stairs from the second floor to rear yard with 33% building lot coverage in a district where the maximum building lot coverage allowed is 20% and b) construction of said stairs with a 2 $\frac{1}{2}$ ' right yard where a 10' side yard is the minimum in this district. The Board voted to **grant** the request as advertised.

Planning Department Comments

The applicant is requesting relief from multiple dimensional standards to demolish the existing structure, construct a single living unit, and add new backyard features.

For this project, the complete demolition of the existing structure creates a vacant lot and will require relief for both the proposed structure dimensions that do not meet zoning and for the non-conforming dimensions of the lot. See Section 10.311 copied below for reference.

10.311 Any lot that has less than the minimum lot area or street frontage required by this Ordinance shall be considered to be nonconforming, and no use or structure shall be established on such lot unless the Board of Adjustment has granted a variance from the applicable requirements of this Ordinance.

At the request of staff, applicant presented the project to the Technical Advisory Committee at a Work session for the review of public infrastructure impacts as they relate to the project.

May 8 Update:

On May 8, 2023 the applicants submitted a revised foundation plan after receiving imput from the abutter. The changes are summarized in the letter dated May 10, 2023 from Jennifer Ramsay and are as follows: the right side of the structure will be 3 feet narrower on the basement level, the new building will be the same in every other aspect and dimension.

Variance Review Criteria

This application must meet all five of the statutory tests for a **variance** (see Section 10.233 of the Zoning Ordinance):

- 1. Granting the variance would not be contrary to the public interest.
- 2. Granting the variance would observe the spirit of the Ordinance.
- 3. Granting the variance would do substantial justice.
- 4. Granting the variance would not diminish the values of surrounding properties.
- 5. The "unnecessary hardship" test:
 - (a) The property has <u>special conditions</u> that distinguish it from other properties in the area. **AND**
 - (b) <u>Owing to these special conditions</u>, a fair and substantial relationship does not exist between the general public purposes of the Ordinance provision and the specific application of that provision to the property; and the proposed use is a reasonable one. OR

<u>Owing to these special conditions</u>, the property cannot be reasonably used in strict conformance with the Ordinance, and a variance is therefore necessary to enable a reasonable use of it.

10.235 Certain Representations Deemed Conditions

Representations made at public hearings or materials submitted to the Board by an applicant for a special exception or variance concerning features of proposed buildings, structures, parking or uses which are subject to regulations pursuant to Subsection 10.232 or 10.233 shall be deemed conditions upon such special exception or variance.

Hoefle, Phoenix, Gormley & Roberts, pllc

ATTORNEYS AT LAW

127 Parrott Avenue | Portsmouth, NH, 03801 Telephone: 603.436.0666 | Facsimile: 603.431.0879 | www.hpgrlaw.com

March 1, 2023

HAND DELIVERED

Peter Stith, Principal Planner Portsmouth City Hall 1 Junkins Avenue Portsmouth, NH 03801

Re: Peter Smith & Cynthia Austin Smith, Owners/Applicants 9 Kent Street Tax Map 113/Lot 42

Dear Mr. Stith & Zoning Board Members:

On behalf of Peter Smith and Cynthia Austin Smith, Owners/Applicants, enclosed please find the following in support of a request for zoning relief:

- Digital Application submitted via Viewpoint earlier today.
- Owner Authorization.
- 3/1/2023 Memorandum and exhibits in support of variance application.

We look forward to presenting this application to the Zoning Board at its March 21, 2023 meeting.

Very truly yours,

R. Timothy Phoenix Monica F. Kieser

Encl.

cc: Peter Smith & Cynthia Austin Smith
 John Chagnon, Ambit Engineering, Inc. (email)
 Jennifer Ramsey, Somma Studios (email)
 Robbi Woodburn, Woodburn & Associates (email)

DANIEL C. HOEFLE R. TIMOTHY PHOENIX LAWRENCE B. GORMLEY STEPHEN H. ROBERTS R. PETER TAYLOR ALEC L. MCEACHERN KEVIN M. BAUM JACOB J.B. MARVELLEY

GREGORY D. ROBBINS PETER V. DOYLE MONICA F. KIESER DUNCAN A. EDGAR STEPHANIE J. JOHNSON OF COUNSEL: SAMUEL R. REID JOHN AHLGREN

OWNER'S AUTHORIZATION

We, Peter Smith and Cynthia Austin Smith, Owners/Applicants of 9 Kent Street, Tax Map 113/Lot 42, hereby authorize law firm Hoefle, Phoenix, Gormley & Roberts, PLLC to represent us before any and all City of Portsmouth Representatives, Boards and Commissions for permitting the project.

Respectfully submitted,

Date:

Cynthia Austin Smith

Date:

MEMORANDUM

TO: FROM:	Portsmouth Zoning Board of Adjustment ("ZBA") R. Timothy Phoenix, Esquire Monica F. Kieser, Esquire
DATE: RE:	March 1, 2023 Owners/Applicants: Peter Smith & Cynthia Austin Smith Property: 9 Kent Street Tax Map 113, Lot 42 General Residence District

Dear Chairman Eldridge and Members of the Zoning Board of Adjustment ("ZBA"):

On behalf of Owners/Applicants Peter Smith & Cynthia Austin Smith ("Smith"), we are pleased to submit this Memorandum and exhibits in support of the requested variances from the Portsmouth Zoning Ordinance ("PZO" or "Ordinance").

I. <u>EXHIBITS</u>

- A. <u>Redevelopment Plan Set</u> Ambit Engineering, Somma Studios, Woodburn & Associates
- B. <u>Average Grade Exhibit</u> Ambit Engineering, Inc.
- C. Front Setback Exhibit Ambit Engineering, Inc
- D. <u>Stormwater Report</u> Ambit Engineering, Inc.
- E. <u>Renderings</u> Tangram.
- F. <u>Site Photographs</u>.
 - Satellite Views
 - Kent Street & Rockland Street Views
- G. <u>Tax Map 113</u>.

II. <u>PROPERTY/PROJECT</u>

9 Kent Street is a 5,000 s.f. (50 ft. wide and 100 ft. deep) corner lot in the General Residence District containing a side-by-side duplex (1,075 s.f.), deck and porch (together 315 s.f.) and one car garage (296 s.f.), but no driveway or off street parking (the "Property"). The home, AC unit, and garage are in the right side yard setback, the garage is in the rear yard setback, and a portion of the existing front deck is slightly within the front yard setback. Smith intends to raze the existing two-family home in favor of a contemporary take on a single-family New Englander (1,353 s.f.) with a lower level garage accessed from Rockland, front/rear porches, and a pervious outdoor living area surrounded by a landscape wall (the "Project"). The outdoor living area an above-grade pervious patio, soaking pool, and pergola. The Project is accompanied by robust landscaping and screening, as well as implementation of a stormwater management system supported by a drainage analysis.

At the outset, Smith engaged their southerly neighbors ("Mikolaites") to discuss their proposed plans, specifically, the interplay between Smith's proposed landscape wall, the installation, and Mikolaites' fence along their mutual boundary. Smith's team also reviewed the Project on two separate occasions with City Staff. At Staff's suggestion, Smith's Engineer sought a work session with the Technical Advisory Committee ("TAC") to discuss the curb cut on Rockland Street, drainage and other aspects of the redevelopment proposal. The latest plan set incorporates feedback received from TAC. In addition, Woodburn & Associates met with the City's Trees and Greenery Committee to review landscaping elements.

After consideration, City Staff have determined that dimensional relief is required for lot area, lot size/dwelling unit, building coverage, and elements within the principal front on Kent, secondary front on Rockland, side, and rear yard setbacks. Notably, the building coverage percentage sought includes the outdoor living area, because Staff have applied the recent zoning amendments related to calculation of building height, which calculates building height in relation to existing average existing grade around the patio. Because the proposed pervious patio is 18 inches above existing grade to provide infiltration and accommodate drainage infrastructure, it is considered a structure for purposes of building coverage calculations. Similarly, any area in the front/side and rear yards where the height of the landscaping wall or pool fencing exceeds 4 feet/6 feet from existing grade, relief from yard setback regulations is required.

III. ADDITIONAL PERMITS REQUIRED

- Demolition Permit ZBA Application will serve this purpose.
- Driveway Permit
- Stormwater Permit
- Building Permit

[RELIEF CHART FOLLOWS]

III. **RELIEF REQUIRED**

Variance Section/Requirement	Existing	Proposed
PZO §10.520/Table §10.521: Dimensional Standards 7,500 s.f. Lot area 7,500 s.f. Lot area/dwelling unit	5,000 s.f. 2,500 s.f./dwelling	No change to lot size 5,000 s.f./dwelling (improved)
PZO §10.520/Table §10.521: Dimensional Standards 10' Front Yard Kent St. ¹ 13' Front Yard Rockland St. ²	Kent: 7.5' (steps) 9.2' (front deck) 17.3' (house) Rockland: 15.7' (house)	Kent: 0' (landscape wall) 6.5' (steps) 9.3' (porch) 14.3' (house) Rockland: 1.0' (landscape wall) 12.5' (steps) 9.7' (overhang) 15.3' (house)
PZO §10.520/Table §10.521: Dimensional Standards 10' Side Yard	0.7' (house) 1.7' (garage)	0.6' (house) 0.5' (landscape wall/pergola) 1.5' (AC unit) 11.5' (pool equipment pad)
PZO §10.520/Table §10.521: Dimensional Standards 20' Rear Yard ³	5.6' (garage)	 4.5' (landscape wall) 4.5' (6 ft. privacy fence/pool)⁴ 10.3' (pool equipment pad)
PZO §10.520/Table §10.521: Dimensional Standards 25% Building Coverage	35%	53% (includes pervious patio 18" above grade)

¹ In accordance with PZO §10.516, 11 ft. is the average front setback derived from lots within 200 ft. on the east side of Kent Street.

² In accordance with PZO §10.516, 13 ft. is the average front setback derived from lots within 200 ft. on the south side of Rockland Street.

³ Staff has differing opinions about whether easterly lot line is a rear lot line, or a side lot line (given the two front yards). We request relief from the more stringent requirement in an abundance of caution. ⁴ Relief is requested for the fence because it will be 6ft. <u>above</u> the patio which is elevated above average grade.

IV. VARIANCE REQUIREMENTS

1. <u>The variances will not be contrary to the public interest.</u>

2. <u>The spirit of the ordinance is observed.</u>

The first step in the ZBA's analysis is to determine whether granting a variance is not contrary to the public interest and is consistent with the spirit and intent of the ordinance, considered together pursuant to <u>Malachy Glen Associates</u>, Inc. v. Town of Chichester, 155 N.H. 102 (2007) and its progeny. Upon examination, it must be determined whether granting a variance "would unduly and to a marked degree conflict with the ordinance such that it violates the ordinance's basic zoning objectives." <u>Id</u>. "Mere conflict with the zoning ordinance is not enough." *Id*.

The purpose of the Portsmouth Zoning Ordinance as set forth in PZO §10.121 is "to promote the health, safety and the general welfare of Portsmouth and its region in accordance with the City of Portsmouth Master Plan... [by] regulating":

- 1. <u>The use of land, buildings and structures for business, industrial, residential and</u> <u>other purposes</u> – The Project proposes a permitted single family home where a duplex presently exists, so redevelopment is more conforming compared to existing conditions.
- 2. <u>The intensity of land use, including lot sizes, building coverage, building height</u> <u>and bulk, yards and open space</u> – The lot is nonconforming as to lot size, density, building coverage, and yards. The Project decreases density by removing one dwelling unit and reconstructs a new home, AC unit and landscaping elements with yard setbacks similar to what exists today.
- 3. <u>The design of facilities for vehicular access, circulation, parking and loading</u> No driveway presently exists, a driveway from Rockland will connect to a below grade garage.
- 4. <u>The impacts on properties of outdoor lighting, noise, vibration, stormwater runoff</u> <u>and flooding</u> – The Project utilizes pervious materials and a Stormwater Management Plan supported by drainage calculations to ensure there is no increase in stormwater runoff. TAC has reviewed the Project and feedback received from TAC informs the current design.
- 5. <u>The preservation and enhancement of the visual environment</u> The Project will replace an aging duplex with a tasteful new home and beautiful landscaping/screening.
- 6. <u>The preservation of historic districts, and buildings and structures of historic or</u> <u>architectural interest</u> – The Property is outside Historic District.
- The protection of natural resources, including groundwater, surface water, wetlands, wildlife habitat and air quality – Redevelopment of the Property has no impact compared to existing conditions.

Based upon the foregoing, then variances do not "in a marked degree conflict with the ordinance such that they violate the ordinance's basic zoning objectives." <u>Malachy Glen, supra</u>, which also held:

One way to ascertain whether granting the variance would violate basic zoning objectives is to examine whether it would <u>alter the</u> <u>essential character of the locality</u>.... Another approach to [determine] whether granting the variance violates basic zoning objectives is to examine whether granting the variance would <u>threaten the public health, safety or welfare</u>. (emphasis added)

The intent of the GRA District is to provide single-family, two-family, or multi-family homes in moderate to high densities (5-12 units/acre) with appropriate accessory uses. The Property is located in thickly settled area of the City. The Project's tasteful single-family home decreases density and provides interior parking, while also affording Smith a secluded outdoor living area surrounded by trees and shrubs. In conjunction with robust plantings, the Project's Stormwater Management Plan ensures stormwater is treated on the lot. Accordingly, granting each requested variance will neither "alter the essential character of the locality," nor "threaten the public health, safety or welfare."

3. Granting the variance will not diminish surrounding property values.

The Project replaces an aging duplex and garage with a tastefully designed single-family home essentially matching the existing yard setbacks, while the defined outdoor living space was developed in consultation with the immediate abutters. Front setback relief is minimal and elements within the rear yard setback will be screened from view by a row of evergreens. In light of these factors, granting the requested variances will not diminish surrounding property values.

4. Denial of the variances results in an unnecessary hardship.

a. <u>Special conditions distinguish the property/project from others in the area.</u>

The Property is small and narrow, with its northeasterly corner sloping toward South Mill Playground; it has no driveway. Although a corner lot, access for a driveway is limited on the Rockland Street side because the public way is not completely paved. These circumstances combine to create special conditions.

b. <u>No fair and substantial relationship exists between the general public purposes of the ordinance and its specific application in this instance</u>.

Lot area, density limits, and building coverage requirements prevent overburdening of the land and overcrowding. Yard setback requirements provide sightlines, promote adequate air, light, space, separation between neighbors and, in conjunction with coverage requirements, ensure adequate space for stormwater treatment. The Project decreases density compared to existing conditions and replaces the existing duplex with a single-family home in a similar location. The outdoor living space is screened by a wall and evergreens and comprised of pervious materials. The Project's drainage analysis and Stormwater Management Plan ensure no increase in stormwater runoff. Accordingly, there is no fair and substantial relationship between the general public purposes of the PZO and its specific application in this instance.

c. <u>The proposed use is reasonable.</u>

If the use is permitted, it is deemed reasonable. <u>Vigeant v. Hudson</u>, 151 N.H. 747 (2005). Residential uses are permitted in the GRA Zone. The Project decreases density and tastefully redevelops an existing lot of record. Accordingly, the proposed use is reasonable and denial of the requested variances would create an unnecessary hardship.

5. Substantial justice will be done by granting the variance.

If "there is no benefit to the public that would outweigh the hardship to the applicant" this factor is satisfied. <u>Harborside Associates, L.P. v. Parade Residence Hotel, LLC</u>, 162 N.H. 508 (2011). That is, "any loss to the [applicant] that is not outweighed by a gain to the general public is an injustice." <u>Malachy Glen, supra</u> at 109.

Applicant is constitutionally entitled to the use of the lot as they see fit; including redevelopment of the Property for a permitted single-family home with an incorporated garage, subject only to its effect on density, coverage requirements, and yard setbacks. "The right to use and enjoy one's property is a fundamental right protected by both the State and Federal Constitutions." N.H. CONST. pt. I, arts. 2, 12; U.S. CONST. amends. V, XIV; <u>Town of Chesterfield v. Brooks</u>, 126 N.H. 64 (1985) at 68. Part I, Article 12 of the New Hampshire Constitution provides in part that "no part of a man's property shall be taken from him, or applied to public uses, without his own consent, or that of the representative body of the people." Thus, our State Constitutional protections limit the police power of the State and its municipalities in their regulation of the use of property. <u>L. Grossman & Sons, Inc. v. Town of Gilford</u>, 118 N.H. 480, 482 (1978). "Property" in the constitutional sense has been interpreted to mean not the tangible property itself, <u>but rather the right to possess, use, enjoy and dispose of it. Burrows v.</u>

<u>City of Keene</u>, 121 N.H. 590, 597 (1981). (emphasis added). The Supreme Court has held that zoning ordinances must be reasonable, not arbitrary and must rest upon some ground of difference having fair and substantial relation to the object of the regulation. <u>Simplex</u> <u>Technologies, Inc. v. Town of Newington</u>, 145 N.H. 727, 731 (2001); <u>Chesterfield</u> at 69.

Granting the requested variances allows for tasteful redevelopment of an existing lot of record in a manner consistent with the surrounding area. Given that the Project's home and patio/pergola match existing yard setbacks; the front relief is slight; the rear yard includes a screen of evergreen trees; and stormwater is infiltrated into the pervious patio, there is no benefit to the public from denial. Conversely, Smith will be greatly harmed by denial as they will lose the opportunity to redevelop the Property as desired. Accordingly, there is no benefit to the public that outweighs the harm to the owner from denial.

VI. <u>CONCLUSION</u>

For all the reasons stated, Smith respectfully requests that the Portsmouth Zoning Board of Adjustment grant each variance.

Respectfully submitted, Peter Smith & Cynthia Austin Smith

By:

R. Timothy Phoenix, Esquire Monica F. Kieser, Esquire



P. 603.766.3760

Date: May 8, 2023

To: City of Portsmouth Planning Department

RE: Zoning Board of Adjustments Submission Updates to 9 Kent Street

This brief is to explain the changes submit on May 8, 2023 for consideration at the Zoning Board of Adjustments hearing on May 16, 2023.

The proposed right-side foundation wall has been moved in 36" from its original location. Above grade, no changes from our original submission will be evident. This change was made in order to alleviate abutter concerns of below grade construction at the property line.

Thank you, Jennifer Ramsey SOMMA Studios **OWNER & APPLICANT:** CYNTHIA AUSTIN SMITH & PETER SMITH 9 KENT STREET PORTSMOUTH, NH 03801 (617) 803–2109

CIVIL ENGINEER & LAND SURVEYOR:

AMBIT ENGINEERING, INC. 200 GRIFFIN ROAD, UNIT 3 PORTSMOUTH, N.H. 03801 TEL. (603) 430–9282 FAX (603) 436-2315

ATTORNEY:

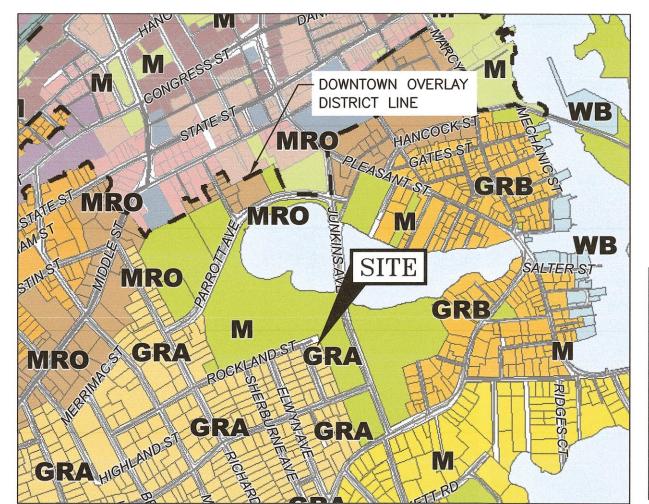
HOEFLE, PHOENIX, GORMLEY & ROBERTS, PLLC 127 PARROTT AVENUE PORTSMOUTH, NH 03801 TEL. (603) 436-0666 FAX (603) 431-0879

LANDSCAPE ARCHITECT:

WOODBURN & COMPANY LANDSCAPE ARCHITECTURE, LLC 103 KENT PLACE NEWMARKET, N.H. 03857 TEL. (603) 659—5949 FAX (603) 659-5939

ARCHITECT:

SOMMA STUDIOS 30 MAPLEWOOD AVENUE PORTSMOUTH NH 03801 TEL. (617) 766–3760 FAX (617) 766-3761

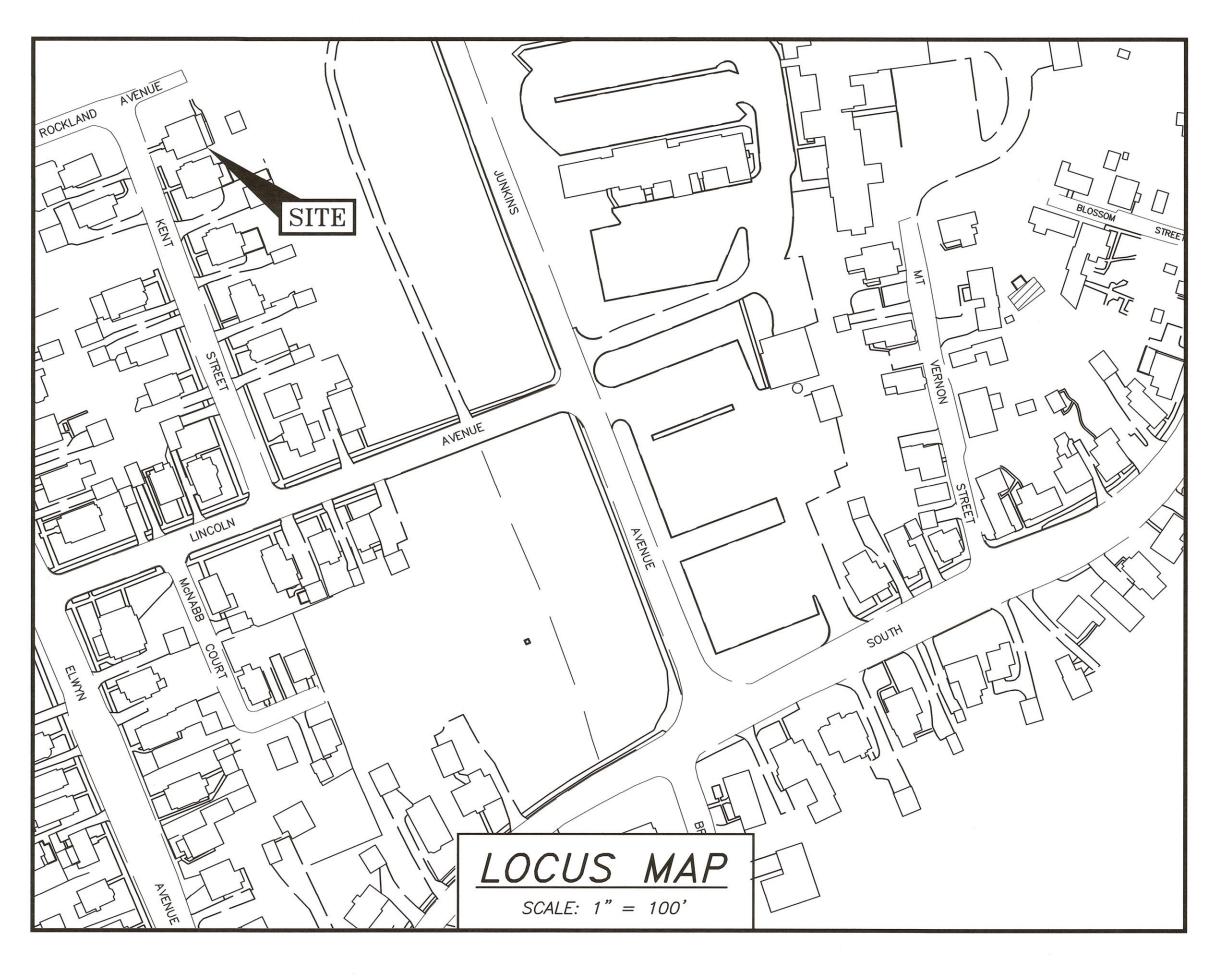


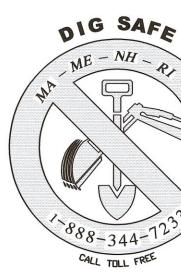
Le	gend	
(Refer	r to Zonin	stricts er-Based Zoning Area g Map Sheet 2 of 2 icts Regulating Plan)
Resid	dential I	Districts
	R	Rural
	SRA	Single Residence A
	SRB	Single Residence B
	GRA	General Residence A
	GRB	General Residence B
	GRC	General Residence C
	GA/MH	Garden Apartment/Mobile Home Park

INDEX OF SHEETS

<u>DWG No.</u>	
	STANDARD BOUNDARY AND TOPOGRAPHIC SU
C1	DEMO PLAN
C2	SITE PLAN
L1	LANDSCAPE PLAN
C3	DRAINAGE & GRADING PLAN
C4	OFF SITE GRADING
C5	UTILITY PLAN
P1	SITE SECTIONS
D1-D3	DETAILS
_	PROPOSED ELEVATIONS
_	PROPOSED FLOOR PLANS

STRUCTURE REPLACEMENT SMITH RESIDENCE 9 - 11 KENT STREET, PORTSMOUTH, NEW HAMPSHIRE **PERMIT PLANS**





UTILITY CONTACTS

ELECTRIC: EVERSOURCE

1700 LAFAYETTE ROAD PORTSMOUTH, N.H. 03801 Tel. (603) 436-7708, Ext. 555.5678 ATTN: MICHAEL BUSBY, P.E. (MANAGER)

SEWER & WATER: PORTSMOUTH DEPARTMENT OF PUBLIC WORKS 680 PEVERLY HILL ROAD PORTSMOUTH, N.H. 03801 Tel. (603) 427-1530 ATTN: JIM TOW

NATURAL GAS:

UNITIL 325 WEST ROAD PORTSMOUTH, N.H. 03801 Tel. (603) 294-5144 ATTN: DAVE BEAULIEU

COMMUNICATIONS: FAIRPOINT COMMUNICATIONS JOE CONSIDINE 1575 GREENLAND ROAD GREENLAND, N.H. 03840 Tel. (603) 427-5525

CABLE: COMCAST 155 COMMERCE WAY PORTSMOUTH, N.H. 03801 Tel. (603) 679-5695 (X1037) ATTN: MIKE COLLINS

SURVEY

Exhibit A

PERMIT LIST: PORTSMOUTH ZONING BOARD: PENDING PORTSMOUTH DRIVEWAY PERMIT: PENDING PORTSMOUTH TREES AND GREENERY: PENDING

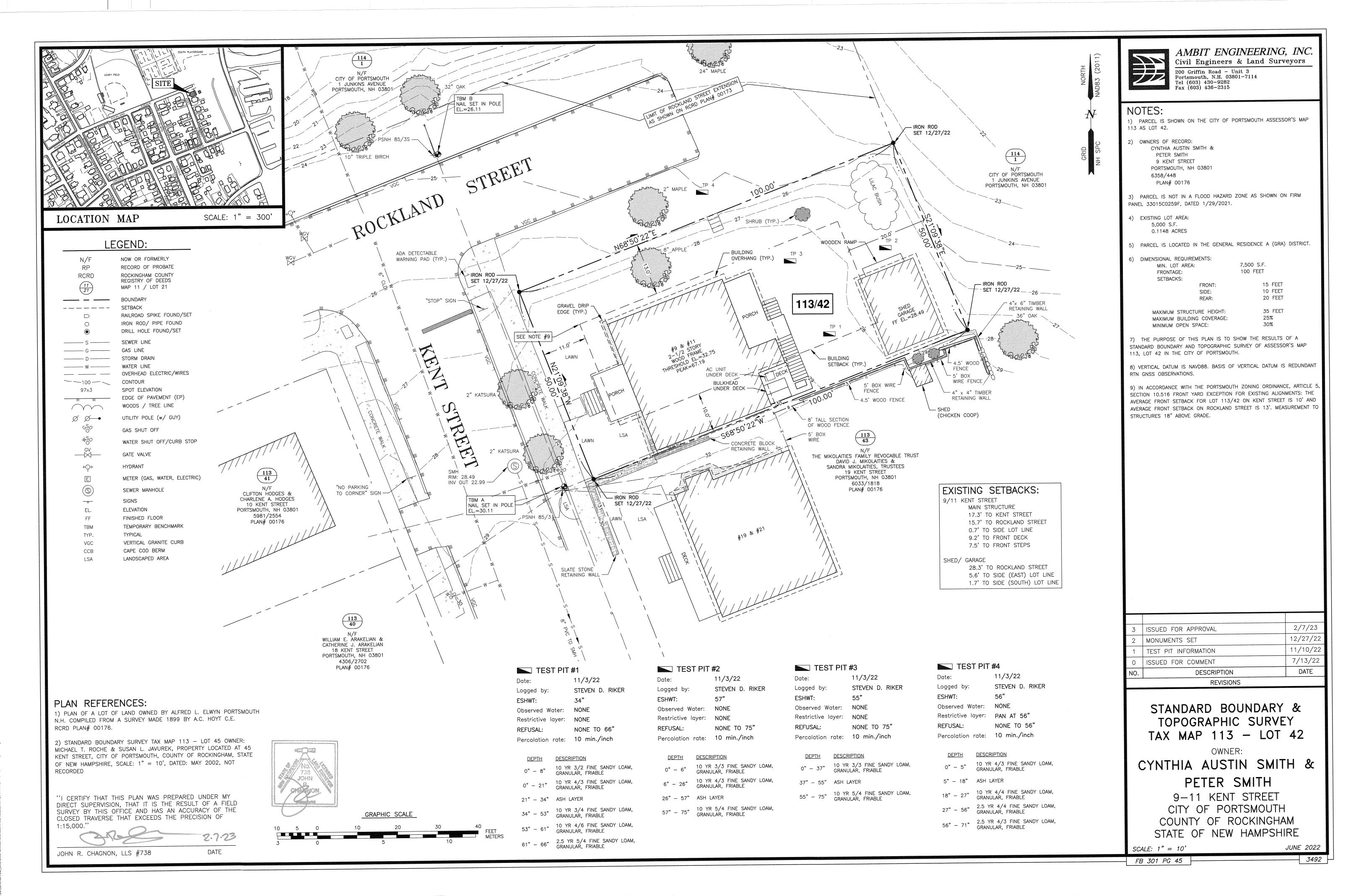
LEGEND:

EXISTING	PROPOSED	
		PROPERTY LINE SETBACK
S	S	SEWER PIPE
SL	SL	SEWER LATERAL
G D	G D	GAS LINE STORM DRAIN
— w —	W	WATER LINE
WS UGE	WS UGE	WATER SERVICE UNDERGROUND ELECTRIC
—— онw ——	OHW	OVERHEAD ELECTRIC/WIRES
	UD	FOUNDATION DRAIN
		EDGE OF PAVEMENT (EP)
	100 98x0	CONTOUR SPOT ELEVATION
-0-		UTILITY POLE
-Ŏ- ''I''	- <u></u> - '////	WALL MOUNTED EXTERIOR LIGHTS
		TRANSFORMER ON CONCRETE PAD
	\bigcirc	ELECTRIC HANDHOLD
120 GS0	NSO GSO	SHUT OFFS (WATER/GAS)
\bowtie	GV	GATE VALVE
	+ + +	HYDRANT
CB	CB	CATCH BASIN
\bigcirc	SMH	SEWER MANHOLE
0	DMH	DRAIN MANHOLE
\bigcirc	O TMH	TELEPHONE MANHOLE
(14)	14)	PARKING SPACE COUNT
PM		PARKING METER
LSA	$\begin{array}{cccc} & \psi & \psi & \psi & \psi \\ \psi & \psi & \psi & \psi \\ \psi & \psi &$	LANDSCAPED AREA
TBD	TBD	TO BE DETERMINED
CI COP	CI COP	CAST IRON PIPE COPPER PIPE
DI	DI	DUCTILE IRON PIPE
PVC	PVC	POLYVINYL CHLORIDE PIPE
RCP AC	RCP —	REINFORCED CONCRETE PIPE ASBESTOS CEMENT PIPE
VC	VC	VITRIFIED CLAY PIPE
EP EL.	EP EL.	EDGE OF PAVEMENT ELEVATION
FF	FF	FINISHED FLOOR
INV	INV	
S = TBM	S = TBM	SLOPE FT/FT TEMPORARY BENCH MARK
TYP	TYP	TYPICAL

STRUCTURE REPLACEMENT SMITH RESIDENCE 9 - 11 KENT STREET PORTSMOUTH, N.H.

