Residential Development Plans

Assessor's Parcel 220-87-2 74 EMERY STREET

&

Assessor's Parcel 220-87-3 64 EMERY STREET Portsmouth, New Hampshire

Issued:

NOVEMBER 6, 2018 PLANNING BOARD APROVAL

OCTOBER 9, 2018 PLANNING BOARD SUBMISSION

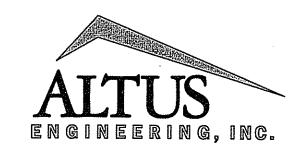
SEPTEMBER 14, 2018 TAC Submission

Owner/Applicant:

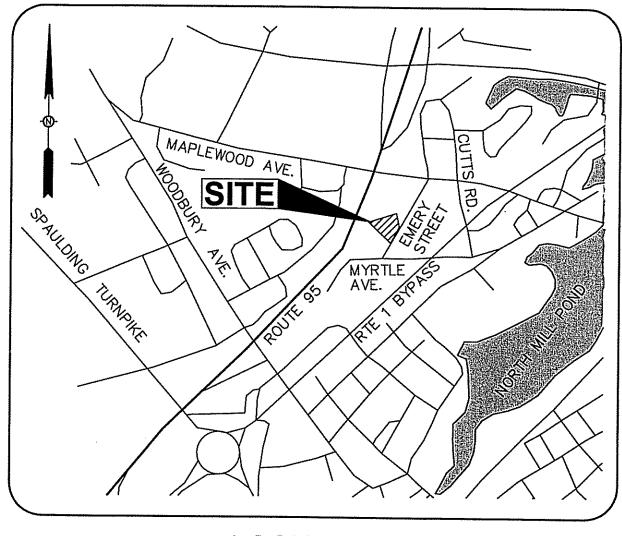
HAPPY MOUNTAIN HOLDINGS, LLC

901 N. MARKET STREET SUITE 705 WILMINGTON, DE 19801

Civil Engineer:



133 COURT STREET PORTSMOUTH, NH 0380 (603) 433-2335 www.ALTUS-ENG.com

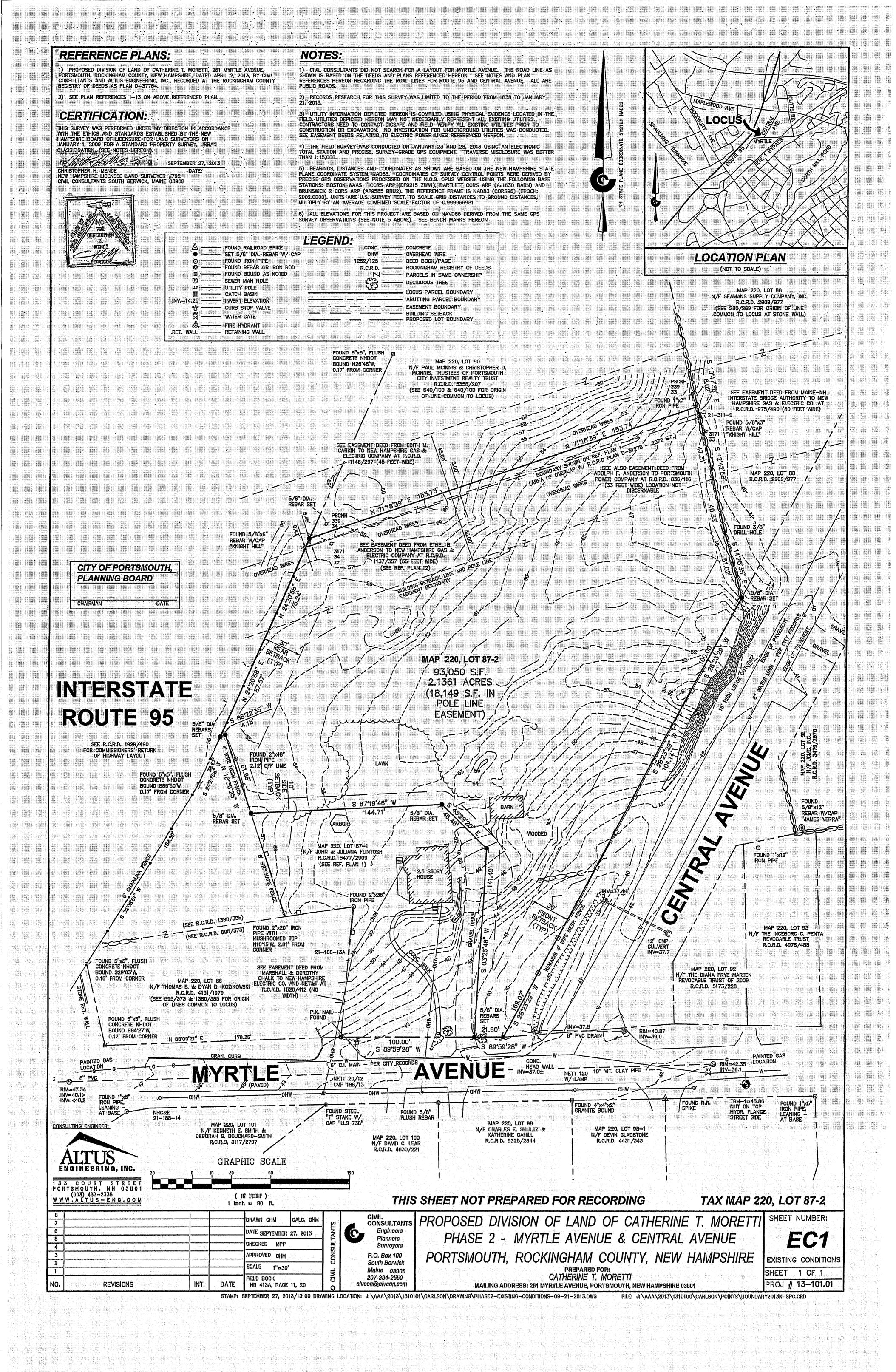


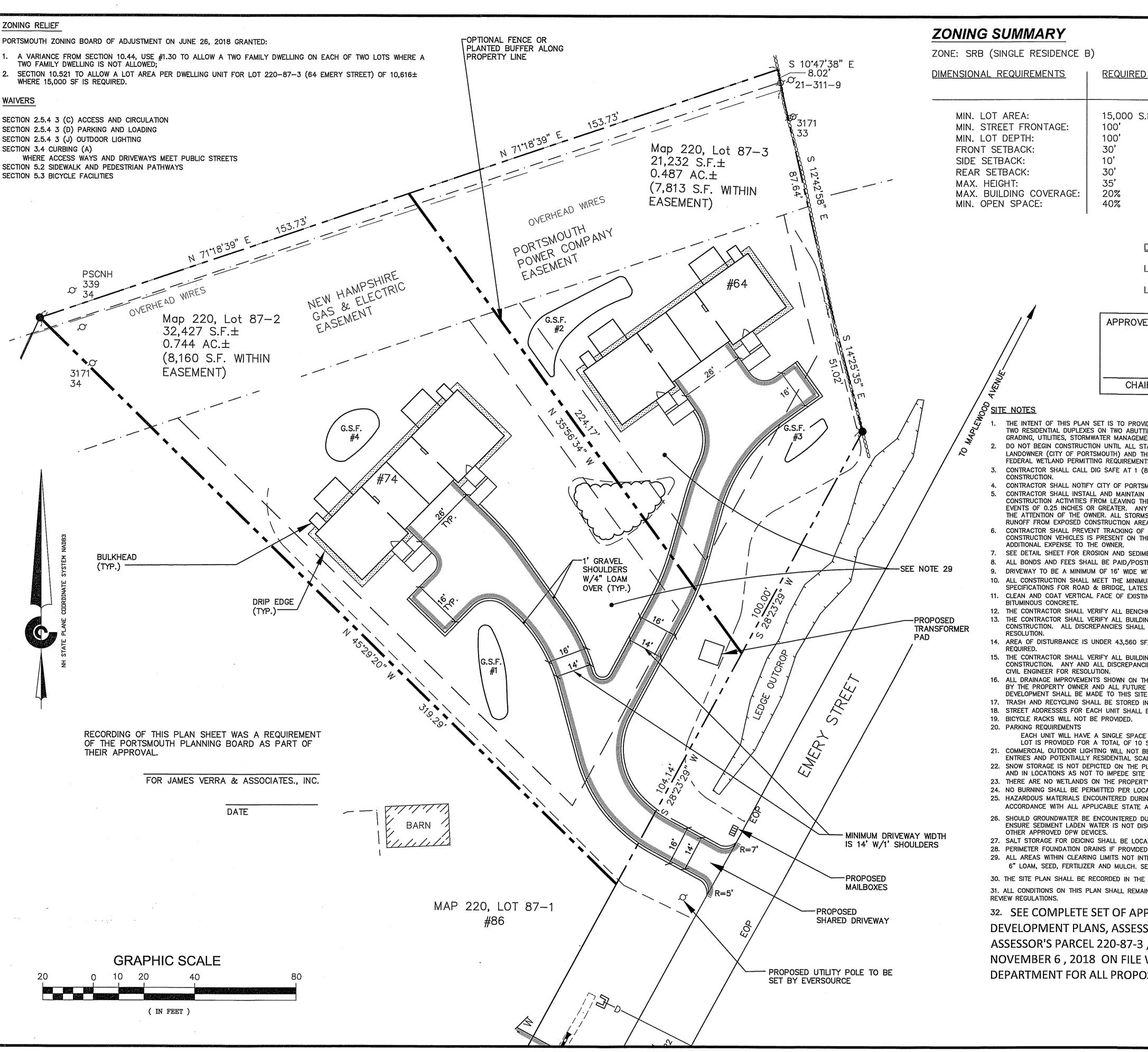
LOCUS MAP

1" = 1,000 FEET +/-

CHAIRMAN	DATE
----------	------

Sheet Index Title	Sheet No.:	Rev.	Date
Existing Conditions Plan (by Civil Consultants, Inc.)	EC-1	0	00 /07 /17
Site Plan		0	09/27/13
	C-1	3	11/06/18
Grading Plan	C-2	3	11/06/18
Utilities Plan	C-3	3	11/06/18
General Notes & Sitework Details	C-4	1	
		I	10/09/18
Sitework Details	C-5	0	09/14/18





ZONE: SRB (SINGLE RESIDENCE B)

DIMENSIONAL REQUIREMENTS	<u>REQUIRED</u>	<u>PROVII</u>	<u>DED</u>	
		ASSESSOR'S PARCEL	ASSESSOR'S PARCEL	
		220-87-2	220-87-3	
MIN. LOT AREA:	15,000 S.F.	32,427 SF	21,232 S.F.	
MIN. STREET FRONTAGE:	100'	104'±	100'	
MIN. LOT DEPTH:	100'	224 ' ±	146'±	
FRONT SETBACK:	30'	125'±	56'±	
SIDE SETBACK:	10'	14'±	14 ' ±	
REAR SETBACK:	30'	75 ' ±	57 ' ±	
MAX. HEIGHT:	35'	<35'	<35'	
MAX. BUILDING COVERAGE:	20%	8.2%±	12.5%±	
MIN. OPEN SPACE:	40%	71% ±	74%±	

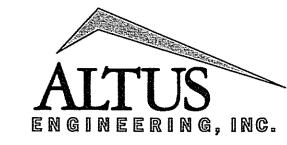
DWELLING DENSITY PER LOT:

LOT 220-87-2: 16,213.5 SF/DWELLING UNIT

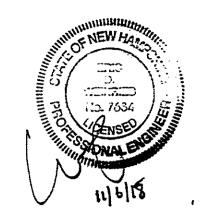
LOT 220-87-3: 10,616 SF/DWELLING UNIT

APPROVED	BY	THE	PORTSMOUTH	PLANNING	BOARD
CHAIRM	AN			DA [*]	TE

- THE INTENT OF THIS PLAN SET IS TO PROVIDE THE NECESSARY INFORMATION FOR THE REVIEW, PERMITTING AND DEVELOPMENT OF TWO RESIDENTIAL DUPLEXES ON TWO ABUTTING LOTS. THESE PLANS PROVIDE DETAILED INFORMATION FOR THE SITE LAYOUT. GRADING, UTILITIES, STORMWATER MANAGEMENT, AND LANDSCAPE IMPROVEMENTS.
- DO NOT BEGIN CONSTRUCTION UNTIL ALL STATE, LOCAL AND FEDERAL PERMITS HAVE BEEN APPLIED FOR AND RECEIVED. THE LANDOWNER (CITY OF PORTSMOUTH) AND THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH LOCAL, STATE AND FEDERAL WETLAND PERMITTING REQUIREMENTS INCLUDING PROTECTION OF NATURAL RESOURCES AND THEIR BUFFERS.
- CONTRACTOR SHALL INSTALL AND MAINTAIN TEMPORARY SEDIMENT AND EROSION CONTROL ITEMS TO PREVENT SEDIMENT FROM CONSTRUCTION ACTIVITIES FROM LEAVING THE SITE. CONTROLS SHALL BE INSPECTED ON A REGULAR BASIS AND AFTER ALL RAIN EVENTS OF 0.25 INCHES OR GREATER. ANY DEFICIENCIES IN THE CONTROLS SHALL BE ADDRESSED IMMEDIATELY AND BROUGHT TO THE ATTENTION OF THE OWNER. ALL STORMS DRAINS WITHIN OR ADJACENT TO THE WORK AREA, WITH THE POTENTIAL TO RECEIVE RUNOFF FROM EXPOSED CONSTRUCTION AREAS, SHALL RECEIVE STORM DRAIN INLET PROTECTION
- CONTRACTOR SHALL PREVENT TRACKING OF DIRT ONTO ANY PUBLIC OR PRIVATE ROADWAYS. IF TRACKING OF DIRT FROM CONSTRUCTION VEHICLES IS PRESENT ON THE OPEN STREETS, CONTRACTOR WILL BE REQUIRED TO SWEEP THE ROADWAY AT NO ADDITIONAL EXPENSE TO THE OWNER.
- SEE DETAIL SHEET FOR EROSION AND SEDIMENT CONTROL NOTES AND DETAILS.
- ALL BONDS AND FEES SHALL BE PAID/POSTED PRIOR TO INITIATING CONSTRUCTION DRIVEWAY TO BE A MINIMUM OF 16' WIDE WITH 2' SHOULDERS. SHOULDERS SHALL BE PLOWED.
- 10. ALL CONSTRUCTION SHALL MEET THE MINIMUM CONSTRUCTION STANDARDS OF THE CITY OF PORTSMOUTH & NHDOT'S STANDARD
- SPECIFICATIONS FOR ROAD & BRIDGE, LATEST EDITION. THE MORE STRINGENT SPECIFICATION SHALL GOVERN. 11. CLEAN AND COAT VERTICAL FACE OF EXISTING PAVEMENT AT SAWCUT LINE WITH RS-1 IMMEDIATELY PRIOR TO PLACING NEW
- 12. THE CONTRACTOR SHALL VERIFY ALL BENCHMARKS AND TOPOGRAPHY IN THE FIELD PRIOR TO CONSTRUCTION. 13. THE CONTRACTOR SHALL VERIFY ALL BUILDING DIMENSIONS WITH THE ARCHITECTURAL AND STRUCTURAL PLANS PRIOR TO
- CONSTRUCTION. ALL DISCREPANCIES SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER FOR
- 14. AREA OF DISTURBANCE IS UNDER 43,560 SF, COVERAGE UNDER EPA NPDES PHASE II CONSTRUCTION GENERAL PERMIT IS NOT
- 15. THE CONTRACTOR SHALL VERIFY ALL BUILDING DIMENSIONS WITH THE ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO CONSTRUCTION. ANY AND ALL DISCREPANCIES SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF BOTH THE ARCHITECT AND
- 16. ALL DRAINAGE IMPROVEMENTS SHOWN ON THIS SITE PLAN SHALL BE CONSTRUCTED AND MAINTAINED IN ACCORDANCE WITH THE PLAN
- BY THE PROPERTY OWNER AND ALL FUTURE PROPERTY OWNERS. NO CHANGES TO THE STORMWATER MANAGMENT ASPECTS OF THIS DEVELOPMENT SHALL BE MADE TO THIS SITE PLAN WITHOUT THE EXPRESS APPROVAL OF THE PORTSMOUTH PLANNING DIRECTOR. 17. TRASH AND RECYCLING SHALL BE STORED INSIDE THE HOMES.
- 18. STREET ADDRESSES FOR EACH UNIT SHALL BE DETERMINED BY FIRE DEPARTMENT & PORTSMOUTH DPW.
- 19. BICYCLE RACKS WILL NOT BE PROVIDED.
- 20. PARKING REQUIREMENTS
- EACH UNIT WILL HAVE A SINGLE SPACE IN THE GARAGE AND SPACE STACKED BEHIND THE GARAGE. ONE ADDITONAL SPACE PER LOT IS PROVIDED FOR A TOTAL OF 10 SPACES FOR 4 RESIDENTIAL UNITS.
- COMMERCIAL OUTDOOR LIGHTING WILL NOT BE PROVIDED. OUTDOOR LIGHTING WILL BE LIMITED TO BUILDING MOUNTED LIGHTS AT ENTRIES AND POTENTIALLY RESIDENTIAL SCALED LIGHTS ALONG THE DRIVEWAY. ALL LIGHTS WILL BE DARK SKY FRIENDLY.
- 22. SNOW STORAGE IS NOT DEPICTED ON THE PLANS. IT WILL BE STORED ALONG THE EDGE OF THE DRIVEWAY ON PRIVATE PROPERTY AND IN LOCATIONS AS NOT TO IMPEDE SITE DISTANCE AT THE DRIVEWAY.
- 23. THERE ARE NO WETLANDS ON THE PROPERTY OR WITHIN 100-FEET OF ANY PROPOSED SITE DISTRUBANCES.
- 24. NO BURNING SHALL BE PERMITTED PER LOCAL REGULATIONS.
- 25. HAZARDOUS MATERIALS ENCOUNTERED DURING CONSTRUCTION ACTIVITIES SHALL BE ABATED IN STRICT ACCORDANCE WITH ALL APPLICABLE STATE AND LOCAL REGULATIONS.
- 26. SHOULD GROUNDWATER BE ENCOUNTERED DURING SITEWORK EXCAVATION, BEST MANAGEMENT PRACTICES SHALL BE EMPLOYED TO ENSURE SEDIMENT LADEN WATER IS NOT DISCHARGED INTO THE CITY DRAINAGE SYSTEM. CONTRACTOR SHALL USE SILT BAGS OR OTHER APPROVED DPW DEVICES.
- 27. SALT STORAGE FOR DEICING SHALL BE LOCATED INDOORS.
- 28. PERIMETER FOUNDATION DRAINS IF PROVIDED MAY REQUIRE PUMPING TO DAYLIGHT. 29. ALL AREAS WITHIN CLEARING LIMITS NOT INTENDED TO BE OTHERWISE TREATED SHALL RECEIVE
- 6" LOAM, SEED, FERTILIZER AND MULCH. SEE GRADING PLAN FOR LIMITS.
- 30. THE SITE PLAN SHALL BE RECORDED IN THE ROCKINGHAM COUNTY REGISTRY OF DEEDS.
- 31. ALL CONDITIONS ON THIS PLAN SHALL REMAIN IN EFFECT IN PERPETUITY PURSUANT TO THE REQUIREMENTS OF THE SITE PLAN
- 32. SEE COMPLETE SET OF APPROVED SITE PLANS TITLED "RESIDENTIAL DEVELOPMENT PLANS, ASSESSOR'S PARCEL 220-87-2, 74 EMERY STREET & ASSESSOR'S PARCEL 220-87-3, 64 EMERY STREET, PORTSMOUTH, NH", DATED NOVEMBER 6, 2018 ON FILE WITH THE CITY OF PORTSMOUTH PLANNING DEPARTMENT FOR ALL PROPOSED IMPROVEMENTS AND CONDITIONS OF APPROVAL



133 COURT STREET PORTSMOUTH, NH 03801 www.ALTUS-ENG.com (603) 433-2335



ISSUED FOR:

PLANNING BOARD APPROVAL

ISSUE DATE:

NOVEMBER 6, 2018

	REV	ISIONS		
	NO.	DESCRIPTION	BY	DATE
ı	0	INITIAL SUBMISSION	EDW	08/07/18
	1	PER TAC COMMENTS	EDW	09/14/18
	2	PB APPROVAL	EDW	10/09/18
	3	PER PB COMMENTS	EDW	11/06/18

RLH DRAWN BY:. EDW APPROVED BY: 4916 SITE.DWG DRAWING FILE:.

SCALE:

11"x17": 1" = 4022"x 34": 1" = 20

APPLICANT/OWNER:

HAPPY MOUNTAIN HOLDINGS,

901 N. MARKET STREET SUITE 705 WILMINGTON, DE 19801

PROJECT:

RESIDENTIAL DEVELOPMENT

ASSESSOR'S PARCEL 220-87-2 74 EMERY STREET

ASSESSOR'S PARCEL 220-87-3 64 EMERY STREET

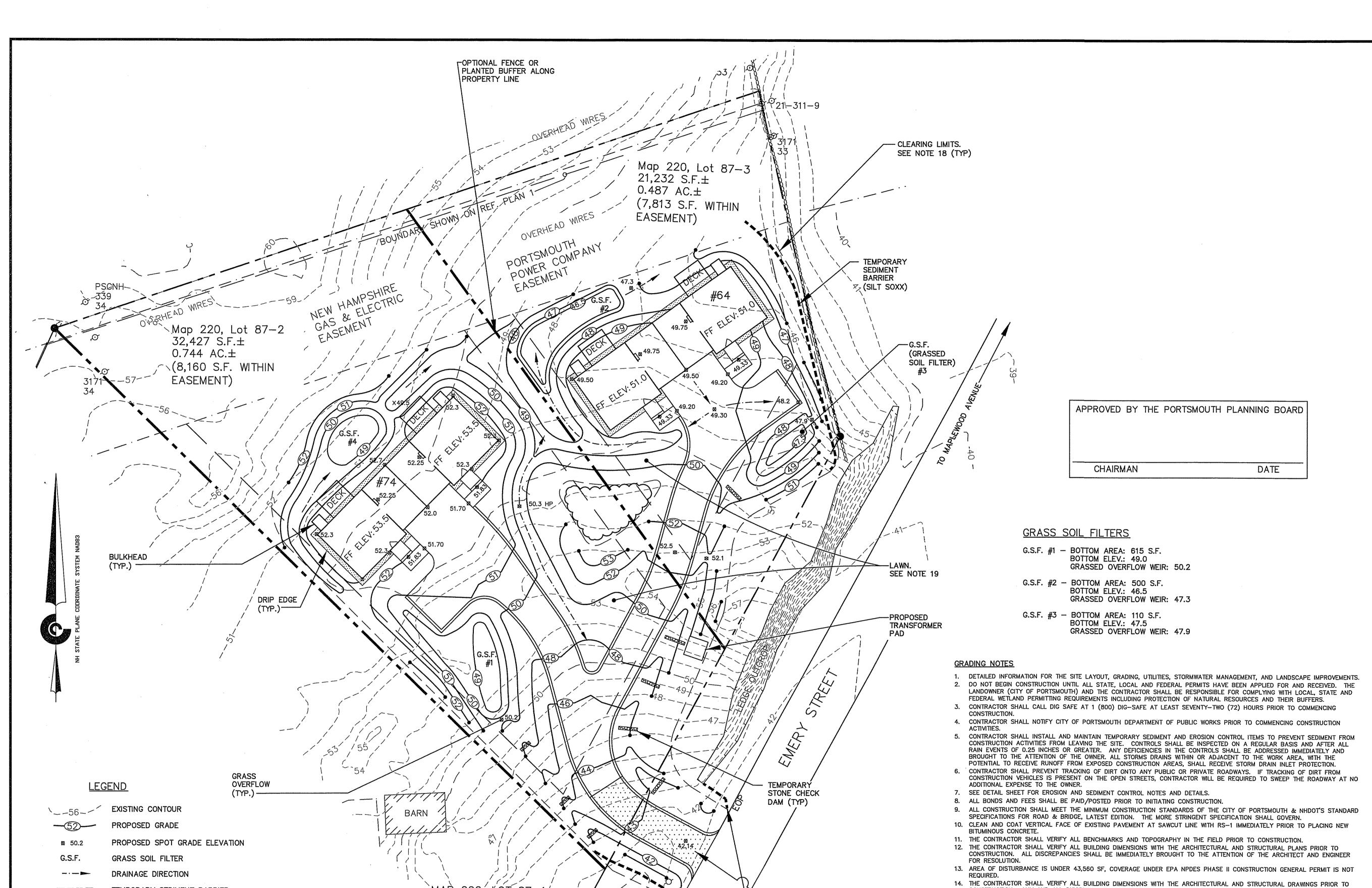
PORTSMOUTH, NEW HAMPSHIRE

TITLE:

RECORDING SITE PLAN

NOTE: SEE COMPLETE SET OF APPROVED SITE PLANS ON FILE WITH THE CITY OF PORTSMOUTH

SHEET NUMBER:



-STABILIZED

TEMPORARY

(SILT SOXX)

SEDIMENT

BARRIER

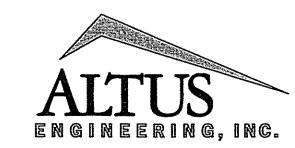
CONSTRUCTION

TEMPORARY SEDIMENT BARRIER

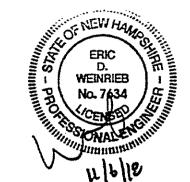
GRAPHIC SCALE

(IN FEET)

CLEARING LIMITS



133 COURT STREET PORTSMOUTH, NH 03801 (603) 433-2335 www.ALTUS-ENG.com



ISSUED FOR:

PLANNING BOARD APPROVAL

ISSUE DATE:

NOVEMBER 6, 2018

RE\	<u> ISIONS</u>		
NO.	DESCRIPTION	BY	DATE
0	INITIAL SUBMISSION	EDW	08/07/18
1	PER TAC COMMENTS	EDW	09/14/18
2	PB APPROVAL	EDW	10/09/18
3	PER PB COMMENTS	EDW	11/06/18

DRAWN BY: RLH

APPROVED BY: EDW

DRAWING FILE: 4916 SITE.DWG

SCALE:

 $11" \times 17"$: 1" = 40' $22" \times 34"$: 1" = 20'

APPLICANT/OWNER:

HAPPY MOUNTAIN HOLDINGS, LLC

901 N. MARKET STREET SUITE 705 WILMINGTON, DE 19801

PROJECT:

RESIDENTIAL DEVELOPMENT

ASSESSOR'S PARCEL 220-87-2 74 EMERY STREET

ASSESSOR'S PARCEL 220-87-3 64 EMERY STREET

PORTSMOUTH, NEW HAMPSHIRE

TITLE:

CONSTRUCTION. ANY AND ALL DISCREPANCIES SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF BOTH THE ARCHITECT

PLAN BY THE PROPERTY OWNER AND ALL FUTURE PROPERTY OWNERS. NO CHANGES TO THE STORMWATER MANAGMENT ASPECTS

OF THIS DEVELOPMENT SHALL BE MADE TO THIS SITE PLAN WITHOUT THE EXPRESS APPROVAL OF THE PORTSMOUTH PLANNING

PROPOSED AND PEAK FLOW RATES ARE MANAGED TO REPLICATE THE EXISTING FLOW CONDITIONS, WITH A VARIANCE OF 0.1± CFS

FOR THE 1" STORM THROUGH THE 25 YEAR STORM EVENT. THERE IS A MINOR INCREASE OF LESS THAN 1 CFS FOR THE NET 50

15. ALL DRAINAGE IMPROVEMENTS SHOWN ON THIS SITE PLAN SHALL BE CONSTRUCTED AND MAINTAINED IN ACCORDANCE WITH THE

17. A DRAINAGE STUDY WAS PREPARED BY ALTUS ENGINEERING, INC. FOR THIS SITE. LOW IMPACT DEVELOPMENT PRACTICES ARE

CONSERVATIVE AND DOES NOT INCLUDE THE ROOF DRIP EDGE THAT WILL INFILTRATE THE MAJORITY OF THE ROOF RUNOFF.