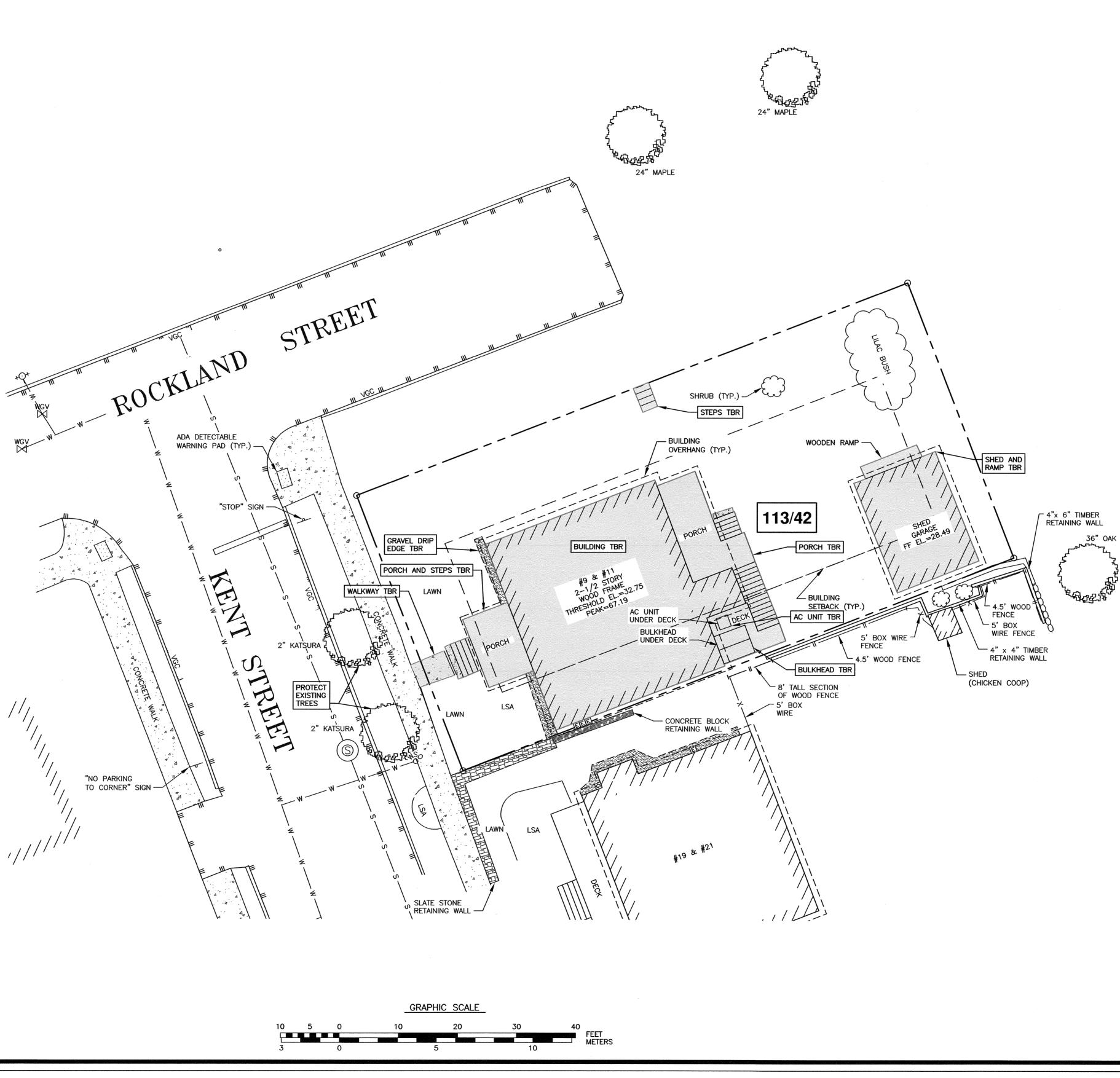


PLAN SET SUBMITTAL DATE: 8 MAY 2023



DEMOLITION NOTES

- A) THE LOCATIONS OF UNDERGROUND UTILITIES ARE APPROXIMATE AND THE LOCATIONS ARE NOT GUARANTEED BY THE OWNER OR THE DESIGNER. IT IS THE CONTRACTORS' RESPONSIBILITY TO LOCATE UTILITIES AND ANTICIPATE CONFLICTS. CONTRACTOR SHALL REPAIR EXISTING UTILITIES DAMAGED BY THEIR WORK AND RELOCATE EXISTING UTILITIES THAT ARE REQUIRED TO BE RELOCATED PRIOR TO COMMENCING ANY WORK IN THE IMPACTED AREA OF THE PROJECT.
- B) ALL MATERIALS SCHEDULED TO BE REMOVED SHALL BECOME THE PROPERTY OF THE CONTRACTORS UNLESS OTHERWISE SPECIFIED. THE CONTRACTOR SHALL DISPOSE OF ALL MATERIALS OFF-SITE IN ACCORDANCE WITH ALL FEDERAL, STATE, AND LOCAL REGULATIONS, ORDINANCES AND CODES. THE CONTRACTOR SHALL COORDINATE REMOVAL, RELOCATION, DISPOSAL, OR SALVAGE OF UTILITIES WITH THE OWNER AND APPROPRIATE UTILITY COMPANY.
- C) ANY EXISTING WORK OR PROPERTY DAMAGED OR DISRUPTED BY CONSTRUCTION / DEMOLITION ACTIVITIES SHALL BE REPLACED OR REPAIRED TO THE ORIGINAL EXISTING CONDITIONS BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER.
- D) THE CONTRACTOR SHALL VERIFY LOCATION OF ALL EXISTING UTILITIES AND CALL DIG SAFE AT LEAST 72 HOURS PRIOR TO THE COMMENCEMENT OF ANY DEMOLITION/CONSTRUCTION ACTIVITIES.
- E) SAWCUT AND REMOVE PAVEMENT ONE FOOT OFF PROPOSED EDGE OF PAVEMENT TRENCH IN AREAS WHERE PAVEMENT IS TO BE REMOVED.
- F) IT IS THE CONTRACTOR'S RESPONSIBILITY TO FAMILIARIZE THEMSELVES WITH THE CONDITIONS OF ALL THE PERMIT APPROVALS.
- G) THE CONTRACTOR SHALL OBTAIN AND PAY FOR ADDITIONAL CONSTRUCTION PERMITS, NOTICES AND FEES NECESSARY TO COMPLETE THE WORK AND ARRANGE FOR AND PAY FOR ANY INSPECTIONS AND APPROVALS FROM THE AUTHORITIES HAVING JURISDICTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL AND OFF-SITE DISPOSAL OF MATERIALS REQUIRED TO COMPLETE THE WORK.
- H) THE CONTRACTOR SHALL REMOVE AND DISPOSE OF ALL EXISTING STRUCTURES, CONCRETE, UTILITIES, VEGETATION, PAVEMENT, AND CONTAMINATED SOIL WITHIN THE WORK LIMITS SHOWN UNLESS SPECIFICALLY IDENTIFIED TO REMAIN. ANY EXISTING DOMESTIC / IRRIGATION SERVICE WELLS IN THE PROJECT AREA IDENTIFIED DURING THE CONSTRUCTION AND NOT CALLED OUT ON THE PLANS SHALL BE BROUGHT TO THE ATTENTION OF THE OWNER AND ENGINEER FOR PROPER CAPPING / RE-USE.
- ALL WORK WITHIN THE CITY OF PORTSMOUTH RIGHT OF WAY SHALL BE COORDINATED WITH THE CITY OF PORTSMOUTH DEPARTMENT OF PUBLIC WORKS (DPW).
-) REMOVE TREES AND BRUSH AS REQUIRED FOR COMPLETION OF WORK. CONTRACTOR SHALL GRUB AND REMOVE ALL SLUMPS WITHIN LIMITS OF WORK AND DISPOSE OF OFF-SITE IN ACCORDANCE WITH FEDERAL, STATE, AND LOCAL LAWS AND REGULATIONS.
- K) CONTRACTOR SHALL PROTECT ALL PROPERTY MONUMENTATION THROUGHOUT DEMOLITION AND CONSTRUCTION OPERATIONS. SHOULD ANY MONUMENTATION BE DISTURBED, THE CONTRACTOR SHALL EMPLOY A NH LICENSED LAND SURVEYOR TO REPLACE THEM.
-) PROVIDE INLET PROTECTION BARRIERS AT ALL CATCH BASINS WITHIN CONSTRUCTION LIMITS AND MAINTAIN FOR THE DURATION OF THE PROJECT. INLET PROTECTION BARRIERS SHALL BE HIGH FLOW SILT SACK BY ACF ENVIRONMENTAL OR APPROVED EQUAL. INSPECT BARRIERS WEEKLY AND AFTER EACH RAIN OF 0.25 INCHES OR GREATER. CONTRACTOR SHALL COMPLETE A MAINTENANCE INSPECTION REPORT AFTER EACH INSPECTION. SEDIMENT DEPOSITS SHALL BE REMOVED AFTER EACH STORM EVENT OR MORE OFTEN IF WARRANTED OR FABRIC BECOMES CLOGGED. EROSION CONTROL MEASURES SHALL BE INSTALLED PRIOR TO THE START OF ANY CLEARING OR DEMOLITION ACTIVITIES.
- THE CONTRACTOR SHALL PAY ALL COSTS NECESSARY FOR TEMPORARY PARTITIONING, BARRICADING, FENCING, SECURITY AND SAFELY DEVICES REQUIRED FOR THE MAINTENANCE OF A CLEAN AND SAFE CONSTRUCTION SITE.
- N) ANY CONTAMINATED MATERIAL REMOVED DURING THE COURSE OF THE WORK WILL REQUIRE HANDLING IN ACCORDANCE WITH NHDES REGULATIONS. CONTRACTOR SHALL HAVE A HEALTH AND SAFETY PLAN IN PLACE, AND COMPLY WITH ALL APPLICABLE PERMITS, APPROVALS, AUTHORIZATIONS, AND REGULATIONS





AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors 200 Griffin Road - Unit 3

Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315

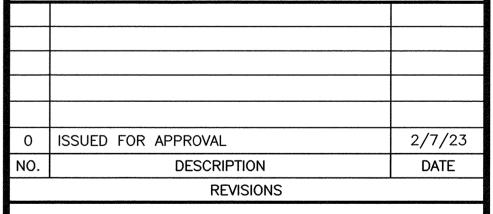
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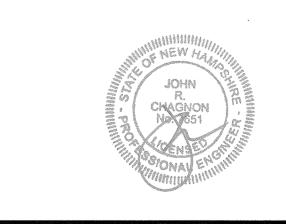
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SMITH RESIDENCE 9 KENT STREET PORTSMOUTH, N.H.





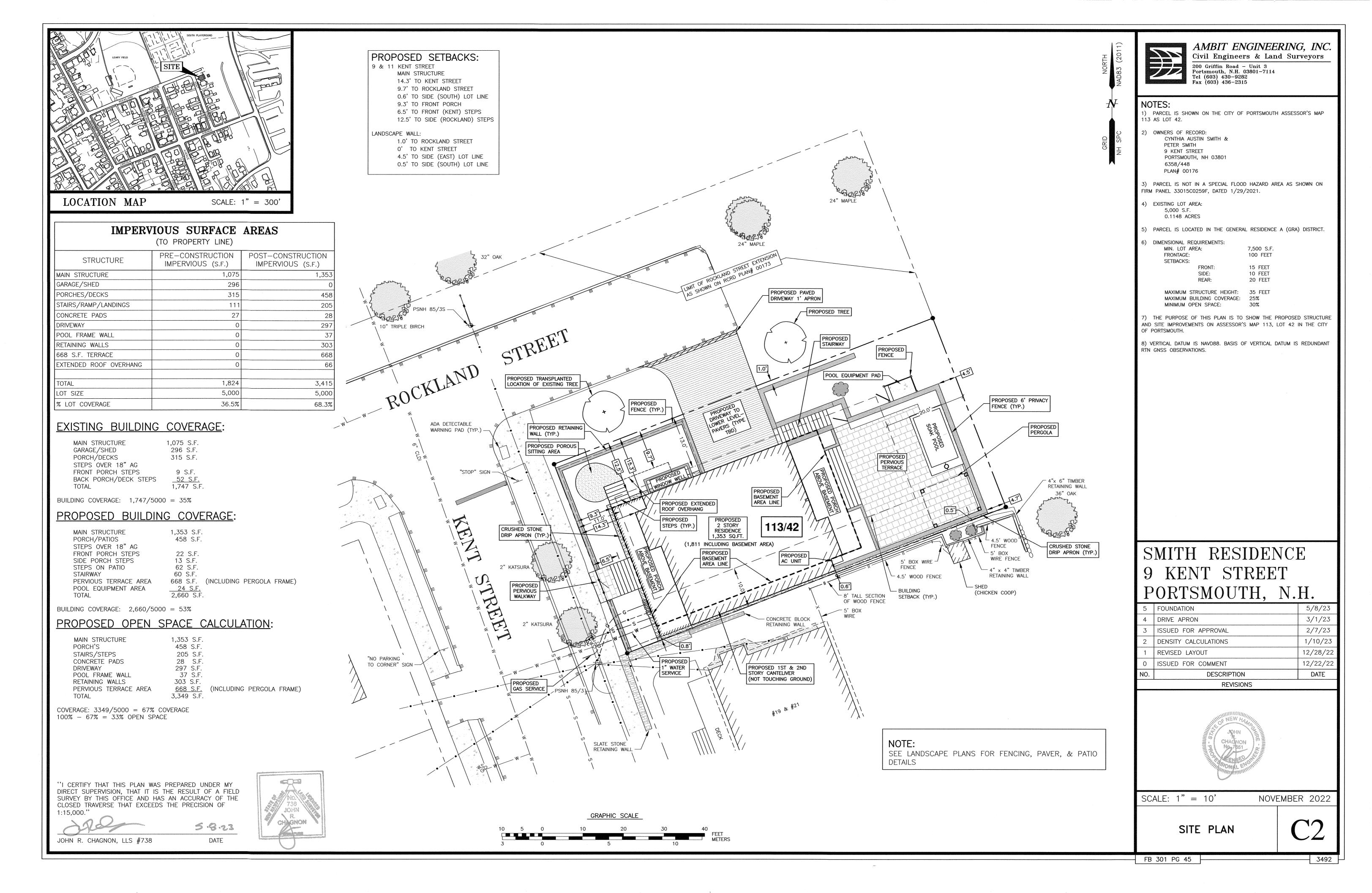
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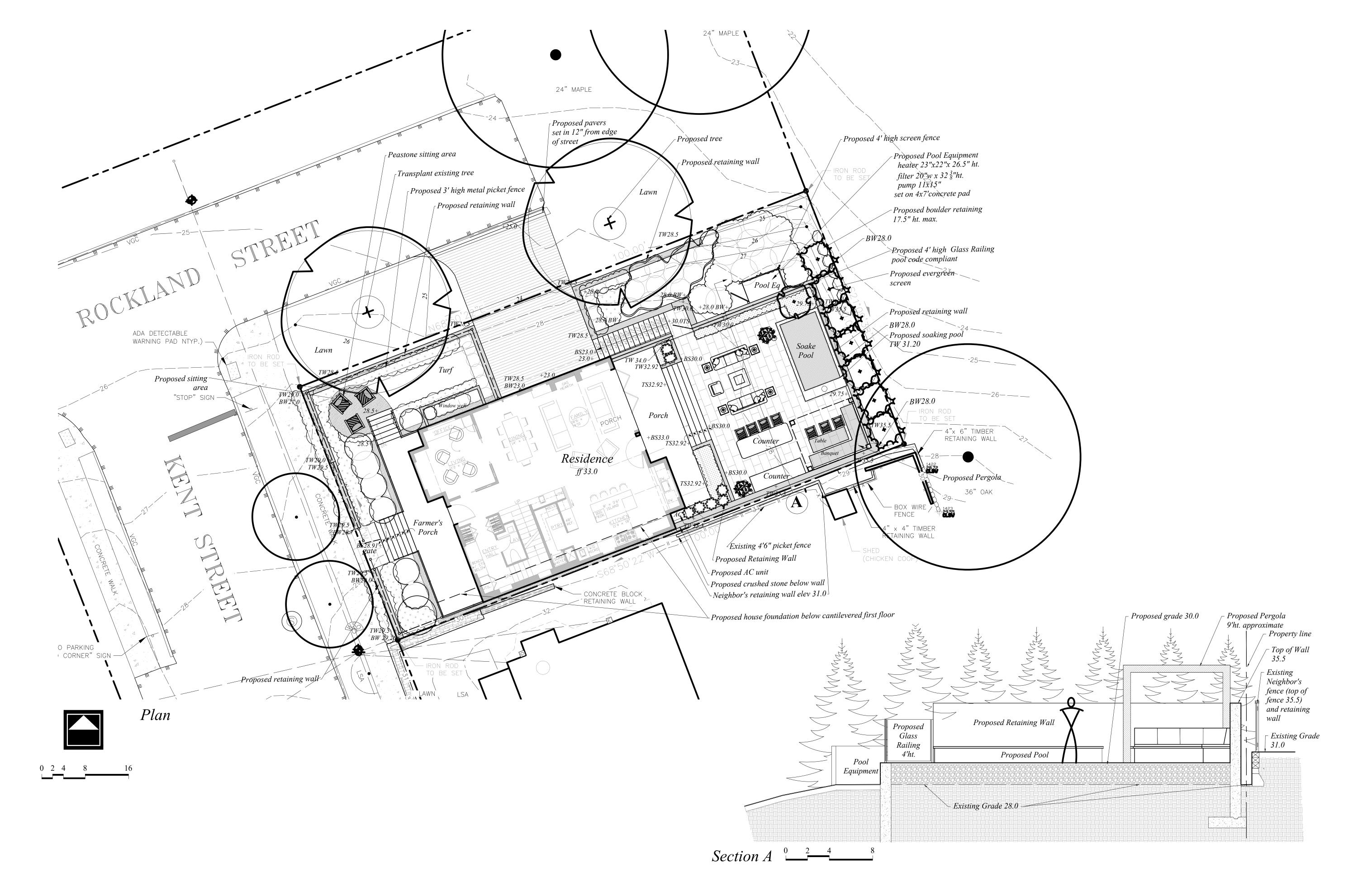
DEMO PLAN

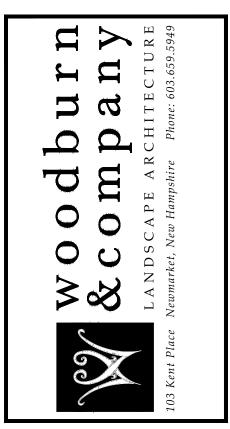
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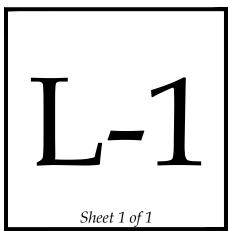




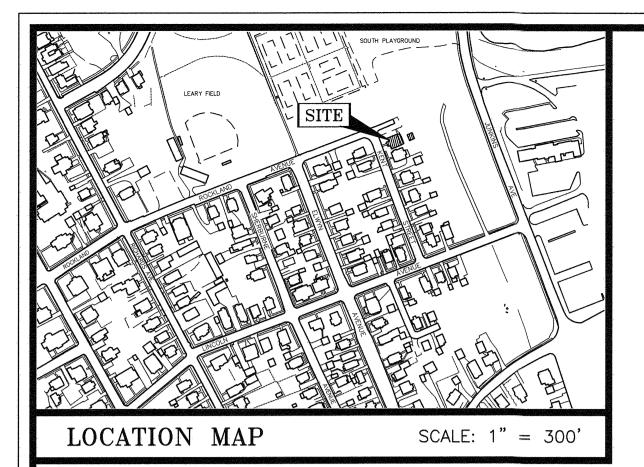
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Scale:	scale
Date:	2023-02-16 issued for ZBA
Revisions:	2023-05-07
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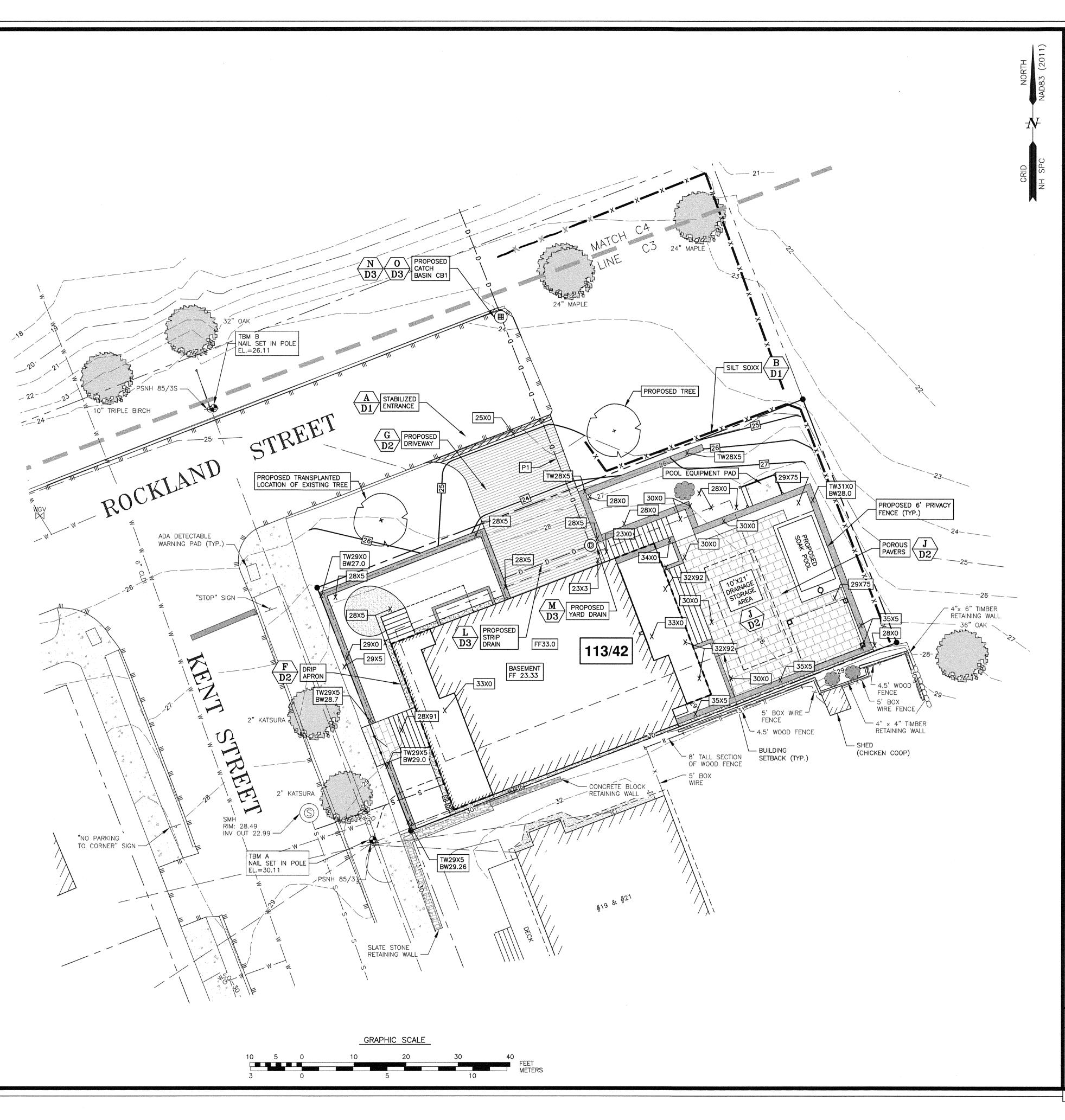


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DRAINAGE STRUCTURE TABLE						
STRUCTURE	PROP/EX	RIM	INVERT IN	INVERT OUT	SUMP	
YARD DRAIN PROP 23.0 21.75 STRIP DRAIN 20.0 HDPE						
CB1	PROP	23.8	19.84	19.74	16.74	
DMH1	PROP	18.5 +/-	14.94	14.50		
DMH 5398	EX	10.6	(12") 7.20	(54") 2.70		

DRAINAGE PIPE SCHEDULE							
PIPE #	PIPE # SIZE LENGTH INVERT IN INVERT OUT SLOPE						
P1	12" HDPE	38'	20	19.84	0.004		
P2	12" HDPE	24'	19.74	14.60	0.20		
P3	12"HDPE	100'	14.50	7.20	0.073		