18. PRIOR TO CLEARING SITE, A LICENSED LAND SURVEYOR SHALL BE ENGAGED TO STAKE THE CLEARING LIMITS TREE PROTECTION SHALL BE IMPLEMENTED TO ENSURE THAT HEALTHY TREES SCHEDULED TO REMAIN ARE NOT DISTURBED BY THE CONSTRUCTION

19. ALL AREAS WITHIN CLEARING LIMITS NOT INTENDED TO BE OTHERWISE TREATED SHALL RECEIVE 6" LOAM, SEED, FERTILIZER AND

PEAK FLOW DISCHARGE FOR THE 50 YEAR STORM EVENT. AS DISCUSSED IN THE DRAINAGE STUDY, THE ANALYSIS IS

AND CIVIL ENGINEER FOR RESOLUTION.

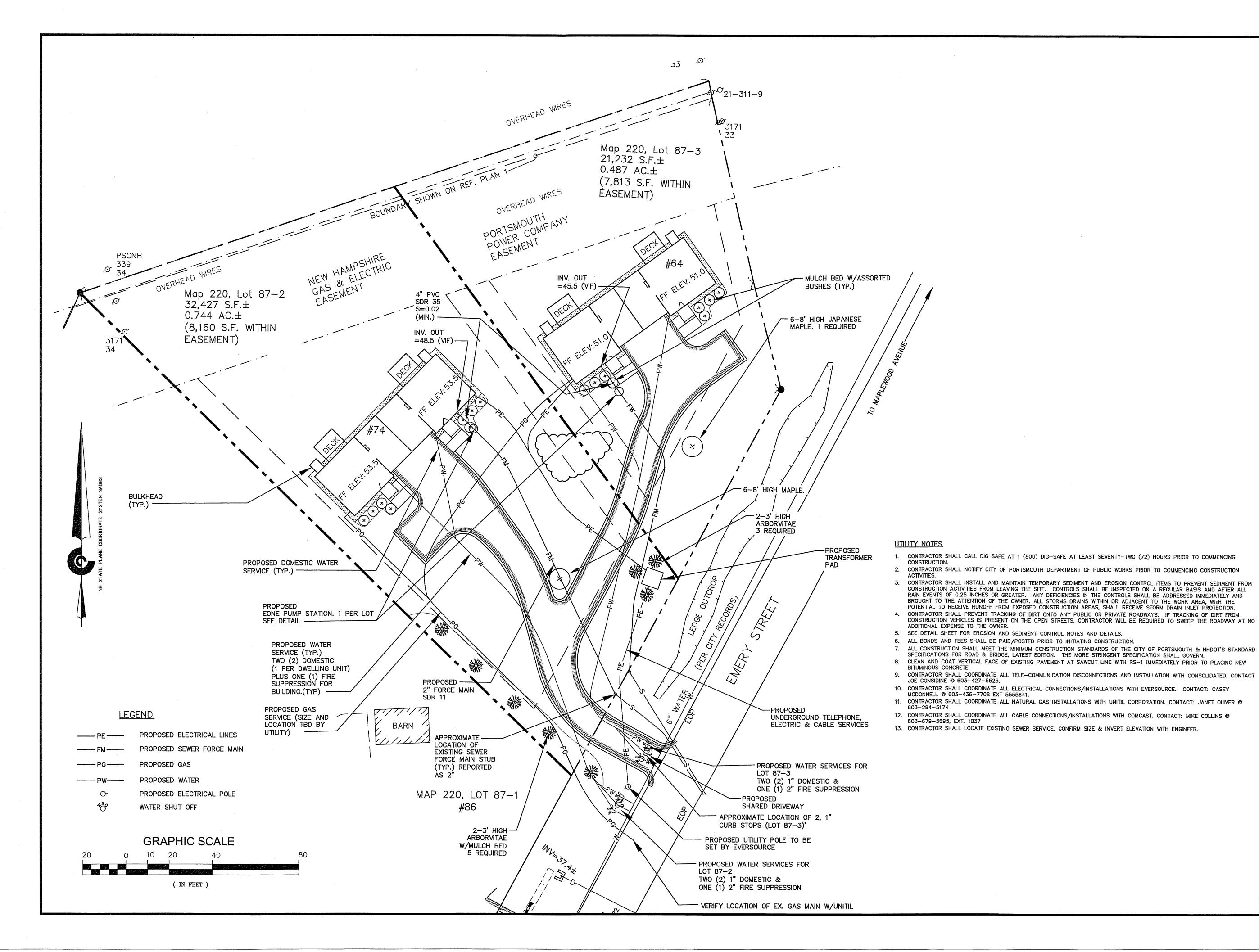
MULCH.

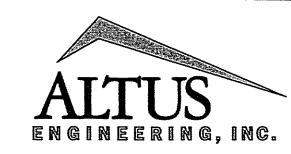
16. PERIMETER FOUNDATION DRAINS IF PROVIDED MAY REQUIRE PUMPING TO DAYLIGHT.

GRADING PLAN

SHEET NUMBER:

C - 2





133 COURT STREET (603) 433-2335

335 www.ALTUS-ENG.com



ISSUED FOR:

PLANNING BOARD APPROVAL

ISSUE DATE:

NOVEMBER 6, 2018

REVISIONS					
NO.	DESCRIPTION	BY	DATE		
0	INITIAL SUBMISSION	EDW	08/07/18		
1	PER TAC COMMENTS	EDW	09/14/18		
2	PB APPROVAL	EDW	10/09/18		
3	PER PB COMMENTS	EDW	11/06/18		
•					

DRAWN BY: RLH

APPROVED BY: EDW

DRAWING FILE: 4916 SITE.DWG

SCALE:

11"x 17": 1" = 40' 22"x 34": 1" = 20'

APPLICANT/OWNER:

HAPPY MOUNTAIN HOLDINGS,

901 N. MARKET STREET SUITE 705 WILMINGTON, DE 19801

PROJECT:

RESIDENTIAL DEVELOPMENT

ASSESSOR'S PARCEL 220-87-2 74 EMERY STREET

ASSESSOR'S PARCEL 220-87-3 64 EMERY STREET

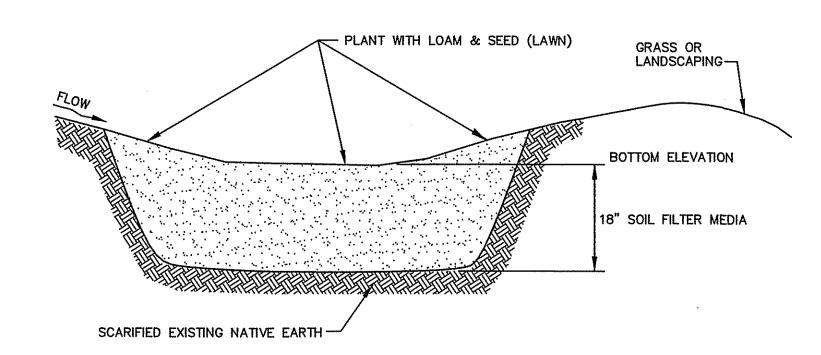
PORTSMOUTH, NEW HAMPSHIRE

TITLE:

UTILITIES PLAN

SHEET NUMBE

C - 3



FIL1	TER MEDIA MI	XTURES			
Percent of Gradation of material					
Component Material	Mixture by Volume	Sieve No.	Percent by Weight Passing Standard Sieve		
	Filter Medi	a			
ASTM C-33 concrete sand	30				
Loamy sand topsoil, with fines as indicated	70	200	15 to 25		

CROSS-SECTION

STRUCTURE DETAIL

SPACING BETWEEN STRUCTURES

- 1. CONTRACTOR SHALL EXCAVATE THE POND AREA TO SUBGRADE AND DESIGN ENGINEER SHALL PERFORM SUBSURFACE EVALUATION PRIOR TO THE PLACEMENT OF ANY SELECT MATERIAL OR
- 2. SOIL FILTER MEDIA SHALL BE PER THE DESIGN FILTER MIXTURE. IF AN ALTERNATIVE MIXTURE IS PROPOSED, IT SHALL BE APPROVED BY THE DESIGN ENGINEER.
- 3. DO NOT PLACE THE POND INTO SERVICE UNTIL THE BMP HAS BEEN PLANTED AND ITS CONTRIBUTING AREAS HAVE BEEN FULLY STABILIZED.
- 4. DO NOT DISCHARGE SEDIMENT-LADEN WATERS FROM CONSTRUCTION ACTIVITIES (RUNOFF, WATER FROM EXCAVATIONS) TO THE POND AREA DURING ANY STAGE OF CONSTRUCTION.
- DO NOT TRAFFIC EXPOSED SOIL SURFACE WITH CONSTRUCTION EQUIPMENT. IF FEASIBLE, PERFORM EXCAVATIONS WITH EQUIPMENT POSITIONED OUTSIDE THE LIMITS OF THE INFILTRATION COMPONENTS

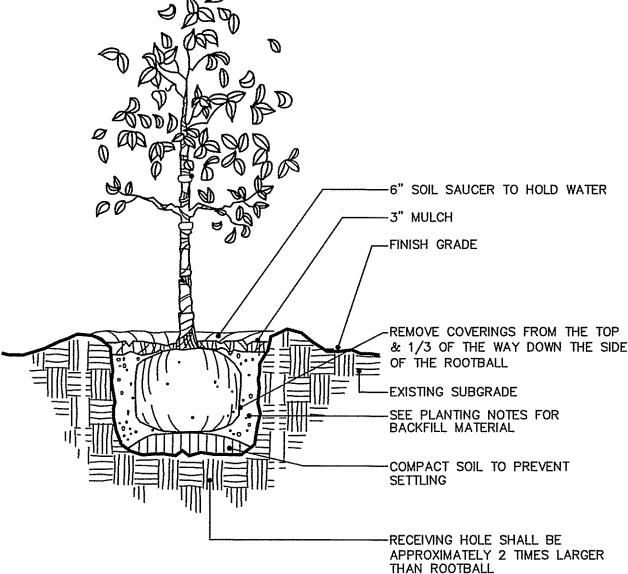
MAINTENANCE REQUIREMENTS

- PONDS SHOULD BE INSPECTED ANNUALLY, AND FOLLOWING ANY RAINFALL EXCEEDING 2.5 INCHES IN A 24-HOUR PERIOD, WITH MAINTENANCE OR REHABILITATION CONDUCTED AS WARRANTED BY
- AT LEAST ONCE ANNUALLY, SYSTEM SHOULD BE INSPECTED FOR DRAWDOWN TIME. IF POND DOES NOT DRAIN WITHIN 72-HOURS FOLLOWING A RAINFALL EVENT, THEN A QUALIFIED PROFESSIONAL SHOULD ASSESS THE CONDITION OF THE FACILITY TO DETERMINE MEASURES REQUIRED TO RESTORE FILTRATION FUNCTION OR INFILTRATION FUNCTION (AS APPLICABLE), INCLUDING BUT NOT LIMITED TO REMOVAL OF ACCUMULATED SEDIMENTS OR RECONSTRUCTION OF THE FILTER MEDIA.
- VEGETATION SHOULD BE INSPECTED AT LEAST ANNUALLY, AND MAINTAINED IN HEALTHY CONDITION, INCLUDING, PRUNING, REMOVAL, AND REPLACEMENT OF DEAD OR DISEASED VEGETATION, AND REMOVAL OF INVASIVE SPECIES.

GRASS SOIL FILTER

2" - 3" STONE

NOT TO SCALE



(Trees with 3" cal. & smaller) SMALL TREE PLANTING/STAKING DETAIL NOT TO SCALE

- DRIP LINE OR MIN. AROUNI TREE OPTION A

6" LOAM AND SEED AS

SPECIFIED, GRADE PER PLANS -

EQUIVALENT APPROVED BY

ARCHITECT OR ENGINEER

2"x12" PT EDGE RESTRAINT OR

BEYOND DRIPLINE

OR AS SPECIFIED

STONE DRIP EDGE

TREE PROTECTION DETAILS

BUILDING OR OTHER

RIGID STRUCTURE

NOT TO SCALE

REQUIRED) -- NHDOT ITEM 304.2 - 12" GRAVEL MINIMUM DRIVEWAY WIDTH TO BE 14' WITH 1' SHOULDER EACH SIDE. NOTE: SUBGRADE AREA TO BE PROOF ROLLED. PAVEMENT CROSS SECTION SHOWN ON PLANS

GRADING & DRAINAGE NOTES

INSULATION AGAINST FREEZING.

SHOULDER TO SHOULDER -

TACK COAT BETWEEN PAVEMENT COURSES-

COMPACTED

NATIVE SUBGRADE

(OR FILL WHERE

1. UNLESS OTHERWISE AGREED IN WRITING, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ESTABLISHING AND MAINTAINING

PROTECTION OF SUBGRADE: THE CONTRACTOR SHALL BE REQUIRED TO MAINTAIN STABLE, DEWATERED SUBGRADES FOR

TO MORE COMPETENT BEARING SOIL AND REPLACED WITH FREE DRAINING STRUCTURAL FILL. IF THE EARTHWORK IS

OF EACH DAY'S OPERATIONS. THE FINAL SUBGRADE ELEVATION WOULD ALSO REQUIRE AN APPROPRIATE DEGREE OF

4. IF SUITABLE, EXCAVATED MATERIALS SHALL BE PLACED AS FILL WITHIN UPLAND AREAS ONLY AND SHALL NOT BE PLACED WITHIN WETLANDS. PLACEMENT OF BORROW MATERIALS SHALL BE PERFORMED IN A MANNER THAT PREVENTS LONG TERM

7. UNLESS OTHERWISE SPECIFIED, ALL DISTURBED AREAS NOT TO BE PAVED OR OTHERWISE TREATED SHALL RECEIVE A MINIMUM

OF SIX (6") INCHES OF LOAM, LIMESTONE, FERTILIZER, SEED, AND HAY MULCH USING APPROPRIATE SOIL STABILIZATION

8. IN ORDER TO PROVIDE VISUAL CLARITY ON THE PLANS, DRAINAGE AND OTHER UTILITY STRUCTURES MAY NOT BE DRAWN TO SCALE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER SIZING AND LOCATION OF ALL STRUCTURES AND IS

NHDOT ITEM 403.12 - HOT

1-1/2" WEARING COURSE, (TYPE 12 mm) 2-1/2" BINDER COURSE, (TYPE 19 mm)

- NHDOT ITEM 304.3 - 6" CRUSHED GRAVEL

AS SHOWN ON PLANS -

50' min.

PLAN VIEW

AS SHOWN ON PLANS --

NOT TO SCALE

MOUNTABLE BERM

FILTER FABRIC

-6" LOAM AND SEED AS SPECIFIED, GRADE PER PLANS (PROVIDE 4" LOAM AND SEED OVER 2'-WIDE GRAVEL SHOULDERS)

BITUMINOUS CONCRETE

PAVEMENT (4" NOMINAL)

- SLOPE AS SHOWN ON PLANS

DIRECTED TO RESOLVE ANY POTENTIAL DISCREPANCY WITH THE ENGINEER PRIOR TO CONSTRUCTION.

DIFFERENTIAL SETTLEMENT. EXCESSIVELY WET MATERIALS SHALL BE STOCKPILED AND ALLOWED TO DRAIN BEFORE

FOUNDATIONS, PAVEMENT AREAS, UTILITY TRENCHES, AND OTHER AREAS DURING CONSTRUCTION. SUBGRADE DISTURBANCE

ACTIVITIES. THE CONTRACTOR SHALL TAKE PRECAUTIONS TO PREVENT SUBGRADE DISTURBANCE. SUCH PRECAUTIONS MAY

INCLUDE DIVERTING STORMWATER RUNOFF AWAY FROM CONSTRUCTION AREAS, REDUCING TRAFFIC IN SENSITIVE AREAS, AND

MAINTAINING AN EFFECTIVE DEWATERING PROGRAM. SOILS EXHIBITING HEAVING OR INSTABILITY SHALL BE OVER EXCAVATED

PERFORMED DURING FREEZING WEATHER, EXPOSED SUBGRADES ARE SUSCEPTIBLE TO FROST. NO FILL OR UTILITIES SHALL

BE PLACED ON FROZEN GROUND. THIS WILL LIKELY REQUIRE REMOVAL OF A FROZEN SOIL CRUST AT THE COMMENCEMENT

MAY BE INFLUENCED BY EXCAVATION METHODS, MOISTURE, PRECIPITATION, GROUNDWATER CONTROL, AND CONSTRUCTION

TEMPORARY BENCHMARKS (TBMS) AND PERFORMING ALL CONSTRUCTION SURVEY LAYOUT.

2. DEWATERING ACTIVITIES SHALL BE DONE IN ACCORDANCE WITH EPA AND NHDES REGULATIONS.

PLACEMENT. FROZEN MATERIAL SHALL NOT BE USED FOR CONSTRUCTION.

TECHNIQUES. SEE DETAILS FOR ADDITIONAL INFORMATION.

5. ALL DRAINAGE PIPE SHALL BE ADS N-12 OR EQUAL APPROVED BY THE ENGINEER.

6. ALL SPOT GRADES ARE AT FINISH GRADE AND BOTTOM OF CURB WHERE APPLICABLE

PROFILE STONE GRADATION TABLE % PASSING SIEVE SIZE BY WEIGHT 1 1/2 inch 3/4 Inch 3/8 Inch

DRIVE WIDTH

CONSTRUCTION SPECIFICATIONS

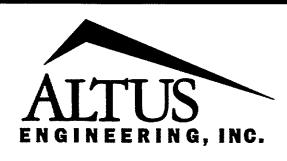
- STONE SIZE NHDOT STANDARD STONE SIZE #4 SECTION 703 OF NHDOT STANDARD. LENGTH - DETAILED ON PLANS (50 FOOT MINIMUM).
- THICKNESS SIX (6) INCHES (MINIMUM).

 WIDTH FULL DRIVE WIDTH UNLESS OTHERWISE SPECIFIED.
- ILTER FABRIC MIRAFI 600X OR EQUAL APPROVED BY ENGINEER. SURFACE WATER CONTROL - ALL SURFACE WATER THAT IS FLOWING TO OR DIVERTED TOWARD THE CONSTRUCTION ENTRANCE SHALL BE PIPED BENEATH THE ENTRANCE. IF PIPING IS IMPRACTICAL, A BERM WITH 5:1 SLOPES THAT CAN BE CROSSED BY VEHICLES
- MAY BE SUBSTITUTED FOR THE PIPE.

 MAINTENANCE THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS—OF—WAY. THIS WILL REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL STONE OR ADDITIONAL LENGTH AS CONDITIONS DEMAND AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC RIGHTS-OF-WAY MUST BE REMOVED IMMEDIATELY.
- WHEELS SHALL BE CLEANED TO REMOVE MUD PRIOR TO ENTRANCE ONTO PUBLIC RIGHTS-OF-WAY. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA
- STABILIZED WITH STONE WHICH DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE. STABILIZED CONSTRUCTION EXITS SHALL BE INSTALLED AT ALL ENTRANCES TO PUBLIC RIGHTS-OF-WAY, AT LOCATIONS SHOWN ON THE PLANS, AND/OR WHERE AS DIRECTED BY

STABILIZED CONSTRUCTION EXIT

NOT TO SCALE



133 COURT STREET PORTSMOUTH, NH 03801 (603) 433-2335 www.ALTUS-ENG.com



ISSUED FOR:

PLANNING BOARD APPROVAL

ISSUE DATE:

OCTOBER 9, 2018

NO. DESCRIPTION DATE EDW 09/14/1 O INITIAL SUBMISSION 1 PB APPROVAL EDW 10/09/1

RLH DRAWN BY: EDW APPROVED BY: _ 4916 DETAILS.DWG DRAWING FILE:

SCALE:

22"x 34": N.T.S.

APPLICANT/OWNER:

HAPPY MOUNTAIN HOLDINGS,

901 N. MARKET STREET SUITE 705 WILMINGTON, DE 19801

PROJECT:

RESIDENTIAL **DEVELOPMENT**

ASSESSOR'S PARCEL 220-87-2 74 EMERY STREET

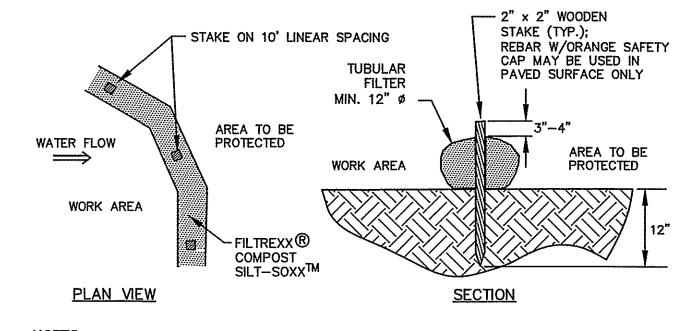
ASSESSOR'S PARCEL 220-87-3 64 EMERY STREET

PORTSMOUTH, NEW HAMPSHIRE

TITLE:

GENERAL NOTES SITEWORK DETAILS

SHEET NUMBER:



CONSTRUCTION SPECIFICATIONS

1. L = DISTANCE SUCH THAT POINTS A AND B ARE OF EQUAL ELEVATION

CHECK DAM SHALL BE CONSTRUCTED OF

2" TO 3" STONE WITH COMPLETE COVERAGE

CENTER OF THE STRUCTURE IS LOWER THAN

TEMPORARY GRADE STABILIZATION STRUCTURES SHOULD BE CHECKED AFTER EACH RAINFALL AND

AT LEAST DAILY DURING PROLONGED STORMS. ANY NECESSARY REPAIRS SHOULD BE MADE

IMMEDIATELY. PARTICULAR ATTENTION SHOULD BE GIVEN TO END RUN AND EROSION AT THE

DOWNSTREAM TOE OF THE STRUCTURE. WHEN THE STRUCTURES ARE REMOVED, THE DISTURBED PORTION SHOULD BE BROUGHT TO THE EXISTING CHANNEL GRADE AND THE AREAS PREPARED,

SEEDED, AND MULCHED. WHILE THIS PRACTICE IS NOT INTENDED TO BE USED PRIMARILY FOR

SEDIMENT TRAPPING, SOME SEDIMENT WILL ACCUMULATE BEHIND THE STRUCTURES, SEDIMENT

SHALL BE REMOVED FROM BEHIND THE STRUCTURES WHEN IT HAS ACCUMULATED TO ONE HALF OF THE ORIGINAL HEIGHT OF THE STRUCTURE.

OF DITCH OR SWALE TO INSURE THAT THE

- 1. STRUCTURES SHALL BE INSTALLED ACCORDING TO THE DIMENSIONS SHOWN ON THE
- PLANS AT THE APPROPRIATE SPACING.

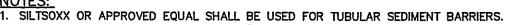
 2. CONSTRUCTION OPERATIONS SHALL BE CARRIED OUT IN SUCH A MANNER SO THAT EROSION AND AIR AND WATER POLLUTION WILL BE MINIMIZED.
- 3. SEEDING, FERTILIZING, AND MULCHING SHALL CONFORM TO THE RECOMMENDATIONS IN THE APPROPRIATE VEGETATIVE BMP.