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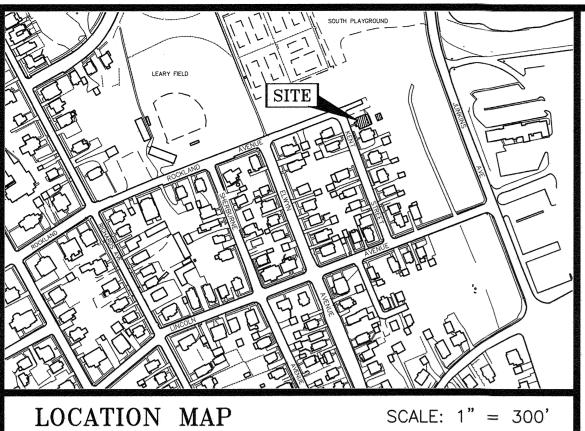
DRAINAGE	3/1/23
ISSUED FOR APPROVAL	2/7/23
DESCRIPTION	DATE
REVISIONS	
	ISSUED FOR APPROVAL DESCRIPTION



SCALE: 1" = 10'

DRAINAGE & GRADING PLAN FEBRUARY 2023

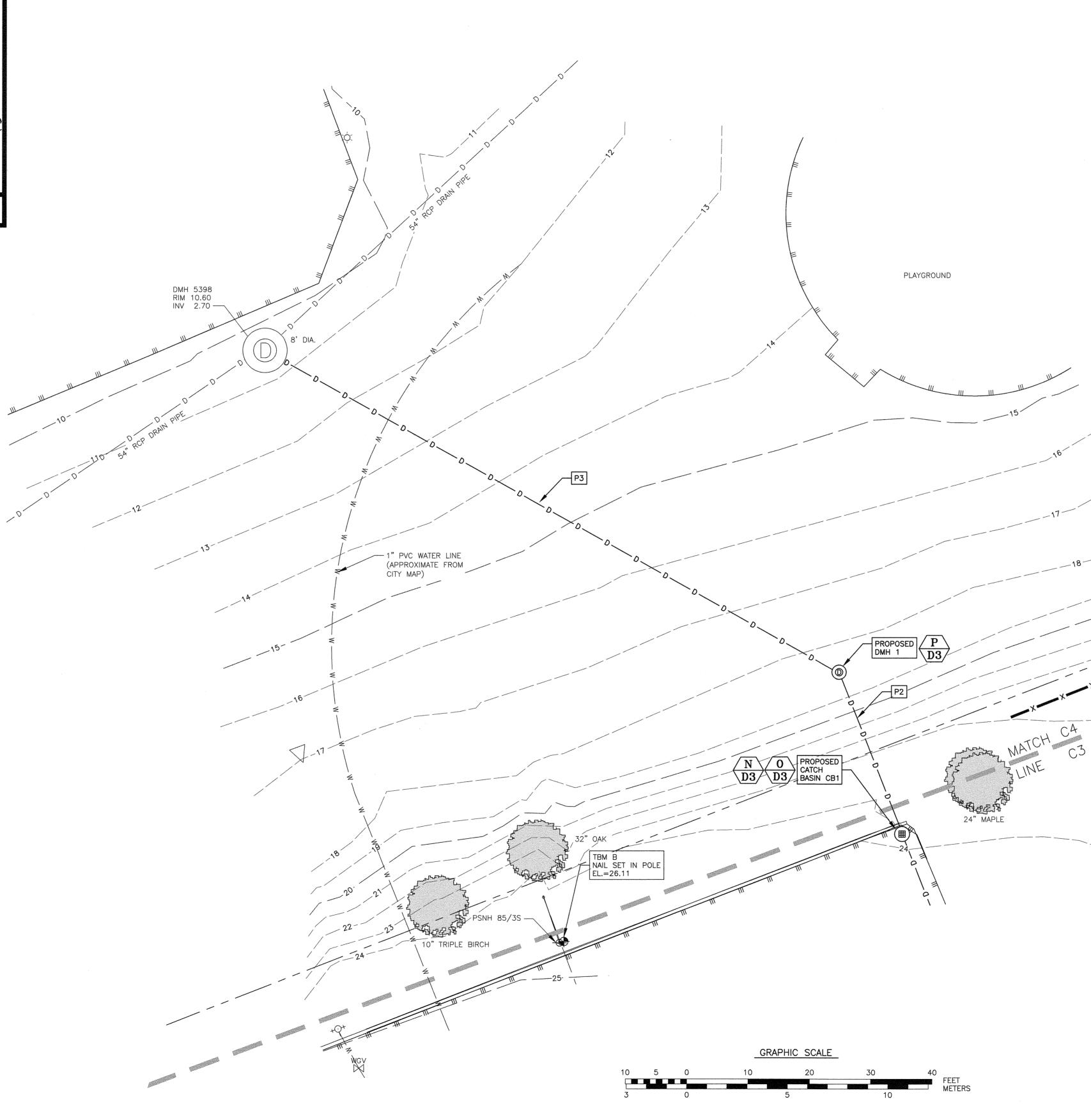
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DRAINAGE STRUCTURE TABLE						
STRUCTURE	PROP/EX	RIM	INVERT IN	INVERT OUT	SUMP	
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DMH 5398	EX	10.6	(12") 7.20	(54") 2.70		

	DRAIN	NAGE	PIPE	SCHEDU	LE
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PIPE #	SIZE	LENGTH	INVERTIN	INVERT OUT	SLOPE	
P1	12" HDPE	38'	20	19.84	0.004	
P2	12" HDPE	24'	19.74	14.60	0.20	
P3	12"HDPE	100'	14.50	7.20	0.073	







24" MAPLE

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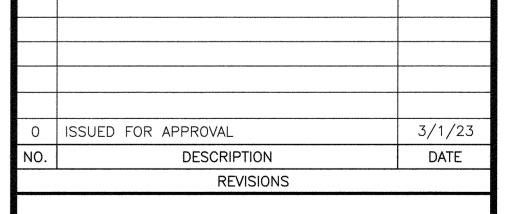
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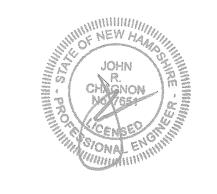
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SMITH RESIDENCE 9 KENT STREET PORTSMOUTH, N.H.





SCALE: 1" = 10'

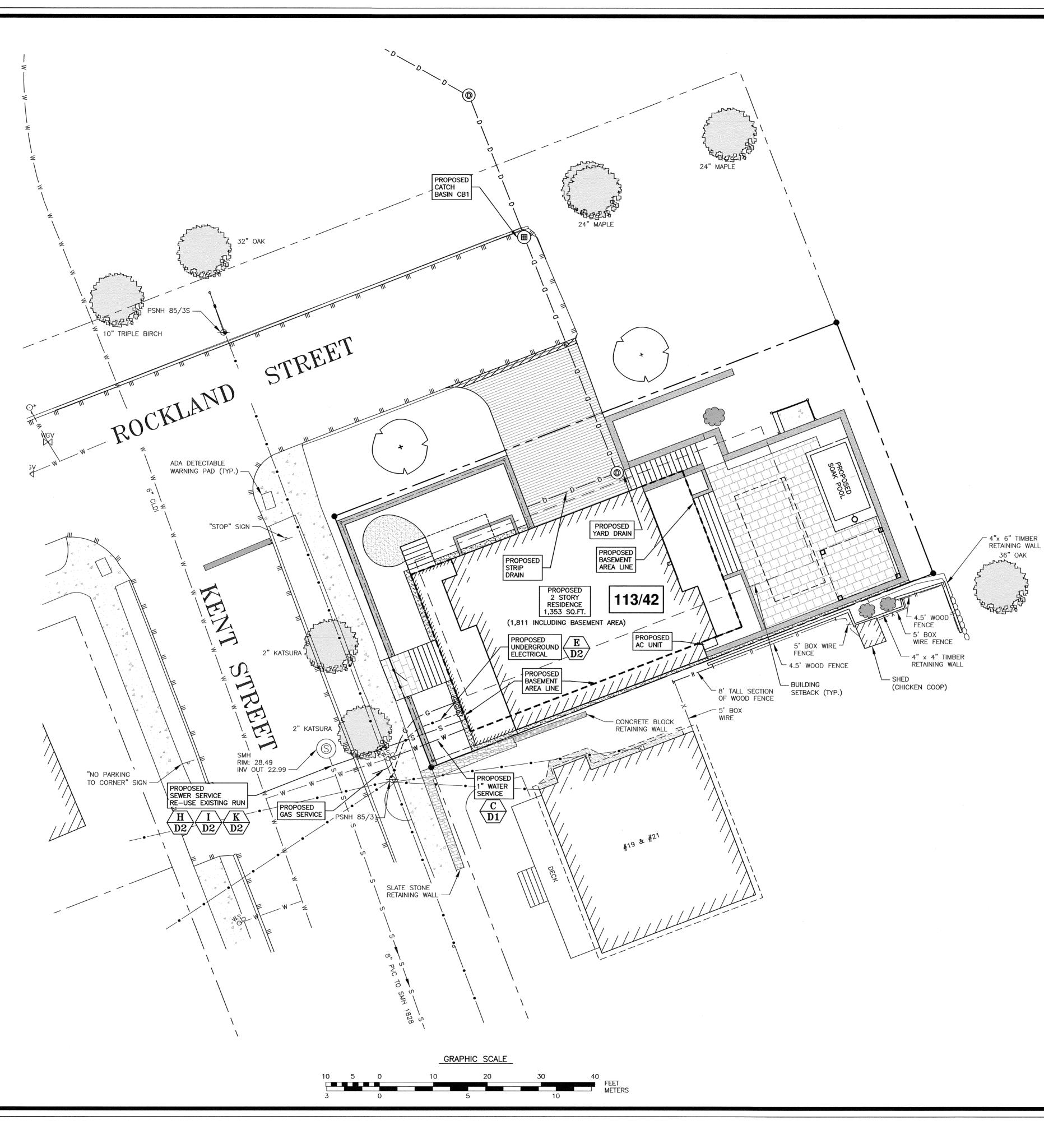
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C4

SEWER STRUCTURE SCHEDULE						
STRUCTURE	PROP/EX	RIM	INVERT IN	INVERT OUT		
SMH 1714	EX	28.49	N/A	22.99		
SMH 1828	EX	28.24	21.29	21.24		





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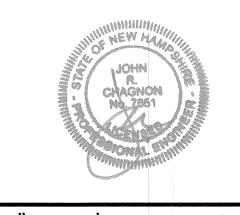
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4) PROTECT EXISTING STREET TREES DURING UTILITY WORK.

5) BUILDING WILL NOT BE SPRINKLED.

SMITH RESIDENCE 9 KENT STREET PORTSMOUTH, N.H.

2	GAS LINE	5/8/23			
1	SEWER LOCATION	3/1/23			
0	ISSUED FOR APPROVAL	2/7/23			
NO.	DESCRIPTION	DATE			
	REVISIONS				

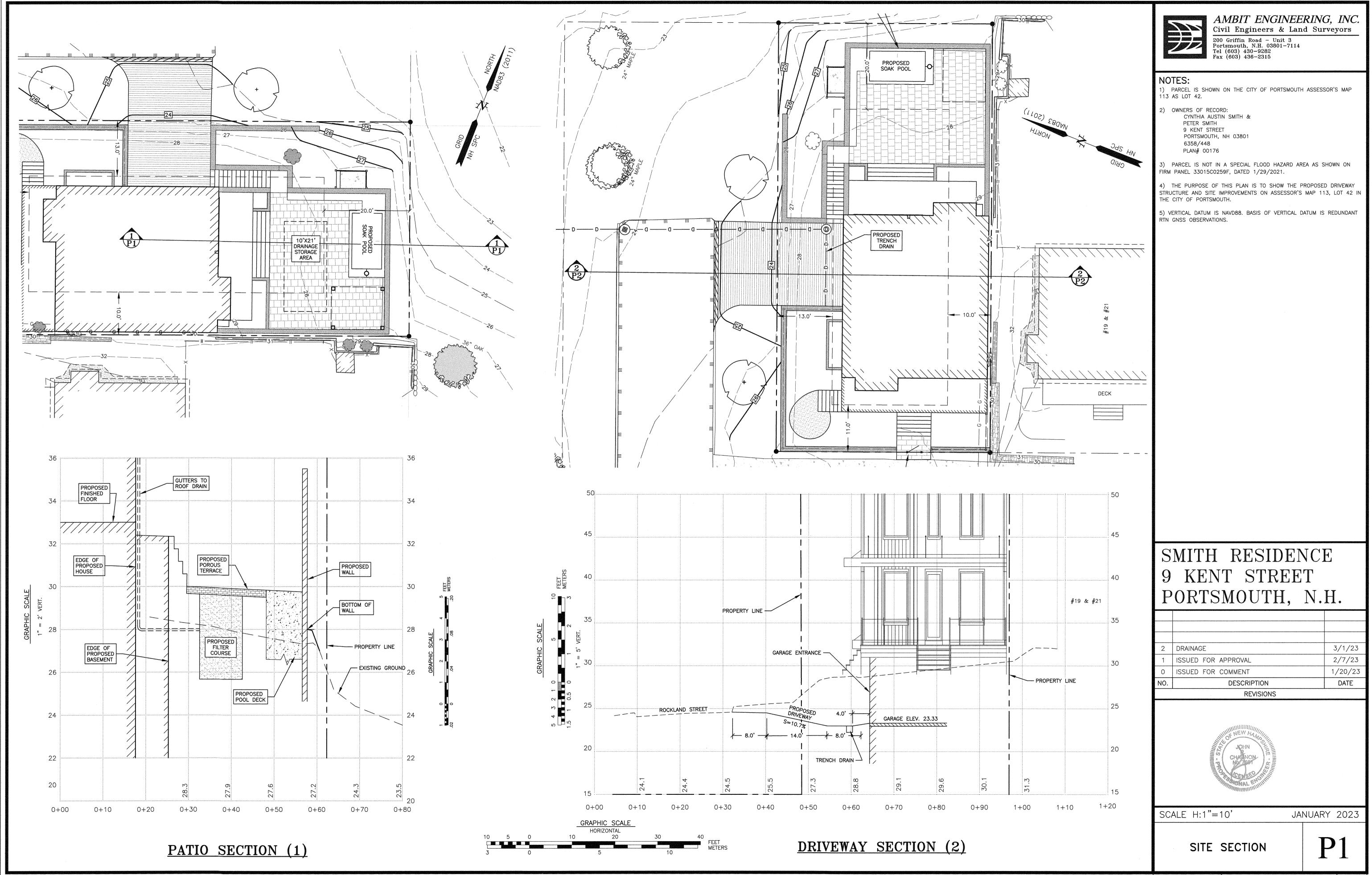


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UTILITY PLAN

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FB 301 PG 45

EROSION CONTROL NOTES

CONSTRUCTION SEQUENCE

DO NOT BEGIN CONSTRUCTION UNTIL ALL LOCAL, STATE AND FEDERAL PERMITS HAVE BEEN APPLIED FOR AND RECEIVED

INSTALL INLET PROTECTION AND PERIMETER CONTROLS, i.e., SILT FENCING OR SILTSOXX AROUND THE LIMITS OF DISTURBANCE AND CATCH BASIN FILTER BEFORE ANY EARTH MOVING OPERATIONS.

CUT AND GRUB ALL TREES, SHRUBS, SAPLINGS, BRUSH, VINES AND REMOVE OTHER DEBRIS AND RUBBISH AS REQUIRED.

REMOVE EXISTING GARAGE.

PERFORM IMPROVEMENTS. CONSTRUCT SITE UTILITIES AND BUILD HOME.

REMOVE TRAPPED SEDIMENTS FROM COLLECTION DEVICES AS APPROPRIATE, AND THEN REMOVE TEMPORARY EROSION CONTROL MEASURES UPON COMPLETION OF FINAL STABILIZATION OF THE SITE.

DURING CONSTRUCTION ACCESS WILL BE PROVIDED TO EXISTING PROPERTY LOCATED ON ROCKLAND

PROJECT DESCRIPTION

THE PROJECT CONSISTS OF A BUILDING REPLACEMENT WITH ASSOCIATED UTILITIES, GRADING, AND PARKING

THE TOTAL AREA TO BE DISTURBED IS APPROXIMATELY 0.115 ACRES.

BASED ON SITE OBSERVATIONS AND TEST PITS THE SOILS ON SITE CONSIST OF FINE SANDY LOAM WHICH ARE WELL DRAINED SOILS WITH A HYDROLOGIC SOIL GROUP RATING OF A.

THE STORMWATER RUNOFF FROM THE SITE WILL BE DISCHARGED TO CITY OF PORTSMOUTH PROPERTY WHICH ULTIMATELY FLOWS TO THE SOUTH MILL POND THEN TO THE PISCATAQUA RIVER.

GENERAL CONSTRUCTION NOTES

THE EROSION CONTROL PROCEDURES SHALL CONFORM TO SECTION 645 OF THE "STANDARD SPECIFICATION FOR ROAD AND BRIDGE CONSTRUCTION" OF THE NHDOT, AND "STORM WATER MANAGEMENT AND EROSION AND SEDIMENT CONTROL HANDBOOK FOR URBAN AND DEVELOPING AREAS IN NEW HAMPSHIRF". THE PROJECT IS TO BE MANAGED IN A MANNER THAT MEETS THE REQUIREMENTS AND INTENT OF RSA 430:53 AND CHAPTER AGR 3800 RELATIVE TO INVASIVE SPECIES.

DURING CONSTRUCTION AND THEREAFTER, EROSION CONTROL MEASURES ARE TO BE IMPLEMENTED AS NOTED. THE SMALLEST PRACTICAL AREA OF LAND SHOULD BE EXPOSED AT ANY ONE TIME DURING DEVELOPMENT. NO DISTURBED AREA SHALL BE LEFT UNSTABILIZED FOR MORE THAN 45 DAYS

ANY DISTURBED AREAS WHICH ARE TO BE LEFT TEMPORARILY, AND WHICH WILL BE REGRADED LATER DURING CONSTRUCTION SHALL BE MACHINE HAY MULCHED AND SEEDED WITH RYE GRASS TO PREVENT EROSION.

THE PROJECT IS TO BE MANAGED IN A MANNER THAT MEETS THE REQUIREMENTS AND INTENT OF RSA 430:53 AND CHAPTER AGR 3800 RELATIVE TO INVASIVE SPECIES.

DUST CONTROL: DUST CONTROL MEASURES SHALL INCLUDE BUT ARE NOT LIMITED TO SPRINKLING WATER ON EXPOSED AREAS, COVERING LOADED DUMP TRUCKS LEAVING THE SITE, AND TEMPORARY MULCHING

DUST CONTROL MEASURES SHALL BE UTILIZED SO AS TO PREVENT THE MIGRATION OF DUST FROM THE SITE TO ABUTTING AREAS. IF TEMPORARY STABILIZATION PRACTICES, SUCH AS TEMPORARY VEGETATION AND MULCHING. DO NOT ADEQUATELY REDUCE DUST GENERATION, APPLICATION OF WATER OR CALCIUM CHLORIDE SHALL BE APPLIED IN ACCORDANCE WITH BEST MANAGEMENT PRACTICES.

SILTSOXX SHALL BE PERIODICALLY INSPECTED DURING THE LIFE OF THE PROJECT AND AFTER EACH STORM, ALL DAMAGED SILTSOXX SHALL BE REPAIRED. SEDIMENT DEPOSITS SHALL PERIODICALLY BE REMOVED AND DISPOSED IN A SECURED LOCATION.

ALL FILLS SHALL BE PLACED AND COMPACTED TO REDUCE EROSION, SLIPPAGE, SETTLEMENT, SUBSIDENCE OR OTHER RELATED PROBLEMS.

ALL NON-STRUCTURAL, SITE-FILL SHALL BE PLACED AND COMPACTED TO 90% MODIFIED PROCTOR DENSITY IN LAYERS NOT EXCEEDING 18 INCHES IN THICKNESS UNLESS OTHERWISE NOTED.

FROZEN MATERIAL OR SOFT, MUCKY OR HIGHLY COMPRESSIBLE MATERIAL, TRASH, WOODY DEBRIS, LEAVES, BRUSH OR ANY DELETERIOUS MATTER SHALL NOT BE INCORPORATED INTO FILLS.

FILL MATERIAL SHALL NOT BE PLACED ON FROZEN FOUNDATION SUBGRADE.

DURING CONSTRUCTION AND UNTIL ALL DEVELOPED AREAS ARE FULLY STABILIZED, ALL EROSION CONTROL MEASURES SHALL BE INSPECTED WEEKLY AND AFTER EACH ONE HALF INCH OF RAINFALL.

THE CONTRACTOR SHALL MODIFY OR ADD EROSION CONTROL MEASURES AS NECESSARY TO ACCOMMODATE PROJECT CONSTRUCTION.

ALL ROADWAYS AND PARKING AREAS SHALL BE STABILIZED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE. ALL CUT AND FILL SLOPES SHALL BE SEEDED/LOAMED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE.

AN AREA SHALL BE CONSIDERED STABLE IF ONE OF THE FOLLOWING HAS OCCURRED:

BASE COURSE GRAVELS HAVE BEEN INSTALLED ON AREAS TO BE PAVED - A MINIMUM OF 85% VEGETATED GROWTH HAS BEEN ESTABLISHED

- A MINIMUM OF 3 INCHES OF NON-EROSIVE MATERIAL SUCH AS STONE OR RIPRAP HAS BEEN INSTALLED

- EROSION CONTROL BLANKETS HAVE BEEN INSTALLED.

- IN AREAS TO BE PAVED, "STABLE" MEANS THAT BASE COURSE GRAVELS MEETING THE REQUIREMENTS OF NHOOT STANDARD FOR ROAD AND BRIDGE CONSTRUCTION, 2016, ITEM 304.2 HAVE BEEN INSTALLED.

STABILIZATION SHALL BE INITIATED ON ALL LOAM STOCKPILES, AND DISTURBED AREAS, WHERE CONSTRUCTION ACTIVITY SHALL NOT OCCUR FOR MORE THAN TWENTY-ONE (21) CALENDAR DAYS BY THE FOURTEENTH (14TH) DAY AFTER CONSTRUCTION ACTIVITY HAS PERMANENTLY OR TEMPORARILY CEASED IN THAT AREA.

STABILIZATION MEASURES TO BE USED INCLUDE: - TEMPORARY SEEDING;

MULCHING.

ALL AREAS SHALL BE STABILIZED WITHIN 45 DAYS OF INITIAL DISTURBANCE. WHEN CONSTRUCTION ACTIVITY PERMANENTLY OR TEMPORARILY CEASES WITHIN 100 FEET OF NEARBY SURFACE WATERS OR DELINEATED WETLANDS, THE AREA SHALL BE STABILIZED WITHIN SEVEN (7) DAYS OR PRIOR TO A RAIN EVENT. ONCE CONSTRUCTION ACTIVITY CEASES PERMANENTLY IN THESE AREAS, SILTSOXX, MULCH BERMS, HAY BALE BARRIERS AND ANY EARTH/DIKES SHALL BE REMOVED ONCE PERMANENT MEASURES ARE ESTABLISHED. . DURING CONSTRUCTION, RUNOFF WILL BE DIVERTED AROUND THE SITE WITH EARTH DIKES.

PIPING OR STABILIZED CHANNELS WHERE POSSIBLE. SHEET RUNOFF FROM THE SITE WILL BE FILTERED THROUGH SILTSOXX, MULCH BERMS, HAY BALE BARRIERS, OR SILT SOCKS. ALL STORM DRAIN BASIN INLETS SHALL BE PROVIDED WITH FLARED END SECTIONS AND TRASH RACKS. THE SITE SHALL BE STABILIZED FOR THE WINTER BY OCTOBER 15.

MAINTENANCE AND PROTECTION

SILTSOXX SHALL BE REMOVED ONCE SITE IS STABILIZED, AND DISTURBED AREAS RESULTING FROM SILTSOXX REMOVAL SHALL BE PERMANENTLY SEEDED.

THE CATCH BASIN INLET BASKET SHALL BE INSPECTED WITHIN 24 HOURS AFTER EACH RAINFALL OR DAILY DURING EXTENDED PERIODS OF PRECIPITATION. REPAIRS SHALL BE MADE IMMEDIATELY, AS NECESSARY, TO PREVENT PARTICLES FROM REACHING THE DRAINAGE SYSTEM AND/OR CAUSING SURFACE FLOODING SEDIMENT DEPOSITS SHALL BE REMOVED AFTER EACH STORM EVENT, OR MORE OFTEN IF THE FABRIC BECOMES CLOGGED.

WINTER NOTES

PROLONGED RAINFALL.

ALL PROPOSED VEGETATED AREAS THAT DO NOT EXHIBIT A MINIMUM OF 85% VEGETATED GROWTH BY OCTOBER 15, OR WHICH ARE DISTURBED AFTER OCTOBER 15, SHALL BE STABILIZED BY SEEDING AND INSTALLING EROSION CONTROL BLANKETS ON SLOPES GREATER THAN 3:1, AND SEEDING AND PLACING 3 TO 4 TONS OF MULCH PER ACRE, SECURED WITH ANCHORED NETTING, ELSEWHERE. THE INSTALLATION OF EROSION CONTROL BLANKETS OR MULCH AND NETTING SHALL NOT OCCUR OVER ACCUMULATED SNOW OR ON FROZEN GROUND AND SHALL BE COMPLETED IN ADVANCE OF THAW OR SPRING MELT EVENTS.

ALL DITCHES OR SWALES WHICH DO NOT EXHIBIT A MINIMUM OF 85 PERCENT VEGETATIVE GROWTH BY OCTOBER 15, OR WHICH ARE DISTURBED AFTER OCTOBER 15, SHALL BE STABILIZED TEMPORARILY WITH STONE OR EROSION CONTROL BLANKETS APPROPRIATE FOR THE DESIGN FLOW CONDITIONS;

AFTER OCTOBER 15, INCOMPLETE ROAD OR PARKING SURFACES, WHERE WORK HAS STOPPED FOR THE WINTER SEASON, SHALL BE PROTECTED WITH A MINIMUM OF 3 INCHES OF CRUSHED GRAVEL PER NHDOT ITEM 304.3, OR IF CONSTRUCTION IS TO CONTINUE THROUGH THE WINTER SEASON BE CLEARED OF ANY ACCUMULATED SNOW AFTER EACH STORM EVENT;

STOCKPILES

LOCATE STOCKPILES A MINIMUM OF 50 FEET AWAY FROM CATCH BASINS, SWALES, AND CULVERTS. 2. ALL STOCKPILES SHOULD BE SURROUNDED WITH TEMPORARY EROSION CONTROL MEASURES

PRIOR TO THE ONSET OF PRECIPITATION PERIMETER BARRIERS SHOULD BE MAINTAINED AT ALL TIMES, AND ADJUSTED AS NEEDED TO 3.

ACCOMMODATE THE DELIVERY AND REMOVAL OF MATERIALS FROM THE STOCKPILE. THE INTEGRITY OF THE BARRIER SHOULD BE INSPECTED AT THE END OF EACH WORKING DAY. PROTECT ALL STOCKPILES FROM STORMWATER RUN-OFF USING TEMPORARY EROSION CONTROL MEASURES SUCH AS BERMS, SILT SOCK, OR OTHER APPROVED PRACTICE TO PREVENT MIGRATION OF MATERIAL BEYOND THE IMMEDIATE CONFINES OF THE STOCKPILES.

CONCRETE WASHOUT AREA

THE FOLLOWING ARE THE ONLY NON-STORMWATER DISCHARGES ALLOWED. ALL OTHER NON-STORMWATER DISCHARGES ARE PROHIBITED ON SITE: THE CONCRETE DELIVERY TRUCKS SHALL, WHENEVER POSSIBLE, USE WASHOUT FACILITIES AT THEIR OWN PLANT OR DISPATCH FAILITY

2. IF IT IS NECESSARY, SITE CONTRACTOR SHALL DESIGNATE SPECIFIC WASHOUT AREAS AND DESIGN FACILITIES TO HANDLE ANTICIPATED WASHOUT WATER; CONTRACTOR SHALL LOCATE WASHOUT AREAS AT LEAST 150 FEET AWAY FROM STORM DRAINS, SWALES AND SURFACE WATERS OR DELINEATED WETLANDS; 4. INSPECT WASHOUT FACILITIES DAILY TO DETECT LEAKS OR TEARS AND TO IDENTIFY WHEN MATERIALS NEED TO BE REMOVED.

ALLOWABLE NON-STORMWATER DISCHARGES

FIRE-FIGHTING ACTIVITIES FIRE HYDRANT FLUSHING:

- WATERS USED TO WASH VEHICLES WHERE DETERGENTS ARE NOT USED;
- WATER USED TO CONTROL DUST; POTABLE WATER INCLUDING UNCONTAMINATED WATER LINE FLUSHING;
- ROUTINE EXTERNAL BUILDING WASH DOWN WHERE DETERGENTS ARE NOT USED:
- PAVEMENT WASH WATERS WHERE DETERGENTS ARE NOT USED;
- UNCONTAMINATED AIR CONDITIONING/COMPRESSOR CONDENSATION; UNCONTAMINATED GROUND WATER OR SPRING WATER;
- FOUNDATION OR FOOTING DRAINS WHICH ARE UNCONTAMINATED;
- UNCONTAMINATED EXCAVATION DEWATERING; 11. 12. LANDSCAPE IRRIGATION.

WASTE DISPOSAL

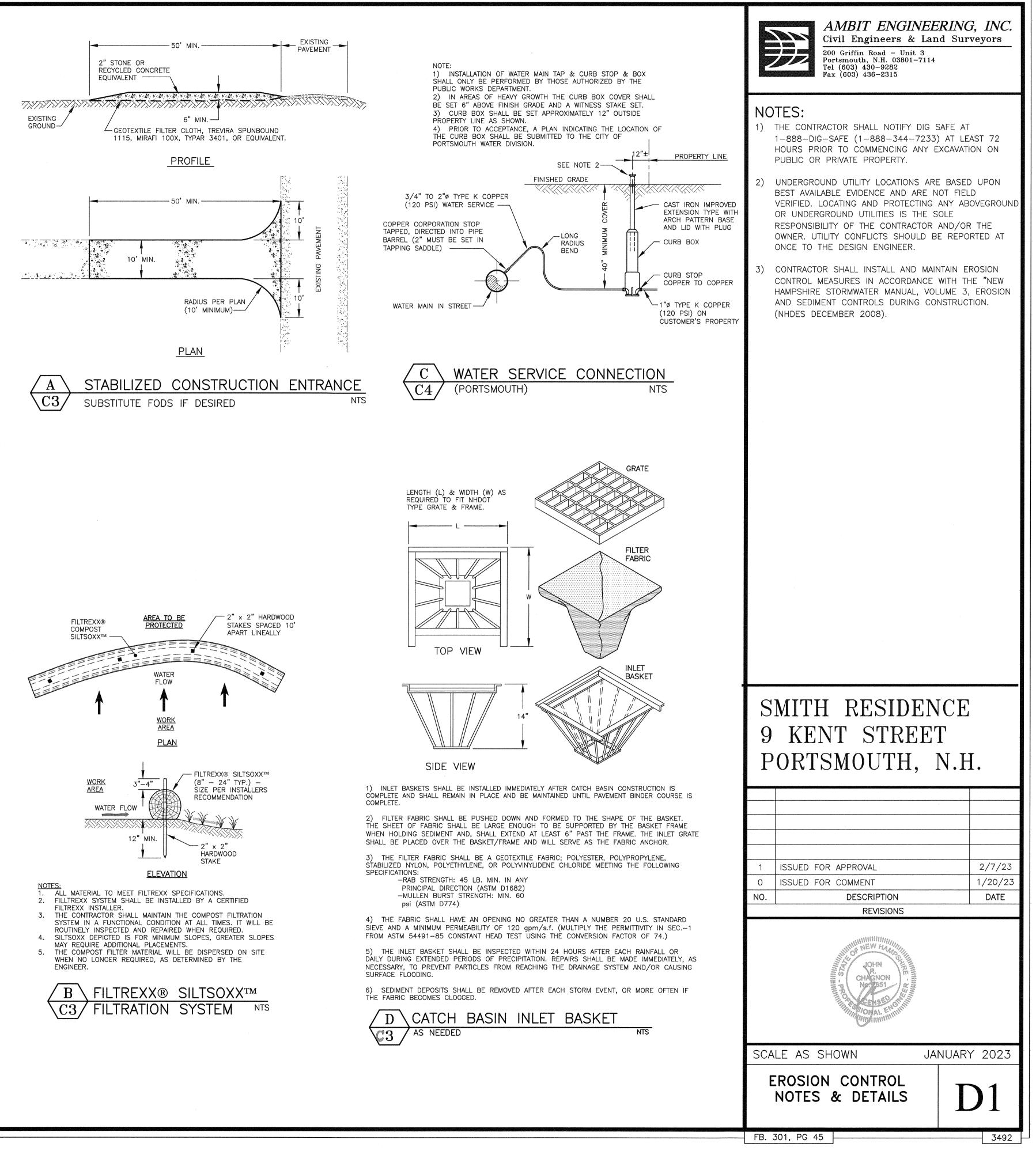
- WASTE MATERIAL
- ALL WASTE MATERIALS SHALL BE COLLECTED AND STORED IN SECURELY LIDDED RECEPTACLES. ALL TRASH AND CONSTRUCTION DEBRIS FROM THE SITE SHALL BE DEPOSITED IN A DUMPSTER:
- NO CONSTRUCTION WASTE MATERIALS SHALL BE BURIED ON SITE; - ALL PERSONNEL SHALL BE INSTRUCTED REGARDING THE CORRECT PROCEDURE FOR WASTE DISPOSAL BY THE SUPERINTENDENT.
- HAZARDOUS WASTE - ALL HAZARDOUS WASTE MATERIALS SHALL BE DISPOSED OF IN THE MANNER SPECIFIED BY LOCAL OR STATE REGULATION OR BY THE MANUFACTURER; - SITE PERSONNEL SHALL BE INSTRUCTED IN THESE PRACTICES BY THE SUPERINTENDENT.
- 3. SANITARY WASTE - ALL SANITARY WASTE SHALL BE COLLECTED FROM THE PORTABLE UNITS A MINIMUM OF ONCE PER WEEK BY A LICENSED SANITARY WASTE MANAGEMENT CONTRACTOR.

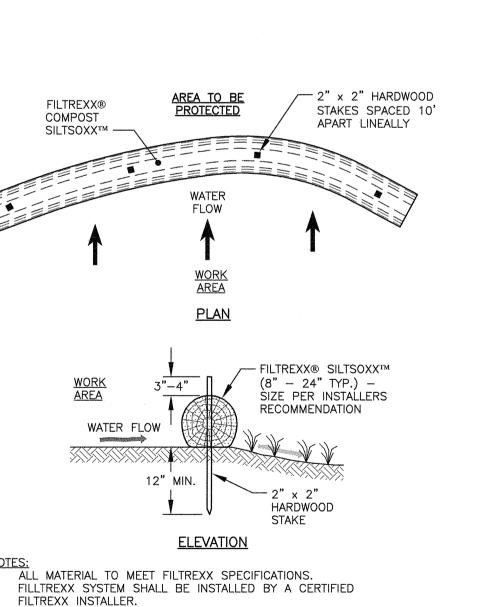
BLASTING NOTES

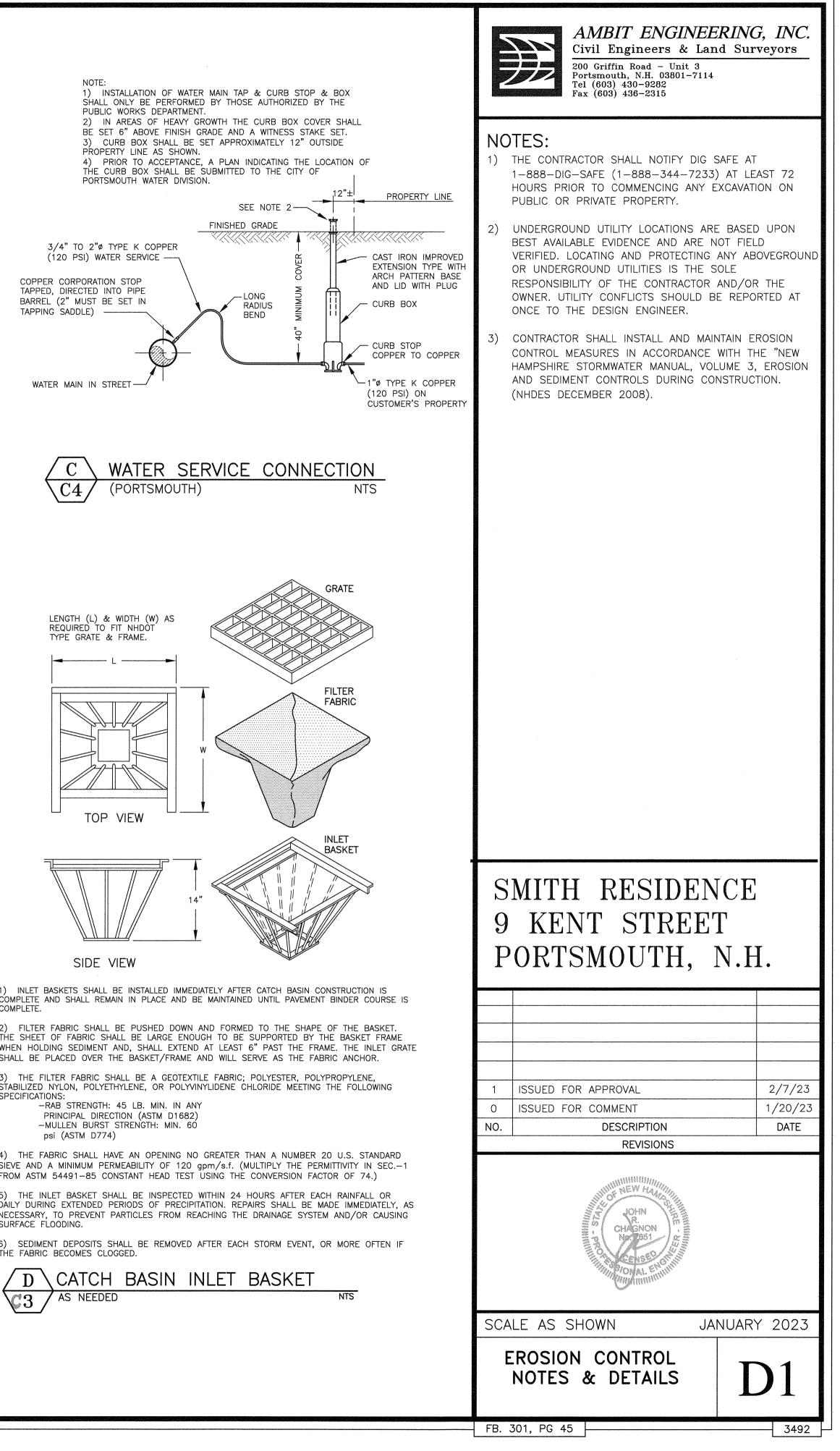
CONTRACTOR SHALL CONTACT THE NHDES AND/OR LOCAL JURISDICTION PRIOR TO COMMENCING ANY BLASTING ACTIVITIES. FOR ANY PROJECT FOR WHICH BLASTING OF BEDROCK IS ANTICIPATED, THE APPLICANT SHALL SUBMIT A BLASTING PLAN THAT IDENTIFIES:

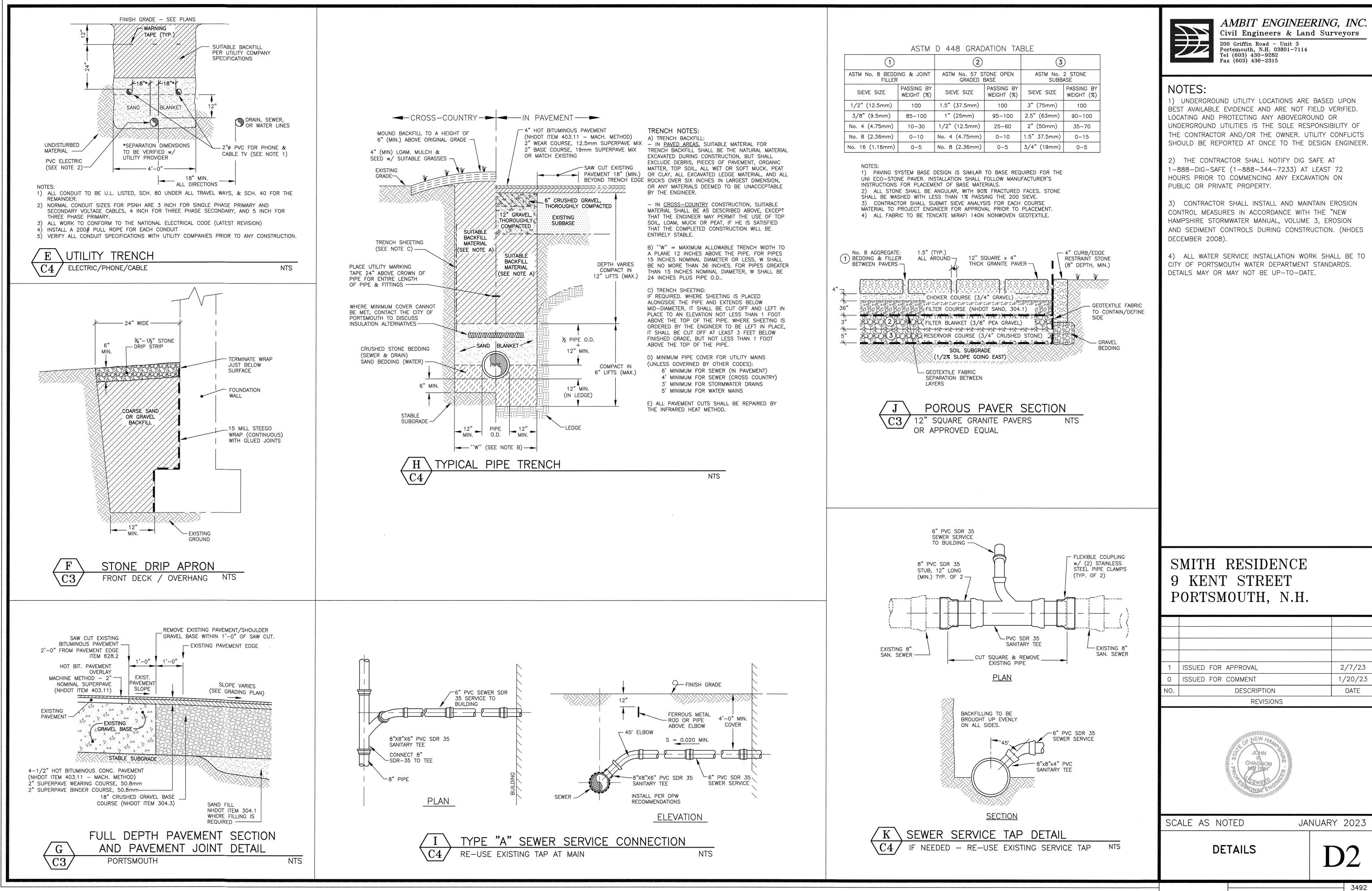
- WHERE THE BLASTING ACTIVITIES ARE ANTICIPATED TO OCCUR; - THE ESTIMATED QUANTITY OF BLAST ROCK IN CUBIC YARDS; AND - SITE-SPECIFIC BLASTING BEST MANAGEMENT PRACTICES.

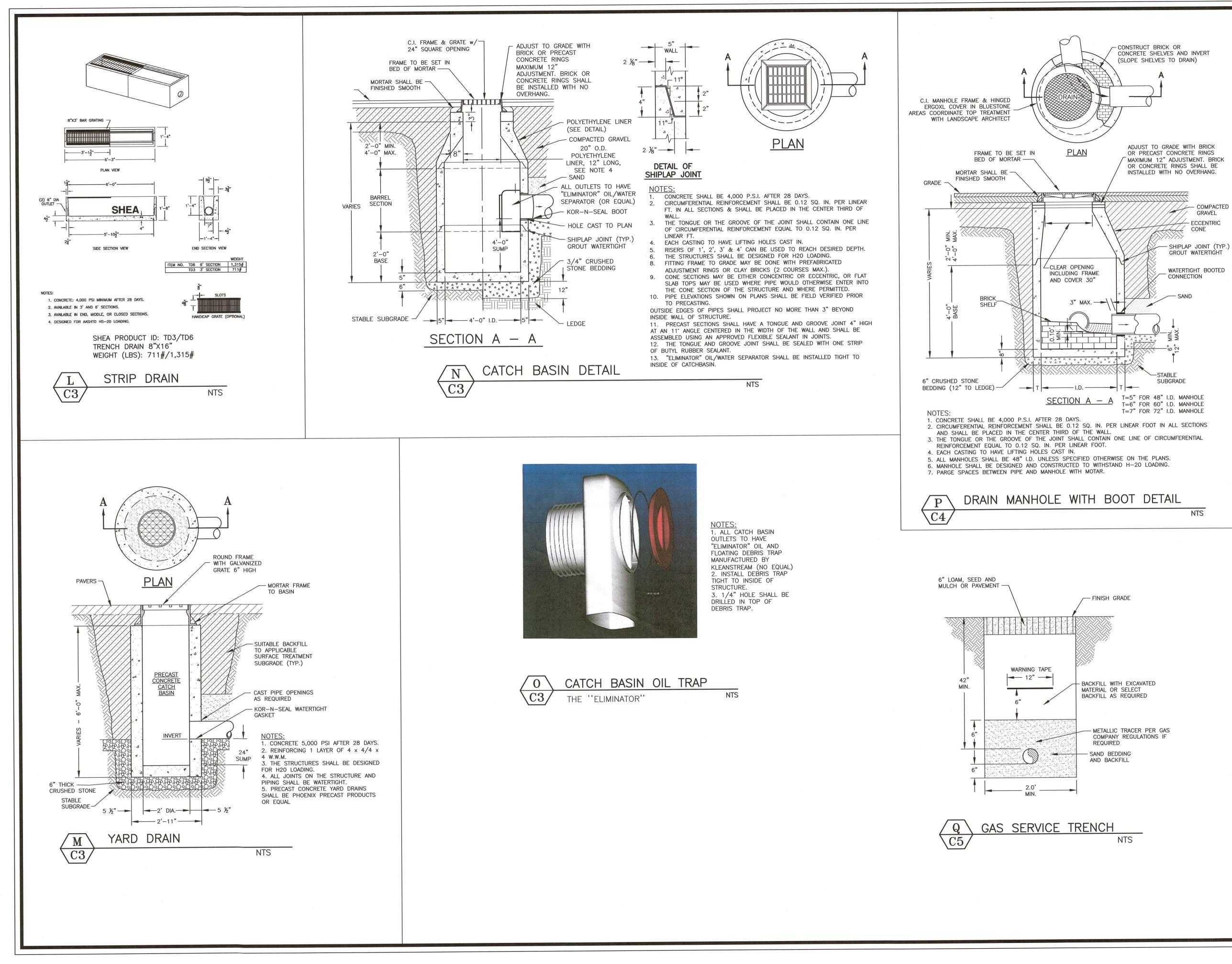
THE SILTSOXX BARRIER SHALL BE CHECKED AFTER EACH RAINFALL AND AT LEAST DAILY DURING













AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315

NOTES:

1) UNDERGROUND UTILITY LOCATIONS ARE BASED UPON BEST AVAILABLE EVIDENCE AND ARE NOT FIELD VERIFIED. LOCATING AND PROTECTING ANY ABOVEGROUND OR UNDERGROUND UTILITIES IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND/OR THE OWNER. UTILITY CONFLICTS SHOULD BE REPORTED AT ONCE TO THE DESIGN ENGINEER.

2) THE CONTRACTOR SHALL NOTIFY DIG SAFE AT 1-888-DIG-SAFE (1-888-344-7233) AT LEAST 72 HOURS PRIOR TO COMMENCING ANY EXCAVATION ON PUBLIC OR PRIVATE PROPERTY.

3) CONTRACTOR SHALL INSTALL AND MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH THE "NEW HAMPSHIRE STORMWATER MANUAL, VOLUME 3, EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION. (NHDES DECEMBER 2008).

SMITH RESIDENCE 9 KENT STREET PORTSMOUTH, N.H.

2	ADD DETAIL Q	5/8/23			
1	DETAILS N, O, P	3/1/23			
0	ISSUED FOR APPROVAL	2/7/23			
NO.	DESCRIPTION	DATE			
	REVISIONS				
State of the second sec					