 4. STRUCTURES SHALL BE REMOVED FROM THE CHANNEL WHEN THEIR USEFUL LIFE HAS

MAINTENANCE

STONE CHECK DAM

NOT TO SCALE



- 2. ALL MATERIAL TO MEET MANUFACTURER'S SPECIFICATIONS.
- 3. COMPOST/SOIL/ROCK/SEED FILL MATERIAL SHALL BE ADJUSTED AS NECESSARY TO MEET THE REQUIREMENTS OF THE SPECIFIC APPLICATION.
- 4. ALL SEDIMENT TRAPPED BY BARRIER SHALL BE DISPOSED OF PROPERLY. 5. STUMPS GRINDINGS MAY BE SUBSTITUTED W/PRIOR APPROVAL FROM ENGINEER,

SILT SOXX BARRIER

NOT TO SCALE

- NON-WOVEN FILTER FABRIC

(5 OZ./SF MIN. WEIGHT)

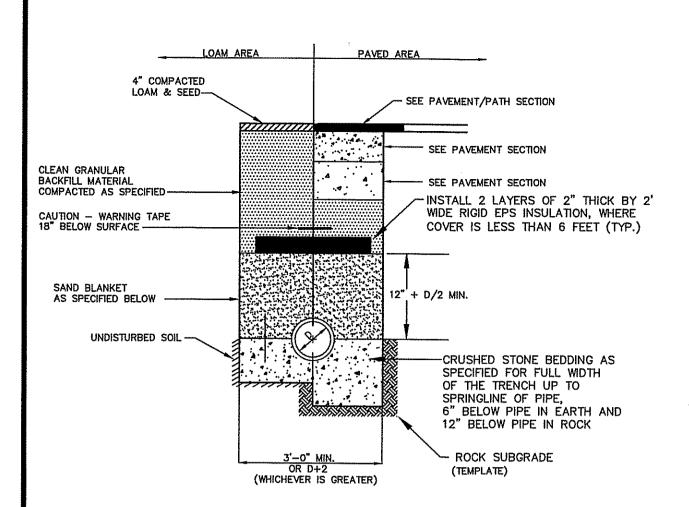
- 6" TAMPED 1"-2" WASHED CRUSHED

STONE OR RIVER STONE. COLOR & TYPE TO BE APPROVED BY OWNER.

6" COMPACTED CRUSHED GRAVEL -

M.D.O.T. TYPE "A" AGGREGATE - 6"

NOT TO SCALE



BACKFILL MATERIAL BELOW PAVED OR CONCRETE AREAS, BEDDING MATERIAL, AND SAND BLANKET SHALL BE COMPACTED TO NOT LESS THAN 95% OF AASHTO T99, METHOD C. SUITABLE BACKFILL MATERIAL BELOW LOAM AREAS SHALL BE COMPACTED TO NOT LESS THAN 90% OF AASHTO T 99,

SAND BLANKET		CRUSHED	STONE BEDDING *
SIEVE SIZE	% FINER BY WEIGHT	SIEVE SIZE	% PASSING BY WEIGHT
1/2"	90 - 100	1"	100
200	0 - 15	3/4"	90 — 100
		3/8"	20 — 55
		# 4	0 - 10
		# 8	0 - 5
* EQUIVALENT 703 OF NH	T TO STANDARD STONE SIZE #1 DOT STANDARD SPECIFICATIONS	67 — SECTION S	

PAVED AREA

SEWER TRENCH SECTION

LOAM AND SEED OR

TREATMENT PER PLANS-

OTHER SURFACE

5' COVER (MIN)

(7' COVER MAX) -

SUITABLE EXCAVATED

BACKFILL OR CLEAN

GRANULAR BACKFILL

MATERIAL COMPACTED

AS SPECIFIED

AS SPECIFIED

6" NOMINAL —

(12" IN LEDGE)

NON-PAVED AREA

NOT TO SCALE

STANDARD TRENCH NOTES:

- 1. ORDERED EXCAVATION OF UNSUITABLE MATERIAL BELOW GRADE: BACKFILL AS STATED IN THE TECHNICAL SPECIFICATIONS OR AS SHOWN OF THE DRAWING.
- 2. BEDDING: SCREENED GRAVEL AND/OR CRUSHED STONE FREE FROM CLAY, LOAM. ORGANIC MATTER AND MEETING ASTM C33, STONE SIZE NO. 67. PASSING 1 INCH SCREEN

PASSING 3/4 INCH SCREEN 20 - 55% PASSING 3/8 INCH SCREEN 0-10% PASSING #4 SIEVE PASSING #8 SIEVE

WHERE ORDERED BY THE ENGINEER TO STABILIZE THE BASE, SCREENED GRAVEL OR CRUSHED STONE 1-1/2 INCH TO 1/2 INCH SHALL BE USED.

- 3. SAND BLANKET: CLEAN SAND FREE FROM ORGANIC MATTER, SO GRADED THAT 90 100% PASSES 1/2 INCH SIEVE AND NOT MORE THAN 15% WILL PASS A #200 SIEVE. BLANKET MAY BE OMITTED FOR CAST-IRON, DUCTILE IRON, AND REINFORCED CONCRETE PIPE PROVIDED HOWEVER, THAT NO STONE LARGER THAN 2" IS IN CONTACT WITH THE PIPE.
- 4. SUITABLE MATERIAL: IN ROADS, ROAD SHOULDERS, WALKWAYS AND TRAVELED WAYS, SUITABLE MATERIAL FOR TRENCH BACKFILL SHALL BE THE NATURAL MATERIAL EXCAVATED DURING THE COURSE OF CONSTRUCTION, BUT SHALL EXCLUDE DEBRIS: PIECES OF PAVEMENT; ORGANIC MATTER; TOP SOIL; ALL WET OR SOFT MUCK. PEAT. OR CLAY: ALL EXCAVATED LEDGE MATERIAL: ALL ROCKS OVER 6 INCHES IN LARGEST DIMENSION; AND ANY MATERIAL WHICH. AS DETERMINED BY THE ENGINEER, WILL NOT PROVIDE SUFFICIENT SUPPORT OR MAINTAIN THE COMPLETED CONSTRUCTION IN A STABLE CONDITION.
- 5. BASE COURSE AND PAVEMENT SHALL MEET THE REQUIREMENTS OF THE NEW HAMPSHIRE DEPARTMENT OF TRANSPORTATION'S LATEST EDITION OF THE STANDARD SPECIFICATIONS FOR HIGHWAYS AND BRIDGES - DIVISIONS 300 AND 400 RESPECTIVELY.
- 6. SHEETING, IF REQUIRED: WHERE SHEETING IS PLACED ALONGSIDE THE PIPE AND EXTENDS BELOW MID—DIAMETER, IT SHALL BE CUT OFF AND LEFT IN PLACE TO AN ELEVATION 1 FOOT ABOVE THE TOP OF PIPE. WHERE SHEETING IS ORDERED BY THE ENGINEER TO BE LEFT IN PLACE, IT SHALL BE CUT OFF AT LEAST 3 FEET BELOW FINISHED GRADE, BUT NOT LESS THAT 1 FOOT ABOVE THE TOP OF THE PIPE.
- 7. W = MAXIMUM ALLOWABLE TRENCH WIDTH TO A PLANE 12 INCHES ABOVE THE PIPE. FOR PIPES 15 INCHES NOMINAL DIAMETER OR LESS, W SHALL BE NO MORE THAN 36 INCHES. FOR PIPES GREATER THAN 15 INCHES IN NOMINAL DIAMETER, W SHALL BE 24 INCHES PLUS PIPE OUTSIDE DIAMETER (O.D.) ALSO, W SHALL BE THE PAYMENT WIDTH FOR LEDGE EXCAVATION AND FOR ORDERED EXCAVATION BELOW GRADE.
- 8. FOR CROSS COUNTRY CONSTRUCTION, BACKFILL OR FILL SHALL BE MOUNDED TO A HEIGHT OF 6 INCHES ABOVE THE ORIGINAL GROUND SURFACE.
- 9. CONCRETE FOR ENCASEMENT SHALL CONFORM TO THE NEW HAMPSHIRE DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS STANDARD SPECIFICATION REQUIREMENTS FOR CLASS A (3000#) CONCRETE AS FOLLOWS: CEMENT: 6.0 BAGS PER CUBIC YARD WATER: 5.75 GALLONS PER BAG CEMENT

MAXIMUM SIZE OF AGGREGATE: 1 INCH CONCRETE ENCASEMENT IS NOT ALLOWED FOR PVC PIPE.

VVII

Patent Numbers: 5,752,315

5,562,254 5,439,180

NA0050P01 Rev C

EXTHEME

- 10. CONCRETE FULL ENCASEMENT: IF FULL ENCASEMENT IS UTILIZED, DEPTH OF CONCRETE BELOW PIPE SHALL BE 1/4 I.D. (4" MINIMUM). BLOCK SUPPORT SHALL BE SOLID
- 11. NEW HAMPSHIRE DEPARTMENT OF ENVIRONMENTAL SERVICES DESIGN STANDARDS REQUIRE TEN FEET (10') SEPARATION BETWEEN WATER AND SEWER. REFER TO CITY'S STANDARD SPECIFICATIONS FOR METHODS OF PROTECTION IN AREAS THAT CANNOT MEET THESE

General Features

Rated for flows of 700 gpd (2650 lpd)

• 70 gallons (265 liters) of capacity

Indoor or outdoor installation

Operational Information

15 gpm at 0 psig (0.95 lps at 0 m)

11 gpm at 40 psig (0.69 lps at 28 m 7.8 gpm at 80 psig (0.49 lps at 56 m)

advanced notice of service requirements.

protection against backflow

Inlet Connections

from the factory

The model DH071 or DR071 grinder pump station is a complete unit that

family homes where codes allow and with consent of the factory.

Standard outdoor heights range from 61 inches to 160 inches

motor controls to the level controls through watertight penetrations.

The DH071 is the "hardwired," or "wired," model where a cable connects the

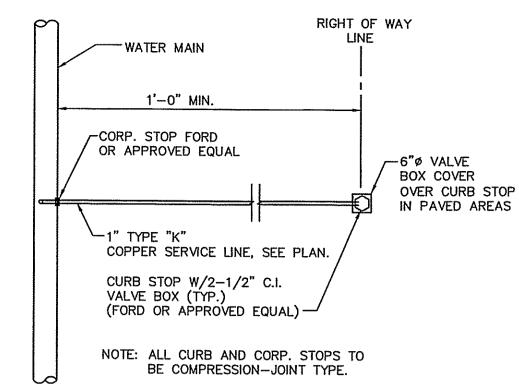
The DR071 is the "radio frequency identification" (RFID), or "wireless," model

Pump discharge terminates in 1.25-Inch NPT female thread. Can easily be adapted to 1.25-inch PVC pipe or any other material required by local codes

E/One requires that the Uni-Lateral, E/One's own stainless steel check valve.

be installed between the grinder pump station and the street main for added

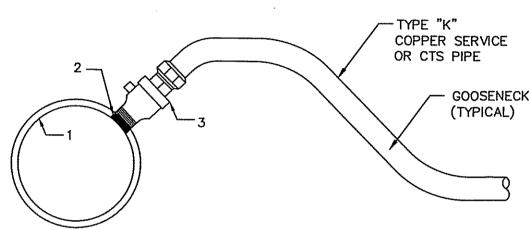
Alarm panels are available with a variety of options, from basic monitoring to



WATER SERVICE CONNECTION TYPICAL

NOTES: REPORTED 6"

- 2. REPORTED 6" x 1" OR 2" TAP. (PER PLANS)
- 3. CORPORATION STOP, FORD OR APPROVED EQUAL TO BE INSTALLED.



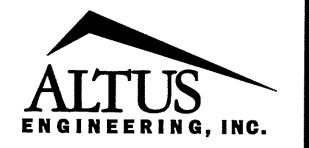
SERVICE TO MAIN CONNECTION DETAIL NOT TO SCALE

GRADING & DRAINAGE NOTES

- UNLESS OTHERWISE AGREED IN WRITING, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ESTABLISHING AND MAINTAINING TEMPORARY BENCHMARKS (TBMS) AND PERFORMING ALL CONSTRUCTION SURVEY LAYOUT.
- 2. DEWATERING ACTIVITIES SHALL BE DONE IN ACCORDANCE WITH EPA AND NHDES REGULATIONS.
- PROTECTION OF SUBGRADE: THE CONTRACTOR SHALL BE REQUIRED TO MAINTAIN STABLE, DEWATERED SUBGRADES FOR FOUNDATIONS, PAVEMENT AREAS, UTILITY TRENCHES, AND OTHER AREAS DURING CONSTRUCTION. SUBGRADE DISTURBANCE MAY BE INFLUENCED BY EXCAVATION METHODS, MOISTURE, PRECIPITATION, GROUNDWATER CONTROL, AND CONSTRUCTION ACTIVITIES. THE CONTRACTOR SHALL TAKE PRECAUTIONS TO PREVENT SUBGRADE DISTURBANCE. SUCH PRECAUTIONS MAY INCLUDE DIVERTING STORMWATER RUNOFF AWAY FROM CONSTRUCTION AREAS, REDUCING TRAFFIC IN SENSITIVE AREAS, AND MAINTAINING AN EFFECTIVE DEWATERING PROGRAM. SOILS EXHIBITING HEAVING OR INSTABILITY SHALL BE OVER EXCAVATED TO MORE COMPETENT BEARING SOIL AND REPLACED WITH FREE DRAINING STRUCTURAL FILL. IF THE EARTHWORK IS PERFORMED DURING FREEZING WEATHER, EXPOSED SUBGRADES ARE SUSCEPTIBLE TO FROST. NO FILL OR UTILITIES SHALL BE PLACED ON FROZEN GROUND. THIS WILL LIKELY REQUIRE REMOVAL OF A FROZEN SOIL CRUST AT THE COMMENCEMENT OF EACH DAY'S OPERATIONS. THE FINAL SUBGRADE ELEVATION WOULD ALSO REQUIRE AN APPROPRIATE DEGREE OF INSULATION AGAINST FREEZING.
- 4. IF SUITABLE, EXCAVATED MATERIALS SHALL BE PLACED AS FILL WITHIN UPLAND AREAS ONLY AND SHALL NOT BE PLACED WITHIN WETLANDS. PLACEMENT OF BORROW MATERIALS SHALL BE PERFORMED IN A MANNER THAT PREVENTS LONG TERM DIFFERENTIAL SETTLEMENT. EXCESSIVELY WET MATERIALS SHALL BE STOCKPILED AND ALLOWED TO DRAIN BEFORE PLACEMENT. FROZEN MATERIAL SHALL NOT BE USED FOR CONSTRUCTION.
- 5. ALL DRAINAGE PIPE SHALL BE ADS N-12 OR EQUAL APPROVED BY THE ENGINEER.
- 6. ALL SPOT GRADES ARE AT FINISH GRADE AND BOTTOM OF CURB WHERE APPLICABLE
- UNLESS OTHERWISE SPECIFIED, ALL DISTURBED AREAS NOT TO BE PAVED OR OTHERWISE TREATED SHALL RECEIVE A MINIMUM OF SIX (6") INCHES OF LOAM, LIMESTONE, FERTILIZER, SEED, AND HAY MULCH USING APPROPRIATE SOIL STABILIZATION TECHNIQUES. SEE DETAILS FOR ADDITIONAL INFORMATION.
- IN ORDER TO PROVIDE VISUAL CLARITY ON THE PLANS, DRAINAGE AND OTHER UTILITY STRUCTURES MAY NOT BE DRAWN TO SCALE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER SIZING AND LOCATION OF ALL STRUCTURES AND IS DIRECTED TO RESOLVE ANY POTENTIAL DISCREPANCY WITH THE ENGINEER PRIOR TO CONSTRUCTION.

CURB BOX (ERIE STYLE W/

5/8" STAINLESS STEEL ROD)



133 COURT STREET PORTSMOUTH, NH 03801 (603) 433-2335 www.ALTUS-ENG.com



ISSUED FOR:

PLANNING BOARD APPROVAL

ISSUE DATE:

SEPTEMBER 14, 2018

REVISIONS NO. DESCRIPTION

O INITIAL SUBMISSION

EDW 09/14/18

DATE

RLH DRAWN BY: __ EDW APPROVED BY: 4916 DETAILS.DWG DRAWING FILE:

SCALE:

STAINLESS STEEL

LATERAL KIT

1-1/4"

SDR II HDPE PIPE

22"x 34": N.T.S.

APPLICANT/OWNER:

HAPPY MOUNTAIN HOLDINGS,

901 N. MARKET STREET SUITE 705 WILMINGTON, DE 19801

PROJECT:

RESIDENTIAL **DEVELOPMENT**

ASSESSOR'S PARCEL 220-87-2 74 EMERY STREET

ASSESSOR'S PARCEL 220-87-3 64 EMERY STREET

PORTSMOUTH, NEW HAMPSHIRE

TITLE:

KIT PARTS ARE NOT ASSEMBLED

SEWER SYSTEMS

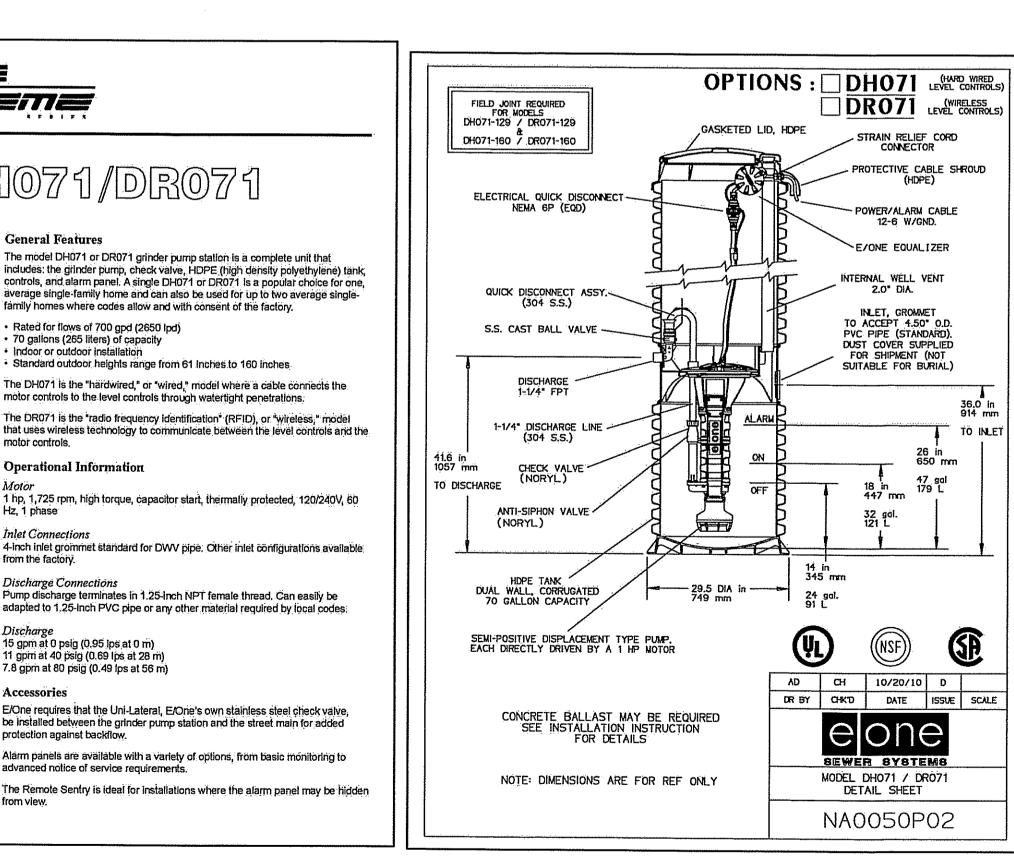
STAINLESS STEEL LATERAL KIT

1-1/4" SDR 11 HDPE PIPE

SITEWORK DETAILS

SHEET NUMBER:

C-5



NHDOT ITEM 403.12 - HOT BITUMINOUS CONCRETE PAVEMENT (4" NOMINAL) 1-1/2" WEARING COURSE, (TYPE 12 mm) 2-1/2" BINDER COURSE, (TYPE 19 mm) -SLOPE AS SHOWN ON PLANS - SEE TRENCH PATCH DETAIL 6" CRUSHED GRAVEL (NHDOT ITEM 304.3) 12" GRAVEL (NHDOT ITEM 304.2) CAUTION TAPE READING "CAUTION WATER LINE BURIED BELOW" SUITABLE EXCAVATED BACKFILL MATERIAL, OR GRANULAR MATERIAL WHERE SPECIFIED, COMPACTED IN 12" LIFTS TO 95% STANDARD PROCTOR MAXIMUM DENSITY. TYPE "K" COPPER WATER SERVICE OR DUCTILE IRON CLASS 52 WATER MAIN SAND BLANKET 3' (MIN)

SAND BLANKET/BARRIER % FINER BY WEIGHT 90 - 100

<u>NOTES</u>

BACKFILL MATERIAL BELOW PAVED OR CONCRETE AREAS, BEDDING MATERIAL, AND SAND BLANKET SHALL BE COMPACTED TO NOT LESS THAN 95% OF AASHTO T 99, METHOD C. SUITABLE BACKFILL MATERIAL BELOW LOAM AREAS SHALL BE COMPACTED TO NOT LESS THAN 90% OF AASHTO T 99.

- 2. WATER MAINS SHALL BE POLY WRAPPED.
- 3. WATER MAINS SHALL HAVE 3 WEDGES PER JOINT.

WATER MAIN TRENCH

NOT TO SCALE

0 - 15

SEWER PUMP STATION (E-ONE) DETAILS

NOT TO SCALE

COMPRESSION ADAPTER FITTING MATERIAL: POLYPROPYLENE COMPRESSION ADAPTER FITTING WITH THREADED END CAP W/INCREASER TO 2"ø TO MAIN TO PUMP VALVE CURB STOP WITH FEMALE PIPE THREADS AND VALVE POSITION STOPS (OPEN/CLOSED) WITH INTEGRAL CHECK VALVE MATERIAL: STAINLESS STEEL 1-1/4" SDR 11 POLYETHYLENE PIPE (CONTRACTOR) COMPRESSION ADAPTER MATERIAL: POLYPROPYLENE SS CURB STOP/CHECK VALVE AND FITTINGS ARE PROVIDED SEPARATELY, TO BE ASSEMBLED BY CONTRACTOR

> STAINLESS STEEL LATERAL KIT - 1 1/4" SDR 11 HDPE PIPE

2. TO ASSEMBLE, APPLY A DOUBLE LAYER OF TEFLON TAPE, AND A LAYER OF PIPE DOPE (SUPPLIED BY OTHERS) TO THE THREADS ON THE PLASTIC FITTINGS AND INSTALL PER THE

TO ORDER SS LATERAL KIT, USE PART NUMBER NC0193G01

CURB BOX IS TO BE ORDERED SEPARATELY, SEE ABOVE

MANUFACTURER'S INSTRUCTIONS

3. ASSEMBLY IS TO BE PRESSURE TESTED

ASSEMBLY IS TO BE USED WITH SDR11 HDPE PIPE

NOT TO SCALE



Civil Site Planning Environmental Engineering

133 Court Street Portsmouth, NH 03801-4413

Juliet T. H. Walker, AICP, Planning Director City of Portsmouth Municipal Complex Planning Department 1 Junkins Avenue Portsmouth, New Hampshire 03801

Re:

Application for Site Plan Review - Planning Board Approval Assessor's Map 220, Lots 87-2 and 87-3 74 and 64 Emery Street Altus Project P4916

Dear Juliet:

At the October 18th Planning Board meeting, the Board voted to postpone the application for Site Plan Approval to the November 15th meeting to allow the applicant to address the issues raised by the Board. The enclosed submittal addresses the following:

- Additional landscaping has been depicted on the plans;
- The drainage computations were amended;
- The landscape and recording waivers have been withdrawn;
- The plan notes have been revised to include the recording requirements.

Enclosed are twelve (12) copies of the following items for consideration at the November 15th Planning Board Meeting:

- Site Plans (two full size (22" x 34") and ten half size (11" x 17"))
- Revised Waiver Requests
- Revised Drainage Study (two full copies and ten summaries)
- CD with pdf copies of the revised application package.

We look forward to presenting the revised application package to the Planning Board. Please contact me directly if you have any questions or need any additional information.

Sincerely

Eric D. Weinrieb, PE

President

wde/4916-PB cvr ltr 11-06-18

Enclosure

Ecopy: Corey Cawthorn

Tel: (603) 433-2335 E-mail: Altus@altus-eng.com



Civil Site Planning Environmental Engineering

133 Court Street Portsmouth, NH 03801-4413

WAIVER REQUESTS
Assessor's Map 220
Lot 87-2 (74 Emery Street)
&
Lot 87-3 (64 Emery Street)
Altus Project P4916
Revised November 6, 2018

On behalf of Happy Mountain Holdings, LLC, Altus Engineering, Inc. request the following waivers from the City of Portsmouth, New Hampshire Site Plan Review Regulations.

Section 2.5.4 3 (C) Access and circulation

Section 2.5.4 3 (D) Parking and loading

Section 2.5.4 3 (J) Outdoor lighting

Section 3.4 Curbing (A) where access ways and driveways meet public streets

Section 5.2 Sidewalk and Pedestrian Pathways

Section 5.3 Bicycle Facilities

This project is unique in the fact that it is the development of two duplex homes on two abutting lots. Because four residential housing units are proposed, the project falls under the criteria for Site Plan Review Regulations. As such, the duplex homes do not require loading, outdoor lighting, curbing at the entrance, bicycle racks and other types of development features that normally are depicted on commercial site developments. We have combined all the waiver requests with a single explanation.