AS NOTED

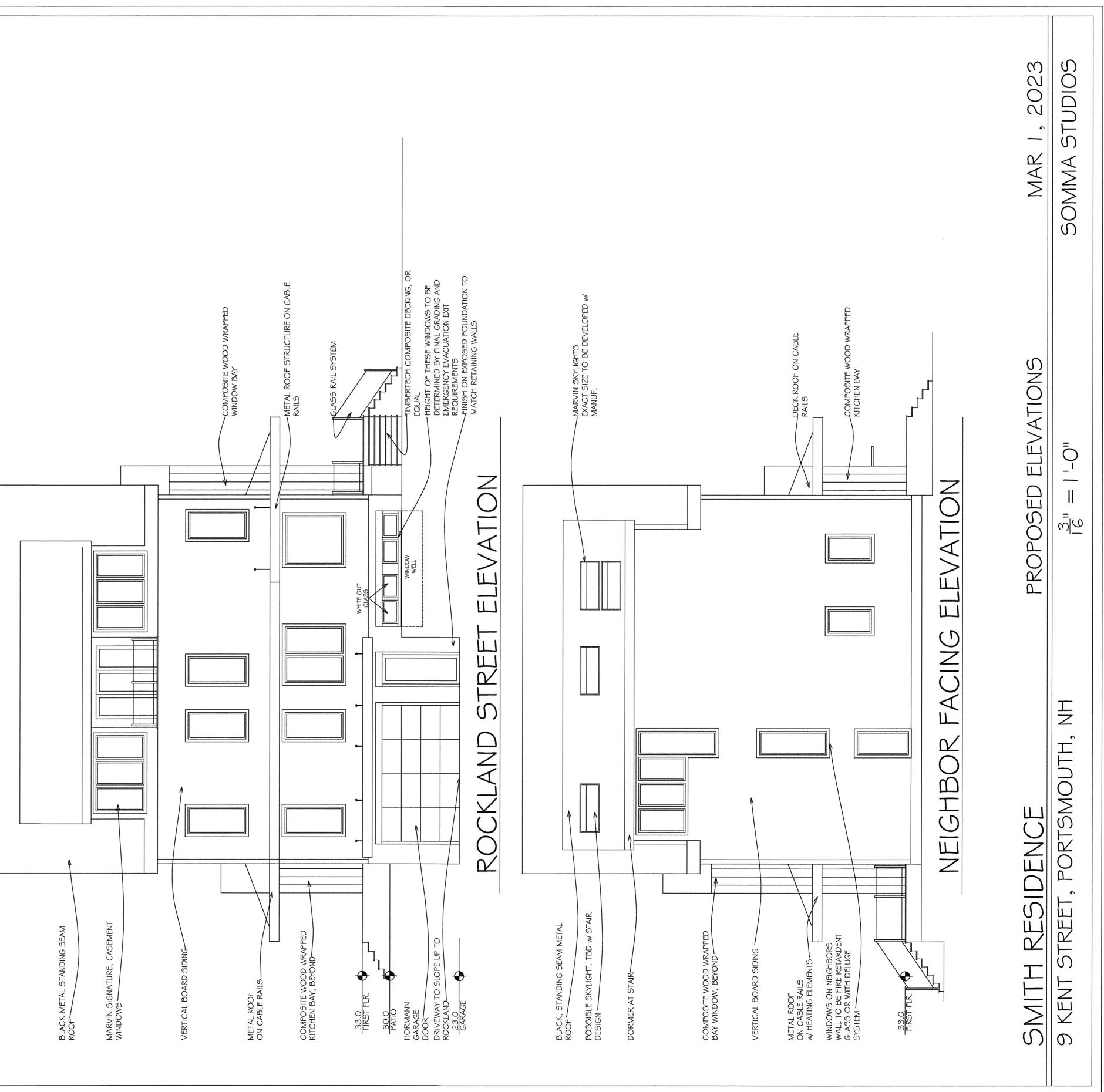
JANUARY 2023



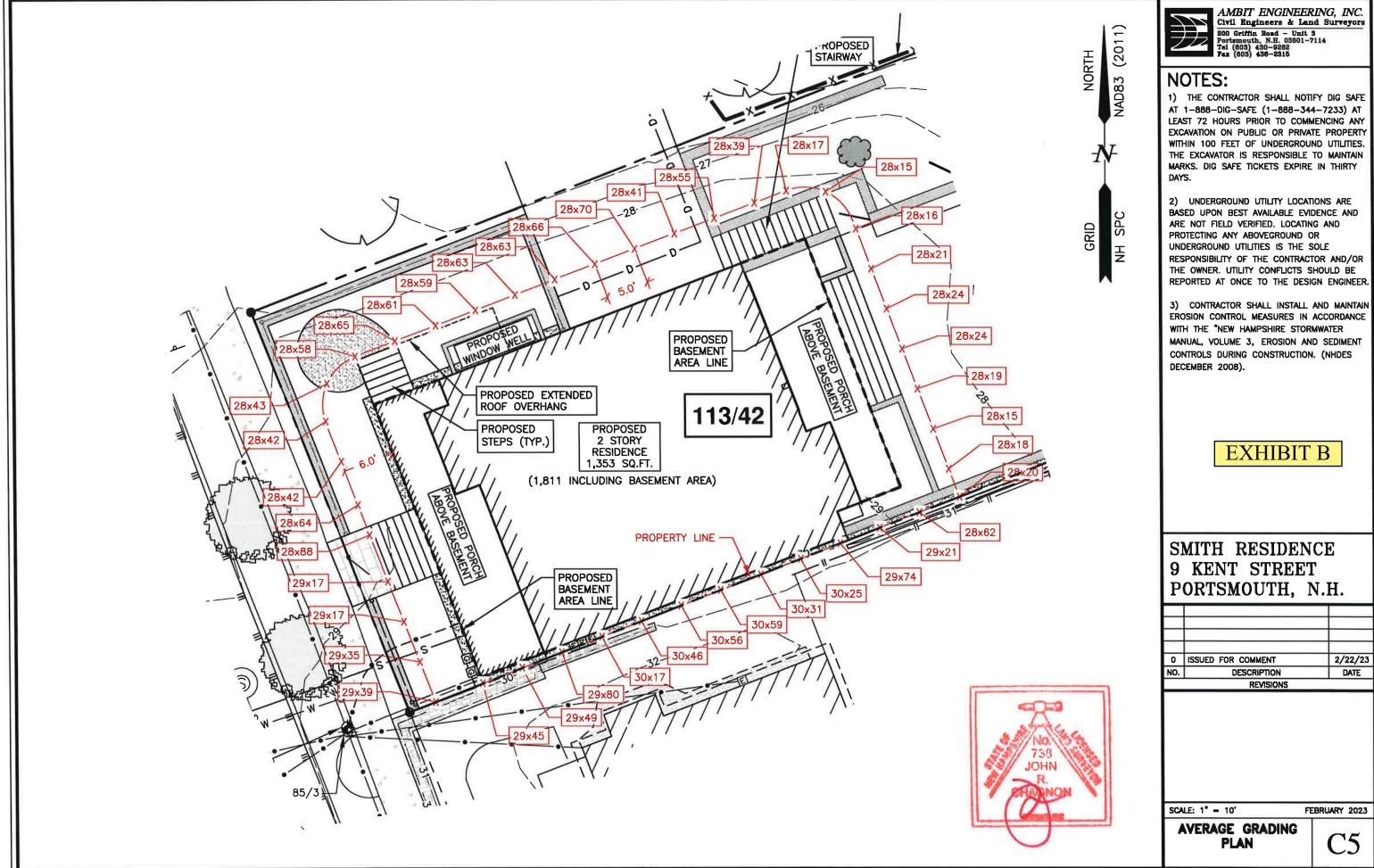
D3

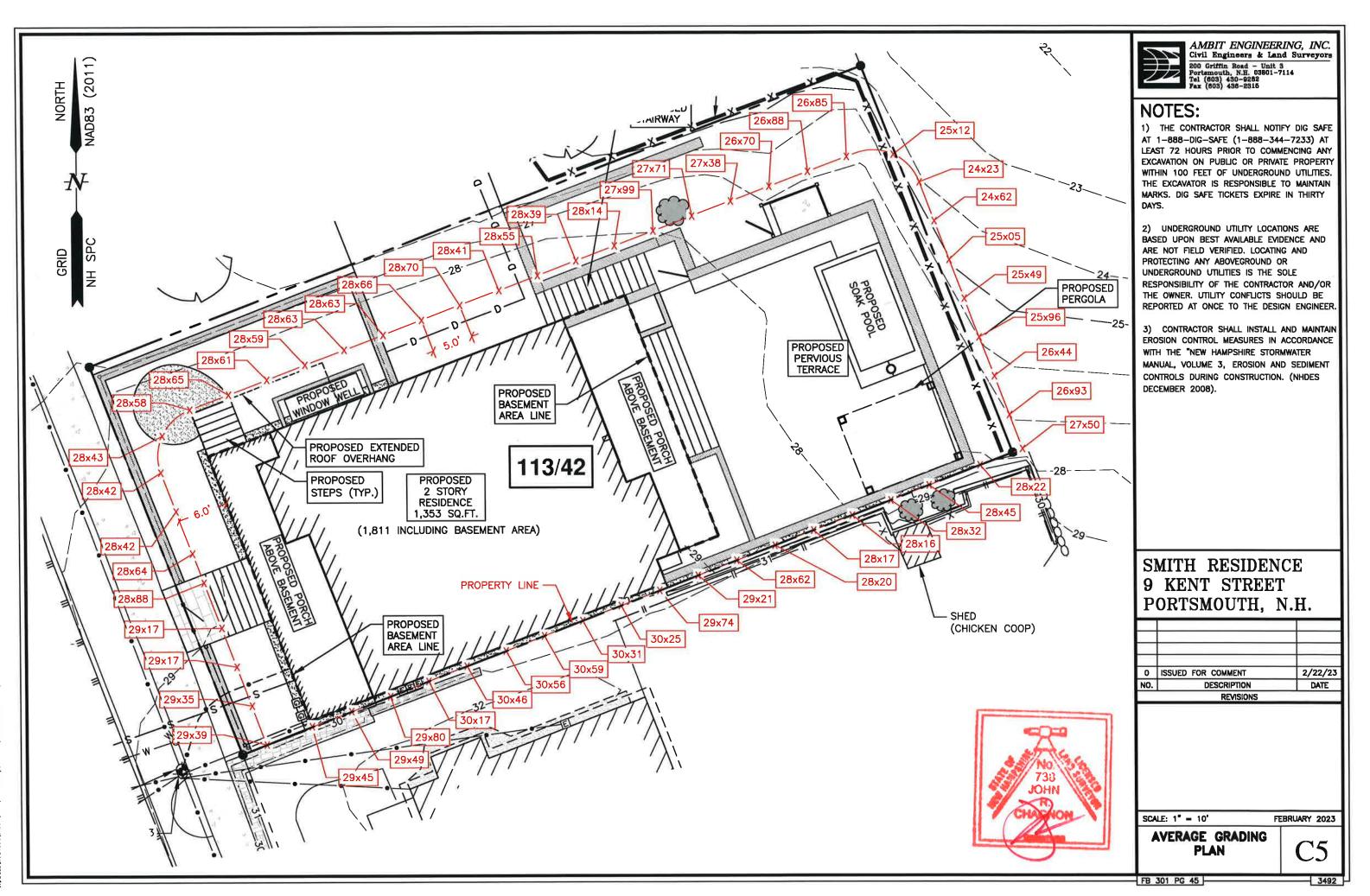
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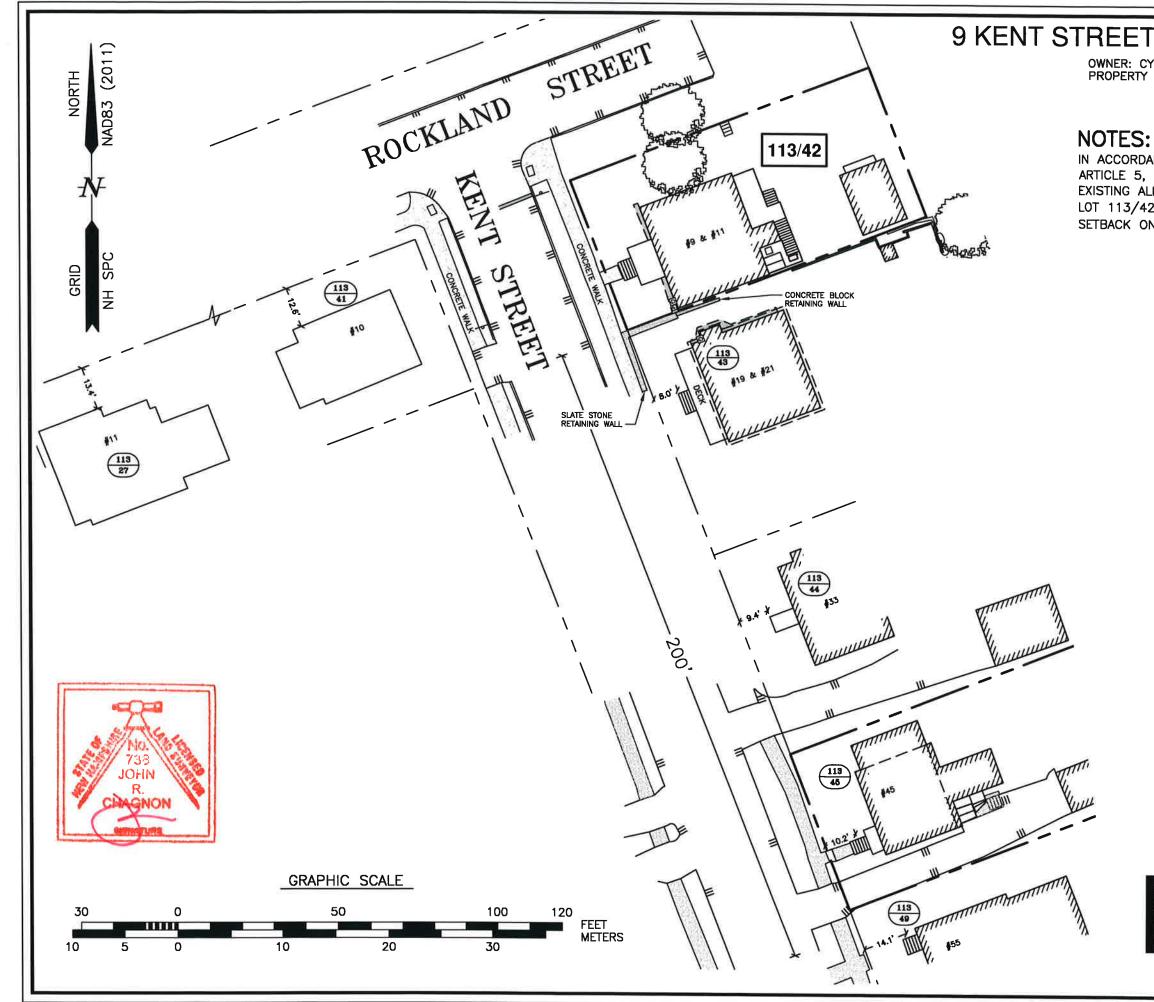






	Average (Grade Wo	ork Sheet	- Buildin	g Only	
Project		Smith R	esidence		Calculated	
Address:	9 Kent Street, Portsmouth, NH				2/23/2023	
6' of	6' offset from Building; Existing Grades 5' OC					
SECTION	Elev	Elev	Elev	Elev	Total	
SOUTH	28.62	29.21	29.74	30.25	117.82	
	30.31	30.59	30.56	30.46	121.92	
	30.17	29.80	29.49	29.45	118.91	
					0.00	
					0.00	AVG PER SECTION
				12.0	358.65	29.89
WEST	29.39	29.35	29.17	29.17	117.08	
	28.88	28.64	28.42	28.42	114.36	
	28.43				28.43	
						AVG PER SECTION
				9.0	259.87	28.87
NORTH	28.58	28.65	28.61	28.59	114.43	
	28.63	28.63	28.66	28.70	114.62	
	28.41	28.55	28.39	28.17	113.52	
					0.00	
					0.00	AVG PER SECTION
				12.0	342.57	28.55
EAST	28.15	28.16	28.21	28.24	112.76	
	28.24	28.19	28.15	28.18	112.76	
	28.20				28.20	
					0.00	
						AVG PER SECTION
				9	253.72	28.19
Total	1,214.81	>	AVERAG	E GRADE		
#	42		28	.92		

Ave	rage Grad	de Work S	Sheet - Bi	uilding an	d Terrace	
Project		Smith R	esidence	Calculated		
Address:	9 Ke	ent Street, F	Portsmouth	, NH	2/23/2023	
6' of	6' offset from Building; Existing Grades 5' OC					
SECTION	Elev	Elev	Elev	Elev	Total	
SOUTH	28.22	28.45	28.32	28.16	113.15	
	28.17	28.20	28.62	29.21	114.20	
	29.74	30.25	30.31	30.59	120.89	
	30.56	30.46	30.17	29.80	120.99	
	29.49	29.45			58.94	AVG PER SECTION
				18.0	528.17	29.34
WEST	29.39	29.35	29.17	29.17	117.08	
	28.88	28.64	28.42	28.42	114.36	
	28.43				28.43	
						AVG PER SECTION
				9.0	259.87	28.87
NORTH	28.58	28.65	28.61	28.59	114.43	
	28.63	28.63	28.66	28.70	114.62	
	28.41	28.55	28.39	28.14	113.49	
	27.99	27.71	27.38	26.70	109.78	
	26.88	26.85			53.73	AVG PER SECTION
				18.0	506.05	28.11
EAST	25.12	24.23	24.62	25.05	99.02	
	25.49	25.96	26.44	26.93	104.82	
	27.50				27.50	
					0.00	
						AVG PER SECTION
				9	231.34	25.70
Total	1,525.43	>	AVERAG	E GRADE		
#	54		28	.25		



9 KENT STREET FRONT SETBACK EXHIBIT

OWNER: CYNTHIA AUSTIN SMITH & PETER SMITH PROPERTY LOCATION: 9-11 KENT STREET CITY OF PORTSMOUTH COUNTY OF ROCKINGHAM STATE OF NEW HAMPSHIRE

IN ACCORDANCE WITH THE PORTSMOUTH ZONING ORDINANCE, ARTICLE 5, SECTION 10.516.10 FRONT YARD EXCEPTION FOR EXISTING ALIGNMENTS: THE AVERAGE FRONT SETBACK FOR LOT 113/42 ON KENT STREET IS 11'. THE AVERAGE SETBACK ON ROCKLAND STREET IS 13'.

KENT STREET					
MAP/LOT	SETBACK	AVERAGE			
113/43	8.0				
113/44	9.4				
113/45	10.2				
113/46	14.1				
4	41.7	11*			

ROCKLAND STREET					
MAP/LOT	AVERAGE				
113/41 12.6					
113/27	13.4				
113/2	13'				

EXHIBIT C

SCALE: 1"=30'

21 DECEMBER, 2022



AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors 200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315

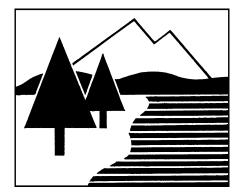
FB 301 PG 45

3492

DRAINAGE ANALYSIS

SMITH RESIDENCE

9 KENT STREET PORTSMOUTH, NH



PREPARED FOR PETER SMITH

06 FEBRUARY 2023 REVISED: 28 FEBRUARY 2023



AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

200 Griffin Road, Unit 3 Portsmouth, NH 03801 Phone: 603.430.9282; Fax: 603.436.2315 E-mail: <u>jrc@ambitengineering.com</u> (Ambit Job Number 3492)

EXHIBIT D

TABLE OF CONTENTS

REPORT

Executive Summary	1
Introduction / Project Description	2
Methodology	2
Site Specific Information	3
Pre-Development Drainage	4
Post-Development Drainage	4
Offsite Infrastructure Capacity	6
Erosion and Sediment Control Practices	6
Conclusion	7
References	7

ATTACHMENTS

Existing Subcatchment Plan Proposed Subcatchment Plan

APPENDIX

Vicinity (Tax) Map	А
Tables, Charts, Etc.	В
HydroCAD Drainage Analysis Calculations	С
Soil Survey Information	D
FEMA FIRM Map	E
Inspection & Long-Term Maintenance Plan	F

EXECUTIVE SUMMARY

This drainage analysis examines the pre-development (existing) and post-development (proposed) stormwater drainage patterns for the Residence Redevelopment at the property known as 9 Kent Street in Portsmouth, NH. The site is shown on the City of Portsmouth Assessor's Tax Map 113 as Lot 42. The total size of the lot is 5000± square feet (0.115 acres) and the associated drainage area is 7,643± square-feet (0.175 acres).

The development will provide for a residence redevelopment, parking, and associated utilities. The development has the potential to increase stormwater runoff to adjacent properties, and should be designed in a manner to prevent that occurrence. The site contains an existing building which will be replaced. The proposed stormwater BMPs will offset the impact caused by the redevelopment. Part of the runoff from the site will be directed to an existing City drainage network using a proposed catch basin.

The hydrologic modeling utilized for this analysis uses the "Extreme Precipitation" values for rainfall from The Northeast Regional Climate Center (Cornell University), with a 15% increase to comply with local ordinance.

INTRODUCTION / PROJECT DESCRIPTION

This drainage report is designed to assist the owner, contractor, regulatory reviewer, and others in understanding the impact of the proposed development project on local surface water runoff and quality. The project site is shown on the City of Portsmouth Assessor's Tax Map 113 as Lot 42. Bounding the site to the north is Rockland Street followed by City property. Bounding the site to the east is City property. Bounding the site to the south is a private residence. Bounding the site to the west is Kent Street, followed by private residences. A vicinity map is included in the Appendix to this report.

The proposed project includes a building redevelopment, associated parking and utilities. This report uses the design to calculate the future impervious coverage of the proposed lot, as required by the City.

This report includes information about the existing site and the proposed site necessary to analyze stormwater runoff and to design any required mitigation. The report includes impervious surface analyses and the associated operations and maintenance manual. The report will provide a narrative of the stormwater runoff. Proposed stormwater management and treatment structures and methods will also be described, as well as erosion and sediment control practices. To fully understand the proposed site development the reader should also review a complete site plan set in addition to this report.

METHODOLOGY

"Extreme Precipitation" values from The Northeast Regional Climate Center (Cornell University) have been used for modeling purposes. These values have been used in this analysis, with a 15% addition to comply with local ordinances.

This report uses the US Soil Conservation Service (SCS) Method for estimating stormwater runoff. The SCS method is published in The National Engineering Handbook (NEH), Section 4 "Hydrology" and includes the Technical Release No. 20, (TR-20) "Computer Program for Project Formulation Hydrology", and Technical Release No. 55 (TR-55) "Urban Hydrology for Small Watersheds" methods. This report uses the HydroCAD version 10.20 program, written by HydroCAD Software Solutions LLC, Chocorua, N.H., to apply these methods for the calculation of runoff and for pond modeling. Rainfall data and runoff curve numbers are taken from "The Stormwater Management and Erosion Control Handbook for Urban and Developing Areas in New Hampshire."

Time of Concentration (Tc) is calculated by entering measured flow path data such as flow path type, length, slope and surface characteristics into the HydroCAD program. For the purposes of this report, a minimum time of concentration of 5 minutes is used. The storm events used for the calculations in this report are the 2-year, 10-year, 25-year, and 50-year (24-hour) storms. Watershed basin boundaries have been delineated using topographic maps prepared by Ambit Engineering and field observations to confirm.

SITE SPECIFIC INFORMATION

Based on the United States Department of Agriculture (USDA) Natural Resources Conservation Service (NRCS), Soil Survey of Rockingham County, New Hampshire the site is made up of two soil types:

Soil Symbol	Soil Name and Slopes
799	Urban land – Canton Complex (3-15% slopes)

Canton complex is well drained with a stated depth to water table and restrictive feature of more than 80 inches. While there is a pond near the site which might suggest high runoff potential, the soil report and test pit observations suggested high infiltrative capacity, so the Hydrologic Soil Group will be assumed to be A, and the design infiltration rate will be 5 inches per hour.

The physical characteristics of the site consist of flat (0-15%) grades that generally slope from the south to the north. Elevations on the site range from 24 to 29 feet above sea level. The existing site is developed and includes an existing building located to the west of the lot, with a shed to the east. Vegetation around the developed portion of the lot consists of established grasses and some landscape areas. According to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) number 33015C0259F (effective date January 29, 2021), the proposed development is located in Zone X and is determined to be outside of the 0.2% annual chance floodplain. A copy of the FIRM map is included in the Appendix.

PRE-DEVELOPMENT DRAINAGE

In the pre-development condition, the site has been analyzed as two subcatchment basins (E1 and E2) based on localized topography and discharge location. Subcatchment E1 contains the west half of the property and flows toward the South Mill Pond (Discharge Point 1 or DP1). Subcatchment E2 contains the east half of the property and flows toward DP1.

 Table 1: Pre-Development Watershed Basin Summary

Watershed	Basin	Тс	CN	10-Year	50-Year	То
Basin ID	Area (SF)	(MIN)		Runoff (CFS)	Runoff (CFS)	Design
						Point
E1	2,922	5.0	56	0.09	0.25	DP1
E2	4,721	5.0	61	0.21	0.48	DP1

POST-DEVELOPMENT DRAINAGE

Proposed subcatchments P1, P2 and P2a occupy the same approximate space as subcatchments E1 and E2, with P1 matching E1 and both P2 and P2a matching E2. All subcatchments flow to the same discharge point. The peak discharge of P1 is mitigated with the use of a drip apron. The peak discharge of subcatchment P2a is mitigated with the use of permeable pavers. The subcatchments were analyzed for peak discharges using HydroCAD.

Watershed	Basin Area	Tc (MIN)	CN	10-Year	50-Year	Design
Basin ID	(SF)			Runoff	Runoff (CFS)	Point
				(CFS)		
P1	2,156	5.0	67	0.13	0.26	DP1
P2	2,820	5.0	55	0.09	0.23	DP1
P2a	2,667	5.0	84	0.27	0.46	DP1

Table 2: Post-Development Watershed Basin Sumn	iary
--	------

The overall impervious coverage of the subcatchment areas analyzed in this report **increases** from 1,824 square-feet (36.5%) in the pre-development condition to 3,415 square-feet (68.3%) in the post-development condition. The project proposes the construction of a drip apron and permeable pavers on site, reducing the peak flow discharge from the site as well as providing treatment.

Table 3 shows a summary of the comparison between pre-developed flows and postdeveloped flows for each design point. The comparison shows the reduced flows as a result of the drip apron and permeable pavers.

Table 3: Pre-Development to Post-Development Comparison

	Q2 (CFS)	Q10	(CFS)	Q50	(CFS)	
Design	Pre	Post	Pre	Post	Pre	Post	Description
Point							
DP1	0.08	0.02	0.30	0.13	0.52	0.49	South Mill Pond

The proposed development has been designed to match the pre-development drainage patterns to the greatest extent feasible. A plan sheet detailing the subcatchments and direction of runoff are included in the Attachments. In the developed condition, the site will have a drip apron and permeable pavers. As a result, discharge point DP1 will experience a net decrease in peak discharge for all design storms in the proposed condition. In addition, a catch basin is proposed adjacent to Rockland Street to help deal with off-site runoff. The catch basin is connected to an existing drainage network north of the property. The proposed drainage to the catch basin is detailed below for design purposes.

Table 4: Proposed Catch Basin Runoff

Design Point	Q2 (CFS)	Q10 (CFS)	Q25 (CFS)	Q50 (CFS)	Contributing
					Area (SF)
CB1	0.30	0.59	1.08	1.36	7,164

OFFSITE INFRASTRUCTURE CAPACITY

There is an overall reduction in off-site flow due to the drip apron and permeable pavers proposed by the project. The new basin will direct flow away from a city playground. The re-direction of flow will provide a net benefit to the City. As a result, there is no anticipated negative impact to City infrastructure.

EROSION AND SEDIMENT CONTROL PRACTICES

The erosion potential for this site as it exists is moderate due to the presence of construction areas that are highly erodible. During construction, the major potential for erosion is wind and stormwater runoff. The contractor will be required to inspect and maintain all necessary erosion control measures, as well as installing any additional measures as required. All erosion control practices shall conform to "The Stormwater Management and Erosion Control Handbook for Urban and Developing Areas in New Hampshire." Some examples of erosion and sediment control measures to be utilized for this project during construction may include:

- Silt Soxx (or approved alternative) located at the toe of disturbed slopes
- Stabilized construction entrance at access point to the site
- Temporary mulching and seeding for disturbed areas
- Spraying water over disturbed areas to minimize wind erosion

After construction, permanent stabilization will be accomplished by permanent seeding, landscaping, and compacting/surfacing the access drives with gravel.

CONCLUSION

The proposed development has been designed to match the pre-development drainage patterns to the greatest extent feasible. With the installation of the drip apron and permeable pavers, the post-development peak runoff will be sufficiently decreased to mitigate any issues caused by the proposed construction. Erosion and sediment control practices will be implemented for both the temporary condition during construction and for final stabilization after construction. The re-direction of street drainage away from the city playground is a net benefit. Therefore, there are no negative impacts to downstream receptors or adjacent properties anticipated as a result of this project.

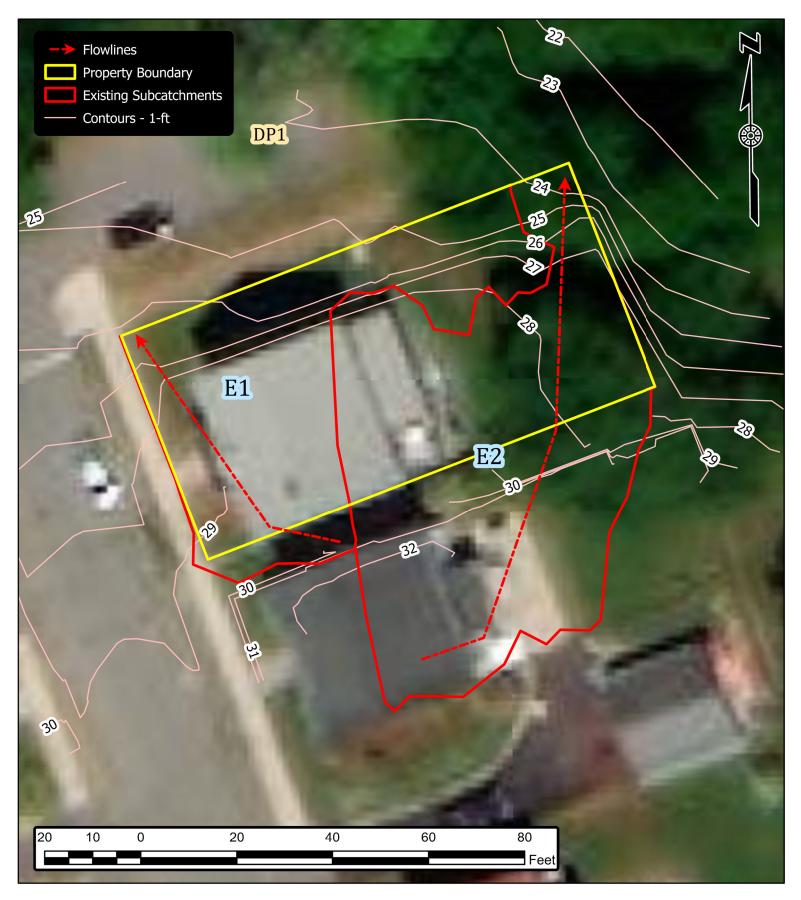
REFERENCES

- Comprehensive Environmental Inc. and New Hampshire Department of Environmental Services. *New Hampshire Stormwater Manual (Volumes 1, 2 and 3)*, December 2008 (Revision 1.0).
- Minnick, E.L. and H.T. Marshall. Stormwater Management and Erosion and Sediment Control Handbook for Urban and Developing Areas in New Hampshire, prepared by Rockingham County Conservation District, prepared for New Hampshire Department of Environmental Services, in cooperation with USDA Soil Conservation Service, August 1992.
- 3. HydroCAD Software Solution, LLC. *HydroCAD Stormwater Modeling System Version 10.20* copyright 2022.

AMBIT ENGINEERING, INC.

Existing Subcatchments

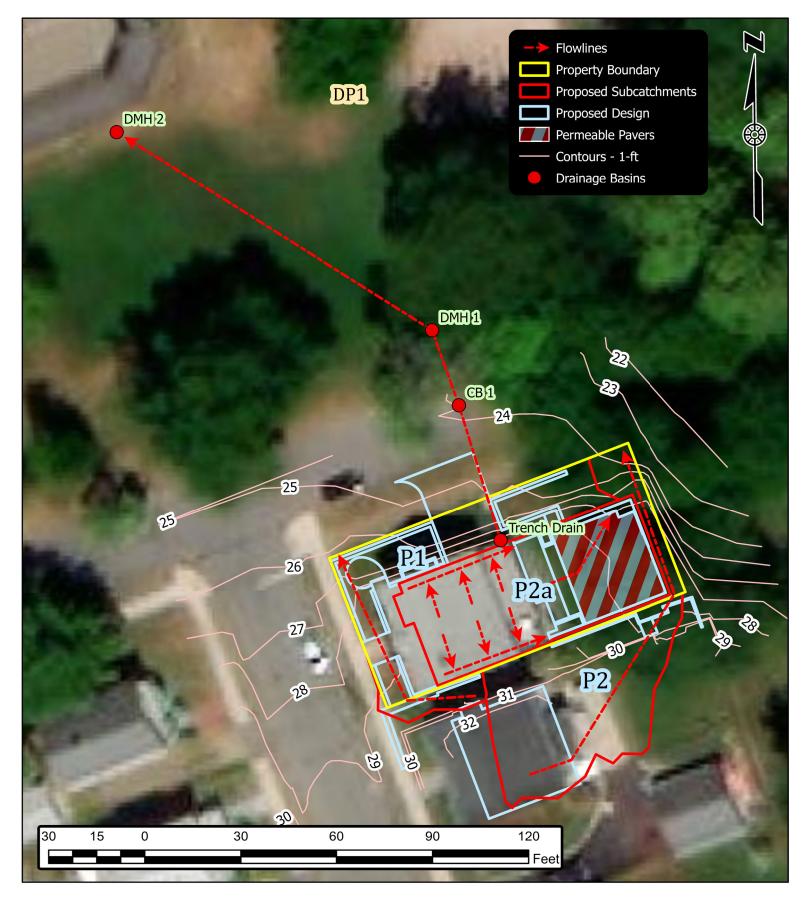
SMITH RESIDENCE 9 KENT STREET PORTSMOUTH, NH JOB NUMBER: 3492 SCALE: 1" = 20' SUBMITTED: 12-23-2022



AMBIT ENGINEERING, INC.

Proposed Subcatchments

SMITH RESIDENCE 9 KENT STREET PORTSMOUTH, NH JOB NUMBER: 3492 SCALE: 1" = 30' SUBMITTED: 02-28-2023



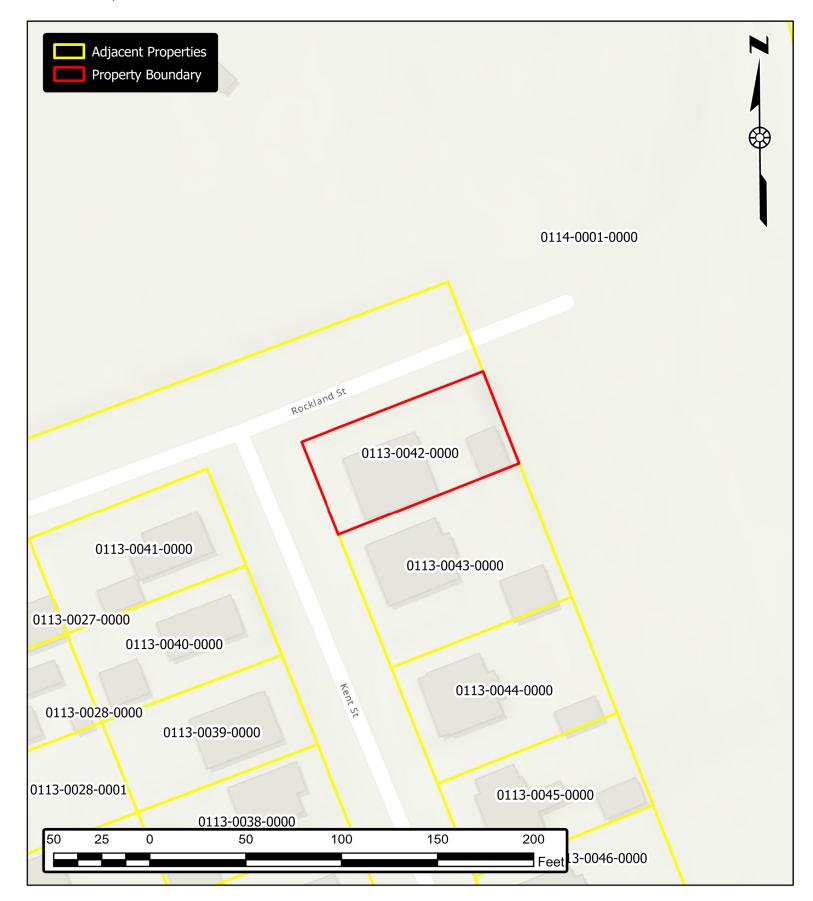
JN 3492

APPENDIX A

VICINITY (TAX) MAP



SMITH RESIDENCE 9 KENT STREET PORTSMOUTH, NH JOB NUMBER: 3492 SCALE: 1" = 50' SUBMITTED: 12-22-2022



JN 3492

APPENDIX B

TABLES, CHARTS, ETC.