The intent of the above sections of the Site Plan Review Regulations are intended for projects where the public will be visiting the property. It is unreasonable to require outdoor bike racks for a duplex. Bicycles will be stored inside the homes just like any other duplex residential unit in Portsmouth. Likewise, the requirement to having loading facilities, curbing at the entrance and sidewalks as well as dedicated pedestrian pathways are not reasonable requests for this property. There will be building mounted lights at the entry doors, garages, and other locations that are typically seen on a residential development.

Wde/4916 waiver rev 11-18

Tel: (603) 433-2335 E-mail: Altus@altus-eng.com

Two Residential Duplexes

64 & 74 Emery Street
Portsmouth, NH
Assessor's Map 220, Lots 87-2 & 87-3

DRAINAGE STUDY

NOVEMBER 2018

Prepared For:

HAPPY MOUNTAIN HOLDINGS, LLC
901 N. MARKET STREET
SUITE 705
WILMINGTON, DE 19801
C/O:
Corey Cawthron
750 Lafayette Road
Portsmouth, NH 03801

Prepared By:

ALTUS ENGINEERING, INC.

133 Court Street Portsmouth, NH 03801 Phone: (603) 433-2335

DRAINAGE STUDY

EXECUTIVE SUMMARY

Happy Mountain Holdings, LLC and Corey Cawthron are planning to develop two residential lots that have recently been approved by the Board of Adjustment to allow each to have a duplex housing. The project involves just lot development as the utility services are available in the public right-of-way. There are no wetlands on the lot or within 100-feet of the lot lines. The lots are encumbered with a utility easement at the rear of the site. No site improvements are proposed in the easement. However, it is expected that the homeowners will mow and maintain the easement areas. The two lots that will be developed are:

Assessor's Parcel	Lot Area
220-87-2	32,427 SF
2220-87-3	21,232 SF

The two lots are 53,659 square feet in size (1.23 acres) and are predominantly wooded lots. The lots were created in 2013. At that time, the City approved the development with a shared driveway and utility cross easements to allow Lot 87-3 to be developed without impacting the ledge outcrop in the Emery Street right-of-way. The two lots are approved developable lots that could be developed with up to 60% impervious area based on zoning regulations, which would allow over 30,000 sf of impervious area. The proposed development will provide approximately 12,000 sf of combined impervious areas as well as three stormwater management ponds to reduce peak flows and provide stormwater treatment.

The proposed project will include the two duplexes, a shared driveway, new utility services and associated site improvements, including; site grading, drainage improvements, and utility service connections. Stormwater ponds will be constructed on each of the two residential lots to manage the storm water flow and provide treatment. The ponds will consist of a depressed lawn area with a loamy-sand material that will promote infiltration, drainage, and provide treatment.

DRAINAGE ANALYSIS

This drainage study is intended to show that the proposed development will manage and treat the stormwater to improve the existing site conditions and minimize impacts from the development. The project was analyzed to compare the ½", 2, 10, 25, and 50 year storm events. As a conservative design approach, which exceeds the city Site Plan Review Regulations, Altus has designed the site following the NHDES Alteration of Terrain rainfall criteria by adding 15-percent to the 24-hour rainfall precipitation for each storm event modeled.

The pre-development subcatchments were modeled and input into HydroCAD for analysis. The "Pre-Development Watershed Plan" illustrates the subcatchments that were modeled for the existing stormwater system. The existing site drains towards the Emery Street right-of-way with a high point near the proposed driveway that directs a portion of the flow to the south towards Myrtle Ave and a portion of the flow to the north towards Maplewood Avenue.

The "Post-Development Watershed Plan" illustrates the proposed stormwater management system. The original subcatchments have been divided into smaller areas to emulate the proposed grading and stormwater management system proposed for construction. The post-development conditions were analyzed at the same primary discharge points examined in the pre-development modeling.

For existing soil conditions the NRCS Web Soil Survey tool was used to determine the existing hydrologic soil groups. The entire site is listed as a type 799 soil series, urban land -canton complex. Utilizing canton as the underlying soil conditions, Hydrologic Soil Group (HSG) Type B was used for the entire site. Referencing the Ksat Values for NH Soils, the low C Ksat value for Canton is 6.0, which is typically multiplied by 0.5 to determine the infiltration design rate. For this analysis, an infiltration design rate of 1.0 was used as a conservative design approach.

The following Stormwater Modelling Summary compares pre-development and post-development peak rates of runoff for all analyzed storm events:

Stormwater Modeling Summary

The Stormwater Modeling Summary Table below shows the results for the peak flow rates for stormwater discharge for the ½" Inch, 2 year, 10 year, 25 year, and 50 year storm events:

Stormwater Modeling Summary Table
(Pre vs. Post-Development Stormwater Peak Runoff Rates)

	1/2"- Storm (0.5 inch)	2-Yr Storm (3.69 inch)	10-Yr Storm (5.60 inch)	25-Yr Storm (7.10 inch)	50-Yr Storm (8.50 inch)
POA #1					
Pre	0.0	0.3	1.5	2.8	4.3
Post	0.0	0.3	1.2	2.6	4.3
Change	0.0	-0.0	-0.3	-0.2	-0.0
POA #2					
Pre	0.0	0.1	0.3	0.5	0.7
Post	0.0	0.1	0.3	0.4	0.5
Change	0.0	-0.0	-0.0	-0.1	-0.2
Net Change	0.0	-0.0	-0.3	-0.3	-0.2

• HydroCAD results are rounded to the nearest 0.1 cfs for modeling tolerances.

As the Stormwater Modeling Summaries demonstrate, the proposed project will manage the stormwater runoff to mitigate impacts to the surrounding areas. The peak flow rates are managed to replicate the existing conditions.

CONCLUSION

The proposed project will not have an adverse effect on abutting properties and infrastructure as a result of stormwater runoff. As the stormwater summary indicates, the peak flow rates discharging from the site will be managed to minimize impacts to the surrounding areas. Four grass soil filter ponds will be constructed to provide retention and treatment of stormwater on site prior discharging to the Emery Street drainage. As noted in the drainage report, the stormwater model utilizes a number of conservative design approaches. Although not a City of Portsmouth requirement, a 15-percent increase was added to each rainfall event, similar to the requirements of NHDES Alteration of Terrain permitting. All of the other storm events analyzed indicate and equal or decreased peak flow rate at both points of analysis and illustrate that the stormwater is being managed to replicated the pre-development conditions and not adversely impact the abutting properties.

In addition to the permanent stormwater management practices, appropriate steps will be taken to properly mitigate erosion and sedimentation during construction through the use of temporary Best Management Practices for sediment and erosion control. In summary, the proposed development will manage stormwater runoff during construction and post development so that there is no adverse impact to the surrounding area as a result of this development.

Methodology

The drainage study was completed using the USDA SCS TR-20 Method within the HydroCAD Stormwater Modeling System. Reservoir routing was performed with the Dynamic Storage Indication method which automates the calculation of Tailwater conditions. A Type III 24-hour rainfall distribution was utilized in analyzing the data for the 0.5", 2, 10, 25, and 50 year 24-hour storm events using rainfall data obtained from the Northeast Regional Climate Center (NRCC) Extreme Precipitation Tables. As a conservative measure, 15-percent has been added to each rainfall mimic the requirements of NHDES Alteration of Terrain Permitting requirements. Site topography, existing features, proposed site improvements, proposed grading, drainage and erosion control measures are shown on the accompanying plans. Recommended erosion control measures are based upon the December 2008 edition of the "New Hampshire Stormwater Manual Volumes 1 through 3" prepared by NHDES and Comprehensive Environmental, Inc. as amended.

Stormwater Modeling Disclaimer

Altus Engineering, Inc. notes that stormwater modeling is limited in its capacity to precisely predict peak rates of runoff and flood elevations. Results should not be considered to represent actual storm events due to the number of variables and assumptions involved in the modeling effort. Surface roughness coefficients (n), entrance loss coefficients (ke), velocity factors (kv) and times of concentration (Tc) are based on subjective field observations and engineering judgment using available data. For design purposes, curve numbers (Cn) describe the average conditions. However, curve numbers will vary from storm to storm depending on the antecedent runoff conditions (ARC) including saturation and frozen ground. Also, higher water elevations than predicted by modeling could occur if drainage channels, closed drain systems or culverts are not maintained and/or become blocked by debris before and/or during a storm event as this will impact flow capacity of the structures. Structures should be re-evaluated if future changes occur within relevant drainage areas in order to assess any required design modification.

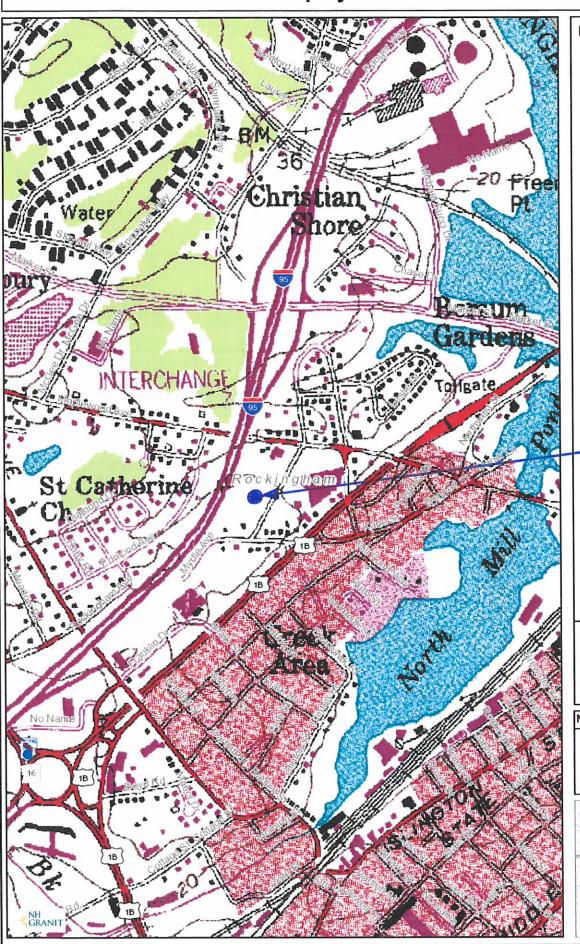
APPENDIX

- A. Site Maps
 - a. USGS Map
 - b. Aerial Image
- B. HydroCAD Modeling Results
 - a. Extreme Precipitation Table
 - b. Pre-Development (2, 10, 25, & 50 Year Storms)
 - c. Post Development (2, 10, 25, & 50 Year Storms)
- C. Web Soil Survey
- D. Plans
 - Project Site Plans (Separate Submittal)
 - Pre-Development Watershed Plan
 - Post-Development Watershed Plan

APPENDIX

- A. Site Maps
 - a. USGS Map
 - b. Aerial Image
- B. HydroCAD Modeling Results
 - a. Extreme Precipitation Table
 - b. Pre-Development (2, 10, 25, & 50 Year Storms)
 - c. Post Development (2, 10, 25, & 50 Year Storms)
- C. Web Soil Survey
- D. Plans
 - Project Site Plans (Separate Submittal)
 - Pre-Development Watershed Plan
 - Post-Development Watershed Plan

Map by NH GRANIT



Legend

- State
- County
- ☐ City/Town

PROJECT LOCATION

Map Scale

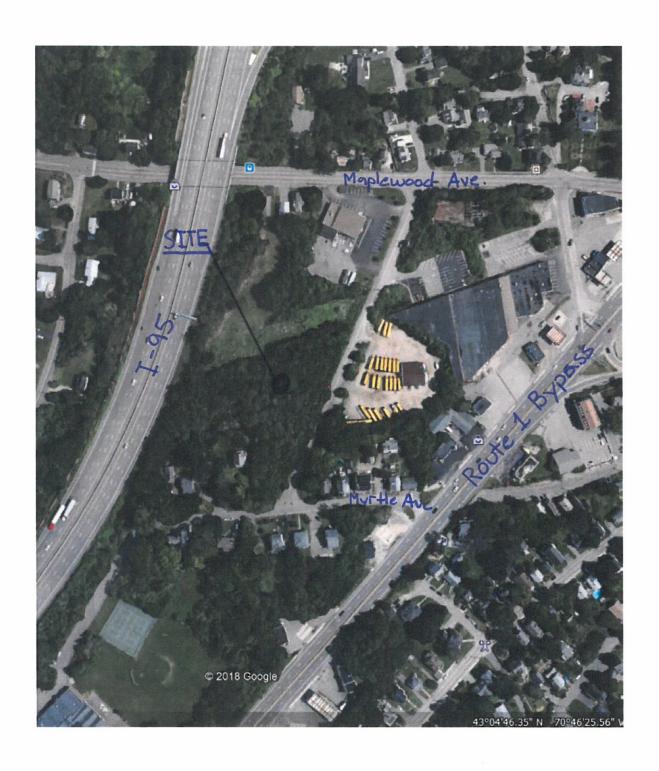
1: 10,000



© NH GRANIT, www.granit.unh.edu Map Generated: 9/14/2018

Notes





AERIAL PHOTOGRAPH LOTS 87-2 AND 87-3



The soil surveys that comprise your AOI were mapped at

Warning: Soil Map may not be valid at this scale.

misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed Enlargement of maps beyond the scale of mapping can cause scale.

Please rely on the bar scale on each map sheet for map measurements, Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

distance and area. A projection that preserves area, such as the Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA=NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Rockingham County, New Hampshire Survey Area Data: Version 19, Sep 11, 2017 Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. Date(s) aerial images were photographed: Dec 31, 2009—Jun

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

MAP LEGEND

Special Line Features Very Stony Spot Story Spot Spoil Area Wet Spot Other Q 8 Soil Map Unit Polygons Area of Interest (AOI) Soil Map Unit Points Soil Map Unit Lines Special Point Features Area of Interest (AOI)

Soils



Borrow Pd

Blowout

9

Clay Spot

Water Features



Closed Depression



Gravelly Spot

Gravel Pit



Lava Flow

Landi























Saline Spot Sandy Spot Severely Eroded Spot ŝ

Sinkhale

Slide or Slip

Sodic Spol

Web Soil Survey National Cooperative Soil Survey

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
799	Urban land-Canton complex, 3 to 15 percent slopes	12,8	100,0%
Totals for Area of Interest		12,8	100,0%

Extreme Precipitation Tables

Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Smoothing

State

New Hampshire

Location

Longitude

70.763 degrees West

Latitude

43.072 degrees North

Elevation

Date/Time

Mon, 30 Jul 2018 11:50:36 -0400

Extreme Precipitation Estimates

5min 10min 15min 30min 60min 120min 1hr 2hr 3hr 6hr 12hr 24hr 0.26 0.40 0.50 0.65 0.81 1.04 0.70 0.98 1.21 1.56 2.66 1yr 1yr 2.03 2yr 0.32 0.50 0.62 0.81 1.02 1.30 2yr 0.88 1.18 1.52 1.94 2.49 3.21 0.37 0.58 0.73 0.98 1.25 1.08 | 1.47 | 1.89 | 2.43 | 5yr 1.61 3.14 4.07 5yr 0.41 0.65 0.82 1.25 | 1.73 | 2.23 | 2.89 | 4.87 10yr 1.12 1.45 1.89 10yr 3.75 25yr 0.48 0.76 0.97 1.34 1.77 2.34 1.53 2.14 2.78 3.63 6.17 25yr 4.74 50yr 0.54 0.86 1.10 1.54 2.07 2.76 50yr | 1.79 | 2.53 | 3.29 | 4.32 | 5.66 7.39 0.60 0.97 1.25 1.77 2.42 3.26 100yr 2.09 2.98 3.90 5.16 8.85 100yr 6.77 2.82 200yr 2.44 3.52 4.62 6.13 8.08 200yr 0.67 1.10 1.43 2.05 3.83 10.61

4.76

- ^			
н	w	ıc	ı
•	-	۰	٦

15%

		1day	2day	4day	7day	10day	
	1yr	2.35	2.81	3.22	3.94	4.55	1yr
3.69	2yr	2.84	3.43	3.94	4.68	5.33	2yr
	5yr	3.60	4.40	5.04	5.94	6.70	5yr
5.60	10yr	4.31	5.32	6.09	7.11	7.98	10yr
7.10	25yr	5.46	6.83	7.80	9.03	10.05	25yr
8.50	50yr	6.54	8.25	9.42	10.81	11.98	50yr
10.18	00yr	7.83	9.98	11.38	12.96	14.27	100yr
	:00yr	9.39	12.07	13.76	15.55	17.02	200yr
	500yr	11.93	15.52	17.67	19.78	21.49	500yr

Lower Confidence Limits

1.31

1.71

2.48

3.48

500yr 0.80

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.23	0.36	0.44	0.59	0.72	0.88	1yr	0.63	0.86	0.92	1.33	1.68	2.24	2.49	1yr	1.98	2.40	2.87	3.18	3.90	1yr
2yr	0.31	0.49	0.60	0.81	1.00	1.19	2yr	0.86	1.16	1.37	1.82	2.34	3.06	3.45	2yr	2.71	3.32	3.82	4.55	5.08	2yr
5yr	0.35	0.54	0.67	0.92	1.17	1.40	5yr	1.01	1.37	1.61	2.12	2.73	3.79	4.19	5yr	3.35	4.03	4.72	5.53	6.24	5yr
10yr	0.39	0.59	0.73	1.03	1.33	1.60	10yr	1.14	1.56	1.80	2.39	3.06	4.37	4.86	10yr	3.87	4.67	5.44	6,41	7.20	10yr
25yr	0.44	0.67	0.83	1.19	1.56	1.90	25yr	1.35	1.86	2.10	2.75	3.53	4.72	5.89	25yr	4.18	5.66	6.65	7.79	8.68	25yr
50yr	0.48	0.73	0.91	1.31	1.76	2.17	50yr	1.52	2.12	2.35	3.07	3.93	5.33	6.80	50yr	4.72	6.54	7.72	9.04	10.02	50yr
100yr	0.54	0.81	1.01	1.47	2.01	2.47	100yr	1.73	2.41	2.63	3.41	4.35	6.00	7.85	100yr	5.31	7.55	8.98	10.51	11.56	100yr
200yr	0.59	0.89	1.13	1.63	2.28	2.81	200yr	1.96	2.75	2.93	3.78	4.79	6.72	9.06	200yr	5.95	8.71	10.42	12.22	13.37	200yr
500yr	0.68	1.02	1.31	1.90	2.71	3.36	500yr	2.34	3.29	3.41	4.31	5.45	7.82	10.94	500yr	6.92	10.52	12.69	14.96	16.19	500yr

500yr 3.00 4.38 5.76 7.70 10.22 13.48

Upper Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.28	0.44	0.54	0.72	0.89	1.08	1yr	0.77	1.06	1.26	1.74	2.21	2.98	3.16	1yr	2.64	3.04	3.58	4.37	5.04	1yr
2yr	0.34	0.52	0.64	0.86	1.07	1.27	2yr	0.92	1.24	1.48	1.96	2.51	3.42	3.70	2yr	3.03	3.56	4.09	4.84	5.63	2yr
5yr	0.40	0.62	0.77	1.05	1.34	1.62	5yr	1.15	1.58	1.88	2.53	3.25	4.34	4.96	5yr	3.84	4.77	5.38	6.37	7.16	5yr
10yr	0.47	0.72	0.89	1.25	1.61	1.98	10yr	1.39	1.93	2.28	3.11	3.95	5.34	6.20	10yr	4.72	5.96	6.82	7.84	8.75	10yr
25yr	0.58	0.88	1.09	1.56	2.05	2.57	25yr	1.77	2.51	2.95	4.07	5.15	7.78	8.34	25yr	6.88	8.02	9.15	10.34	11.41	25yr
50yr	0.67	1.02	1.27	1.83	2.46	3.13	50yr	2.12	3.06	3.60	5.00	6.32	9.74	10.46	50yr	8.62	10.06	11.44	12.72	13.96	50yr
100yr	0.79	1.19	1.49	2.16	2.96	3.81	100yr	2.55	3.72	4.37	6.16	7.76	12.18	13.10	100yr	10.78	12.60	14.31	15.69	17.09	100yr
200yr	0.92	1.39	1.76	2.55	3.56	4.65	200yr	3.07	4.55	5.34	7.58	9.54	15.28	16.44	200yr	13.53	15.81	17.92	19.35	20.92	200yr
500yr	1.15	1.71	2.19	3.19	4.53	6.04	500yr	3.91	5.90	6.93	10.02	12.56	20.65	22.20	500yr	18.27	21.34	24.13	25.51	27.34	500yr



POA #1



POA #2









Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.634	48	Brush, Good, HSG B (1S)
1.629	55	Woods, Good, HSG B (1S, 2S)
2.263	53	TOTAL AREA

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
2.263	HSG B	1S, 2S
0.000	HSG C	
0.000	HSG D	
0.000	Other	
2.263		TOTAL AREA

4916 pre
Prepared by Altus Engineering, Inc.
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 9/14/2018 Page 4

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.634	0.000	0.000	0.000	0.634	Brush, Good	18
0.000	1.629	0.000	0.000	0.000	1.629	Woods, Good	1S, 2S
0.000	2.263	0.000	0.000	0.000	2.263	TOTAL	
						AREA	

4916 pre

Type III 24-hr 0.5 Inch storm Rainfall=0.50"

Prepared by Altus Engineering, Inc.

Printed 9/14/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 5

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: POA #1

Runoff Area=88,216 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=425' Tc=21.5 min CN=53 Runoff=0.00 cfs 0.000 af

Subcatchment 2S: POA #2

Runoff Area=10,360 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=100' Slope=0.1000 '/' Tc=10.5 min CN=55 Runoff=0.00 cfs 0.000 af

Total Runoff Area = 2.263 ac Runoff Volume = 0.000 af Average Runoff Depth = 0.00" 100.00% Pervious = 2.263 ac 0.00% Impervious = 0.000 ac

Page 6

Printed 9/14/2018

Summary for Subcatchment 1S: POA #1

Runoff = 0.00 cfs @

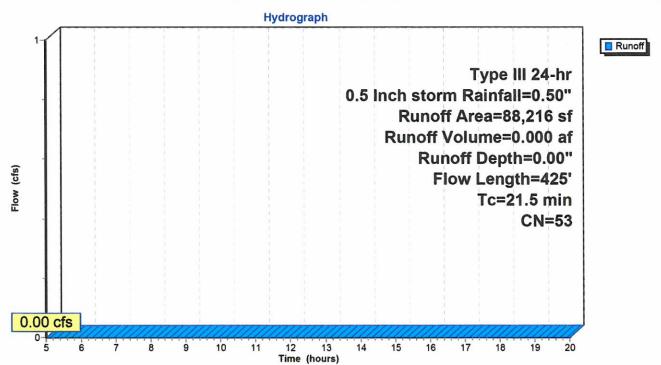
5.00 hrs, Volume=

0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

	Α	rea (sf)	CN [Description		
		27,600	48 E	Brush, Goo	d, HSG B	
		60,616	55 V	Voods, Go	od, HSG B	
		88,216	53 V	Veighted A	verage	
		88,216	1	100.00% Pe	ervious Are	a
	Тс	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	15.2	100	0.0400	0.11		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.69"
	6.3	325	0.0300	0.87		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	21.5	425	Total			

Subcatchment 1S: POA #1



Printed 9/14/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 7

Summary for Subcatchment 2S: POA #2

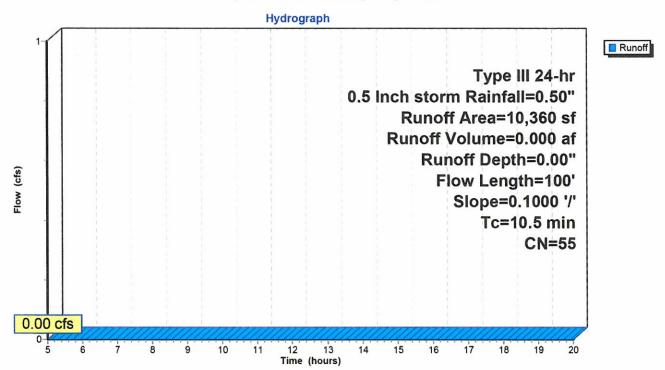
Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

_	A	rea (sf)	CN	Description							
_		10,360	55	Voods, Good, HSG B							
		10,360		100.00% Pe	00.00% Pervious Area						
_	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description					
	10.5	100	0.1000	0.16		Sheet Flow,					

Woods: Light underbrush n= 0.400 P2= 3.69"

Subcatchment 2S: POA #2



Type III 24-hr 2-yr storm Rainfall=3.69"

Prepared by Altus Engineering, Inc.

Printed 9/14/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 8

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: POA #1

Runoff Area=88,216 sf 0.00% Impervious Runoff Depth>0.29" Flow Length=425' Tc=21.5 min CN=53 Runoff=0.27 cfs 0.048 af

Subcatchment 2S: POA #2

2 Runoff Area=10,360 sf 0.00% Impervious Runoff Depth>0.35" Flow Length=100' Slope=0.1000 '/' Tc=10.5 min CN=55 Runoff=0.05 cfs 0.007 af

Total Runoff Area = 2.263 ac Runoff Volume = 0.055 af Average Runoff Depth = 0.29" 100.00% Pervious = 2.263 ac 0.00% Impervious = 0.000 ac

Page 9

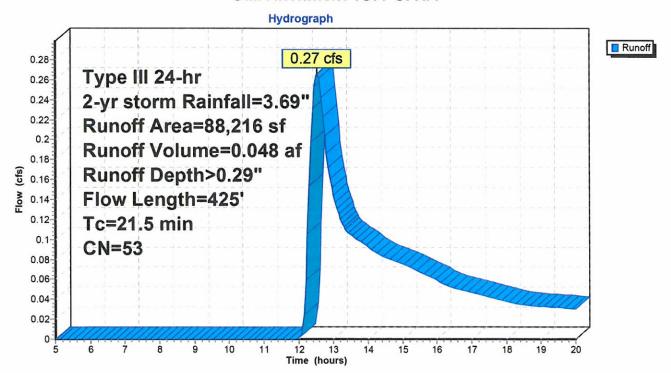
Summary for Subcatchment 1S: POA #1

Runoff = 0.27 cfs @ 12.53 hrs, Volume= 0.048 af, Depth> 0.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

	Α	rea (sf)	CN [Description				
		27,600	48 E	Brush, Goo	d, HSG B			
_		60,616	55 \	Voods, God	od, HSG B			
		88,216	53 \	Veighted A	verage	, , , , , , , , , , , , , , , , , , , ,	5000	
	88,216 100.00% Pervious Area							
	Tc	Length	Slope		Capacity	Description		
-	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	15.2	100	0.0400	0.11		Sheet Flow,		
						Woods: Light underbrush n= 0.400	P2= 3.69"	
	6.3	325	0.0300	0.87		Shallow Concentrated Flow,		
						Woodland Kv= 5.0 fps		
	21.5	425	Total					

Subcatchment 1S: POA #1



Summary for Subcatchment 2S: POA #2

Runoff

=

0.05 cfs @ 12.28 hrs, Volume=

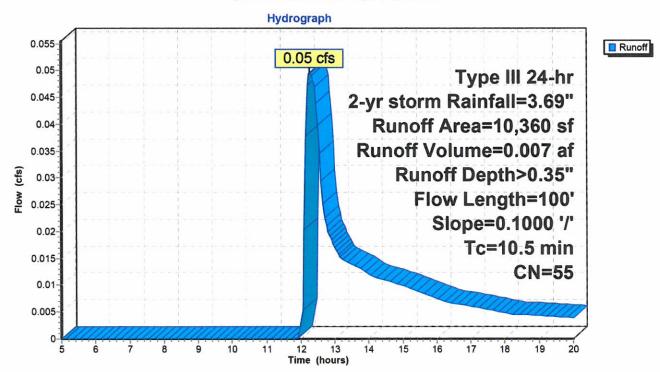
0.007 af, Depth> 0.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

	Α	rea (sf)	CN I	Description	1						
		10,360	55 \	Noods, Go	od, HSG B						
		10,360		100.00% P	0.00% Pervious Area						
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
_	10.5	100	0.1000	0.16		Sheet Flow,	85 00 000 W/10004750 W/87000	2010 Sept.			

Woods: Light underbrush n= 0.400 P2= 3.69"

Subcatchment 2S: POA #2



4916 pre

Type III 24-hr 10-yr storm Rainfall=5.60"

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 9/14/2018

Page 11

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: POA #1

Runoff Area=88,216 sf 0.00% Impervious Runoff Depth>1.02" Flow Length=425' Tc=21.5 min CN=53 Runoff=1.46 cfs 0.172 af

Subcatchment 2S: POA #2

Runoff Area=10,360 sf 0.00% Impervious Runoff Depth>1.16" Flow Length=100' Slope=0.1000 '/' Tc=10.5 min CN=55 Runoff=0.26 cfs 0.023 af

Total Runoff Area = 2.263 ac Runoff Volume = 0.195 af Average Runoff Depth = 1.04" 100.00% Pervious = 2.263 ac 0.00% Impervious = 0.000 ac

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 12

Summary for Subcatchment 1S: POA #1

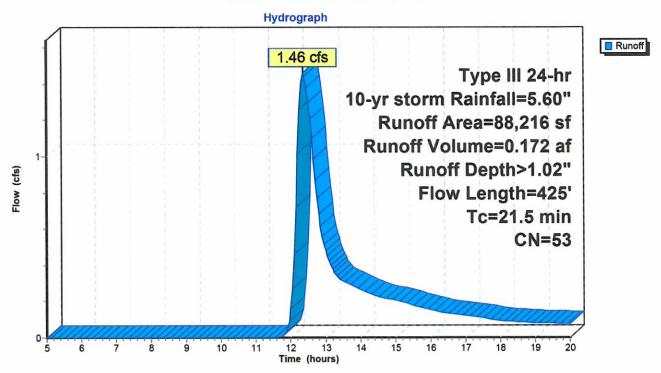
Runoff = 1.46 cfs @ 12.36 hrs, Volume=

0.172 af, Depth> 1.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

	Α	rea (sf)	CN [Description					
_		27,600	48 E	Brush, Goo	d, HSG B				
		60,616	55 \	Woods, Good, HSG B					
		88,216	53 \	Neighted A	verage				
	а								
	Tc (min)	Length (feet)	Slope (ft/ft)	•	Capacity (cfs)	Description			
-	15.2	100	0.0400	0.11		Sheet Flow,			
	6.3	325	0.0300	0.87		Woods: Light underbrush n= 0.400 P2= 3.69" Shallow Concentrated Flow, Woodland Kv= 5.0 fps			
	21.5	425	Total						

Subcatchment 1S: POA #1



HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 13

Summary for Subcatchment 2S: POA #2

Runoff =

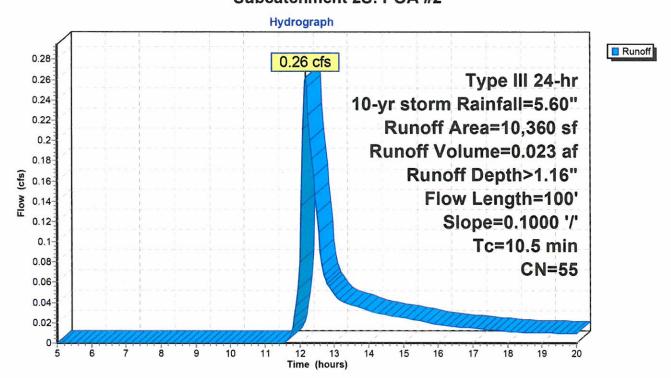
0.26 cfs @ 12.17 hrs, Volume=

0.023 af, Depth> 1.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

	Α	rea (sf)	CN	Description				
_		10,360	55	Woods, Go	od, HSG B			
		10,360						
	Tc (min)	Length (feet)	Slop (ft/ft		Capacity (cfs)	Description		
	10.5	100	0.100	0.16		Sheet Flow, Woods: Light underbrush	n= 0.400	P2= 3.69"

Subcatchment 2S: POA #2



4916 pre

Type III 24-hr 25-yr storm Rainfall=7.10"

Prepared by Altus Engineering, Inc.

Printed 9/14/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 14

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: POA #1

Runoff Area=88,216 sf 0.00% Impervious Runoff Depth>1.80" Flow Length=425' Tc=21.5 min CN=53 Runoff=2.82 cfs 0.303 af

Subcatchment 2S: POA #2

2 Runoff Area=10,360 sf 0.00% Impervious Runoff Depth>1.99" Flow Length=100' Slope=0.1000 '/' Tc=10.5 min CN=55 Runoff=0.49 cfs 0.039 af

Total Runoff Area = 2.263 ac Runoff Volume = 0.343 af Average Runoff Depth = 1.82" 100.00% Pervious = 2.263 ac 0.00% Impervious = 0.000 ac

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 15

Summary for Subcatchment 1S: POA #1

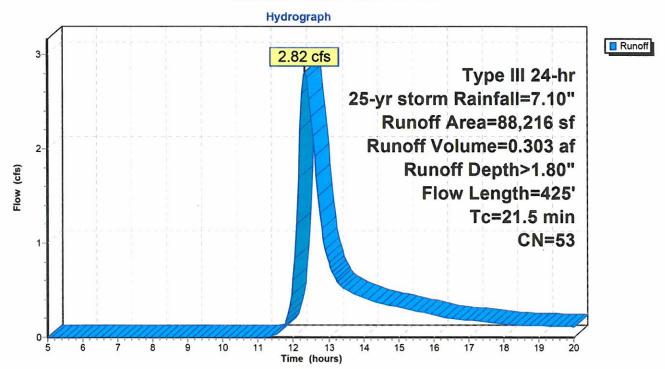
Runoff = 2.82 cfs @ 12.33 hrs, Volume=

0.303 af, Depth> 1.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

_	Α	rea (sf)	CN [Description		
		27,600	48 E	Brush, Goo	d, HSG B	
_		60,616	55 \	Noods, Go	od, HSG B	
		88,216	53 \	Veighted A	verage	
		88,216	•	100.00% Pe	ervious Are	a
	Тс	Length	Slope	•	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	15.2	100	0.0400	0.11		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.69"
	6.3	325	0.0300	0.87		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	21.5	425	Total			

Subcatchment 1S: POA #1



Page 16

Summary for Subcatchment 2S: POA #2

Runoff

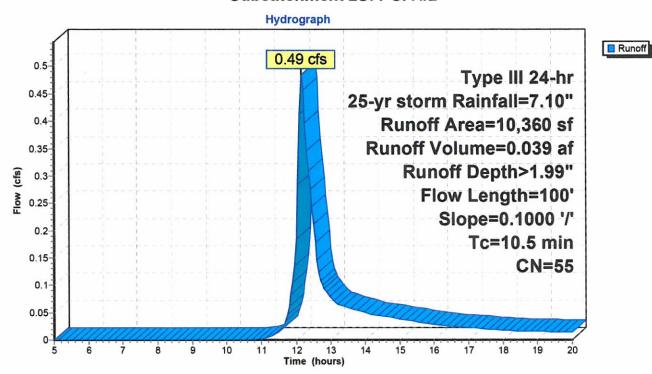
0.49 cfs @ 12.16 hrs, Volume=

0.039 af, Depth> 1.99"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

	Α	rea (sf)	CN	Description	escription						
		10,360	55	Woods, Go	od, HSG B						
		10,360		100.00% P	00.00% Pervious Area						
	Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description					
-	10.5	100	0.1000	0.16		Sheet Flow, Woods: Light underbrush	n= 0.400	P2= 3.69"			

Subcatchment 2S: POA #2



Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 9/14/2018 Page 17

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: POA #1

Runoff Area=88,216 sf 0.00% Impervious Runoff Depth>2.63" Flow Length=425' Tc=21.5 min CN=53 Runoff=4.28 cfs 0.445 af

Subcatchment 2S: POA #2

Runoff Area=10,360 sf 0.00% Impervious Runoff Depth>2.87" Flow Length=100' Slope=0.1000 '/' Tc=10.5 min CN=55 Runoff=0.72 cfs 0.057 af

Total Runoff Area = 2.263 ac Runoff Volume = 0.501 af Average Runoff Depth = 2.66" 100.00% Pervious = 2.263 ac 0.00% Impervious = 0.000 ac

Printed 9/14/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 18

Summary for Subcatchment 1S: POA #1

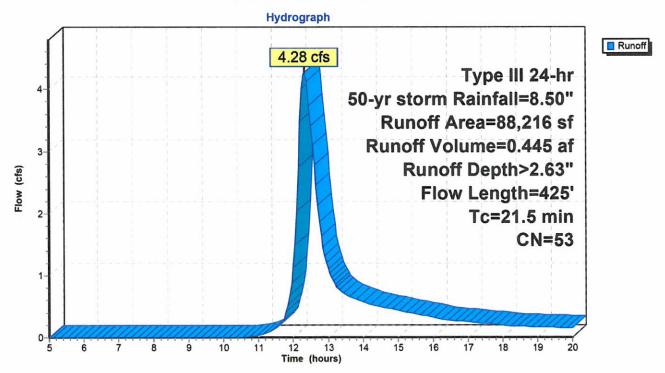
Runoff = 4.28 cfs @ 12.32 hrs, Volume=

0.445 af, Depth> 2.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

	Α	rea (sf)	CN [Description		
		27,600	48 E	Brush, Goo	d, HSG B	
		60,616	55 \	Voods, Go	od, HSG B	
_		88,216	53 \	Veighted A	verage	
		88,216	1	00.00% Pe	ervious Are	a
	Тс	Length	Slope	•	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	15.2	100	0.0400	0.11		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.69"
	6.3	325	0.0300	0.87		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	21.5	425	Total			

Subcatchment 1S: POA #1



Page 19

Summary for Subcatchment 2S: POA #2

Runoff =

0.72 cfs @ 12.16 hrs, Volume=

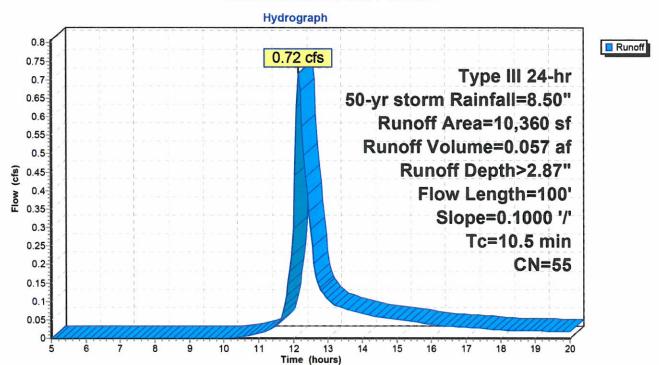
0.057 af, Depth> 2.87"

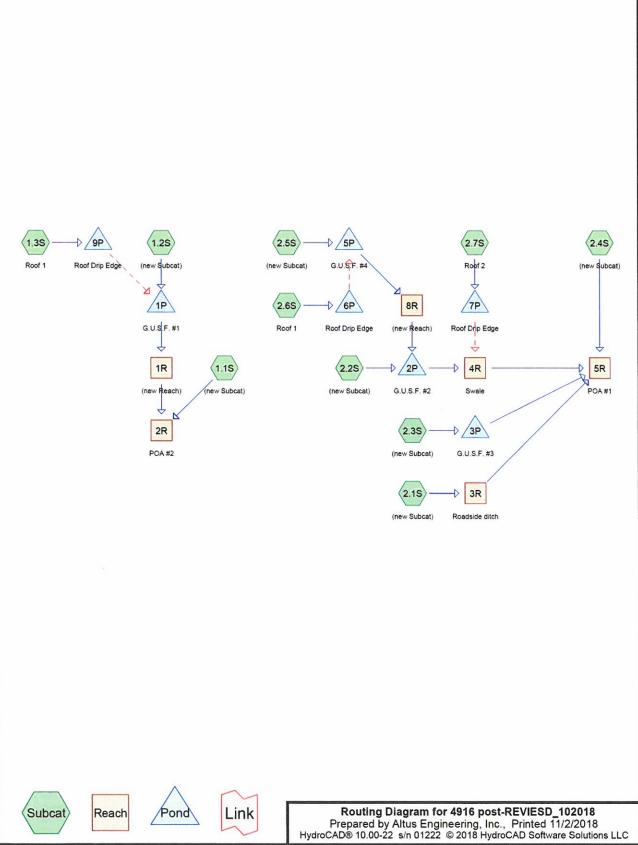
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

	Α	rea (sf)	CN I	Description			
		10,360	55 \	Noods, Go	od, HSG B		
		10,360		100.00% P	ervious Are	а	
112	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	10.5	100	0.1000	0.16		Sheet Flow,	

Woods: Light underbrush n= 0.400 P2= 3.69"

Subcatchment 2S: POA #2





4916 post-REVIESD_102018
Prepared by Altus Engineering, Inc.
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 11/2/2018 Page 2

Area Listing (all nodes)

Area	Area CN Description	
(acres)	(subcatchment-numbers)
0.479	9 61	>75% Grass cover, Good, HSG B (1.1S, 1.2S, 1.3S, 2.1S, 2.2S, 2.3S, 2.4S, 2.5S, 2.6S, 2.7S)
0.58	7 48	Brush, Good, HSG B (2.2S, 2.4S, 2.5S)
0.160	98	Paved parking, HSG B (1.1S, 1.2S, 2.1S, 2.2S, 2.3S, 2.4S)
0.093	3 98	Roofs, HSG B (1.2S, 1.3S, 2.2S, 2.3S, 2.6S, 2.7S)
0.939	9 55	Woods, Good, HSG B (1.1S, 1.2S, 2.1S, 2.3S, 2.4S, 2.5S)
2.26	3 59	TOTAL AREA

4916 post-REVIESD_102018
Prepared by Altus Engineering, Inc.
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 11/2/2018 Page 3

Soil Listing (all nodes)

,	Area Soil	Subcatchment
(ac	cres) Grou	o Numbers
0	.000 HSG	A
2	.263 HSG	B 1.1S, 1.2S, 1.3S, 2.1S, 2.2S, 2.3S, 2.4S, 2.5S, 2.6S, 2.7S
0	.000 HSG	С
0	.000 HSG	D
0	.000 Other	•
2	.263	TOTAL AREA

4916 post-REVIESD_102018
Prepared by Altus Engineering, Inc.
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 11/2/2018 Page 4

Ground Covers (all nodes)

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
0.000	0.479	0.000	0.000	0.000	0.479	>75% Grass cover, Good	1.1S,
							1.2S,
							1.3S,
							2.1S,
							2.2S,
							2.3S,
							2.4S,
							2.5S,
							2.6S,
							2.78
0.000	0.587	0.000	0.000	0.000	0.587	Brush, Good	2.2S,
							2.4S,
							2.58
0.000	0.166	0.000	0.000	0.000	0.166	Paved parking	1.1S,
							1.2S,
							2.1S,
							2.28,
							2.3S,
							2.48
0.000	0.093	0.000	0.000	0.000	0.093	Roofs	1.28,
							1.3S,
							2.28,
							2.38,
							2.6S,
							2.7S
0.000	0.939	0.000	0.000	0.000	0.939	Woods, Good	1.1S,
							1.2S,
							2.1S,
							2.3S,
							2.4S,
. 12/2/2		121 2 2101	8 6 5 5		10 101011		2.5S
0.000	2.263	0.000	0.000	0.000	2.263	TOTAL AREA	

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Type III 24-hr 0.5 Inch storm Rainfall=0.50" Printed 11/2/2018

Page 5

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1.1S: (new Su	bcat) Runoff Area=3,988 sf 25.45% Impervious Runoff Depth=0.00" bw Length=130' Slope=0.0750 '/' Tc=6.0 min CN=70 Runoff=0.00 cfs 0.000 af
Subcatchment 1.2S: (new Su	bcat) Runoff Area=5,446 sf 25.27% Impervious Runoff Depth=0.00" Flow Length=230' Tc=18.1 min CN=69 Runoff=0.00 cfs 0.000 af
Subcatchment 1.3S: Roof 1	Runoff Area=410 sf 85.37% Impervious Runoff Depth>0.10" Tc=6.0 min CN=93 Runoff=0.00 cfs 0.000 af
Subcatchment 2.1S: (new Su	Runoff Area=8,038 sf 36.17% Impervious Runoff Depth=0.00" Flow Length=95' Tc=6.0 min CN=73 Runoff=0.00 cfs 0.000 af
Subcatchment 2.2S: (new Su	Runoff Area=10,052 sf 9.19% Impervious Runoff Depth=0.00" Flow Length=200' Tc=12.3 min CN=61 Runoff=0.00 cfs 0.000 af
Subcatchment 2.3S: (new Su	Runoff Area=3,804 sf 34.23% Impervious Runoff Depth=0.00" Flow Length=65' Tc=6.0 min CN=72 Runoff=0.00 cfs 0.000 af
Subcatchment 2.4S: (new Su	Runoff Area=19,654 sf 0.74% Impervious Runoff Depth=0.00" Flow Length=130' Tc=7.5 min CN=50 Runoff=0.00 cfs 0.000 af
Subcatchment 2.5S: (new Su	Runoff Area=43,461 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=320' Tc=17.7 min CN=54 Runoff=0.00 cfs 0.000 af
Subcatchment 2.6S: Roof 1	Runoff Area=1,740 sf 87.36% Impervious Runoff Depth>0.10" Tc=6.0 min CN=93 Runoff=0.00 cfs 0.000 af
Subcatchment 2.7S: Roof 2	Runoff Area=2,000 sf 86.00% Impervious Runoff Depth>0.10" Tc=6.0 min CN=93 Runoff=0.00 cfs 0.000 af
Reach 1R: (new Reach)	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=100.0' S=0.1020 '/' Capacity=6.31 cfs Outflow=0.00 cfs 0.000 af
Reach 2R: POA #2	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=0.00 cfs 0.000 af
Reach 3R: Roadside ditch	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=160.0' S=0.0125 '/' Capacity=2.21 cfs Outflow=0.00 cfs 0.000 af
Reach 4R: Swale	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=85.0' S=0.0624 '/' Capacity=4.93 cfs Outflow=0.00 cfs 0.000 af
Reach 5R: POA #1	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=1.0' S=0.1000'/' Capacity=6.24 cfs Outflow=0.00 cfs 0.000 af
Reach 8R: (new Reach)	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=80.0' S=0.0250 '/' Capacity=3.12 cfs Outflow=0.00 cfs 0.000 af

4916 pos	t-R	EVIE	SD	10201	8
----------	-----	------	----	-------	---

Type III 24-hr 0.5 Inch storm Rainfall=0.50"

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 11/2/2018 Page 6

Pond 1P: G.U.S.F.#1			Storage=0 cf Inflow=0.00 c	
	Discarded=0.00 cfs 0.000	af Primary=0.00	cfs 0.000 af Outflow=0.00 c	fs 0.000 af
Pond 2P: G.U.S.F. #2		Peak Elev=45.00'	Storage=0 cf Inflow=0.00 c	fs 0 000 af
	Discarded=0.00 cfs 0.000		cfs 0.000 af Outflow=0.00 c	
Pond 3P: G.U.S.F. #3		Peak Flev=46 00'	Storage=0 cf Inflow=0.00 c	fs 0.000 af
1 0110 01 1 010.0.1 1 110	Discarded=0.00 cfs 0.000		cfs 0.000 af Outflow=0.00 c	
Pond 5P: G.U.S.F.#4		Dock Flour-49 FO	Storage=0 of Inflow=0.00 o	fo 0.000 of
Pond 5P: G.U.S.F. #4	Discarded=0.00 cfs 0.000		Storage=0 cf Inflow=0.00 c cfs 0.000 af Outflow=0.00 c	
Pond 6P: Roof Drip Edge		Peak Elev=50.02'	Storage=1 cf Inflow=0.00 c	fs 0.000 af
D	iscarded=0.00 cfs 0.000 af	Secondary=0.00	cfs 0.000 af Outflow=0.00 c	fs 0.000 af
Pond 7P: Roof Drip Edge		Peak Elev=48.01'	Storage=2 cf Inflow=0.00 c	fs 0.000 af
			cfs 0.000 af Outflow=0.00 c	
Pond 9P: Roof Drip Edge		Peak Flav-50 01'	Storage=0 cf Inflow=0.00 c	ofo 0.000 of
U	iscarded-0.00 dis 0.000 ar	Secondary=0.00	cfs 0.000 af Outflow=0.00 c	is 0.000 at

Total Runoff Area = 2.263 ac Runoff Volume = 0.001 af Average Runoff Depth = 0.00" 88.58% Pervious = 2.005 ac 11.42% Impervious = 0.258 ac

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 7

Summary for Subcatchment 1.1S: (new Subcat)

Runoff

0.00 cfs @ 5.00 hrs, Volume=

0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

_	Α	rea (sf)	CN [Description							
		1,015	98 F	Paved parki	aved parking, HSG B						
		2,373	61 >	75% Grass cover, Good, HSG B							
_		600	55 \	Voods, Good, HSG B							
		3,988	70 \	Neighted A	eighted Average						
		2,973	7	74.55% Per	vious Area						
		1,015	2	25.45% Imp	ervious Are	ea					
	Tc	Length	Slope	(A. 1900)	Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	0.5	130	0.0750	4.11		Shallow Concentrated Flow,					
						Grassed Waterway Kv= 15.0 fps					
	0.5	130	Total.	Increased t	o minimum	Tc = 6.0 min					

Summary for Subcatchment 1.2S: (new Subcat)

Runoff

0.00 cfs @ 5.00 hrs, Volume=

0.000 af, Depth= 0.00"

	Α	rea (sf)	CN [Description						
		150	98 F	Roofs, HSG B						
		1,226	98 F	Paved park	ing, HSG B					
		2,570		>75% Grass cover, Good, HSG B						
_		1,500	55 \	Woods, Good, HSG B						
		5,446		Neighted Average						
		4,070			∿ious Area					
		1,376	- 2	25.27% lmp	pervious Ar	ea				
	_									
	Tc	Length	Slope		Capacity	Description				
_	(min)	(feet)	(ft/ft)		(cfs)					
	15.2	100	0.0400	0.11		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 3.69"				
	2.7	100	0.0150	0.61		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	0.2	30	0.0200	2.12		Shallow Concentrated Flow,				
_						Grassed Waterway Kv= 15.0 fps				
	19 1	230	Total							

Type III 24-hr 0.5 Inch storm Rainfall=0.50"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 8

Summary for Subcatchment 1.3S: Roof 1

Runoff

0.00 cfs @ 12.11 hrs, Volume=

0.000 af, Depth> 0.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

	Α	rea (sf)	CN	Description								
		350	98	Roofs, HSG	Roofs, HSG B							
_		60	61	>75% Gras	75% Grass cover, Good, HSG B							
		410	93	Weighted A	/eighted Average							
		60		14.63% Pervious Area								
		350		85.37% Imp	ervious Ar	rea						
	Тс	Length	Slope		Capacity	Description						
	(min)	(feet)	(ft/ft	(ft/sec)	(cfs)							
	6.0					Direct Entry,						

Summary for Subcatchment 2.1S: (new Subcat)