Extreme Precipitation Tables

Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Smoothing	Yes
State	New Hampshire
Location	
Longitude	70.756 degrees West
Latitude	43.071 degrees North
Elevation	0 feet
Date/Time	Thu, 22 Dec 2022 14:33:27 -0500

Extreme Precipitation Estimates

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.26	0.40	0.50	0.65	0.81	1.04	1yr	0.70	0.98	1.21	1.56	2.03	2.66	2.92	1yr	2.35	2.81	3.22	3.94	4.55	1yr
2yr	0.32	0.50	0.62	0.81	1.02	1.30	2yr	0.88	1.18	1.52	1.94	2.49	3.21	3.57	2yr	2.84	3.43	3.94	4.68	5.33	2yr
5yr	0.37	0.58	0.73	0.98	1.25	1.61	5yr	1.08	1.47	1.89	2.43	3.14	4.07	4.58	5yr	3.60	4.40	5.04	5.94	6.70	5yr
10yr	0.41	0.65	0.82	1.12	1.45	1.89	10yr	1.25	1.73	2.23	2.89	3.75	4.86	5.53	10yr	4.31	5.32	6.09	7.11	7.98	10yr
25yr	0.48	0.76	0.97	1.34	1.78	2.34	25yr	1.53	2.14	2.78	3.63	4.74	6.17	7.10	25yr	5.46	6.83	7.81	9.03	10.05	25yr
50yr	0.54	0.86	1.10	1.54	2.08	2.76	50yr	1.79	2.53	3.29	4.33	5.66	7.39	8.58	50yr	6.54	8.25	9.43	10.81	11.97	50yr
100yr	0.60	0.97	1.25	1.77	2.42	3.26	100yr	2.09	2.98	3.91	5.16	6.77	8.85	10.38	100yr	7.83	9.98	11.39	12.96	14.27	100yr
200yr	0.68	1.10	1.43	2.05	2.83	3.84	200yr	2.44	3.52	4.62	6.14	8.08	10.60	12.55	200yr	9.38	12.06	13.76	15.55	17.01	200yr
500yr	0.80	1.32	1.72	2.49	3.49	4.78	500yr	3.01	4.39	5.78	7.71	10.22	13.47	16.14	500yr	11.92	15.52	17.68	19.78	21.48	500yr

Lower Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.23	0.36	0.44	0.59	0.72	0.88	1yr	0.63	0.86	0.93	1.33	1.69	2.24	2.49	1yr	1.98	2.39	2.87	3.19	3.90	1yr
2yr	0.31	0.49	0.60	0.81	1.00	1.19	2yr	0.86	1.16	1.37	1.82	2.34	3.06	3.45	2yr	2.71	3.32	3.82	4.55	5.09	2yr
5yr	0.35	0.54	0.67	0.92	1.17	1.40	5yr	1.01	1.37	1.61	2.12	2.73	3.78	4.19	5yr	3.35	4.03	4.72	5.53	6.24	5yr
10yr	0.39	0.59	0.73	1.03	1.33	1.60	10yr	1.14	1.56	1.80	2.39	3.05	4.37	4.85	10yr	3.87	4.67	5.43	6.41	7.19	10yr
25yr	0.44	0.67	0.83	1.19	1.56	1.90	25yr	1.35	1.86	2.10	2.75	3.53	4.73	5.88	25yr	4.19	5.65	6.64	7.78	8.67	25yr
50yr	0.48	0.73	0.91	1.31	1.76	2.17	50yr	1.52	2.12	2.35	3.06	3.92	5.35	6.78	50yr	4.73	6.52	7.71	9.03	10.00	50yr
100yr	0.54	0.81	1.01	1.46	2.01	2.47	100yr	1.73	2.41	2.62	3.40	4.33	6.02	7.82	100yr	5.32	7.52	8.95	10.49	11.55	100yr
200yr	0.59	0.89	1.13	1.63	2.27	2.81	200yr	1.96	2.75	2.93	3.77	4.77	6.75	9.02	200yr	5.97	8.68	10.38	12.20	13.35	200yr
500yr	0.68	1.02	1.31	1.90	2.71	3.36	500yr	2.33	3.28	3.41	4.30	5.43	7.86	10.89	500yr	6.95	10.47	12.63	14.92	16.17	500yr

Upper Confidence Limits

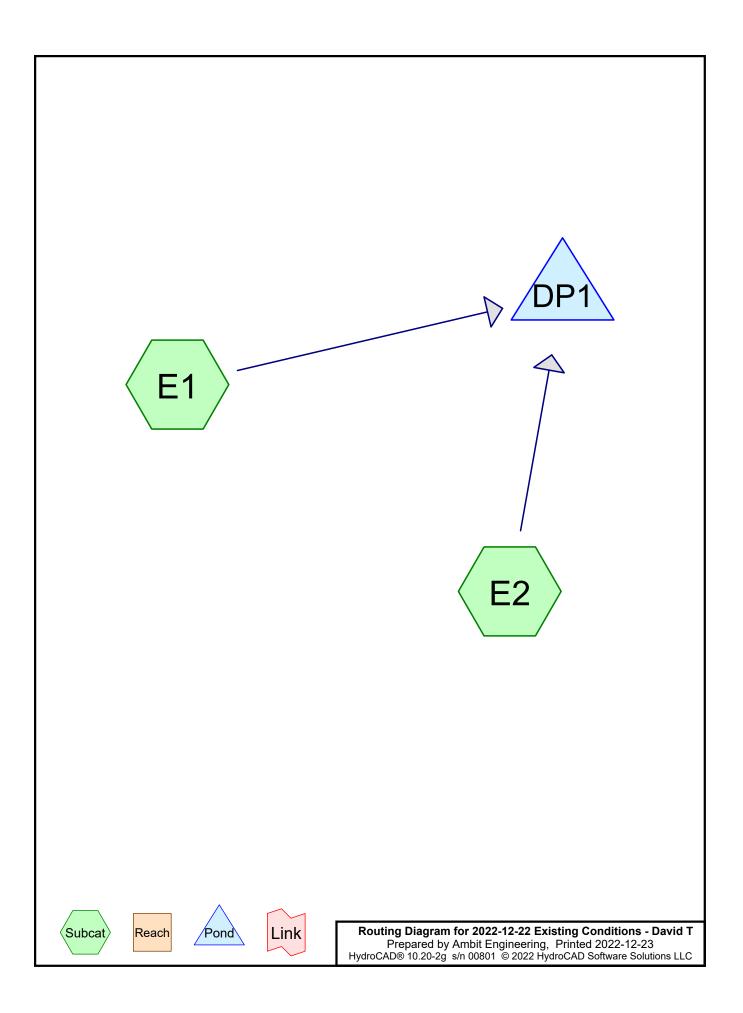
	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.28	0.44	0.54	0.72	0.89	1.08	1yr	0.77	1.06	1.26	1.74	2.20	2.98	3.17	1yr	2.64	3.05	3.58	4.37	5.04	1yr
2yr	0.34	0.52	0.64	0.87	1.07	1.27	2yr	0.92	1.24	1.48	1.96	2.52	3.42	3.71	2yr	3.03	3.56	4.09	4.84	5.63	2yr
5yr	0.40	0.62	0.77	1.05	1.34	1.62	5yr	1.15	1.59	1.89	2.54	3.25	4.34	4.97	5yr	3.84	4.78	5.38	6.38	7.16	5yr
10yr	0.47	0.72	0.89	1.25	1.61	1.98	10yr	1.39	1.93	2.28	3.11	3.96	5.34	6.21	10yr	4.72	5.97	6.83	7.85	8.76	10yr
25yr	0.58	0.88	1.09	1.56	2.05	2.57	25yr	1.77	2.52	2.96	4.08	5.16	7.76	8.36	25yr	6.87	8.04	9.17	10.35	11.42	25yr
50yr	0.67	1.02	1.27	1.83	2.47	3.13	50yr	2.13	3.06	3.60	5.01	6.34	9.71	10.48	50yr	8.59	10.08	11.48	12.74	13.98	50yr
100yr	0.79	1.20	1.50	2.16	2.97	3.82	100yr	2.56	3.73	4.38	6.17	7.79	12.15	13.14	100yr	10.75	12.63	14.36	15.72	17.11	100yr
200yr	0.93	1.39	1.77	2.56	3.57	4.66	200yr	3.08	4.56	5.35	7.60	9.57	15.23	16.48	200yr	13.48	15.85	18.00	19.38	20.94	200yr
500yr	1.15	1.71	2.20	3.20	4.55	6.06	500yr	3.93	5.92	6.94	10.05	12.62	20.58	22.27	500yr	18.21	21.41	24.26	25.55	27.37	500yr



APPENDIX C

HYDROCAD DRAINAGE

ANALYSIS CALCULATIONS



Project Notes

Defined 5 rainfall events from output (21) IDF

Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
	Name				(hours)		(inches)	
1	2-yr	Type III 24-hr		Default	24.00	1	3.69	2
2	10-yr	Type III 24-hr		Default	24.00	1	5.59	2
3	25-yr	Type III 24-hr		Default	24.00	1	7.10	2
4	50-yr	Type III 24-hr		Default	24.00	1	8.50	2

Rainfall Events Listing (selected events)

2022-12-22 Existing Conditions - David T Prepared by Ambit Engineering HydroCAD® 10.20-2g s/n 00801 © 2022 HydroCAD Software Solutions LLC

Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.115	39	>75% Grass cover, Good, HSG A (E1, E2)
0.001	50	Drip Apron (E1)
0.008	98	Paved parking, HSG A (E1, E2)
0.002	98	Paved parking, HSG A, Retwall (E1, E2)
0.050	98	Roofs, HSG A (E1, E2)
0.175	59	TOTAL AREA

2022-12-22 Existing Conditions - David T Prepared by Ambit Engineering HydroCAD® 10.20-2g s/n 00801 © 2022 HydroCAD Software Solutions LLC

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.175	HSG A	E1, E2
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.001	Other	E1
0.175		TOTAL AREA

2022-12-22 Existing Conditions - David T Prepared by Ambit Engineering HydroCAD® 10.20-2g s/n 00801 © 2022 HydroCAD Software Solutions LLC

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment
0.115	0.000	0.000	0.000	0.000	0.115	>75% Grass cover, Good	E1, E2
0.000	0.000	0.000	0.000	0.001	0.001	Drip Apron	E1
0.010	0.000	0.000	0.000	0.000	0.010	Paved parking	E1, E2
0.050	0.000	0.000	0.000	0.000	0.050	Roofs	E1, E2
0.175	0.000	0.000	0.000	0.001	0.175	TOTAL AREA	

Ground Covers (all nodes)

2022-12-22 Existing Conditions - David T	Type III 24-hr	2-yr Rainfall=3.69"
Prepared by Ambit Engineering		Printed 2022-12-23
HydroCAD® 10.20-2g s/n 00801 © 2022 HydroCAD Software Solutions LL	С	Page 7
Time span=0.00-24.00 hrs, dt=0.05 hrs, 4 Runoff by SCS TR-20 method, UH=SCS, W		

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment E1:

Runoff Area=2,922 sf 28.78% Impervious Runoff Depth>0.45" Tc=5.0 min CN=56 Runoff=0.02 cfs 0.003 af

Subcatchment E2:

Runoff Area=4,721 sf 37.60% Impervious Runoff Depth>0.66" Tc=5.0 min CN=61 Runoff=0.07 cfs 0.006 af

Pond DP1:

Inflow=0.08 cfs 0.008 af Primary=0.08 cfs 0.008 af

Total Runoff Area = 0.175 acRunoff Volume = 0.008 afAverage Runoff Depth = 0.58"65.77% Pervious = 0.115 ac34.23% Impervious = 0.060 ac

Summary for Subcatchment E1:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.02 cfs @ 12.12 hrs, Volume= Routed to Pond DP1 : 0.003 af, Depth> 0.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr Rainfall=3.69"

	A	rea (sf)	CN	Description					
		2,045	39	>75% Gras	s cover, Go	ood, HSG A			
*		49	98	Paved park	ing, HSG A	A, Retwall			
		733	98	Roofs, HSC	βĂ				
		59	98	Paved park	ing, HSG A	Ą			
*		36	50	Drip Apron	-				
		2,922	56	Weighted Average					
		2,081		71.22% Pervious Area					
		841		28.78% Impervious Area					
	Тс	Length	Slope	e Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
	5.0					Direct Entry,			
						-			

Summary for Subcatchment E2:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.07 cfs @ 12.10 hrs, Volume= 0.006 af, Depth> 0.66" Routed to Pond DP1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr Rainfall=3.69"

Α	rea (sf)	CN	Description					
	2,946	39	>75% Gras	s cover, Go	bood, HSG A			
*	23	98	Paved park	ing, HSG A	A, Retwall			
	1,451	98	Roofs, HSC	θĂ.				
	301	98	Paved park	ing, HSG A	Α			
	4,721	61	Weighted Average					
	2,946		62.40% Pervious Area					
	1,775		37.60% Imp	pervious Ar	rea			
-		~		A				
Tc	Length	Slope	,	Capacity	1			
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)				
5.0					Direct Entry,			

Summary for Pond DP1:

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	=	0.175 ac, 34.23% Imp	ervious, Inflow De	epth > 0.58"	for 2-yr event
Inflow =	=	0.08 cfs @ 12.11 hrs,	Volume=	0.008 af	
Primary =	=	0.08 cfs @ 12.11 hrs,	Volume=	0.008 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

2022-12-22 Existing Conditions - David T	Type III 24-hr 10-yr Rainfall=5.59"
Prepared by Ambit Engineering	Printed 2022-12-23
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Time span=0.00-24.00 hrs, dt=0.05 hrs Runoff by SCS TR-20 method, UH=SCS,	Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment E1:

Runoff Area=2,922 sf 28.78% Impervious Runoff Depth>1.36" Tc=5.0 min CN=56 Runoff=0.09 cfs 0.008 af

Subcatchment E2:

Runoff Area=4,721 sf 37.60% Impervious Runoff Depth>1.73" Tc=5.0 min CN=61 Runoff=0.21 cfs 0.016 af

Pond DP1:

Inflow=0.30 cfs 0.023 af Primary=0.30 cfs 0.023 af

Total Runoff Area = 0.175 acRunoff Volume = 0.023 afAverage Runoff Depth = 1.59"65.77% Pervious = 0.115 ac34.23% Impervious = 0.060 ac

Summary for Subcatchment E1:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.09 cfs @ 12.09 hrs, Volume= Routed to Pond DP1 : 0.008 af, Depth> 1.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr Rainfall=5.59"

	A	rea (sf)	CN	Description					
		2,045	39	>75% Gras	s cover, Go	ood, HSG A			
*		49	98	Paved park	ing, HSG A	A, Retwall			
		733	98	Roofs, HSC	βĂ				
		59	98	Paved park	ing, HSG A	Ą			
*		36	50	Drip Apron	а.				
		2,922	56	Weighted Average					
		2,081		71.22% Pervious Area					
		841		28.78% Impervious Area					
	Тс	Length	Slope	e Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
	5.0					Direct Entry,			
						-			

Summary for Subcatchment E2:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.21 cfs @ 12.09 hrs, Volume= 0.016 af, Depth> 1.73" Routed to Pond DP1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr Rainfall=5.59"

A	vrea (sf)	CN	Description						
	2,946	39	⊳75% Grass cover, Good, HSG A						
*	23	98	Paved park	ing, HSG A	A, Retwall				
	1,451	98	Roofs, HSC	βĂ					
	301	98	Paved park	ing, HSG A	Α				
	4,721	61	Weighted Average						
	2,946		62.40% Pervious Area						
	1,775		37.60% Impervious Area						
Tc	Length	Slope		Capacity	I I I I I I I I I I I I I I I I I I I				
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)					
5.0					Direct Entry,				

Summary for Pond DP1:

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	0.175 ac, 34.23% Impervious, Inflow Depth > 1.59" for 10-yr event
Inflow	=	0.30 cfs @ 12.09 hrs, Volume= 0.023 af
Primary	=	0.30 cfs @ 12.09 hrs, Volume= 0.023 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

2022-12-22 Existing Conditions - David T	Type III 24-hr 25-yr Rainfall=7.10"
Prepared by Ambit Engineering	Printed 2022-12-23
HydroCAD® 10.20-2g s/n 00801 © 2022 HydroCAD Software Solutions	LLC Page 13
Time span=0.00-24.00 hrs, dt=0.05 hrs Runoff by SCS TR-20 method, UH=SCS, Reach routing by Stor-Ind+Trans method - Pond ro	Weighted-CN

Subcatchment E1:

Runoff Area=2,922 sf 28.78% Impervious Runoff Depth>2.28" Tc=5.0 min CN=56 Runoff=0.17 cfs 0.013 af

Subcatchment E2:

Runoff Area=4,721 sf 37.60% Impervious Runoff Depth>2.77" Tc=5.0 min CN=61 Runoff=0.35 cfs 0.025 af

Pond DP1:

Inflow=0.52 cfs 0.038 af Primary=0.52 cfs 0.038 af

Total Runoff Area = 0.175 acRunoff Volume = 0.038 afAverage Runoff Depth = 2.58"65.77% Pervious = 0.115 ac34.23% Impervious = 0.060 ac

Summary for Subcatchment E1:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.17 cfs @ 12.09 hrs, Volume= Routed to Pond DP1 : 0.013 af, Depth> 2.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr Rainfall=7.10"

	A	rea (sf)	CN	Description					
		2,045	39	>75% Grass cover, Good, HSG A					
*		49	98	Paved park	ing, HSG A	A, Retwall			
		733	98	Roofs, HSC	βĂ				
		59	98	Paved park	ing, HSG A	Α			
*		36	50	Drip Apron	-				
		2,922	56	Weighted A	verage				
		2,081		71.22% Per	vious Area	а			
		841		28.78% Impervious Area					
	Тс	Length	Slope	e Velocity	Capacity	Description			
(r	min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
	5.0					Direct Entry,			

Summary for Subcatchment E2:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.35 cfs @ 12.08 hrs, Volume= 0.025 af, Depth> 2.77" Routed to Pond DP1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr Rainfall=7.10"

	Area (sf)	CN	Description				
	2,946	39	>75% Grass cover, Good, HSG A				
*	23	98	Paved park	ing, HSG A	A, Retwall		
	1,451	98	Roofs, HSC	βĂ			
	301	98	Paved park	ing, HSG A	Α		
	4,721	61	Weighted Average				
	2,946		62.40% Pervious Area				
	1,775		37.60% Impervious Area				
т.	1	<u>Olan</u>		0	Description		
Tc	5	Slope	,	Capacity	1		
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)			
5.0					Direct Entry,		

Summary for Pond DP1:

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =		0.175 ac, 34.23% Impervious, Inflow Depth > 2.58" for 25-yr event	
Inflow	=	0.52 cfs @ 12.08 hrs, Volume= 0.038 af	
Primary	=	0.52 cfs @ 12.08 hrs, Volume= 0.038 af, Atten= 0%, Lag= 0.0 min	

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

2022-12-22 Existing Conditions - David T	Type III 24-hr 50-y	r Rainfall=8.50"
Prepared by Ambit Engineering	Prir	nted 2022-12-23
HydroCAD® 10.20-2g s/n 00801 © 2022 HydroCAD Software Solutions	LLC	Page 16
Time span=0.00-24.00 hrs, dt=0.05 hrs, Runoff by SCS TR-20 method, UH=SCS, Reach routing by Stor-Ind+Trans method - Pond rou	Weighted-CN	od

Subcatchment E1:

Runoff Area=2,922 sf 28.78% Impervious Runoff Depth>3.24" Tc=5.0 min CN=56 Runoff=0.25 cfs 0.018 af

Subcatchment E2:

Runoff Area=4,721 sf 37.60% Impervious Runoff Depth>3.83" Tc=5.0 min CN=61 Runoff=0.48 cfs 0.035 af

Pond DP1:

Inflow=0.73 cfs 0.053 af Primary=0.73 cfs 0.053 af

Total Runoff Area = 0.175 acRunoff Volume = 0.053 afAverage Runoff Depth = 3.60"65.77% Pervious = 0.115 ac34.23% Impervious = 0.060 ac

Summary for Subcatchment E1:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.25 cfs @ 12.08 hrs, Volume= Routed to Pond DP1 : 0.018 af, Depth> 3.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr Rainfall=8.50"

	A	rea (sf)	CN	Description				
		2,045	39	>75% Gras	s cover, Go	ood, HSG A		
*		49	98	Paved park	ing, HSG A	, Retwall		
		733	98	Roofs, HSC	βĂ			
		59	98	Paved park	ing, HSG A	١		
*		36	50	Drip Apron	-			
		2,922	56	Weighted A	verage			
		2,081		71.22% Pervious Area				
		841		28.78% Impervious Area				
	Тс	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	5.0					Direct Entry,		
						-		

Summary for Subcatchment E2:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.48 cfs @ 12.08 hrs, Volume= 0.035 af, Depth> 3.83" Routed to Pond DP1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr Rainfall=8.50"

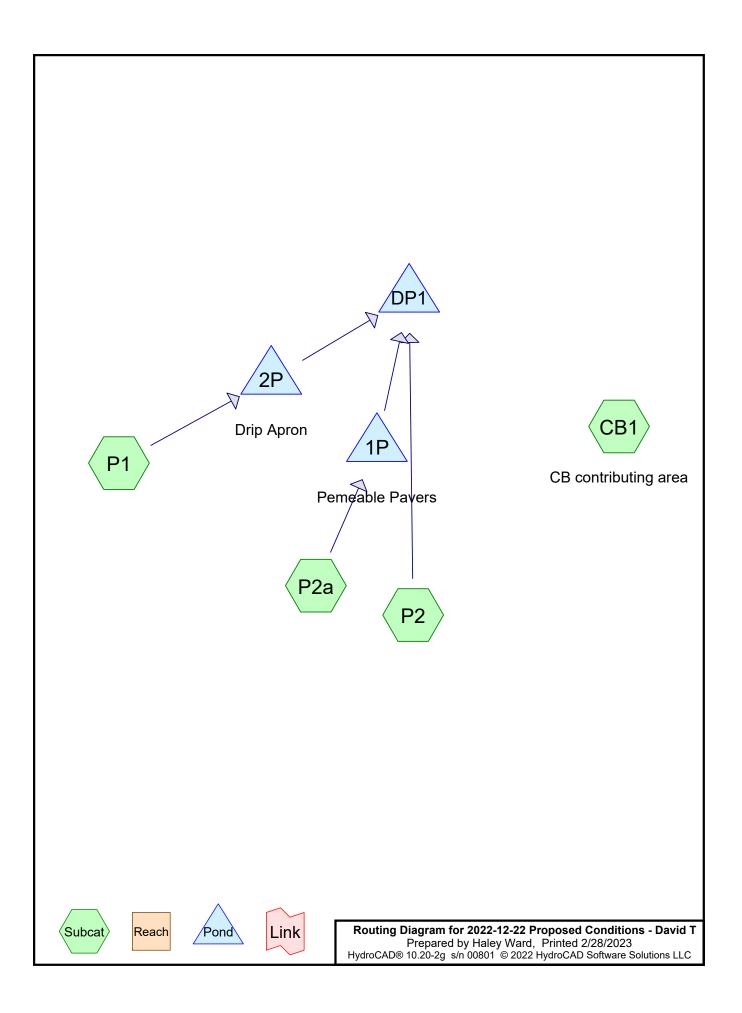
A	vrea (sf)	CN	Description				
	2,946	39	>75% Grass cover, Good, HSG A				
*	23	98	Paved park	ing, HSG A	A, Retwall		
	1,451	98	Roofs, HSC	βĂ			
	301	98	Paved park	ing, HSG A	Α		
	4,721	61	Weighted Average				
	2,946		62.40% Pervious Area				
	1,775		37.60% Impervious Area				
Tc	Length	Slope		Capacity	I		
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)			
5.0					Direct Entry,		

Summary for Pond DP1:

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =		0.175 ac, 34.23% Impervious, Inflo	ow Depth > 3.60" for 50-yr event	
Inflow	=	0.73 cfs @ 12.08 hrs, Volume=	0.053 af	
Primary	=	0.73 cfs @ 12.08 hrs, Volume=	0.053 af, Atten= 0%, Lag= 0.0 min	I

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



Project Notes

Defined 5 rainfall events from output (21) IDF

Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
	Name				(hours)		(inches)	
1	2-yr	Type III 24-hr		Default	24.00	1	3.69	2
2	10-yr	Type III 24-hr		Default	24.00	1	5.59	2
3	25-yr	Type III 24-hr		Default	24.00	1	7.10	2
4	50-yr	Type III 24-hr		Default	24.00	1	8.50	2

Rainfall Events Listing (selected events)

Area Listing (selected nodes)

Area	a CN	Description	
(acres)	(subcatchment-numbers)	
0.13	3 39	>75% Grass cover, Good, HSG A (CB1, P1, P2, P2a)	
0.004	4 50	Drip Apron (CB1, P1, P2)	
0.004	4 96	Gravel surface, HSG A (CB1, P1)	
0.114	4 98	Paved parking, HSG A (CB1, P1, P2a)	
0.00	9 98	Paved parking, HSG A, Retwall (P1, P2, P2a)	
0.01	6 50	Permeable Pavers (P1, P2a)	
0.05	9 98	Roofs, HSG A (CB1, P2, P2a)	
0.00	3 98	Water Surface, HSG A (P2a)	
0.34	0 72	TOTAL AREA	

Soil Listing (selected nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.321	HSG A	CB1, P1, P2, P2a
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.019	Other	CB1, P1, P2, P2a
0.340		TOTAL AREA

Printed 2/28/2023 Page 6

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
 (acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
 0.133	0.000	0.000	0.000	0.000	0.133	>75% Grass cover, Good	CB1, P1,
							P2, P2a
0.000	0.000	0.000	0.000	0.004	0.004	Drip Apron	CB1, P1,
							P2
0.004	0.000	0.000	0.000	0.000	0.004	Gravel surface	CB1, P1
0.123	0.000	0.000	0.000	0.000	0.123	Paved parking	CB1, P1,
							P2, P2a
0.000	0.000	0.000	0.000	0.016	0.016	Permeable Pavers	P1, P2a
0.059	0.000	0.000	0.000	0.000	0.059	Roofs	CB1, P2,
							P2a
0.003	0.000	0.000	0.000	0.000	0.003	Water Surface	P2a
0.321	0.000	0.000	0.000	0.019	0.340	TOTAL AREA	

Ground Covers (selected nodes)

2022-12-22 Proposed Conditions - De Prepared by Haley Ward	avid T Type III 24-hr 2-yr Rainfall=3.69" Printed 2/28/2023					
HydroCAD® 10.20-2g s/n 00801 © 2022 HydroC	CAD Software Solutions LLC Page 7					
Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method						
Subcatchment CB1: CB contributing area Flow Length=189'	Runoff Area=7,163 sf 62.40% Impervious Runoff Depth>1.57" Slope=0.0258 '/' Tc=5.7 min CN=77 Runoff=0.30 cfs 0.022 af					
SubcatchmentP1:	Runoff Area=2,156 sf 42.39% Impervious Runoff Depth>0.96" Tc=5.0 min CN=67 Runoff=0.05 cfs 0.004 af					
Subcatchment P2:	Runoff Area=2,820 sf 25.92% Impervious Runoff Depth>0.41" Tc=5.0 min CN=55 Runoff=0.02 cfs 0.002 af					
SubcatchmentP2a:	Runoff Area=2,667 sf 71.69% Impervious Runoff Depth>2.10" Tc=5.0 min CN=84 Runoff=0.15 cfs 0.011 af					
Pond 1P: Pemeable Pavers Discarded=0.04 cfs	Peak Elev=0.74' Storage=0.002 af Inflow=0.15 cfs 0.011 af 0.011 af Primary=0.00 cfs 0.000 af Outflow=0.04 cfs 0.011 af					