Runoff

0.00 cfs @ 5.00 hrs, Volume=

0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

A	rea (sf)	CN	Description		
	2,907	98	Paved park	ing, HSG B	
	3,899	61	>75% Grass	s cover, Go	ood, HSG B
	1,232	55	Woods, Go	od, HSG B	2
	8,038	73	Weighted A	verage	
	5,131		63.83% Per	vious Area	
	2,907		36.17% Imp	ervious Are	ea
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.3	60	0.2000	0.19		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.2	35	0.0600	3.67		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
5.5	95	Total,	Increased t	o minimum	Tc = 6.0 min

Summary for Subcatchment 2.2S: (new Subcat)

Runoff

0.00 cfs @ 5.00 hrs, Volume=

0.000 af, Depth= 0.00"

Type III 24-hr 0.5 Inch storm Rainfall=0.50"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 9

_	A	rea (sf)	CN E	escription							
		150	98 F	98 Roofs, HSG B							
		774	98 F	aved park	ing, HSG B						
		6,280	61 >	75% Grass	s cover, Go	ood, HSG B					
_		2,848	48 E	Brush, Goo	d, HSG B						
		10,052	61 V	Veighted A	verage						
		9,128	9	0.81% Per	vious Area						
		924	9	.19% Impe	ervious Area	a					
	Tc	Length	Slope	Velocity	Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	11.5	100	0.0800	0.15		Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 3.69"					
	8.0	100	0.0200	2.12		Shallow Concentrated Flow,					
						Grassed Waterway Kv= 15.0 fps					
	12.3	200	Total								

Summary for Subcatchment 2.3S: (new Subcat)

Runoff

0.00 cfs @ 5.00 hrs, Volume=

0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

A	rea (sf)	CN	Description					
	150	98	Roofs, HSG B					
	1,152	98	Paved park	ing, HSG B				
	1,622	61	>75% Gras	s cover, Go	ood, HSG B			
	880	_ 55	Woods, Go	od, HSG B				
	3,804	72	Weighted Average					
	2,502		65.77% Per	rvious Area				
	1,302		34.23% Imp	pervious Ar	ea			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
4.3	40	0.1500	0.16		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.69"			
0.1	25	0.1800	6.36		Shallow Concentrated Flow,			
					Grassed Waterway Kv= 15.0 fps			
4.4	0.5	T-4-1	1 1 (T 00:			

4.4 65 Total, Increased to minimum Tc = 6.0 min

Summary for Subcatchment 2.4S: (new Subcat)

Runoff 0.00 cfs @ 5.00 hrs, Volume=

0.000 af, Depth= 0.00"

Type III 24-hr 0.5 Inch storm Rainfall=0.50"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 10

	A	rea (sf)	CN [Description							
		145	98 F	Paved parking, HSG B							
		1,100	61 >	75% Gras	s cover, Go	ood, HSG B					
		17,252	48 E	Brush, Goo	d, HSG B						
_		1,157	55 V	Voods, Go	od, HSG B						
		19,654	50 V	Veighted A	verage						
		19,509	9	9.26% Per	vious Area						
		145	(.74% Impe	ervious Area	a					
	Tc	Length	Slope	Velocity	Capacity	Description					
1924	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	6.4	50	0.0875	0.13	E	Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 3.69"					
	1.1	80	0.0625	1.25		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
	7.5	130	Total								

Summary for Subcatchment 2.5S: (new Subcat)

Runoff

0.00 cfs @ 5.00 hrs, Volume=

0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

_	A	rea (sf)	CN [Description								
		2,480	61 >	>75% Grass	75% Grass cover, Good, HSG B							
		5,450	48 E	Brush, Goo	d, HSG B							
_		35,531	55 \	Noods, Go	od, HSG B							
		43,461		Neighted A								
		43,461	•	100.00% Pe	ervious Are	a						
	Тс	Length	Slope	•	Capacity	Description						
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	15.2	100	0.0400	0.11		Sheet Flow,						
						Woods: Light underbrush n= 0.400 P2= 3.69"						
	1.2	50	0.0200	0.71		Shallow Concentrated Flow,						
				Woodland Kv= 5.0 fps								
	1.3	170	0.0200	2.12		Shallow Concentrated Flow,						
Grassed Waterway Kv= 15.0 fps						Grassed Waterway Kv= 15.0 fps						
	17.7	320	Total									

Summary for Subcatchment 2.6S: Roof 1

Runoff 0.00 cfs @ 12.11 hrs, Volume= 0.000 af, Depth> 0.10"

Type III 24-hr 0.5 Inch storm Rainfall=0.50"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 11

	Α	rea (sf)	CN	Description				_			
		1,520	98	Roofs, HSG	Roofs, HSG B						
722		220	61	>75% Gras	75% Grass cover, Good, HSG B						
1.77		1,740	93	Weighted A	Veighted Average						
		220		12.64% Per	12.64% Pervious Area						
		1,520		87.36% Imp	ervious Are	ea					
	Тс	Length	Slop	,	Capacity	Description					
	(min)	(feet)	(ft/f	(ft/sec)	(cfs)			_			
	6.0					Direct Entry.					

Summary for Subcatchment 2.7S: Roof 2

Runoff = 0.00 cfs @ 12.11 hrs, Volume=

0.000 af, Depth> 0.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 lnch storm Rainfall=0.50"

A	rea (sf)	CN	Description								
	1,720	98	Roofs, HSG	Roofs, HSG B							
	280	61	>75% Gras	s cover, Go	ood, HSG B						
	2,000	93	Weighted A	Veighted Average							
	280		14.00% Per	vious Area							
	1,720		86.00% Imp	ervious Ar	ea						
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description						
6.0		1,	Lanca de la constante de la co		Direct Entry,						

Visitati di Angelia di Gilia di Angelia di A

Summary for Reach 1R: (new Reach)

Inflow Area = 0.125 ac, 25.27% Impervious, Inflow Depth = 0.00" for 0.5 Inch storm event

Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min

Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.31 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight

Length= 100.0' Slope= 0.1020 '/'

Inlet Invert= 50.20', Outlet Invert= 40.00'

Prepared by Altus Engineering, Inc.

Type III 24-hr 0.5 Inch storm Rainfall=0.50"

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 12



Summary for Reach 2R: POA #2

Inflow Area = 0.217 ac, 25.34% Impervious, Inflow Depth = 0.00" for 0.5 Inch storm event

Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight

Length= 1.0' Slope= 0.1000 '/'

Inlet Invert= 40.00'. Outlet Invert= 39.90'



Summary for Reach 3R: Roadside ditch

Inflow Area = 0.185 ac, 36.17% Impervious, Inflow Depth = 0.00" for 0.5 Inch storm event

Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 2.21 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight

Length= 160.0' Slope= 0.0125 '/'

Inlet Invert= 42.00', Outlet Invert= 40.00'

Type III 24-hr 0.5 Inch storm Rainfall=0.50"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 13



Summary for Reach 4R: Swale

Inflow Area = 1.228 ac, 1.73% Impervious, Inflow Depth = 0.00" for 0.5 Inch storm event *

Inflow 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Outflow 5.00 hrs, Volume= 0.00 cfs @ 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 4.93 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight

Length= 85.0' Slope= 0.0624 '/'

Inlet Invert= 47.30', Outlet Invert= 42.00'



Summary for Reach 5R: POA #1

Inflow Area = 6.21% Impervious, Inflow Depth = 0.00" for 0.5 Inch storm event

Inflow 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Outflow 5.00 hrs, Volume= 0.00 cfs @ 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min

Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight

Length= 1.0' Slope= 0.1000 '/'

Inlet Invert= 40.00', Outlet Invert= 39.90'

Type III 24-hr 0.5 Inch storm Rainfall=0.50" Printed 11/2/2018

Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 14



Summary for Reach 8R: (new Reach)

Inflow Area = 0.998 ac, 0.00% Impervious, Inflow Depth = 0.00" for 0.5 Inch storm event

Inflow 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Outflow 0.00 cfs @ 5.00 hrs. Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 3.12 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight

Length= 80.0' Slope= 0.0250 '/'

Inlet Invert= 50.00'. Outlet Invert= 48.00'



Summary for Pond 1P: G.U.S.F. #1

Inflow Area = 0.125 ac, 25.27% Impervious, Inflow Depth = 0.00" for 0.5 Inch storm event

Inflow 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Outflow 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Discarded = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af Primary 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 48.50' @ 5.00 hrs Surf.Area= 300 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no inflow)

Volume	Invert	Avail.Storage	Storage Description
#1	48.50'	1,552 cf	Custom Stage Data (Prismatic)Listed below

Davisa Bautina

Type III 24-hr 0.5 Inch storm Rainfall=0.50"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 15

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
48.50	300	0.0	0	0
49.00	300	40.0	60	60
50.00	1,115	100.0	708	768
50.60	1,500	100.0	785	1,552

Device	Routing	inveπ	Outlet Devices				
#1	Discarded	48.50'	1.000 in/hr Exfiltration over Surface area				
#2	Primary	50.20'	4.0' long x 8.0' breadth Broad-Crested Rectangular Weir				
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00				
			2.50 3.00 3.50 4.00 4.50 5.00 5.50				
			Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64				
			2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74				

Discarded OutFlow Max=0.00 cfs @ 5.00 hrs HW=48.50' (Free Discharge) 1=Exfiltration (Passes 0.00 cfs of 0.01 cfs potential flow)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=48.50' (Free Discharge) —2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 2P: G.U.S.F. #2

Inflow Area =	1.228 ac,	1.73% Impervious, Inflow Do	epth = 0.00" fe	or 0.5 Inch storm event
Inflow =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	
Outflow =	0.00 cfs @	5.00 hrs, Volume=	0.000 af, Atten	= 0%, Lag= 0.0 min
Discarded =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	
Primary =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 45.00' @ 5.00 hrs Surf.Area= 500 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no inflow)

Volume	Invert	Ava	il.Storage	Storage Descr	ription	
#1	45.00'		1,236 c	f Custom Stag	e Data (Prismatio	:)Listed below
Elevation		urf.Area	Voids	Inc.Store	Cum.Store	
(fee	The second secon	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
45.0	00	500	0.0	0	0	
46.5	50	500	40.0	300	300	
47.0	00	697	100.0	299	599	
47.7	75	1,000	100.0	636	1,236	
					•	
Device	Routing	In	vert Ou	ıtlet Devices		
#1	Discarded	45	5.00' 1.0	000 in/hr Exfiltra	tion over Surface	area
#2	Primary	47	'.30' 4.0)' long x 8.0' bre	adth Broad-Cres	ted Rectangular Weir
	,					00 1.20 1.40 1.60 1.80 2.00
				강경에 가장 그렇게 되는 것이 없는 그렇게 없는 그 없다고 있다.	00 4.50 5.00 5.50	
				, , ,		2.68 2.68 2.66 2.64 2.64
			26	64 265 265 26	66 266 268 27	N 274

Type III 24-hr 0.5 Inch storm Rainfall=0.50"

Prepared by Altus Engineering, Inc.
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 11/2/2018

Page 16

Discarded OutFlow Max=0.00 cfs @ 5.00 hrs HW=45.00' (Free Discharge) 1=Exfiltration (Passes 0.00 cfs of 0.01 cfs potential flow)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=45.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 3P: G.U.S.F. #3

 Inflow Area =
 0.087 ac, 34.23% Impervious, Inflow Depth = 0.00" for 0.5 Inch storm event

 Inflow =
 0.00 cfs @ 5.00 hrs, Volume=
 0.000 af

 Outflow =
 0.00 cfs @ 5.00 hrs, Volume=
 0.000 af, Atten= 0%, Lag= 0.0 min

 Discarded =
 0.00 cfs @ 5.00 hrs, Volume=
 0.000 af

 Primary =
 0.00 cfs @ 5.00 hrs, Volume=
 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 46.00' @ 5.00 hrs Surf.Area= 110 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Avail Storage Storage Description

Center-of-Mass det. time= (not calculated: no inflow)

Valuma

volume	Inve	en Ava	II.Storage	Storage Descrip	otion	
#1	46.0	00'	347 cf	Custom Stage	Data (Prismatic)	Listed below
Elevation (fee		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
46.0	00	110	0.0	0	0	
47.5	50	110	40.0	66	66	
48.0	00	302	100.0	103	169	
48.	50	410	100.0	178	347	
Device	Routing	In	vert Ou	tlet Devices		
#1	Discarde	d 46	6.00' 1.0	00 in/hr Exfiltration	on over Surface	area
#2	Primary	47	'.90' 4.0	long x 8.0 brea	dth Broad-Crest	ed Rectangular Weir
	-		He	ad (feet) 0.20 0.4	0 0.60 0.80 1.0	0 1.20 1.40 1.60 1.80 2.00
			2.5	0 3.00 3.50 4.00	4.50 5.00 5.50	
			Co	ef. (English) 2.43	2.54 2.70 2.69	2.68 2.68 2.66 2.64 2.64
			2.6	4 2.65 2.65 2.66	2.66 2.68 2.70	2.74

Discarded OutFlow Max=0.00 cfs @ 5.00 hrs HW=46.00' (Free Discharge) 1=Exfiltration (Passes 0.00 cfs of 0.00 cfs potential flow)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=46.00' (Free Discharge)

2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 5P: G.U.S.F. #4

Type III 24-hr 0.5 Inch storm Rainfall=0.50"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 17

Inflow Area =	0.998 ac,	0.00% Impervious, Inflow D	Depth = 0.00"	for 0.5 Inch storm event
Inflow =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	
Outflow =	0.00 cfs @	5.00 hrs, Volume=	0.000 af, Atte	en= 0%, Lag= 0.0 min
Discarded =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	
Primary =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 48.50' @ 5.00 hrs Surf.Area= 300 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no inflow)

Volume	Inve	ert Ava	il.Storage	Storage Descri	ption	
#1	48.5	50'	1,540 cf	Custom Stage	Data (Prismatic)Listed below
Elevatio		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
48.5	50	300	0.0	0	0	
49.0	00	300	40.0	60	60	
50.0	00	1,100	100.0	700	760	
50.6	0	1,500	100.0	780	1,540	
Device	Routing	In	vert Outl	et Devices		
#1	Discarde	d 48	.50' 1.00	0 in/hr Exfiltrati	on over Surface	area
#2	Primary	50	.00' 4.0'	long x 8.0' brea	adth Broad-Cres	ted Rectangular Weir
			Hea	d (feet) 0.20 0.4	40 0.60 0.80 1.0	00 1.20 1.40 1.60 1.80 2.00
			2.50	3.00 3.50 4.00	0 4.50 5.00 5.50)
			Coe	f. (English) 2.43	2.54 2.70 2.69	2.68 2.68 2.66 2.64 2.64
			2.64	2.65 2.65 2.66	6 2.66 2.68 2.70	2.74

Discarded OutFlow Max=0.00 cfs @ 5.00 hrs HW=48.50' (Free Discharge) 1=Exfiltration (Passes 0.00 cfs of 0.01 cfs potential flow)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=48.50' (Free Discharge) —2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 6P: Roof Drip Edge

Inflow Area =	0.040 ac, 87.36% Impervious, Inflow Depth > 0.10"	for 0.5 Inch storm event
Inflow =	0.00 cfs @ 12.11 hrs, Volume= 0.000 af	
Outflow =	0.00 cfs @ 12.21 hrs, Volume= 0.000 af, Atte	en= 28%, Lag= 6.2 min
Discarded =	0.00 cfs @ 12.21 hrs, Volume= 0.000 af	
Secondary =	0.00 cfs @ 5.00 hrs, Volume= 0.000 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.02' @ 12.21 hrs Surf.Area= 220 sf Storage= 1 cf

Plug-Flow detention time= 7.2 min calculated for 0.000 af (99% of inflow) Center-of-Mass det. time= 5.7 min (840.9 - 835.2)

Type III 24-hr 0.5 Inch storm Rainfall=0.50"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 18

Volume	Invert	Avail.S	torage	Storage Descrip	ption	
#1	50.00'		286 cf	Custom Stage	Data (Prismatic)Listed below
Elevation (fee		rf.Area Vo	oids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
50.0	00	220	0.0	0	0	
52.0	00	220 4	0.0	176	176	
52.5	50	220 10	0.00	110	286	
#1 #2 #3	Routing Discarded Discarded Secondary	50.50 50.00 52.00	Limi 1.00 1 1.00 1 110 Hea 2.50 Coe	.0' long x 2.0' bid (feet) 0.20 0.40 3.00 3.50	Grate C= 0.600 at low heads fon over Surface readth Broad-Cr 40 0.60 0.80 1.0 2.61 2.61 2.60	e area rested Rectangular Weir 200 1.20 1.40 1.60 1.80 2.00 2.66 2.70 2.77 2.89 2.88

Discarded OutFlow Max=0.01 cfs @ 12.21 hrs HW=50.02' (Free Discharge)

1=Orifice/Grate (Controls 0.00 cfs)
2=Exfiltration (Exfiltration Controls 0.01 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=50.00' (Free Discharge)
—3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 7P: Roof Drip Edge

Inflow Area =	0.046 ac, 86.00% Impervious, Inflow De	epth > 0.10" for 0.5 Inch storm event
Inflow =	0.00 cfs @ 12.11 hrs, Volume=	0.000 af
Outflow =	0.00 cfs @ 12.21 hrs, Volume=	0.000 af, Atten= 28%, Lag= 6.2 min
Discarded =	0.00 cfs @ 12.21 hrs, Volume=	0.000 af
Secondary =	0.00 cfs @ 5.00 hrs. Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 48.01' @ 12.21 hrs Surf.Area= 280 sf Storage= 2 cf

Plug-Flow detention time= 7.2 min calculated for 0.000 af (99% of inflow) Center-of-Mass det. time= 5.7 min (840.9 - 835.2)

Volume	Invert Ava	il.Storage	Storage Descrip	tion	
#1	48.00'	364 cf	Custom Stage	Data (Prismatic)Listed b	elow
Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
48.00	280	0.0	0	0	
50.00	280	40.0	224	224	
50.50	280	100.0	140	364	

Type III 24-hr 0.5 Inch storm Rainfall=0.50"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 19

Device	Routing	Invert	Outlet Devices
#1	Discarded	48.50'	6.0" Vert. Orifice/Grate C= 0.600
#2	Discarded	48.00'	1.000 in/hr Exfiltration over Surface area
#3	Secondary	50.00'	140.0' long x 2.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50
			Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88
			2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.01 cfs @ 12.21 hrs HW=48.01' (Free Discharge)

1=Orifice/Grate (Controls 0.00 cfs)

Invert

Volume

-2=Exfiltration (Exfiltration Controls 0.01 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=48.00' (Free Discharge)
—3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 9P: Roof Drip Edge

Inflow Area =	0.009 ac, 85.37% Impervious, Inflow De	epth > 0.10" for 0.5 Inch storm event
Inflow =	0.00 cfs @ 12.11 hrs, Volume=	0.000 af
Outflow =	0.00 cfs @ 12.21 hrs, Volume=	0.000 af, Atten= 28%, Lag= 6.2 min
Discarded =	0.00 cfs @ 12.21 hrs, Volume=	0.000 af
Secondary =	0.00 cfs @ 5.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.01' @ 12.21 hrs Surf.Area= 60 sf Storage= 0 cf

Plug-Flow detention time= 7.2 min calculated for 0.000 af (99% of inflow) Center-of-Mass det. time= 5.7 min (840.9 - 835.2)

Avail.Storage Storage Description

				Ctorage Becompact	
#1	50.00'		78 cf	Custom Stage Data (Prismatic)Listed below	
Elevation (fee		f.Area (sq-ft)	Voids (%)	Inc.Store Cum.Store (cubic-feet) (cubic-feet)	
50.0		60	0.0	0 0	
52.0	00	60	40.0	48 48	
52.5	50	60	100.0	30 78	
Device #1 #2 #3	Routing Discarded Discarded Secondary	50. 50.	.50' 6.0" .00' 1.00 .00' 110. Hea 2.50 Coe	let Devices "Vert. Orifice/Grate C= 0.600 00 in/hr Exfiltration over Surface area 0.0' long x 3.0' breadth Broad-Crested Rectangular Weir ad (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.0 0 3.00 3.50 4.00 4.50 ef. (English) 2.44 2.58 2.68 2.67 2.65 2.64 2.64 2.68 2.68 2 2.81 2.92 2.97 3.07 3.32	

Type III 24-hr 0.5 Inch storm Rainfall=0.50"

Printed 11/2/2018

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 20

Discarded OutFlow Max=0.00 cfs @ 12.21 hrs HW=50.01' (Free Discharge)
—1=Orifice/Grate (Controls 0.00 cfs)
—2=Exfiltration (Exfiltration Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=50.00' (Free Discharge)
—3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Type III 24-hr 2-yr storm Rainfall=3.69" Printed 11/2/2018

Page 21

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1.1S: (new Subcat	Cat) Runoff Area=3,988 sf 25.45% Impervious Runoff Depth>1.03" Length=130' Slope=0.0750'/' Tc=6.0 min CN=70 Runoff=0.11 cfs 0.008 af
Subcatchment 1.2S: (new Subc	Runoff Area=5,446 sf 25.27% Impervious Runoff Depth>0.97" Flow Length=230' Tc=18.1 min CN=69 Runoff=0.10 cfs 0.010 af
Subcatchment 1.3S: Roof 1	Runoff Area=410 sf 85.37% Impervious Runoff Depth>2.76" Tc=6.0 min CN=93 Runoff=0.03 cfs 0.002 af
Subcatchment 2.1S: (new Subc	Runoff Area=8,038 sf 36.17% Impervious Runoff Depth>1.20" Flow Length=95' Tc=6.0 min CN=73 Runoff=0.27 cfs 0.018 af
Subcatchment 2.2S: (new Subc	Runoff Area=10,052 sf 9.19% Impervious Runoff Depth>0.58" Flow Length=200' Tc=12.3 min CN=61 Runoff=0.11 cfs 0.011 af
Subcatchment 2.3S: (new Subc	Runoff Area=3,804 sf 34.23% Impervious Runoff Depth>1.14" Flow Length=65' Tc=6.0 min CN=72 Runoff=0.12 cfs 0.008 af
Subcatchment 2.4S: (new Subc	Runoff Area=19,654 sf 0.74% Impervious Runoff Depth>0.20" Flow Length=130' Tc=7.5 min CN=50 Runoff=0.04 cfs 0.008 af
Subcatchment 2.5S: (new Subc	Runoff Area=43,461 sf 0.00% Impervious Runoff Depth>0.32" Flow Length=320' Tc=17.7 min CN=54 Runoff=0.16 cfs 0.027 af
Subcatchment 2.6S: Roof 1	Runoff Area=1,740 sf 87.36% Impervious Runoff Depth>2.76" Tc=6.0 min CN=93 Runoff=0.13 cfs 0.009 af
Subcatchment 2.7S: Roof 2	Runoff Area=2,000 sf 86.00% Impervious Runoff Depth>2.76" Tc=6.0 min CN=93 Runoff=0.15 cfs 0.011 af
Reach 1R: (new Reach)	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=100.0' S=0.1020 '/' Capacity=6.31 cfs Outflow=0.00 cfs 0.000 af
Reach 2R: POA #2	Avg. Flow Depth=0.08' Max Vel=2.86 fps Inflow=0.11 cfs 0.008 af n=0.022 L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=0.11 cfs 0.008 af
Reach 3R: Roadside ditch	Avg. Flow Depth=0.18' Max Vel=1.78 fps Inflow=0.27 cfs 0.018 af n=0.022 L=160.0' S=0.0125 '/' Capacity=2.21 cfs Outflow=0.25 cfs 0.018 af
Reach 4R: Swale	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=85.0' S=0.0624 '/' Capacity=4.93 cfs Outflow=0.00 cfs 0.000 af
Reach 5R: POA #1	Avg. Flow Depth=0.11' Max Vel=3.70 fps Inflow=0.26 cfs 0.027 af n=0.022 L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=0.26 cfs 0.027 af
Reach 8R: (new Reach)	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=80.0' S=0.0250 '/' Capacity=3.12 cfs Outflow=0.00 cfs 0.000 af

4916	post-RE\	/IESD	102018
------	----------	-------	--------

Type III 24-hr 2-yr storm Rainfall=3.69"

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 11/2/2018
Page 22

Pond 1P: G.U.S.F.#1	Peak Elev=49.22' Storage=214 cf Inflow=0.10 cfs 0.0 Discarded=0.01 cfs 0.007 af Primary=0.00 cfs 0.000 af Outflow=0.01 cfs 0.0	
Pond 2P: G.U.S.F.#2	Peak Elev=46.11' Storage=223 cf Inflow=0.11 cfs 0.0 Discarded=0.01 cfs 0.008 af Primary=0.00 cfs 0.000 af Outflow=0.01 cfs 0.000 af Outflow=0.000 af	
Pond 3P: G.U.S.F. #3	Peak Elev=47.92' Storage=152 cf Inflow=0.12 cfs 0.0 Discarded=0.01 cfs 0.004 af Primary=0.03 cfs 0.001 af Outflow=0.03 cfs 0.001	
Pond 5P: G.U.S.F.#4	Peak Elev=49.82' Storage=636 cf Inflow=0.16 cfs 0. Discarded=0.02 cfs 0.013 af Primary=0.00 cfs 0.000 af Outflow=0.02 cfs	
Pond 6P: Roof Drip Edge	Peak Elev=50.58' Storage=51 cf Inflow=0.13 cfs 0. scarded=0.13 cfs 0.009 af Secondary=0.00 cfs 0.000 af Outflow=0.13 cfs 0.	
Pond 7P: Roof Drip Edge D	Peak Elev=48.72' Storage=81 cf Inflow=0.15 cfs 0. scarded=0.14 cfs 0.011 af Secondary=0.00 cfs 0.000 af Outflow=0.14 cfs 0.	
Pond 9P: Roof Drip Edge	Peak Elev=50.60' Storage=14 cf Inflow=0.03 cfs 0. scarded=0.03 cfs 0.002 af Secondary=0.00 cfs 0.000 af Outflow=0.03 cfs 0.	

Total Runoff Area = 2.263 ac Runoff Volume = 0.112 af Average Runoff Depth = 0.59" 88.58% Pervious = 2.005 ac 11.42% Impervious = 0.258 ac

Type III 24-hr 2-yr storm Rainfall=3.69"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 23

Summary for Subcatchment 1.1S: (new Subcat)

Runoff = 0.11 cfs @ 12.10 hrs, Volume=

0.008 af, Depth> 1.03"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

	Α	rea (sf)	CN [Description					
		1,015	98 F	Paved park	ing, HSG B				
		2,373	61 >	75% Gras	s cover, Go	ood, HSG B			
		600	55 \	Voods, Go	od, HSG B				
		3,988	70 \	Veighted A	verage				
		2,973	7	4.55% Per	vious Area				
		1,015	2	25.45% Imp	pervious Are	ea			
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	0.5	130	0.0750	4.11		Shallow Concentrated Flow,			
						Grassed Waterway Kv= 15.0 fps			
177	0.5	130	Total,	II, Increased to minimum Tc = 6.0 min					

Summary for Subcatchment 1.2S: (new Subcat)

Runoff = 0.10 cfs @ 12.28 hrs, Volume=

0.010 af, Depth> 0.97"

A	rea (sf)	CN E	escription		
	150	98 F	Roofs, HSG	В	
	1,226	98 F	aved park	ng, HSG B	
	2,570	61 >	75% Gras	s cover, Go	od, HSG B
	1,500	55 V	Voods, Go	od, HSG B	
	5,446	69 V	Veighted A	verage	
	4,070	7	4.73% Per	vious Area	
	1,376	2	25.27% Imp	ervious Are	ea
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
15.2	100	0.0400	0.11		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
2.7	100	0.0150	0.61		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.2	30	0.0200	2.12		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
18.1	230	Total			

Type III 24-hr 2-yr storm Rainfall=3.69"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 24

Summary for Subcatchment 1.3S: Roof 1

Runoff = 0.03 cfs @ 12.09 hrs, Volume=

0.002 af, Depth> 2.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

_	A	rea (sf)	CN	Description								
-		350	98	Roofs, HSG	Roofs, HSG B							
-		60	61	>75% Gras	s cover, Go	ood, HSG B						
-		410	93	Weighted A	eighted Average							
		60		14.63% Per	vious Area							
		350		85.37% Imp	ervious Ar	ea						
	Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description						
	6.0					Direct Entry,						

Summary for Subcatchment 2.1S: (new Subcat)

Runoff = 0.27 cfs @ 12.10 hrs, Volume=

0.018 af, Depth> 1.20"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

	A	rea (sf)	CN	Description		
		2,907	98	Paved parki	ing, HSG B	
		3,899	61	>75% Grass	s cover, Go	od, HSG B
_		1,232	55	Woods, God	od, HSG B	
		8,038	73	Weighted A	verage	
		5,131		63.83% Per	vious Area	
		2,907		36.17% Imp	ervious Are	ea
	Тс	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	5.3	60	0.2000	0.19		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.69"
	0.2	35	0.0600	3.67		Shallow Concentrated Flow,
						Grassed Waterway Kv= 15.0 fps
	5.5	95	Total,	Increased t	o minimum	Tc = 6.0 min

Summary for Subcatchment 2.2S: (new Subcat)

Runoff = 0.11 cfs @ 12.22 hrs, Volume=

0.011 af, Depth> 0.58"

Type III 24-hr 2-yr storm Rainfall=3.69"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 25

_	Α	rea (sf)	CN [Description								
		150	98 F	Roofs, HSG B								
		774	98 F	Paved park	ing, HSG B							
		6,280	61	75% Gras	s cover, Go	ood, HSG B						
		2,848	48 E	Brush, Goo	d, HSG B	4410						
		10,052	61 \	Weighted A	verage							
		9,128	9	90.81% Per	vious Area							
		924	9	9.19% Impe	ervious Area	a						
	Tc	Length	Slope		Capacity	Description						
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	11.5	100	0.0800	0.15		Sheet Flow,						
						Woods: Light underbrush n= 0.400 P2= 3.69"						
	8.0	100	0.0200	2.12		Shallow Concentrated Flow,						
_						Grassed Waterway Kv= 15.0 fps						
	12.3	200	Total									

Summary for Subcatchment 2.3S: (new Subcat)

Runoff = 0.12 cfs @ 12.10 hrs, Volume=

0.008 af, Depth> 1.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

A	rea (sf)	CN I	Description		
	150	98	Roofs, HSG	ВВ	
	1,152	98	Paved park	ing, HSG B	
	1,622	61	>75% Gras	s cover, Go	ood, HSG B
	880	55 \	Noods, Go	od, HSG B	
	3,804	72 \	Neighted A	verage	
	2,502	(55.77% Per	vious Area	
	1,302	:	34.23% Imp	ervious Ar	ea
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
4.3	40	0.1500	0.16		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.1	25	0.1800	6.36		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
4 4	O.F.	Total			T- 00-1

4.4 65 Total, Increased to minimum Tc = 6.0 min

Summary for Subcatchment 2.4S: (new Subcat)

Runoff = 0.04 cfs @ 12.39 hrs, Volume=

0.008 af, Depth> 0.20"

Type III 24-hr 2-yr storm Rainfall=3.69"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 26

	Α	rea (sf)	CN	Description		
		145	98	Paved park	ing, HSG B	
		1,100	61	>75% Gras	s cover, Go	ood, HSG B
		17,252	48	Brush, Goo	d, HSG B	
		1,157	55	Woods, Go	od, HSG B	
		19,654	50	Weighted A	verage	
		19,509	0	99.26% Per	vious Area	
		145		0.74% Impe	ervious Area	a
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.4	50	0.0875	0.13		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.69"
	1.1	80	0.0625	1.25		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	7.5	130	Total			

Summary for Subcatchment 2.5S: (new Subcat)

Runoff = 0.16 cfs @ 12.45 hrs, Volume=

0.027 af, Depth> 0.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

	Α	rea (sf)	CN E	escription		
0.77		2,480				ood, HSG B
		5,450		Brush, Good	**************************************	
_		35,531	THE RESERVE TO STATE OF THE PARTY.	Voods, God	NAME OF TAXABLE PARTY.	
		43,461		Veighted A		
		43,461	1	00.00% Pe	ervious Are	a
	75-20	2 22	12/8	4 0 0 101		
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	15.2	100	0.0400	0.11		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.69"
	1.2	50	0.0200	0.71		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	1.3	170	0.0200	2.12		Shallow Concentrated Flow,
						Grassed Waterway Kv= 15.0 fps
	17 7	320	Total			

Summary for Subcatchment 2.6S: Roof 1

Runoff = 0.13 cfs @ 12.09 hrs, Volume= 0.009 af, Depth> 2.76"

Type III 24-hr 2-yr storm Rainfall=3.69"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 27

A	rea (sf)	CN	Description		
	1,520	98	Roofs, HSG	ВВ	
	220	61	>75% Gras	s cover, Go	ood, HSG B
	1,740	93	Weighted A	verage	
	220		12.64% Per	vious Area	a ·
	1,520		87.36% Imp	ervious Are	rea
Тс	Length	Slope	e Velocity	Capacity	Description
(min)	(feet)	(ft/ft	,	(cfs)	Bookiption
6.0					Direct Entry,

Summary for Subcatchment 2.7S: Roof 2

Runoff

0.15 cfs @ 12.09 hrs, Volume=

0.011 af, Depth> 2.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

A	rea (sf)	CN	Description				
	1,720	98	Roofs, HSG	ВВ			
	280	61	>75% Gras	s cover, Go	ood, HSG B		
	2,000 280 1,720		Weighted Average 14.00% Pervious Area 86.00% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description		
6.0					Direct Entry,		

Summary for Reach 1R: (new Reach)

0.125 ac, 25.27% Impervious, Inflow Depth = 0.00" for 2-yr storm event Inflow Area = Inflow

0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Outflow 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min

Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.31 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 100.0' Slope= 0.1020 '/' Inlet Invert= 50.20', Outlet Invert= 40.00'

Type III 24-hr 2-yr storm Rainfall=3.69"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC Page 28



Summary for Reach 2R: POA #2

Inflow Area = 0.217 ac, 25.34% Impervious, Inflow Depth > 0.43" for 2-yr storm event

Inflow = 0.11 cfs @ 12.10 hrs, Volume= 0.008 af

Outflow = 0.11 cfs @ 12.10 hrs, Volume= 0.008 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.86 fps, Min. Travel Time= 0.0 min Avg. Velocity = 1.23 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.10 hrs Average Depth at Peak Storage= 0.08' Bank-Full Depth= 0.50' Flow Area= 0.7 sf. Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



Summary for Reach 3R: Roadside ditch

Inflow Area = 0.185 ac, 36.17% Impervious, Inflow Depth > 1.20" for 2-yr storm event

Inflow = 0.27 cfs @ 12.10 hrs, Volume= 0.018 af

Outflow = 0.25 cfs @ 12.15 hrs, Volume= 0.018 af, Atten= 6%, Lag= 2.9 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.78 fps, Min. Travel Time= 1.5 min Avg. Velocity = 0.75 fps, Avg. Travel Time= 3.6 min

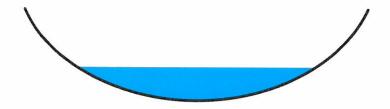
Peak Storage= 23 cf @ 12.12 hrs Average Depth at Peak Storage= 0.18' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 2.21 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 160.0' Slope= 0.0125 '/' Inlet Invert= 42.00', Outlet Invert= 40.00'

Type III 24-hr 2-yr storm Rainfall=3.69" Printed 11/2/2018

Prepared by Altus Engineering, Inc.
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 29



Summary for Reach 4R: Swale

Inflow Area = 1.228 ac, 1.73% Impervious, Inflow Depth = 0.00" for 2-yr storm event

Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 4.93 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight

Length= 85.0' Slope= 0.0624 '/'

Inlet Invert= 47.30', Outlet Invert= 42.00'



Summary for Reach 5R: POA #1

Inflow Area = 1.952 ac, 6.21% Impervious, Inflow Depth > 0.17" for 2-yr storm event

Inflow = 0.26 cfs @ 12.16 hrs, Volume= 0.027 af

Outflow = 0.26 cfs @ 12.16 hrs, Volume= 0.027 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.70 fps, Min. Travel Time= 0.0 min

Avg. Velocity = 1.81 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.16 hrs

Average Depth at Peak Storage= 0.11'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight

Length= 1.0' Slope= 0.1000 '/'

Inlet Invert= 40.00', Outlet Invert= 39.90'

Type III 24-hr 2-yr storm Rainfall=3.69"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 30



Summary for Reach 8R: (new Reach)

Inflow Area = 0.998 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-yr storm event

Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 3.12 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight

Length= 80.0' Slope= 0.0250 '/'

Inlet Invert= 50.00', Outlet Invert= 48.00'



Summary for Pond 1P: G.U.S.F. #1

Inflow Area = 0.125 ac, 25.27% Impervious, Inflow Depth > 0.97" for 2-yr storm event

Inflow = 0.10 cfs @ 12.28 hrs, Volume= 0.010 af

Outflow = 0.01 cfs @ 14.76 hrs, Volume= 0.007 af, Atten= 89%, Lag= 148.9 min

Discarded = 0.01 cfs @ 14.76 hrs, Volume= 0.007 af Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 49.22' @ 14.76 hrs Surf.Area= 477 sf Storage= 214 cf

Plug-Flow detention time= 192.2 min calculated for 0.007 af (69% of inflow)

Center-of-Mass det. time= 120.2 min (952.7 - 832.4)

Volume	Invert	Avail.Storage	Storage Description
#1	48.50'	1,552 cf	Custom Stage Data (Prismatic)Listed below

Type III 24-hr 2-yr storm Rainfall=3.69"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 31

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
48.50	300	0.0	0	0
49.00	300	40.0	60	60
50.00	1,115	100.0	708	768
50.60	1,500	100.0	785	1,552

Device	Routing	Invert	Outlet Devices			
#1	Discarded	48.50'	1.000 in/hr Exfiltration over Surface area			
#2	Primary	50.20'	4.0' long x 8.0' breadth Broad-Crested Rectangular Weir			
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00			
			2.50 3.00 3.50 4.00 4.50 5.00 5.50			
			Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64			
			2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74			

Discarded OutFlow Max=0.01 cfs @ 14.76 hrs HW=49.22' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=48.50' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 2P: G.U.S.F. #2

Inflow Area :	=	1.228 ac,	1.73% Impervious,	Inflow Depth >	0.11" fo	r 2-yr storm event
Inflow =	=	0.11 cfs @	12.22 hrs, Volume	e= 0.011	af	
Outflow =	=	0.01 cfs @	12.05 hrs, Volume	e= 0.008	af, Atten=	89%, Lag= 0.0 min
Discarded =	=	0.01 cfs @	12.05 hrs, Volume	e= 0.008	af	
Primary =	=	0.00 cfs @	5.00 hrs, Volume	e= 0.000	af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 46.11' @ 15.59 hrs Surf.Area= 500 sf Storage= 223 cf

Plug-Flow detention time= 186.9 min calculated for 0.008 af (69% of inflow) Center-of-Mass det. time= 110.8 min (960.3 - 849.4)

Volume	Inve	rt Ava	il.Stora	ge Storage Descr	iption	
#1	45.00	ם'	1,236	cf Custom Stage	e Data (Prismatio	Listed below
Elevatio		Surf.Area (sq-ft)	Voids (%)		Cum.Store (cubic-feet)	
45.0	00	500	0.0	0	0	
46.5	50	500	40.0	300	300	
47.0	00	697	100.0	299	599	
47.7	'5	1,000	100.0	636	1,236	
Device	Routing	In	vert (Outlet Devices		
#1	Discarded	45	5.00'	1.000 in/hr Exfiltrat	ion over Surface	area
#2	#2 Primary 47.30'		'.30' ⁴	4.0' long x 8.0' breadth Broad-Crested Rectangular Weir		
						00 1.20 1.40 1.60 1.80 2.00
			:	2.50 3.00 3.50 4.0	0 4.50 5.00 5.50	0
						2.68 2.68 2.66 2.64 2.64
				2.64 2.65 2.65 2.6	66 2.66 2.68 2.70	0 2.74

Type III 24-hr 2-yr storm Rainfall=3.69"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 32

Discarded OutFlow Max=0.01 cfs @ 12.05 hrs HW=45.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=45.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 3P: G.U.S.F. #3

Inflow Area = 0.087 ac, 34.23% Impervious, Inflow Depth > 1.14" for 2-yr storm event
Inflow = 0.12 cfs @ 12.10 hrs, Volume= 0.008 af
Outflow = 0.03 cfs @ 12.52 hrs, Volume= 0.005 af, Atten= 71%, Lag= 25.2 min
Discarded = 0.03 cfs @ 12.52 hrs, Volume= 0.004 af
Primary = 0.03 cfs @ 12.52 hrs, Volume= 0.001 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 47.92' @ 12.52 hrs Surf.Area= 271 sf Storage= 152 cf

Plug-Flow detention time= 163.4 min calculated for 0.005 af (66% of inflow) Center-of-Mass det. time= 87.8 min (904.2 - 816.4)

Volume	Inver	t Ava	il.Storag	age Storage Description			
#1	46.00)'	347	cf Custom Stage	e Data (Prismatio	:)Listed below	
Elevatio		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
46.0		110	0.0	0	0		
47.5		110	40.0	66	66		
48.0		302	100.0	103	169		
48.5	50	410	100.0	178	347		
Device	Routing	In	vert C	Outlet Devices			
#1	Discarded	46	3.00' 1	.000 in/hr Exfiltrat	ion over Surface	area	
#2	Primary	47		4.0' long x 8.0' breadth Broad-Crested Rectangular Weir			
			H	lead (feet) 0.20 0	.40 0.60 0.80 1.	00 1.20 1.40 1.60 1.80 2.00	
			2	.50 3.00 3.50 4.0	0 4.50 5.00 5.5	0	
			C	coef. (English) 2.43	3 2.54 2.70 2.69	2.68 2.68 2.66 2.64 2.64	
			2	.64 2.65 2.65 2.6	66 2.66 2.68 2.7	0 2.74	

Discarded OutFlow Max=0.01 cfs @ 12.52 hrs HW=47.92' (Free Discharge)

—1=Exfiltration (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.02 cfs @ 12.52 hrs HW=47.92' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 0.02 cfs @ 0.32 fps)

Summary for Pond 5P: G.U.S.F. #4

Type III 24-hr 2-yr storm Rainfall=3.69"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 33

Inflow Area = 0.998 ac, 0.00% Impervious, Inflow Depth > 0.32" for 2-yr storm event Inflow = 0.16 cfs @ 12.45 hrs, Volume= 0.027 af

Outflow = 0.02 cfs @ 17.50 hrs, Volume= 0.013 af, Atten= 86%, Lag= 302.9 min
Discarded = 0.02 cfs @ 17.50 hrs, Volume= 0.013 af
Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 49.82' @ 17.50 hrs Surf.Area= 958 sf Storage= 636 cf

Plug-Flow detention time= 214.4 min calculated for 0.013 af (48% of inflow) Center-of-Mass det. time= 105.5 min (986.0 - 880.5)

Volume	Inve	ert Ava	il.Storage	e Storage Descr	iption	
#1	48.5	50'	1,540 c	f Custom Stage	e Data (Prismatio	:)Listed below
Elevatio (fee		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
48.5	0	300	0.0	0	0	
49.0	0	300	40.0	60	60	
50.0	0	1,100	100.0	700	760	
50.6	0	1,500	100.0	780	1,540	
Device	Routing	In	vert O	utlet Devices		
#1	Discarde	ed 48	3.50' 1.	000 in/hr Exfiltrat	tion over Surface	area
#2	Primary	50	0.00' 4.0	0' long x 8.0' bre	adth Broad-Cres	ted Rectangular Weir
						00 1.20 1.40 1.60 1.80 2.00
			2.	50 3.00 3.50 4.0	0 4.50 5.00 5.5	0
			Co	pef. (English) 2.43	3 2.54 2.70 2.69	2.68 2.68 2.66 2.64 2.64
			2.	64 2.65 2.65 2.6	66 2.66 2.68 2.7	0 2.74

Discarded OutFlow Max=0.02 cfs @ 17.50 hrs HW=49.82' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=48.50' (Free Discharge) —2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 6P: Roof Drip Edge

Inflow Area =	0.040 ac, 87.36% Impervious, Inflow	Depth > 2.76" for 2-yr storm event
Inflow =	0.13 cfs @ 12.09 hrs, Volume=	0.009 af
Outflow =	0.13 cfs @ 12.10 hrs, Volume=	0.009 af, Atten= 0%, Lag= 0.7 min
Discarded =	0.13 cfs @ 12.10 hrs, Volume=	0.009 af
Secondary =	0.00 cfs @ 5.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.58' @ 12.10 hrs Surf.Area= 220 sf Storage= 51 cf

Plug-Flow detention time= 47.3 min calculated for 0.009 af (98% of inflow) Center-of-Mass det. time= 40.9 min (799.4 - 758.5)

Type III 24-hr 2-yr storm Rainfall=3.69"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 34

Volume	Invert	Avail.S	torage	Storage Descrip	tion	
#1	50.00'		286 cf	Custom Stage	Data (Prismatic	Listed below
Elevation (fee		urf.Area V (sq-ft)	oids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
50.0	00	220	0.0	0	0	
52.0	00		40.0	176	176	
52.5	50	220 1	0.00	110	286	
Device #1	Routing Discarded	Inve	0' 6.0" Limi	et Devices Horiz. Orifice/Greated to weir flow at	low heads	
#2	Discarded	50.00		0 in/hr Exfiltration		
#3	Secondary	52.00	Hea 2.50 Coe	d (feet) 0.20 0.4 3.00 3.50	0 0.60 0.80 1.0 2.61 2.61 2.60	ested Rectangular Weir 00 1.20 1.40 1.60 1.80 2.00 2.66 2.70 2.77 2.89 2.88

Discarded OutFlow Max=0.13 cfs @ 12.10 hrs HW=50.58' (Free Discharge)

1=Orifice/Grate (Weir Controls 0.12 cfs @ 0.94 fps)
2=Exfiltration (Exfiltration Controls 0.01 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=50.00' (Free Discharge)
—3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 7P: Roof Drip Edge

Inflow Area =	0.046 ac, 86.00% Impervious, Inflow	Depth > 2.76" for 2-yr storm event
Inflow =	0.15 cfs @ 12.09 hrs, Volume=	0.011 af
Outflow =	0.14 cfs @ 12.12 hrs, Volume=	0.011 af, Atten= 5%, Lag= 1.8 min
Discarded =	0.14 cfs @ 12.12 hrs, Volume=	0.011 af
Secondary =	0.00 cfs @ 5.00 hrs. Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 48.72' @ 12.12 hrs Surf.Area= 280 sf Storage= 81 cf

Plug-Flow detention time= 51.2 min calculated for 0.011 af (100% of inflow) Center-of-Mass det. time= 49.4 min (807.9 - 758.5)

Volume	Invert	Ava	I.Storage	Storage Descrip	tion	
#1	48.00'		364 cf	Custom Stage I	Data (Prismatic)Listed below	
Elevation (feet)	Surf./	Area sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
48.00		280	0.0	0	0	
50.00		280	40.0	224	224	
50.50		280	100.0	140	364	

Type III 24-hr 2-yr storm Rainfall=3.69"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 35

Device	Routing	Invert	Outlet Devices
#1	Discarded	48.50'	6.0" Vert. Orifice/Grate C= 0.600
#2	Discarded	48.00'	1.000 in/hr Exfiltration over Surface area
#3	Secondary	50.00'	140.0' long x 2.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50
			Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88
			2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.13 cfs @ 12.12 hrs HW=48.72' (Free Discharge)

-1=Orifice/Grate (Orifice Controls 0.13 cfs @ 1.58 fps)

-2=Exfiltration (Exfiltration Controls 0.01 cfs)

Volume

Invert

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=48.00' (Free Discharge)
—3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 9P: Roof Drip Edge

Inflow Area =	0.009 ac, 85.37% Impervious, Inflo	w Depth > 2.76"	for 2-yr storm event
Inflow =	0.03 cfs @ 12.09 hrs, Volume=	0.002 af	The state of the s
Outflow =	0.03 cfs @ 12.10 hrs, Volume=	0.002 af, Atte	en= 1%, Lag= 0.8 min
Discarded =	0.03 cfs @ 12.10 hrs, Volume=	0.002 af	
Secondary =	0.00 cfs @ 5.00 hrs, Volume=	0.000 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.60' @ 12.10 hrs Surf.Area= 60 sf Storage= 14 cf

Plug-Flow detention time= 50.0 min calculated for 0.002 af (100% of inflow) Center-of-Mass det. time= 48.9 min (807.4 - 758.5)

Avail.Storage Storage Description

#1	50.00'		78 cf	Custom Stage	Data (Prismatic)Lis	ted below
Elevation (fee		f.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
50.0 52.0	00	60 60	0.0 40.0	0 48	0 48	
52.		60	100.0	30	78	
Device	Routing	In	vert Out	let Devices		
#1 #2 #3	Discarded Discarded Secondary	50	0.00' 1.00 0.00' 110 Hea 2.50	.0' long x 3.0' br ad (feet) 0.20 0.4 0 3.00 3.50 4.00	on over Surface are eadth Broad-Crest 0 0.60 0.80 1.00 4.50 2.58 2.68 2.67 2.0	ea ed Rectangular Weir 1.20 1.40 1.60 1.80 2.00 65 2.64 2.64 2.68 2.68

Type III 24-hr 2-yr storm Rainfall=3.69"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 36

Discarded OutFlow Max=0.03 cfs @ 12.10 hrs HW=50.60' (Free Discharge)
1=Orifice/Grate (Orifice Controls 0.03 cfs @ 1.06 fps)
2=Exfiltration (Exfiltration Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=50.00' (Free Discharge)
—3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Type III 24-hr 10-yr storm Rainfall=5.60" Printed 11/2/2018