Pond 2P: Drip ApronPeak Elev=1.41' Storage=0.001 af Inflow=0.05 cfs 0.004 af
Discarded=0.01 cfs 0.004 af Primary=0.00 cfs 0.000 af Outflow=0.01 cfs 0.004 af

Pond DP1:

Inflow=0.02 cfs 0.002 af Primary=0.02 cfs 0.002 af

Total Runoff Area = 0.340 acRunoff Volume = 0.038 afAverage Runoff Depth = 1.36"45.79% Pervious = 0.156 ac54.21% Impervious = 0.184 ac

Summary for Subcatchment CB1: CB contributing area

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.30 cfs @ 12.09 hrs, Volume= 0.022 af, Depth> 1.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr Rainfall=3.69"

	A	rea (sf)	CN	Description			
		3,815	98	Paved park	ing, HSG A	١	
*		43	50	Drip Apron			
		82	96	Gravel surfa	ace, HSG A	A	
		168	98	Paved park	ing, HSG A	١	
		487	98	Roofs, HSC	βA		
		2,568	39	>75% Gras	s cover, Go	ood, HSG A	
		7,163	77	Weighted A	verage		
		2,693		37.60% Pei	vious Area		
		4,470		62.40% Imp	pervious Ar	ea	
	Тс	Length	Slope		Capacity	Description	
(m	nin)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	5.7	189	0.0258	0.55		Lag/CN Method,	

Summary for Subcatchment P1:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.05 cfs @ 12.09 hrs, Volume= 0.004 af, Depth> 0.96" Routed to Pond 2P : Drip Apron

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr Rainfall=3.69"

	A	rea (sf)	CN	Description		
		1,082	39	>75% Gras	s cover, Go	bod, HSG A
*		62	50	Drip Apron		
*		16	50	Permeable	Pavers	
		82	96	Gravel surfa	ace, HSG A	4
*		226	98	Paved park	ing, HSG A	A, Retwall
		688	98	Paved park	ing, HSG A	Α
		2,156	67	Weighted A	verage	
		1,242		57.61% Per	vious Area	l
		914		42.39% Imp	ervious Are	ea
(Tc min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description
	5.0					Direct Entry,

Summary for Subcatchment P2:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.02 cfs @ 12.14 hrs, Volume= Routed to Pond DP1 : 0.002 af, Depth> 0.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr Rainfall=3.69"

_	A	rea (sf)	CN	Description		
*		54	50	Drip Apron		
		2,035	39	>75% Gras	s cover, Go	lood, HSG A
*		23	98	Paved park	ing, HSG A	A, Retwall
_		708	98	Roofs, HSC	6 A	
		2,820	55	Weighted A	verage	
		2,089		74.08% Per	rvious Area	а
		731		25.92% Imp	pervious Ar	rea
	-				A	
	ŢĊ	Length	Slope		Capacity	
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
	5.0					Direct Entry,

Summary for Subcatchment P2a:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.15 cfs @ 12.08 hrs, Volume= 0.011 af, Depth> 2.10" Routed to Pond 1P : Pemeable Pavers

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr Rainfall=3.69"

	A	ea (sf)	CN	Description		
*		665	50	Permeable	Pavers	
		289	98	Paved park	ing, HSG A	4
		131	98	Water Surfa	ace, HSG A	4
		90	39	>75% Gras	s cover, Go	ood, HSG A
*		135	98	Paved park	ing, HSG A	A, Retwall
		680	98	Roofs, HSC	6 A	
		677	98	Roofs, HSC	6 A	
		2,667	84	Weighted A	verage	
		755		28.31% Per	vious Area	3
		1,912		71.69% Imp	pervious Ar	rea
	Тс	Length	Slop	e Velocity	Capacity	Description
(n	nin)	(feet)	(ft/f) (ft/sec)	(cfs)	
	5.0					Direct Entry,

Summary for Pond 1P: Pemeable Pavers

Inflow Area = 0.061 ac, 71.69% Impervious, Inflow Depth > 2.10" for 2-yr event Inflow 0.15 cfs @ 12.08 hrs, Volume= 0.011 af = 0.04 cfs @ 11.85 hrs, Volume= Outflow = 0.011 af, Atten= 74%, Lag= 0.0 min 0.04 cfs @ 11.85 hrs, Volume= Discarded = 0.011 af Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af Routed to Pond DP1 :

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 0.74' @ 12.46 hrs Surf.Area= 0.008 ac Storage= 0.002 af

Plug-Flow detention time= 14.9 min calculated for 0.011 af (100% of inflow) Center-of-Mass det. time= 14.5 min (836.3 - 821.8)

Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	0.012 af	24.00'W x 14.00'L x 4.00'H Prismatoid
			0.031 af Overall x 40.0% Voids
#2	4.00'	0.008 af	24.00'W x 14.00'L x 1.00'H Prismatoid
		0.020 af	Total Available Storage
Device	Routing	Invert Ou	tlet Devices

#1	Discarded	0.00'	5.000 in/hr Exfiltration over Horizontal area Phase-In= 0.01'
#2	Primary	4.00'	38.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
			0.5' Crest Height

Discarded OutFlow Max=0.04 cfs @ 11.85 hrs HW=0.05' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=0.00' (Free Discharge) ←2=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 2P: Drip Apron

Inflow Area =	0.049 ac, 42.39% Imp	pervious, Inflow De	epth > 0.96"	for 2-yr event		
Inflow =	0.05 cfs @ 12.09 hrs	, Volume=	0.004 af	-		
Outflow =	0.01 cfs @ 11.90 hrs	, Volume=	0.004 af, Atte	en= 82%, Lag= 0.0 min		
Discarded =	0.01 cfs @ 11.90 hrs	, Volume=	0.004 af	-		
Primary =	0.00 cfs @ 0.00 hrs	, Volume=	0.000 af			
Routed to Pond DP1 :						

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 1.41' @ 12.67 hrs Surf.Area= 0.002 ac Storage= 0.001 af

Plug-Flow detention time= 35.0 min calculated for 0.004 af (100% of inflow) Center-of-Mass det. time= 34.6 min (908.5 - 874.0)

2022-12-22 Proposed Conditions - David T

Prepared by Haley Ward

 Type III 24-hr
 2-yr Rainfall=3.69"

 Printed
 2/28/2023

 C
 Page 11

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Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	0.002 af	13.00'W x 6.00'L x 3.00'H Prismatoid
			0.005 af Overall x 40.0% Voids
#2	3.00'	0.002 af	13.00'W x 6.00'L x 1.00'H Prismatoid
		0.004 af	Total Available Storage
Device	Routing	Invert O	utlet Devices
#1	Discarded	0.00' 5 .	.000 in/hr Exfiltration over Horizontal area Phase-In= 0.01'
#2	Primary		4.0' Iong Sharp-Crested Rectangular Weir 0 End Contraction(s)
		0.	.5' Crest Height
	_		

Discarded OutFlow Max=0.01 cfs @ 11.90 hrs HW=0.04' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=0.00' (Free Discharge) ←2=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond DP1:

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	0.175 ac, 46.54% Impervious, Inflow Depth > 0.15" for 2-yr	event
Inflow	=	0.02 cfs @ 12.14 hrs, Volume= 0.002 af	
Primary	=	0.02 cfs @ 12.14 hrs, Volume= 0.002 af, Atten= 0%, L	_ag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

2022-12-22 Proposed Conditions - I Prepared by Haley Ward	David T	Type III 24-hr	<i>10-yr Rainfall=5.59"</i> Printed 2/28/2023			
HydroCAD® 10.20-2g s/n 00801 © 2022 Hydro	CAD Software Solutions	LLC	Page 12			
Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method						
Subcatchment CB1: CB contributing area Flow Length=189			ous Runoff Depth>3.12" Runoff=0.59 cfs 0.043 af			
SubcatchmentP1:			ous Runoff Depth>2.22" Runoff=0.13 cfs 0.009 af			
SubcatchmentP2:			ous Runoff Depth>1.29" Runoff=0.09 cfs 0.007 af			
Subcatchment P2a:			ous Runoff Depth>3.81" Runoff=0.27 cfs 0.019 af			
Pond 1P: Pemeable Pavers Discarded=0.04 c		•	Inflow=0.27 cfs 0.019 af utflow=0.04 cfs 0.019 af			
Pond 2P: Drip Apron Discarded=0.02 c			Inflow=0.13 cfs 0.009 af utflow=0.08 cfs 0.009 af			
Pond DP1:		Ρ	Inflow=0.12 cfs 0.008 af rimary=0.12 cfs 0.008 af			
Total Runoff Area = 0.340 ac Runoff Volume = 0.078 af Average Runoff Depth = 2.77" 45.79% Pervious = 0.156 ac 54.21% Impervious = 0.184 ac						

Summary for Subcatchment CB1: CB contributing area

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.59 cfs @ 12.09 hrs, Volume= 0.043 af, Depth> 3.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr Rainfall=5.59"

	A	rea (sf)	CN	Description					
		3,815	98	Paved park	ing, HSG A	١			
*		43	50	Drip Apron					
		82	96	Gravel surfa	ace, HSG A	A			
		168	98	Paved park	ing, HSG A	١			
		487	98	Roofs, HSC	βA				
		2,568	39	>75% Grass cover, Good, HSG A					
		7,163	77	77 Weighted Average					
		2,693		37.60% Pervious Area					
		4,470		62.40% Imp	pervious Ar	ea			
	Тс	Length	Slope		Capacity	Description			
(m	nin)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	5.7	189	0.0258	0.55		Lag/CN Method,			

Summary for Subcatchment P1:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.13 cfs @ 12.08 hrs, Volume= 0.009 af, Depth> 2.22" Routed to Pond 2P : Drip Apron

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr Rainfall=5.59"

	A	rea (sf)	CN	Description				
		1,082	39	>75% Gras	s cover, Go	bod, HSG A		
*		62	50	Drip Apron				
*		16	50	Permeable	Pavers			
		82	96	Gravel surfa	ace, HSG A	4		
*		226	98	Paved park	ing, HSG A	A, Retwall		
		688	98	8 Paved parking, HSG A				
		2,156	156 67 Weighted Average					
		1,242		57.61% Per	vious Area	l		
		914		42.39% Imp	ervious Are	ea		
(Tc min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description		
	5.0					Direct Entry,		

Summary for Subcatchment P2:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.09 cfs @ 12.09 hrs, Volume= Routed to Pond DP1 : 0.007 af, Depth> 1.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr Rainfall=5.59"

	A	rea (sf)	CN	Description					
*		54	50	Drip Apron					
		2,035	39	>75% Gras	s cover, Go	ood, HSG A			
*		23	98	Paved park	ing, HSG A	A, Retwall			
		708	98	Roofs, HSC	β A				
		2,820 55 Weighted Average							
		2,089	2,089 74.08% Pervious Area						
		731		25.92% Imp	pervious Ar	rea			
	Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)				
	5.0					Direct Entry,			

Summary for Subcatchment P2a:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.27 cfs @ 12.07 hrs, Volume= 0.019 a Routed to Pond 1P : Pemeable Pavers

0.019 af, Depth> 3.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr Rainfall=5.59"

	Area	(sf)	CN	Description		
*		665	50	Permeable	Pavers	
		289	98	Paved park	ing, HSG A	4
		131	98	Water Surfa	ace, HSG A	4
		90	39	>75% Gras	s cover, Go	ood, HSG A
*		135		Paved park		A, Retwall
		680	98	Roofs, HSC	6 Á	
		677	98	Roofs, HSC	6 A	
	2,	667	84	Weighted A	verage	
		755		28.31% Pei	vious Area	3
	1,	912		71.69% Imp	ervious Ar	rea
				-		
	Tc Le	ength	Slope	e Velocity	Capacity	Description
(m	in) ((feet)	(ft/ft) (ft/sec)	(cfs)	
ł	5.0					Direct Entry,

Summary for Pond 1P: Pemeable Pavers

Inflow Area = 0.061 ac, 71.69% Impervious, Inflow Depth > 3.81" for 10-yr event Inflow 0.27 cfs @ 12.07 hrs, Volume= 0.019 af = 0.04 cfs @ 11.70 hrs, Volume= Outflow = 0.019 af, Atten= 86%, Lag= 0.0 min 0.04 cfs @ 11.70 hrs, Volume= Discarded = 0.019 af Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af Routed to Pond DP1 :

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 1.96' @ 12.59 hrs Surf.Area= 0.008 ac Storage= 0.006 af

Plug-Flow detention time= 45.8 min calculated for 0.019 af (100% of inflow) Center-of-Mass det. time= 45.4 min (850.3 - 804.9)

Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	0.012 af	24.00'W x 14.00'L x 4.00'H Prismatoid
			0.031 af Overall x 40.0% Voids
#2	4.00'	0.008 af	24.00'W x 14.00'L x 1.00'H Prismatoid
		0.020 af	Total Available Storage
Device	Routing	Invert Ou	tlet Devices

#1	Discarded	0.00'	5.000 in/hr Exfiltration over Horizontal area Phase-In= 0.01'
#2	Primary	4.00'	38.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
			0.5' Crest Height

Discarded OutFlow Max=0.04 cfs @ 11.70 hrs HW=0.06' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=0.00' (Free Discharge) ←2=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 2P: Drip Apron

Inflow Area =	0.049 ac, 4	2.39% Impervious, Inflov	v Depth > 2.22"	for 10-yr event				
Inflow =	0.13 cfs @	12.08 hrs, Volume=	0.009 af	-				
Outflow =	0.08 cfs @	12.21 hrs, Volume=	0.009 af, Atte	n= 35%, Lag= 7.9 min				
Discarded =	0.02 cfs @	12.20 hrs, Volume=	0.008 af					
Primary =	0.06 cfs @	12.21 hrs, Volume=	0.001 af					
Routed to Pond DP1 :								

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 3.00' @ 12.20 hrs Surf.Area= 0.004 ac Storage= 0.002 af

Plug-Flow detention time= 80.5 min calculated for 0.009 af (100% of inflow) Center-of-Mass det. time= 80.1 min (927.9 - 847.7)

2022-12-22 Proposed Conditions - David T

Prepared by Haley Ward

Type III 24-hr 10-yr Rainfall=5.59" Printed 2/28/2023 LC Page 16

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Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	0.002 af	
			0.005 af Overall x 40.0% Voids
#2	3.00'	0.002 af	13.00'W x 6.00'L x 1.00'H Prismatoid
		0.004 af	Total Available Storage
			-
Device	Routing	Invert Ou	utlet Devices
#1	Discarded	0.00' 5.	000 in/hr Exfiltration over Horizontal area Phase-In= 0.01'
#2	Primary		I.0' Iong Sharp-Crested Rectangular Weir 0 End Contraction(s)
		0.9	5' Crest Height

Discarded OutFlow Max=0.02 cfs @ 12.20 hrs HW=3.00' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.01 cfs @ 12.21 hrs HW=3.00' (Free Discharge) **2=Sharp-Crested Rectangular Weir** (Weir Controls 0.01 cfs @ 0.16 fps)

Summary for Pond DP1:

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =		0.175 ac, 46.54% Impervious, Inflow Depth > 0.55" for 10-yr event
Inflow	=	0.12 cfs @ 12.21 hrs, Volume= 0.008 af
Primary	=	0.12 cfs $@$ 12.21 hrs, Volume= 0.008 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

2022-12-22 Proposed Conditions - Prepared by Haley Ward	David T	Type III 24-hr 25-yr Rainfall=7.10" Printed 2/28/2023						
HydroCAD® 10.20-2g s/n 00801 © 2022 Hydro	oCAD Software Solutions							
Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method								
Subcatchment CB1: CB contributing area Flow Length=189		62.40% Impervious Runoff Depth>4.45" 5.7 min CN=77 Runoff=0.84 cfs 0.061 af						
Subcatchment P1:		42.39% Impervious Runoff Depth>3.38" 5.0 min CN=67 Runoff=0.20 cfs 0.014 af						
Subcatchment P2:		25.92% Impervious Runoff Depth>2.19" 5.0 min CN=55 Runoff=0.16 cfs 0.012 af						
Subcatchment P2a:		71.69% Impervious Runoff Depth>5.23" 5.0 min CN=84 Runoff=0.37 cfs 0.027 af						
Pond 1P: Pemeable Pavers Discarded=0.04 c		Storage=0.009 af Inflow=0.37 cfs 0.027 af 00 cfs 0.000 af Outflow=0.04 cfs 0.027 af						
Pond 2P: Drip Apron Discarded=0.02 c		Storage=0.002 af Inflow=0.20 cfs 0.014 af 82 cfs 0.004 af Outflow=0.34 cfs 0.014 af						
Pond DP1:		Inflow=0.48 cfs 0.015 af Primary=0.48 cfs 0.015 af						
Total Runoff Area = 0.340 a	ac Runoff Volume = (45.79% Pervious = 0.1	U I						

Summary for Subcatchment CB1: CB contributing area

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.84 cfs @ 12.09 hrs, Volume= 0.061 af, Depth> 4.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr Rainfall=7.10"

vrea (sf)	CN	Description					
3,815	98	Paved park	ing, HSG A	L .			
43	50	Drip Apron					
82	96	Gravel surfa	ace, HSG A	۱.			
168	98	Paved park	ing, HSG A	l l			
487	98	Roofs, HSG	ΪĂ				
2,568	39 :	>75% Grass cover, Good, HSG A					
7,163	77	77 Weighted Average					
2,693		37.60% Pervious Area					
4,470	(52.40% Imp	ervious Ar	ea			
Length	Slope	Velocity	Capacity	Description			
(feet)	(ft/ft)	(ft/sec)	(cfs)				
189	0.0258	0.55		Lag/CN Method,			
	43 82 168 487 2,568 7,163 2,693 4,470 Length (feet)	3,815 98 1 43 50 1 82 96 0 168 98 1 487 98 1 2,568 39 2 7,163 77 1 2,693 3 3 4,470 6 Length Slope (feet) (ft/ft)	3,815 98 Paved parki 43 50 Drip Apron 82 96 Gravel surfa 168 98 Paved parki 487 98 Roofs, HSG 2,568 39< >75% Grass 7,163 77 Weighted A 2,693 37.60% Per 4,470 62.40% Imp Length Slope Velocity (feet) (ft/ft) (ft/sec)	3,81598Paved parking, HSG A4350Drip Apron8296Gravel surface, HSG A16898Paved parking, HSG A48798Roofs, HSG A2,56839>75% Grass cover, Go7,16377Weighted Average2,69337.60% Pervious Area4,47062.40% Impervious AreaLengthSlopeVelocity(feet)(ft/ft)(ft/sec)(cfs)	3,81598Paved parking, HSG A4350Drip Apron8296Gravel surface, HSG A16898Paved parking, HSG A48798Roofs, HSG A2,56839>75% Grass cover, Good, HSG A7,16377Weighted Average2,69337.60% Pervious Area4,47062.40% Impervious AreaLengthSlopeVelocity(feet)(ft/ft)(ft/sec)(cfs)		

Summary for Subcatchment P1:

[49] Hint: Tc<2dt may require smaller dt

Runoff	=	0.20 cfs @	12.08 hrs,	Volume=	0.014 af,	Depth> 3.38"
Routed	to Pond	2P : Drip Ap	ron			-

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr Rainfall=7.10"

	Area (sf)	CN	Description		
	1,082	39	>75% Grass c	over, Go	bod, HSG A
*	62	50	Drip Apron		
*	16	50	Permeable Pa	vers	
	82	96	Gravel surface	, HSG A	Α
*	226	98	Paved parking	, HSG A	A, Retwall
	688	98	Paved parking	, HSG A	Α
	2,156	67	Weighted Aver	rage	
	1,242		57.61% Pervio	ous Area	1
	914		42.39% Imperv	vious Are	ea
(mi	Tc Length n) (feet)	Slop (ft/f		apacity (cfs)	Description
5	0.0				Direct Entry,

Summary for Subcatchment P2:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.16 cfs @ 12.09 hrs, Volume= Routed to Pond DP1 : 0.012 af, Depth> 2.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr Rainfall=7.10"

_	A	rea (sf)	CN	Description		
*		54	50	Drip Apron		
		2,035	39	>75% Gras	s cover, Go	ood, HSG A
*		23	98	Paved park	ing, HSG A	A, Retwall
_		708	98	Roofs, HSC	βA	
		2,820	55	Weighted A	verage	
		2,089		74.08% Pei	rvious Area	а
		731		25.92% Imp	pervious Are	rea
	-		01		0	
	Tc	Length	Slope		Capacity	
_	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
	5.0					Direct Entry,

Summary for Subcatchment P2a:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.37 cfs @ 12.07 hrs, Volume= Routed to Pond 1P : Pemeable Pavers 0.027 af, Depth> 5.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr Rainfall=7.10"

	Area (sf)	CN	Description		
*	665	50	Permeable	Pavers	
	289	98	Paved park	ing, HSG A	4
	131	98	Water Surfa	ace, HSG A	4
	90	39	>75% Gras	s cover, Go	ood, HSG A
*	135	98	Paved park	ing, HSG A	A, Retwall
	680	98	Roofs, HSC	6 A	
	677	98	Roofs, HSC	β A	
	2,667	84	Weighted A	verage	
	755		28.31% Per	vious Area	3
	1,912		71.69% Imp	pervious Ar	rea
			-		
-	Tc Length	Slop	e Velocity	Capacity	Description
(mi	n) (feet)	(ft/	ft) (ft/sec)	(cfs)	
5	.0				Direct Entry,

Summary for Pond 1P: Pemeable Pavers

Inflow Area = 0.061 ac. 71.69% Impervious. Inflow Depth > 5.23" for 25-vr event Inflow 0.37 cfs @ 12.07 hrs, Volume= 0.027 af = 0.04 cfs @ 11.60 hrs, Volume= Outflow = 0.027 af, Atten= 89%, Lag= 0.0 min 0.04 cfs @ 11.60 hrs, Volume= 0.027 af Discarded = Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af Routed to Pond DP1 : Routing by Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs. Peak Elev= 3.05' @ 12.82 hrs Surf.Area= 0.008 ac Storage= 0.009 af Plug-Flow detention time= 78.2 min calculated for 0.027 af (100% of inflow) Center-of-Mass det. time= 77.9 min (873.8 - 796.0) Volume Invert Avail.Storage Storage Description 0.012 af 24.00'W x 14.00'L x 4.00'H Prismatoid #1 0.00' 0.031 af Overall x 40.0% Voids 4.00' 0.008 af 24.00'W x 14.00'L x 1.00'H Prismatoid #2 0.020 af Total Available Storage Routing Invert Outlet Devices Device #1 Discarded 0.00' 5.000 in/hr Exfiltration over Horizontal area Phase-In= 0.01' #2 Primary 4.00' 38.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) 0.5' Crest Height

Discarded OutFlow Max=0.04 cfs @ 11.60 hrs HW=0.05' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=0.00' (Free Discharge) ←2=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 2P: Drip Apron

[88] Warning: Qout>Qin may require smaller dt or Finer Routing [85] Warning: Oscillations may require smaller dt or Finer Routing (severity=8)

Inflow Area =	0.049 ac, 4	12.39% Impervious,	Inflow Depth > 3.38"	for 25-yr event		
Inflow =	0.20 cfs @	12.08 hrs, Volume=	= 0.014 af			
Outflow =	0.34 cfs @	12.10 hrs, Volume=	= 0.014 af, Att	en= 0%, Lag= 1.2 min		
Discarded =	0.02 cfs @	12.10 hrs, Volume=	= 0.010 af			
Primary =	0.32 cfs @	12.10 hrs, Volume=	= 0.004 af			
Routed to Pond DP1 :						

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 3.01' @ 12.10 hrs Surf.Area= 0.004 ac Storage= 0.002 af

Plug-Flow detention time= 66.5 min calculated for 0.014 af (100% of inflow) Center-of-Mass det. time= 66.0 min (901.5 - 835.4) 2022-12-22 Proposed Conditions - David T

Prepared by Haley Ward

 Type III 24-hr
 25-yr Rainfall=7.10"

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 LC
 Page 21

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Volume	Invert	Avail.Storage	Storage Description				
#1	0.00'	0.002 af	13.00'W x 6.00'L x 3.00'H Prismatoid				
			0.005 af Overall x 40.0% Voids				
#2	3.00'	0.002 af	13.00'W x 6.00'L x 1.00'H Prismatoid				
		0.004 af	Total Available Storage				
Device	Routing	Invert O	utlet Devices				
#1	Discarded	0.00' 5.	.000 in/hr Exfiltration over Horizontal area Phase-In= 0.01'				
#2	Primary		4.0' long Sharp-Crested Rectangular Weir 0 End Contraction(s) .5' Crest Height				
$\tilde{\mathbf{v}}$							

Discarded OutFlow Max=0.02 cfs @ 12.10 hrs HW=3.01' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.19 cfs @ 12.10 hrs HW=3.01' (Free Discharge) **2=Sharp-Crested Rectangular Weir** (Weir Controls 0.19 cfs @ 0.39 fps)

Summary for Pond DP1:

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	0.175 ac, 46.54% Impervious, Inflow Depth > 1.06" for 25-yr even	t
Inflow	=	0.48 cfs @ 12.10 hrs, Volume= 0.015 af	
Primary	=	0.48 cfs @ 12.10 hrs, Volume= 0.015 af, Atten= 0%, Lag= 0	.0 min

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

2022-12-22 Proposed Conditions - I Prepared by Haley Ward	David T	Type III 24-hr 50-yr Rainfall=8.50" Printed 2/28/2023						
HydroCAD® 10.20-2g s/n 00801 © 2022 Hydro	CAD Software Solutions							
Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method								
Subcatchment CB1: CB contributing area Flow Length=189		f 62.40% Impervious Runoff Depth>5.73" 5.7 min CN=77 Runoff=1.08 cfs 0.079 af						
SubcatchmentP1:		f 42.39% Impervious Runoff Depth>4.54" 5.0 min CN=67 Runoff=0.26 cfs 0.019 af						
SubcatchmentP2:		f 25.92% Impervious Runoff Depth>3.13" 5.0 min CN=55 Runoff=0.23 cfs 0.017 af						
SubcatchmentP2a:		f 71.69% Impervious Runoff Depth>6.57" 5.0 min CN=84 Runoff=0.46 cfs 0.034 af						
Pond 1P: Pemeable Pavers Discarded=0.07 c		Storage=0.012 af Inflow=0.46 cfs 0.034 af 00 cfs 0.000 af Outflow=0.07 cfs 0.034 af						
Pond 2P: Drip Apron Discarded=0.02 c		Storage=0.002 af Inflow=0.26 cfs 0.019 af 26 cfs 0.007 af Outflow=0.28 cfs 0.019 af						
Pond DP1:		Inflow=0.49 cfs 0.023 af Primary=0.49 cfs 0.023 af						
Total Runoff Area = 0.340 ac Runoff Volume = 0.148 af Average Runoff Depth = 5.21" 45.79% Pervious = 0.156 ac 54.21% Impervious = 0.184 ac								

Summary for Subcatchment CB1: CB contributing area

[49] Hint: Tc<2dt may require smaller dt

Runoff = 1.08 cfs @ 12.09 hrs, Volume= 0.079 af, Depth> 5.73"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr Rainfall=8.50"

	Area (sf)	CN	Description				
	3,815	98	Paved park	ing, HSG A	۱.		
*	43	50	Drip Apron				
	82	96	Gravel surfa	ace, HSG A	A Contraction of the second seco		
	168	98	Paved park	ing, HSG A	l l		
	487	98	Roofs, HSO	βA			
	2,568	39	>75% Gras	s cover, Go	ood, HSG A		
	7,163	77	77 Weighted Average				
	2,693		37.60% Pei	vious Area			
	4,470		62.40% Imp	pervious Ar	ea		
Тс	Length	Slope		Capacity	Description		
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)			
5.7	189	0.0258	0.55		Lag/CN Method,		
5.7	105	0.0200	0.00		Eug/ort Method,		

Summary for Subcatchment P1:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.26 cfs @ 12.08 hrs, Volume= 0.019 af, Depth> 4.54" Routed to Pond 2P : Drip Apron

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr Rainfall=8.50"

	Area (sf)	CN	Description		
	1,082	39	>75% Grass c	over, Go	bod, HSG A
*	62	50	Drip Apron		
*	16	50	Permeable Pa	vers	
	82	96	Gravel surface	, HSG A	Α
*	226	98	Paved parking	, HSG A	A, Retwall
	688	98	Paved parking	, HSG A	Α
	2,156	67	Weighted Aver	rage	
	1,242		57.61% Pervio	ous Area	1
	914		42.39% Imperv	vious Are	ea
(mi	Tc Length n) (feet)	Slop (ft/f		apacity (cfs)	Description
5	0.0				Direct Entry,

Summary for Subcatchment P2:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.23 cfs @ 12.08 hrs, Volume= Routed to Pond DP1 : 0.017 af, Depth> 3.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr Rainfall=8.50"

	A	rea (sf)	CN	Description		
*		54	50	Drip Apron		
		2,035	39	>75% Gras	s cover, Go	ood, HSG A
*		23	98	Paved park	ing, HSG A	A, Retwall
		708	98	Roofs, HSC	β A	
		2,820	55	Weighted A	verage	
		2,089		74.08% Per	vious Area	a
		731		25.92% Imp	pervious Ar	rea
	Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	
	5.0					Direct Entry,

Summary for Subcatchment P2a:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.46 cfs @ 12.07 hrs, Volume= Routed to Pond 1P : Pemeable Pavers 0.034 af, Depth> 6.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr Rainfall=8.50"

	A	ea (sf)	CN	Description		
*		665	50	Permeable	Pavers	
		289	98	Paved park	ing, HSG A	4
		131	98	Water Surfa	ace, HSG A	4
		90	39	>75% Gras	s cover, Go	ood, HSG A
*		135	98	Paved park	ing, HSG A	A, Retwall
		680	98	Roofs, HSC	6 A	
		677	98	Roofs, HSC	6 A	
		2,667	84	Weighted A	verage	
		755		28.31% Per	vious Area	3
		1,912		71.69% Imp	pervious Ar	rea
	Тс	Length	Slop	e Velocity	Capacity	Description
(n	nin)	(feet)	(ft/f) (ft/sec)	(cfs)	
	5.0					Direct Entry,

Summary for Pond 1P: Pemeable Pavers

Inflow Area = 0.061 ac. 71.69% Impervious. Inflow Depth > 6.57" for 50-vr event Inflow 0.46 cfs @ 12.07 hrs, Volume= 0.034 af = 0.07 cfs @ 12.57 hrs, Volume= Outflow = 0.034 af, Atten= 84%, Lag= 30.0 min 0.07 cfs @ 12.57 hrs, Volume= Discarded = 0.034 af 0.00 cfs @ 0.00 hrs, Volume= Primary = 0.000 af Routed to Pond DP1 : Routing by Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs. Peak Elev= 3.99' @ 12.57 hrs Surf.Area= 0.008 ac Storage= 0.012 af Plug-Flow detention time= 105.1 min calculated for 0.033 af (100% of inflow) Center-of-Mass det. time= 104.5 min (894.3 - 789.7) Volume Invert Avail.Storage Storage Description 0.012 af 24.00'W x 14.00'L x 4.00'H Prismatoid #1 0.00' 0.031 af Overall x 40.0% Voids 4.00' 0.008 af 24.00'W x 14.00'L x 1.00'H Prismatoid #2 0.020 af Total Available Storage Device Routing Invert Outlet Devices #1 Discarded 0.00' 5.000 in/hr Exfiltration over Horizontal area Phase-In= 0.01' #2 Primary 4.00' 38.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) 0.5' Crest Height Discarded OutFlow Max=0.04 cfs @ 12.57 hrs HW=3.99' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=0.00' (Free Discharge)

Summary for Pond 2P: Drip Apron

[88] Warning: Qout>Qin may require smaller dt or Finer Routing [85] Warning: Oscillations may require smaller dt or Finer Routing (severity=14)

Inflow Area =	0.049 ac, 4	12.39% Impervious,	Inflow Depth > 4.54	for 50-yr event		
Inflow =	0.26 cfs @	12.08 hrs, Volume	= 0.019 af			
Outflow =	0.28 cfs @	12.06 hrs, Volume	= 0.019 af, A	tten= 0%, Lag= 0.0 min		
Discarded =	0.02 cfs @	12.00 hrs, Volume	= 0.012 af			
Primary =	0.26 cfs @	12.06 hrs, Volume	= 0.007 af			
Routed to Pond DP1 :						

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 3.01' @ 12.05 hrs Surf.Area= 0.004 ac Storage= 0.002 af

Plug-Flow detention time= 58.4 min calculated for 0.019 af (100% of inflow) Center-of-Mass det. time= 58.0 min (884.9 - 827.0) 2022-12-22 Proposed Conditions - David T

Prepared by Haley Ward

 Type III 24-hr
 50-yr Rainfall=8.50"

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 LC
 Page 26

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Volume	Invert	Avail.Storage	e Storage Description				
#1	0.00'	0.002 a	f 13.00'W x 6.00'L x 3.00'H Prismatoid				
			0.005 af Overall x 40.0% Voids				
#2	3.00'	0.002 a [.]	f 13.00'W x 6.00'L x 1.00'H Prismatoid				
		0.004 a	f Total Available Storage				
Device	Routing	Invert C	Outlet Devices				
#1	Discarded	0.00' 5	.000 in/hr Exfiltration over Horizontal area Phase-In= 0.01'				
#2	Primary	3.00' 3	4.0' Iong Sharp-Crested Rectangular Weir 0 End Contraction(s)				
	-	0	.5' Crest Height				
Disconded OutFlow Movel 02 of @ 12.00 hrs. UW/=2.011 (Free Discharge)							

Discarded OutFlow Max=0.02 cfs @ 12.00 hrs HW=3.01' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.13 cfs @ 12.06 hrs HW=3.01' (Free Discharge) **2=Sharp-Crested Rectangular Weir** (Weir Controls 0.13 cfs @ 0.34 fps)

Summary for Pond DP1:

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	0.175 ac, 46.54% Impervious, Inflow Depth > 1.60" for 50-yr event	
Inflow	=	0.49 cfs @ 12.07 hrs, Volume= 0.023 af	
Primary	=).49 cfs $\overline{@}$ 12.07 hrs, Volume= 0.023 af, Atten= 0%, Lag= 0.0 m	in

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

APPENDIX D

SOIL SURVEY INFORMATION



United States Department of Agriculture

Natural Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants Custom Soil Resource Report for Rockingham County, New Hampshire





MAP LEGEND				MAP INFORMATION		
Area of Interest (AOI)		Spoil Area	The soil surveys that comprise your AOI were mapped at 1:24,000.			
	Area of Interest (AOI)	۵	Stony Spot	1.24,000.		
Soils	Soil Map Unit Polygons	0	Very Stony Spot	Warning: Soil Map may not be valid at this scale.		
	Soil Map Unit Lines	Ŷ	Wet Spot			
~	^		Other	Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil		
Soil Map Unit Points			Special Line Features	line placement. The maps do not show the small areas of		
Special (0)	Point Features Blowout	Water Features		contrasting soils that could have been shown at a more detailed scale.		
×	Borrow Pit	\sim	Streams and Canals			
×	Clay Spot	Transport		Please rely on the bar scale on each map sheet for map		
0	Closed Depression	+++	Rails	measurements.		
*	Gravel Pit	~	Interstate Highways	Source of Map: Natural Resources Conservation Service		
X	Gravelly Spot	~	US Routes	Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)		
	Landfill	~	Major Roads			
٩	Lava Flow	~	Local Roads	Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts		
A.		Backgrou		distance and area. A projection that preserves area, such as the		
Let Marsh or swamp		Aerial Photography		Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.		
~	Mine or Quarry					
 Miscellaneous Water Perennial Water 				This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.		
\vee	Rock Outcrop			Soil Survey Area: Rockingham County, New Hampshire Survey Area Data: Version 25, Sep 12, 2022		
+ Saline Spot				Survey Area Data. Version 25, Sep 12, 2022		
Sandy Spot				Soil map units are labeled (as space allows) for map scales		
-	Severely Eroded Spot			1:50,000 or larger.		
\diamond	Sinkhole			Date(s) aerial images were photographed: Jun 19, 2020—Sep		
≫	Slide or Slip			20, 2020		
ø	Sodic Spot			The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.		

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
799	Urban land-Canton complex, 3 to 15 percent slopes	0.1	100.0%
Totals for Area of Interest		0.1	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Rockingham County, New Hampshire

799—Urban land-Canton complex, 3 to 15 percent slopes

Map Unit Setting

National map unit symbol: 9cq0 Elevation: 0 to 1,000 feet Mean annual precipitation: 42 to 46 inches Mean annual air temperature: 45 to 48 degrees F Frost-free period: 120 to 160 days Farmland classification: Not prime farmland

Map Unit Composition

Urban land: 55 percent *Canton and similar soils:* 20 percent *Minor components:* 25 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Canton

Setting

Parent material: Till

Typical profile

H1 - 0 to 5 inches: gravelly fine sandy loam *H2 - 5 to 21 inches:* gravelly fine sandy loam *H3 - 21 to 60 inches:* loamy sand

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Low (about 5.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2e Hydrologic Soil Group: A Ecological site: F144AY034CT - Well Drained Till Uplands Hydric soil rating: No

Minor Components

Udorthents

Percent of map unit: 5 percent *Hydric soil rating:* No

Scituate and newfields

Percent of map unit: 4 percent Hydric soil rating: No

Chatfield

Percent of map unit: 4 percent *Hydric soil rating:* No

Boxford and eldridge

Percent of map unit: 4 percent Hydric soil rating: No

Walpole

Percent of map unit: 4 percent Landform: Depressions Hydric soil rating: Yes

Squamscott and scitico

Percent of map unit: 4 percent Landform: Marine terraces Hydric soil rating: Yes JN 3492

<u>APPENDIX E</u>

FEMA FIRM MAP



SMITH RESIDENCE 9 KENT STREET PORTSMOUTH, NH



JOB NUMBER: 3492 SCALE: 1" = 200' SUBMITTED: 12-22-2022



<u>APPENDIX F</u> INSPECTION & LONG TERM

MAINTENANCE PLAN

INSPECTION & LONG-TERM MAINTENANCE PLAN FOR SMITH RESIDENCE

AMBIT ENGINEERING, INC.

9 KENT STREET PORTSMOUTH, NH

Introduction

The intent of this plan is to provide Peter Smith (herein referred to as "owner") with a list of procedures that document the inspection and maintenance requirements of the stormwater management system for this development. Specifically, the proposed drip apron and permeable pavers (collectively referred to as the "Stormwater Management System"). The contact information for the owner shall be kept current, and if there is a change of ownership of the property this plan must be transferred to the new owner.

The following inspection and maintenance program is necessary to keep the stormwater management system functioning properly and will help in maintaining a high quality of stormwater runoff to minimize potential environmental impacts. By following the enclosed procedures, the owner will be able to maintain the functional design of the stormwater management system and maximize its ability to remove sediment and other contaminants from site generated stormwater runoff.

<u>Annual Report</u>

The owner shall prepare an annual Inspection & Maintenance Report. The report shall include a summary of the system's maintenance and repair by transmission of the Inspection & Maintenance Log and other information as required. A copy of the report shall be delivered annually to the Portsmouth DPW, if required.

Inspection & Maintenance Checklist/Log

The following pages contain the Stormwater Management System Inspection & Maintenance Requirements and a blank copy of the Stormwater Management System Inspection & Maintenance Log. These forms are provided to the owner as a guideline for performing the inspection and maintenance of the Stormwater Management System. This is a guideline and should be periodically reviewed for conformance with current practice and standards.

Stormwater Management System Components

The Stormwater Management System is designed to mitigate the quality of site-generated stormwater runoff. As a result, the design includes the following elements:

Non-Structural BMPs

Non-Structural best management practices (BMP's) include temporary and permanent measures that typically require less labor and capital inputs and are intended to provide protection against erosion of soils. Examples of non-structural BMP's on this project include but are not limited to:

- Temporary and Permanent mulching
- Temporary and Permanent grass cover
- Trees
- Shrubs and ground covers
- Miscellaneous landscape plantings
- Dust control
- Tree protection
- Topsoiling
- Sediment barriers
- Stabilized construction entrance

Structural BMPs

Structural BMPs are more labor and capital-intensive structures or installations that require more specialized personnel to install. Examples on this project include but are not limited to:

- Permeable pavers
- Drip apron
- Catch basin with deep sump

Inspection and Maintenance Requirements

The following summarizes the inspection and maintenance requirements for the various BMP's that may be found on this project.

- 1. Grassed areas (until established): After each rain event of 0.5" or more during a 24-hour period, inspect grassed areas for signs of disturbance, such as erosion. If damaged areas are discovered, immediately repair the damage. Repairs may include adding new topsoil, lime, seed, fertilizer and mulch.
- 2. Plantings: Planting and landscaping (trees, shrubs) shall be monitored bi-monthly during the first year to insure viability and vigorous growth. Replace dead or dying vegetation with new stock and make adjustments to the conditions that caused the dead or dying vegetation. During dryer times of the year, provide weekly watering or irrigation during the establishment period of the first year. Make the necessary adjustments to ensure long-term health of the vegetated covers, i.e. provide

more permanent mulch or compost or other means of protection.

- **3. Permeable Pavers:** Ensure that sediments do not enter and plug pavement. Remove sediments, trash, and debris, as necessary. Repair outlet structures and appurtenances, as necessary. Vacuum at least twice annually.
- 4. **Drip Apron:** Ensure that sediments do not enter and plug drip apron surface. If system does not drain within 72 hours of a rainfall event, consult a qualified professional about restoration of function of the drip apron.
- 5. Storm Drain and Catch Basin Inlets/Outlets: Monitor drain inlets and outlet for excessive accumulation of sediments, in excess of 1 foot in the sump, monthly for the first year following construction, every other month thereafter. Remove sediments as required.
- 6. Gutters: Monitor gutters after leaf fall events (abscission) for clogging and overflowing. Remove debris as necessary.

Pollution Prevention

The following pollution prevention activities shall be undertaken to minimize potential impacts on stormwater runoff quality. The Contractor is responsible for all activities during construction. The Owner is responsible thereafter.

Spill Procedures

Any discharge of waste oil or other pollutant shall be reported immediately to the New Hampshire Department of Environmental Services (NHDES). The Contractor/Owner will be responsible for any incident of groundwater contamination resulting from the improper discharge of pollutants to the stormwater system, and may be required by NHDES to remediate incidents that may impact groundwater quality. If the property ownership is transferred, the new owner will be informed of the legal responsibilities associated with operation of the stormwater system, as indicated above.

Sanitary Facilities

Sanitary facilities shall be provided during all phases of construction.

Material Storage

No on site trash facility is provided until homes are constructed. The contractors are required to remove trash from the site. Hazardous material storage is prohibited.

Material Disposal

All waste material, trash, sediment, and debris shall be removed from the site and disposed of in accordance with applicable local, state, and federal guidelines and regulations. Removed sediments shall be if necessary dewatered prior to disposal.

Invasive Species

Monitor the Stormwater Management System for signs of invasive species growth. If caught early, their eradication is much easier. The most likely places where invasions start is in wetter, disturbed soils or detention ponds. Species such as phragmites and purple loose-strife are common invaders in these wetter areas. If they are found, the owner shall refer to the fact-sheet created by the University of New Hampshire Cooperative Extension or contact a wetlands scientist with experience in invasive species control to implement a plan of action for eradication. Measures that do not require the application of chemical herbicides should be the first line of defense.



Figure 1: Lythrum salicaria, Purple Loosestrife. Photo by Liz West. Figure 2: Phragmites australis. Photo by Le Loup Gris

CLOSED DRAINAGE STRUCTURE LONG-TERM MAINTENANCE SHEET

INSPECTION REQUIREMENTS			
ACTION TAKEN	FREQUENCY	MAINTENANCE REQUIREMENTS	
-Drain Manholes -Catch Basins	Monthly for 1 year following construction, Every other Month thereafter	Check for erosion or short-circuiting Check for sediment accumulation Check for floatable contaminants	
-Drainage Pipes	Monthly for 1 year following construction, 1 time per 2 years thereafter	Check for sediment accumulation/clogging, or soiled runoff. Check for erosion at outlets.	
-Gutters	After leaf fall events	Check for clogging, overflowing along roof edges.	
	MAINTENANC	E LOG	
PROJECT NAME			
INSPECTOR NAME	INSPECTOR CONTAC	CT INFO	
DATE OF INSPECTION	REASON FOR INSPECTION		
	□LARGE STORM EVENT □PERIODIC CHECK-IN		
IS CORRECTIVE ACTION NEEDED?	DESCRIBE ANY PRO	BLEMS, NEEDED MAINTENANCE	
DATE OF MAINTENANCE	PERFORMED BY		

DRIP APRON LONG-TERM MAINTENANCE SHEET

INSPECTION REQUIREMENTS		
ACTION TAKEN	FREQUENCY	MAINTENANCE REQUIREMENTS
-Inspect drip apron for the occurrence of silt or vegetation -Check to see if trench drains within 72 hours of rainfall.	Bi-Yearly and following major storm events	 -Ensure that sediments do not enter and plug drip apron surface. -if system does not drain within 72 hours of a rainfall event, consult a qualified professional about restoration of function of the drip apron.

MAINTENANCE LOG			
PROJECT NAME			
INSPECTOR NAME	INSPECTOR CONTACT INFO		
DATE OF INSPECTION	REASON FOR INSPECTION		
	□LARGE STORM EVENT □PERIODIC CHECK-IN		
IS CORRECTIVE ACTION NEEDED?	DESCRIBE ANY PROBLEMS, NEEDED MAINTENANCE		
□YES □NO			
DATE OF MAINTENANCE	PERFORMED BY		
NOTES			

PERMEABLE PAVER LONG-TERM MAINTENANCE SHEET

INSPECTION REQUIREMENTS		
ACTION TAKEN	FREQUENCY	MAINTENANCE REQUIREMENTS
-Inspect pavement surface for the occurrence of sediment, trash, debris, or structural damage. -Check pavement for surface ponding	Frequently in first few months following construction, Bi- annually after	 -Ensure that sediments do not enter and plug pavement. Remove sediments, trash, and debris, as necessary. -Repair outlet structures and appurtenances, as necessary. -Vacuum pavement at least twice annually. -Prevent vehicles with muddy wheels from accessing permeable pavement.
-No winter sanding permitted -Minimize application of salt	Continuous practice	

MAINTENANCE LOG			
PROJECT NAME			
INSPECTOR NAME	INSPECTOR CONTACT INFO		
DATE OF INSPECTION	REASON FOR INSPECTION		
	LARGE STORM EVENT PERIODIC CHECK-IN		
IS CORRECTIVE ACTION NEEDED?	DESCRIBE ANY PROBLEMS, NEEDED MAINTENANCE		
DATE OF MAINTENANCE	PERFORMED BY		
NOTES			

STABILIZED CONSTRUCTION ENTRANCE CONSTRUCTION MAINTENANCE SHEET

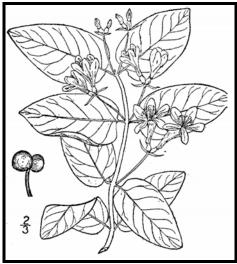
INSPECTION REQUIREMENTS			
ACTION TAKEN	FREQUENCY	MAINTENANCE REQUIREMENTS	
ENTRANCE SURFACE	After heavy rains,	-Top dress pad with new stone.	
-Check for sediment	as necessary	-Replace stone completely if completely	
accumulation/clogging of stone		clogged.	
-Check Vegetative filter strips		-Maintain vigorous stand of vegetation.	
WASHING FACILITIES (if	As often as	-Remove Sediments from traps.	
applicable)	necessary		
-Monitor Sediment Accumulation			

MAINTENANCE LOG		
PROJECT NAME		
INSPECTOR NAME	INSPECTOR CONTACT INFO	
DATE OF INSPECTION	REASON FOR INSPECTION	
	□LARGE STORM EVENT □PERIODIC CHECK-IN	
IS CORRECTIVE ACTION NEEDED?	DESCRIBE ANY PROBLEMS, NEEDED MAINTENANCE	
DATE OF MAINTENANCE	PERFORMED BY	
NOTES		



Methods for Disposing Non-Native Invasive Plants

Prepared by the Invasives Species Outreach Group, volunteers interested in helping people control invasive plants. Assistance provided by the Piscataquog Land Conservancy and the NH Invasives Species Committee. Edited by Karen Bennett, Extension Forestry Professor and Specialist.



Tatarian honeysuckle Lonicera tatarica USDA-NRCS PLANTS Database / Britton, N.L., and A. Brown. 1913. An illustrated flora of the northern United States, Canada and the British Possessions. Vol. 3: 282.

Non-native invasive plants crowd out natives in natural and managed landscapes. They cost taxpayers billions of dollars each year from lost agricultural and forest crops, decreased biodiversity, impacts to natural resources and the environment, and the cost to control and eradicate them.

Invasive plants grow well even in less than desirable conditions such as sandy soils along roadsides, shaded wooded areas, and in wetlands. In ideal conditions, they grow and spread even faster. There are many ways to remove these nonnative invasives, but once removed, care is needed to dispose the removed plant material so the plants don't grow where disposed.

Knowing how a particular plant reproduces indicates its method of spread and helps determine

the appropriate disposal method. Most are spread by seed and are dispersed by wind, water, animals, or people. Some reproduce by vegetative means from pieces of stems or roots forming new plants. Others spread through both seed and vegetative means.

Because movement and disposal of viable plant parts is restricted (see NH Regulations), viable invasive parts can't be brought to most transfer stations in the state. Check with your transfer station to see if there is an approved, designated area for invasives disposal. This fact sheet gives recommendations for rendering plant parts nonviable.

Control of invasives is beyond the scope of this fact sheet. For information about control visit <u>www.nhinvasives.org</u> or contact your UNH Cooperative Extension office.

New Hampshire Regulations

Prohibited invasive species shall only be disposed of in a manner that renders them nonliving and nonviable. (Agr. 3802.04)

No person shall collect, transport, import, export, move, buy, sell, distribute, propagate or transplant any living and viable portion of any plant species, which includes all of their cultivars and varieties, listed in Table 3800.1 of the New Hampshire prohibited invasive species list. (Agr 3802.01)

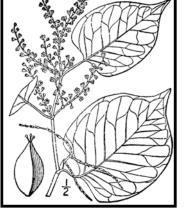
How and When to Dispose of Invasives?

To prevent seed from spreading remove invasive plants before seeds are set (produced). Some plants continue to grow, flower and set seed even after pulling or cutting. Seeds can remain viable in the ground for many years. If the plant has flowers or seeds, place the flowers and seeds in a heavy plastic bag "head first" at the weeding site and transport to the disposal site. The following are general descriptions of disposal methods. See the chart for recommendations by species.

Burning: Large woody branches and trunks can be used as firewood or burned in piles. For outside burning, a written fire permit from the local forest fire warden is required unless the ground is covered in snow. Brush larger than 5 inches in diameter can't be burned. Invasive plants with easily airborne seeds like black swallow-wort with mature seed pods (indicated by their brown color) shouldn't be burned as the seeds may disperse by the hot air created by the fire.

Bagging (solarization): Use this technique with softertissue plants. Use heavy black or clear plastic bags (contractor grade), making sure that no parts of the plants poke through. Allow the bags to sit in the sun for several weeks and on dark pavement for the best effect.

Tarping and Drying: Pile material on a sheet of plastic



Japanese knotweed Polygonum cuspidatum USDA-NRCS PLANTS Database / Britton, N.L., and A. Brown. 1913. An illustrated flora of the northern United States, Canada and the British Possessions. Vol. 1: 676.

and cover with a tarp, fastening the tarp to the ground and monitoring it for escapes. Let the material dry for several weeks, or until it is clearly nonviable.

Chipping: Use this method for woody plants that don't reproduce vegetatively.

Burying: This is risky, but can be done with watchful diligence. Lay thick plastic in a deep pit before placing the cut up plant material in the hole. Place the material away from the edge of the plastic before covering it with more heavy plastic. Eliminate as much air as possible and toss in soil to weight down the material in the pit. Note that the top of the buried material should be at least three feet underground. Japanese knotweed should be at least 5 feet underground!

Drowning: Fill a large barrel with water and place soft-tissue plants in the water. Check after a few weeks and look for rotted plant material (roots, stems, leaves, flowers). Well-rotted plant material may be composted. A word of caution- seeds may still be viable after using this method. Do this before seeds are set. This method isn't used often. Be prepared for an awful stink!

Composting: Invasive plants can take root in compost. Don't compost any invasives unless you know there is no viable (living) plant material left. Use one of the above techniques (bagging, tarping, drying, chipping, or drowning) to render the plants nonviable before composting. Closely examine the plant before composting and avoid composting seeds.

Be diligent looking for seedlings for years in areas where removal and disposal took place.

Suggested Disposal Methods for Non-Native Invasive Plants

This table provides information concerning the disposal of removed invasive plant material. If the infestation is treated with herbicide and left in place, these guidelines don't apply. Don't bring invasives to a local transfer station, unless there is a designated area for their disposal, or they have been rendered non-viable. This listing includes wetland and upland plants from the New Hampshire Prohibited Invasive Species List. The disposal of aquatic plants isn't addressed.

Woody Plants	Method of Reproducing	Methods of Disposal
Norway maple (Acer platanoides) European barberry (Berberis vulgaris) Japanese barberry (Berberis thunbergii) autumn olive (Elaeagnus umbellata) burning bush (Euonymus alatus) Morrow's honeysuckle (Lonicera morrowii) Tatarian honeysuckle (Lonicera tatarica) showy bush honeysuckle (Lonicera x bella) common buckthorn (Rhamnus cathartica) glossy buckthorn (Frangula alnus)	Fruit and Seeds	 Prior to fruit/seed ripening Seedlings and small plants Pull or cut and leave on site with roots exposed. No special care needed. Larger plants Use as firewood. Make a brush pile. Chip. Burn. After fruit/seed is ripe Don't remove from site. Burn. Make a covered brush pile. Chip once all fruit has dropped from branches. Leave resulting chips on site and monitor.
oriental bittersweet (<i>Celastrus orbiculatus</i>) multiflora rose (<i>Rosa multiflora</i>)	Fruits, Seeds, Plant Fragments	 Prior to fruit/seed ripening Seedlings and small plants Pull or cut and leave on site with roots exposed. No special care needed. Larger plants Make a brush pile. Burn. After fruit/seed is ripe Don't remove from site. Burn. Make a covered brush pile. Chip – only after material has fully dried (1 year) and all fruit has dropped from branches. Leave resulting chips on site and monitor.

Non-Woody Plants	Method of Reproducing	Methods of Disposal
<pre>garlic mustard (Alliaria petiolata) spotted knapweed (Centaurea maculosa) • Sap of related knapweed can cause skin irritation and tumors. Wear gloves when handling. black swallow-wort (Cynanchum nigrum) • May cause skin rash. Wear gloves and long sleeves when handling. pale swallow-wort (Cynanchum rossicum) giant hogweed (Heracleum mantegazzianum) • Can cause major skin rash. Wear gloves and long sleeves when handling. dame's rocket (Hesperis matronalis) perennial pepperweed (Lepidium latifolium) purple loosestrife (Lythrum salicaria) Japanese stilt grass (Microstegium vimineum) mile-a-minute weed (Polygonum perfoliatum)</pre>	Fruits and Seeds	 Prior to flowering Depends on scale of infestation Small infestation Pull or cut plant and leave on site with roots exposed. Large infestation Pull or cut plant and pile. (You can pile onto or cover with plastic sheeting). Monitor. Remove any re-sprouting material. During and following flowering Do nothing until the following year or remove flowering heads and bag and let rot. Small infestation Pull or cut plant and leave on site with roots exposed. Large infestation Pull or cut plant and pile remaining material. Unity of the plastic or cover with plastic sheeting). Monitor. Remove any re-sprouting material.
common reed (<i>Phragmites australis</i>) Japanese knotweed (<i>Polygonum cuspidatum</i>) Bohemian knotweed (<i>Polygonum x bohemicum</i>)	Fruits, Seeds, Plant Fragments Primary means of spread in these species is by plant parts. Although all care should be given to preventing the dispersal of seed during control activities, the presence of seed doesn't materially influence disposal activities.	 Small infestation Bag all plant material and let rot. Never pile and use resulting material as compost. Burn. Large infestation Remove material to unsuitable habitat (dry, hot and sunny or dry and shaded location) and scatter or pile. Monitor and remove any sprouting material. Pile, let dry, and burn.

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TANGRAM 3DS









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