Page 37

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Reach routing by	Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method					
Subcatchment 1.1S: (new Sub	Runoff Area=3,988 sf 25.45% Impervious Runoff Depth>2.30" w Length=130' Slope=0.0750 '/' Tc=6.0 min CN=70 Runoff=0.26 cfs 0.018 af					
Subcatchment 1.2S: (new Sub	Runoff Area=5,446 sf 25.27% Impervious Runoff Depth>2.21" Flow Length=230' Tc=18.1 min CN=69 Runoff=0.24 cfs 0.023 af					
Subcatchment 1.3S: Roof 1	Runoff Area=410 sf 85.37% Impervious Runoff Depth>4.52" Tc=6.0 min CN=93 Runoff=0.05 cfs 0.004 af					
Subcatchment 2.1S: (new Sub	Runoff Area=8,038 sf 36.17% Impervious Runoff Depth>2.56" Flow Length=95' Tc=6.0 min CN=73 Runoff=0.58 cfs 0.039 af					
Subcatchment 2.25: (new Sub	Runoff Area=10,052 sf 9.19% Impervious Runoff Depth>1.58" Flow Length=200' Tc=12.3 min CN=61 Runoff=0.35 cfs 0.030 af					
Subcatchment 2.35: (new Sub	Runoff Area=3,804 sf 34.23% Impervious Runoff Depth>2.47" Flow Length=65' Tc=6.0 min CN=72 Runoff=0.27 cfs 0.018 af					
Subcatchment 2.4S: (new Sub	Runoff Area=19,654 sf 0.74% Impervious Runoff Depth>0.84" Flow Length=130' Tc=7.5 min CN=50 Runoff=0.34 cfs 0.032 af					
Subcatchment 2.5S: (new Sub	Runoff Area=43,461 sf 0.00% Impervious Runoff Depth>1.09" Flow Length=320' Tc=17.7 min CN=54 Runoff=0.84 cfs 0.090 af					
Subcatchment 2.6S: Roof 1	Runoff Area=1,740 sf 87.36% Impervious Runoff Depth>4.52" Tc=6.0 min CN=93 Runoff=0.20 cfs 0.015 af					
Subcatchment 2.7S: Roof 2	Runoff Area=2,000 sf 86.00% Impervious Runoff Depth>4.52" Tc=6.0 min CN=93 Runoff=0.23 cfs 0.017 af					
Reach 1R: (new Reach)	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=100.0' S=0.1020 '/' Capacity=6.31 cfs Outflow=0.00 cfs 0.000 af					
Reach 2R: POA #2	Avg. Flow Depth=0.11' Max Vel=3.69 fps Inflow=0.26 cfs 0.018 af n=0.022 L=1.0' S=0.1000'/' Capacity=6.24 cfs Outflow=0.26 cfs 0.018 af					
Reach 3R: Roadside ditch	Avg. Flow Depth=0.26' Max Vel=2.25 fps Inflow=0.58 cfs 0.039 af n=0.022 L=160.0' S=0.0125 '/' Capacity=2.21 cfs Outflow=0.55 cfs 0.039 af					
Reach 4R: Swale	Avg. Flow Depth=0.18' Max Vel=3.89 fps Inflow=0.53 cfs 0.053 af n=0.022 L=85.0' S=0.0624 '/' Capacity=4.93 cfs Outflow=0.54 cfs 0.053 af					
Reach 5R: POA #1	Avg. Flow Depth=0.23' Max Vel=5.72 fps Inflow=1.18 cfs 0.134 af n=0.022 L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=1.18 cfs 0.134 af					
Reach 8R: (new Reach)	Avg. Flow Depth=0.23' Max Vel=2.89 fps Inflow=0.58 cfs 0.056 af n=0.022 L=80.0' S=0.0250 '/' Capacity=3.12 cfs Outflow=0.58 cfs 0.056 af					

4916	post-REVIESD	102018
------	--------------	--------

Type III 24-hr 10-yr storm Rainfall=5.60"

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 11/2/2018
Page 38

Pond 1P: G.U.S.F.#1	Discarded=0.02 cfs			of Inflow=0.24 cfs Outflow=0.02 cfs	
Pond 2P: G.U.S.F. #2	Discarded=0.02 cfs			of Inflow=0.74 cfs Outflow=0.55 cfs	
Pond 3P: G.U.S.F.#3	Discarded=0.01 cfs			of Inflow=0.27 cfs Outflow=0.30 cfs	
Pond 5P: G.U.S.F.#4	Discarded=0.03 cfs			of Inflow=0.84 cfs Outflow=0.61 cfs	
Pond 6P: Roof Drip Edge D	iscarded=0.20 cfs 0.0			of Inflow=0.20 cfs Outflow=0.20 cfs	
Pond 7P: Roof Drip Edge D	iscarded=0.23 cfs 0.0			of Inflow=0.23 cfs Outflow=0.23 cfs	
Pond 9P: Roof Drip Edge		ak Elev=50.63	Storage=15	of Inflow=0.05 cfs	0.004 af

Total Runoff Area = 2.263 ac Runoff Volume = 0.286 af Average Runoff Depth = 1.52" 88.58% Pervious = 2.005 ac 11.42% Impervious = 0.258 ac

Discarded=0.05 cfs 0.003 af Secondary=0.00 cfs 0.000 af Outflow=0.05 cfs 0.003 af

Type III 24-hr 10-yr storm Rainfall=5.60"

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 11/2/2018

Page 39

Summary for Subcatchment 1.1S: (new Subcat)

Runoff = 0.26 cfs @ 12.10 hrs, Volume=

0.018 af, Depth> 2.30"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

	Α	rea (sf)	CN E	escription						
27		1,015	98 F	aved park						
		2,373	61 >	75% Gras	ood, HSG B					
		600	55 V	Voods, Go	oods, Good, HSG B					
		3,988	70 V	Veighted A	eighted Average					
		2,973	7	74.55% Pervious Area						
		1,015	2	5.45% Imp	pervious Are	ea				
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	0.5	130	0.0750	4.11		Shallow Concentrated Flow,				
_				111.000		Grassed Waterway Kv= 15.0 fps				
	0.5	130	Total I	ncreased t	o minimum	Tc = 6.0 min				

Summary for Subcatchment 1.2S: (new Subcat)

Runoff = 0.24 cfs @ 12.26 hrs, Volume=

0.023 af, Depth> 2.21"

	Α	rea (sf)	CN [Description							
		150	98 F	Roofs, HSG B							
		1,226	98 F	Paved parking, HSG B							
		2,570	61 >	>75% Grass cover, Good, HSG B Woods, Good, HSG B							
		1,500	55 \								
	5,446 69 Weighted Average										
		4,070	7	74.73% Per	vious Area						
		1,376	2	25.27% Imp	pervious Are	ea					
	Tc	Length	Slope		Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	15.2	100	0.0400	0.11		Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 3.69"					
	2.7	100	0.0150	0.61		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
	0.2	30	0.0200	2.12		Shallow Concentrated Flow,					
_						Grassed Waterway Kv= 15.0 fps					
	18.1	230	Total								

Type III 24-hr 10-yr storm Rainfall=5.60"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 40

Summary for Subcatchment 1.3S: Roof 1

Runoff

=

0.05 cfs @ 12.09 hrs, Volume=

0.004 af, Depth> 4.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

A	rea (sf)	CN	Description	escription						
	350	98	Roofs, HSG	oofs, HSG B 75% Grass cover, Good, HSG B						
	60	61	>75% Gras							
	410	93	Weighted A	Veighted Average						
	60		14.63% Per	14.63% Pervious Area						
	350		85.37% Imp	35.37% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)						
6.0					Direct Entry,					

Summary for Subcatchment 2.1S: (new Subcat)

Runoff

0.58 cfs @ 12.09 hrs, Volume=

0.039 af, Depth> 2.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

	Α	rea (sf)	CN	Description						
		2,907	98	Paved park						
		3,899	61	>75% Gras	od, HSG B					
_		1,232	55	Woods, Go						
		8,038	73	Weighted Average						
		5,131		63.83% Per	vious Area					
		2,907		36.17% Impervious Area						
	Тс	Length	Slope		Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	5.3	60	0.2000	0.19		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 3.69"				
	0.2	35	0.0600	3.67		Shallow Concentrated Flow,				
						Grassed Waterway Kv= 15.0 fps				
	5.5	95	Total,	Increased t	o minimum	Tc = 6.0 min				

Summary for Subcatchment 2.2S: (new Subcat)

Runoff = 0.35 cfs @ 12.19 hrs, Volume=

0.030 af, Depth> 1.58"

Type III 24-hr 10-yr storm Rainfall=5.60"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 41

		, .									
	A	rea (sf)	CN [Description							
		150	98 F	Roofs, HSG B							
		774	98 F	Paved parking, HSG B							
		6,280	61 >								
		2,848	48 E	Brush, Goo	d, HSG B						
		10,052	61 Weighted Average								
		9,128			vious Area						
		924	ç	1.19% Impe	rvious Area	a					
				•							
	Tc	Length	Slope	Velocity	Capacity	Description					
-	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	3 1000 B 20 1					
	11.5	100	0.0800	0.15		Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 3.69"					
	8.0	100	0.0200	2.12		Shallow Concentrated Flow,					
						Grassed Waterway Kv= 15.0 fps					
	12.3	200	Total								

Summary for Subcatchment 2.3S: (new Subcat)

Runoff = 0.27 cfs @ 12.09 hrs, Volume= 0.018 af, Depth> 2.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

	A	rea (sf)	CN	N Description						
		150	98	Roofs, HSG						
		1,152	98	Paved park	}					
		1,622	61	>75% Gras	ood, HSG B					
_		880	55	5 Woods, Good, HSG B						
		3,804	72	Weighted Average						
		2,502		65.77% Pervious Area						
		1,302		34.23% Imp	ervious Are	ea				
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	4.3	40	0.1500	0.16		Sheet Flow,				
				Woods: Light underbrush n= 0.400 P2= 3.69"						
	0.1	25	0.1800	6.36		Shallow Concentrated Flow,				
_						Grassed Waterway Kv= 15.0 fps				
	4 4	65	Total	Increased t	o minimum	Tc = 6.0 min				

4.4 65 Total, Increased to minimum Tc = 6.0 min

Summary for Subcatchment 2.4S: (new Subcat)

Runoff = 0.34 cfs @ 12.14 hrs, Volume= 0.032 af, Depth> 0.84"

Type III 24-hr 10-yr storm Rainfall=5.60"

Prepared by Altus Engineering, Inc.
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 11/2/2018 Page 42

	A	rea (sf)	CN [Description							
		145	98 F	Paved parking, HSG B							
		1,100		>75% Grass cover, Good, HSG B Brush, Good, HSG B							
		17,252									
		1,157			od, HSG B						
	19,654 50 Weighted Average										
		19,509									
		145	Č	0.74% Impe	ervious Area	a					
	Tc	Length	Slope	Velocity	Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
-	6.4	50	0.0875	0.13		Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 3.69"					
	1.1	80	0.0625	1.25		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
-	7.5	130	Total								

Summary for Subcatchment 2.5S: (new Subcat)

Runoff = 0.84 cfs @ 12.29 hrs, Volume=

0.090 af, Depth> 1.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

Α	rea (sf)	CN I	Description							
	2,480 5,450		>75% Grass cover, Good, HSG B Brush, Good, HSG B Woods, Good, HSG B							
	35,531									
43,461 54 Weighted Average 43,461 100.00% Pervious Area										
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description					
15.2	100	0.0400	0.11		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.69"					
1.2	50	0.0200	0.71		Shallow Concentrated Flow, Woodland Kv= 5.0 fps					
1.3 170 0.0200 2.12					Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps					
17.7	320	Total								

Summary for Subcatchment 2.6S: Roof 1

Runoff = 0.20 cfs @ 12.09 hrs, Volume=

0.015 af, Depth> 4.52"

Type III 24-hr 10-yr storm Rainfall=5.60"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 43

	Α	rea (sf)	CN	Description	escription						
-		1,520	98	Roofs, HSG	oofs, HSG B						
_		220	61	>75% Grass cover, Good, HSG B							
		1,740	93	Weighted Average							
		220		12.64% Pervious Area							
		1,520		87.36% Imp	37.36% Impervious Area						
	Тс	Length	Slope		Capacity	Description					
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)						
	6.0					Direct Entry					

Summary for Subcatchment 2.7S: Roof 2

Runoff

0.23 cfs @ 12.09 hrs, Volume=

0.017 af, Depth> 4.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

A	rea (sf)	CN	Description							
	1,720	98	Roofs, HSG	toofs, HSG B						
	280	61	>75% Gras	75% Grass cover, Good, HSG B						
.30	2,000	93	Weighted Average							
	280		14.00% Pervious Area							
	1,720		86.00% Impervious Area							
_										
Tc	-	Slope		Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Reach 1R: (new Reach)

Inflow Area = 0.125 ac, 25.27% Impervious, Inflow Depth = 0.00" for 10-yr storm event

Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min

Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.31 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight

Length= 100.0' Slope= 0.1020 '/'

Inlet Invert= 50.20', Outlet Invert= 40.00'

Type III 24-hr 10-yr storm Rainfall=5.60"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 44



Summary for Reach 2R: POA #2

Inflow Area = 0.217 ac, 25.34% Impervious, Inflow Depth > 0.97" for 10-yr storm event

0.26 cfs @ 12.10 hrs, Volume= Inflow 0.018 af

Outflow 0.26 cfs @ 12.10 hrs, Volume= 0.018 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.69 fps, Min. Travel Time= 0.0 min Avg. Velocity = 1.46 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.10 hrs

Average Depth at Peak Storage= 0.11'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight

Length= 1.0' Slope= 0.1000 '/'

Inlet Invert= 40.00', Outlet Invert= 39.90'



Summary for Reach 3R: Roadside ditch

Inflow Area = 0.185 ac, 36.17% Impervious, Inflow Depth > 2.56" for 10-yr storm event

0.58 cfs @ 12.09 hrs, Volume= 0.55 cfs @ 12.13 hrs, Volume= Inflow 0.039 af

Outflow 0.039 af, Atten= 6%, Lag= 2.2 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.25 fps, Min. Travel Time= 1.2 min Avg. Velocity = 0.88 fps, Avg. Travel Time= 3.0 min

Peak Storage= 41 cf @ 12.11 hrs

Average Depth at Peak Storage= 0.26'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 2.21 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight

Length= 160.0' Slope= 0.0125 '/'

Inlet Invert= 42.00', Outlet Invert= 40.00'

Type III 24-hr 10-yr storm Rainfall=5.60" Printed 11/2/2018

Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 45



Summary for Reach 4R: Swale

Inflow Area = 1.228 ac, 1.73% Impervious, Inflow Depth > 0.52" for 10-yr storm event

Inflow = 0.53 cfs @ 12.72 hrs, Volume= 0.053 af

Outflow = 0.54 cfs @ 12.72 hrs, Volume= 0.053 af, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.89 fps, Min. Travel Time= 0.4 min Avg. Velocity = 1.99 fps, Avg. Travel Time= 0.7 min

Peak Storage= 12 cf @ 12.71 hrs Average Depth at Peak Storage= 0.18'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 4.93 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight

Length= 85.0' Slope= 0.0624 '/'

Inlet Invert= 47.30', Outlet Invert= 42.00'



Summary for Reach 5R: POA #1

Inflow Area = 1.952 ac, 6.21% Impervious, Inflow Depth > 0.83" for 10-yr storm event

Inflow = 1.18 cfs @ 12.12 hrs, Volume= 0.134 af

Outflow = 1.18 cfs @ 12.12 hrs, Volume= 0.134 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity = 5.72 fps, Min. Travel Time = 0.0 min Avg. Velocity = 2.61 fps, Avg. Travel Time = 0.0 min

Peak Storage= 0 cf @ 12.12 hrs

Average Depth at Peak Storage= 0.23'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight

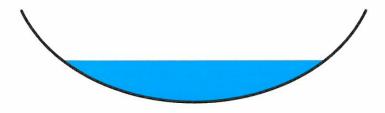
Length= 1.0' Slope= 0.1000 '/'

Inlet Invert= 40.00', Outlet Invert= 39.90'

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 46



Summary for Reach 8R: (new Reach)

0.998 ac, 0.00% Impervious, Inflow Depth > 0.67" for 10-yr storm event Inflow Area =

0.58 cfs @ 12.54 hrs, Volume= Inflow 0.056 af

0.58 cfs @ 12.56 hrs, Volume= 0.056 af, Atten= 0%, Lag= 1.0 min Outflow

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.89 fps, Min. Travel Time= 0.5 min Avg. Velocity = 1.48 fps, Avg. Travel Time= 0.9 min

Peak Storage= 16 cf @ 12.55 hrs Average Depth at Peak Storage= 0.23'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 3.12 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight

Length= 80.0' Slope= 0.0250 '/'

Inlet Invert= 50.00', Outlet Invert= 48.00'



Summary for Pond 1P: G.U.S.F. #1

Inflow Area = 0.125 ac, 25.27% Impervious, Inflow Depth > 2.21" for 10-yr storm event

0.24 cfs @ 12.26 hrs, Volume= 0.02 cfs @ 14.99 hrs, Volume= Inflow 0.023 af

Outflow 0.013 af, Atten= 92%, Lag= 163.5 min

Discarded = 0.02 cfs @ 14.99 hrs, Volume= 0.013 af 0.000 af Primary = 0.00 cfs @ 5.00 hrs, Volume=

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 49.71' @ 14.99 hrs Surf.Area= 882 sf Storage= 566 cf

Plug-Flow detention time= 209.7 min calculated for 0.013 af (57% of inflow) Center-of-Mass det. time= 130.1 min (944.2 - 814.1)

Volume	Invert	Avail.Storage	Storage Description
#1	48.50'	1,552 cf	Custom Stage Data (Prismatic)Listed below

Type III 24-hr 10-yr storm Rainfall=5.60"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 47

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
48.50	300	0.0	0	0
49.00	300	40.0	60	60
50.00	1,115	100.0	708	768
50.60	1,500	100.0	785	1,552

Device	Routing	Invert	Outlet Devices
#1	Discarded	48.50'	1.000 in/hr Exfiltration over Surface area
#2	Primary	50.20'	4.0' long x 8.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64
			2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74

Discarded OutFlow Max=0.02 cfs @ 14.99 hrs HW=49.71' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=48.50' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 2P: G.U.S.F. #2

Inflow Area =	1.228 ac,	1.73% Impervious, Inflow D	Depth > 0.84"	for 10-yr storm event
Inflow =	0.74 cfs @	12.54 hrs, Volume=	0.086 af	•
Outflow =	0.55 cfs @	12.72 hrs, Volume=	0.066 af, Atte	en= 25%, Lag= 10.7 min
Discarded =	0.02 cfs @	12.72 hrs, Volume=	0.013 af	
Primary =	0.53 cfs @	12.72 hrs, Volume=	0.053 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 47.44' @ 12.72 hrs Surf.Area= 876 sf Storage= 976 cf

Plug-Flow detention time= 87.8 min calculated for 0.066 af (77% of inflow) Center-of-Mass det. time= 34.7 min (877.0 - 842.3)

Volume	Invert	Avai	I.Storage	Storage Descri	ption	
#1	45.00	ľ	1,236 cf	Custom Stage	Data (Prismatic	:)Listed below
Elevatio		urf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
45.0	00	500	0.0	0	0	
46.5	50	500	40.0	300	300	
47.0	00	697	100.0	299	599	
47.7	' 5	1,000	100.0	636	1,236	
Device	Routing	In	vert Out	tlet Devices		
#1	Discarded	45	.00' 1.0	00 in/hr Exfiltrat	ion over Surface	area
#2	Primary	47	.30' 4.0	long x 8.0' bre	adth Broad-Cres	ted Rectangular Weir
	•					00 1.20 1.40 1.60 1.80 2.00
					0 4.50 5.00 5.50	
			Co	ef. (English) 2.43	3 2.54 2.70 2.69	2.68 2.68 2.66 2.64 2.64
					6 2.66 2.68 2.70	

Volume

Prepared by Altus Engineering, Inc.

Invert

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 11/2/2018 Page 48

Discarded OutFlow Max=0.02 cfs @ 12.72 hrs HW=47.44' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.52 cfs @ 12.72 hrs HW=47.44' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 0.52 cfs @ 0.91 fps)

Summary for Pond 3P: G.U.S.F. #3

Inflow Area = 0.087 ac, 34.23% Impervious, Inflow Depth > 2.47" for 10-yr storm event 0.27 cfs @ 12.09 hrs, Volume= 0.018 af 0.30 cfs @ 12.11 hrs, Volume= 0.015 af, Atten= 0%, Lag= 1.1 min 0.30 cfs @ 12.12 hrs, Volume= 0.005 af 0.30 cfs @ 12.11 hrs, Volume= 0.010 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 48.00' @ 12.12 hrs Surf.Area= 302 sf Storage= 169 cf

Plug-Flow detention time= 76.4 min calculated for 0.015 af (81% of inflow) Center-of-Mass det. time= 25.4 min (824.5 - 799.1)

Avail.Storage Storage Description

					11-11	
#1	46.00)'	347 cf	Custom Stage	Data (Prismatic)Lis	sted below
Elevation (fee	5 (S) (S)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
46.0	00	110	0.0	0	0	
47.5 48.0		110 302	40.0 100.0	66 103	66 169	
48.5	50	410	100.0	178	347	
Device	Routing	In	vert Ou	tlet Devices		
#1	Discarded	1 46	6.00' 1.0	00 in/hr Exfiltration	on over Surface ar	ea
#2	Primary	47				l Rectangular Weir
						1.20 1.40 1.60 1.80 2.00
				0 3.00 3.50 4.00		
						.68 2.68 2.66 2.64 2.64
			2.6	4 2.65 2.65 2.66	2.66 2.68 2.70 2	2.74

Discarded OutFlow Max=0.01 cfs @ 12.12 hrs HW=47.99' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.27 cfs @ 12.11 hrs HW=47.99' (Free Discharge)
—2=Broad-Crested Rectangular Weir (Weir Controls 0.27 cfs @ 0.73 fps)

Summary for Pond 5P: G.U.S.F. #4

Type III 24-hr 10-yr storm Rainfall=5.60"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 49

0.00% Impervious, Inflow Depth > 1.09" for 10-yr storm event Inflow Area = 0.998 ac, Inflow 0.84 cfs @ 12.29 hrs, Volume= 0.090 af

Outflow 0.61 cfs @ 12.54 hrs, Volume= 0.073 af, Atten= 28%, Lag= 14.7 min

0.03 cfs @ 12.54 hrs, Volume= Discarded = 0.017 af 0.58 cfs @ 12.54 hrs, Volume= 0.056 af Primary

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.15' @ 12.54 hrs Surf.Area= 1,202 sf Storage= 958 cf

Plug-Flow detention time= 85.4 min calculated for 0.072 af (80% of inflow) Center-of-Mass det. time= 32.4 min (876.9 - 844.5)

Volume	Invert	Ava	il.Storage	Storage Descri	ption	
#1	48.50'		1,540 c	f Custom Stage	Data (Prismatic)Listed	below
Elevation (feet)	Sur	f.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
48.50		300	0.0	0	0	
49.00		300	40.0	60	60	
50.00		1,100	100.0	700	760	
50.60		1,500	100.0	780	1,540	
Device R	outing	In	vert Ou	utlet Devices		,
	iscarded				ion over Surface area	

Primary 50.00' 4.0' long x 8.0' breadth Broad-Crested Rectangular Weir #2 Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74

Discarded OutFlow Max=0.03 cfs @ 12.54 hrs HW=50.15' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=0.57 cfs @ 12.54 hrs HW=50.15' (Free Discharge) —2=Broad-Crested Rectangular Weir (Weir Controls 0.57 cfs @ 0.95 fps)

Summary for Pond 6P: Roof Drip Edge

Inflow Area =	0.040 ac, 87.36% Impervious, Inflov	w Depth > 4.52" for 10-yr storm even	ıt
Inflow =	0.20 cfs @ 12.09 hrs, Volume=	0.015 af	
Outflow =	0.20 cfs @ 12.10 hrs, Volume=	0.014 af, Atten= 0%, Lag= 0.6 mir	١
Discarded =	0.20 cfs @ 12.10 hrs, Volume=	0.014 af	
Secondary =	0.00 cfs @ 5.00 hrs, Volume=	0.000 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.61' @ 12.10 hrs Surf.Area= 220 sf Storage= 54 cf

Plug-Flow detention time= 35.9 min calculated for 0.014 af (96% of inflow) Center-of-Mass det. time= 19.7 min (768.9 - 749.2)

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 50

## F0.00! 000 f 0 1 01 D 1 /D 1 11 11 11 1	
#1 50.00' 286 cf Custom Stage Data (Prismatic)Listed below	1
Elevation Surf.Area Voids Inc.Store Cum.Store (feet) (sq-ft) (%) (cubic-feet) (cubic-feet)	
50.00 220 0.0 0 0	
52.00 220 40.0 176 176	
52.50 220 100.0 110 286	
Device Routing Invert Outlet Devices	1.60 1.80 2.00

Discarded OutFlow Max=0.20 cfs @ 12.10 hrs HW=50.61' (Free Discharge)

-1=Orifice/Grate (Weir Controls 0.20 cfs @ 1.10 fps)

-2=Exfiltration (Exfiltration Controls 0.01 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=50.00' (Free Discharge)
—3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 7P: Roof Drip Edge

Inflow Area =	0.046 ac, 86.00% Impervious, Inflow D	epth > 4.52" for 10-yr storm event
Inflow =	0.23 cfs @ 12.09 hrs, Volume=	0.017 af
Outflow =	0.23 cfs @ 12.11 hrs, Volume=	0.017 af, Atten= 4%, Lag= 1.4 min
Discarded =	0.23 cfs @ 12.11 hrs, Volume=	0.017 af
Secondary =	0.00 cfs @ 5.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 48.79' @ 12.11 hrs Surf.Area= 280 sf Storage= 89 cf

Plug-Flow detention time= 40.8 min calculated for 0.017 af (96% of inflow) Center-of-Mass det. time= 25.5 min (774.7 - 749.2)

Volume	Invert	Ava	il.Storage	Storage Descrip	tion	
#1	48.00'		364 cf	Custom Stage I	Data (Prismatic)Listed below	
Elevation (feet)	Surf./	Area sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
48.00		280	0.0	0	0	
50.00		280	40.0	224	224	
50.50		280	100.0	140	364	

Type III 24-hr 10-yr storm Rainfall=5.60"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 51

Device	Routing	Invert	Outlet Devices
#1	Discarded	48.50'	6.0" Vert. Orifice/Grate C= 0.600
#2	Discarded	48.00'	1.000 in/hr Exfiltration over Surface area
#3	Secondary	50.00'	140.0' long x 2.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50
			Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88
			2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.22 cfs @ 12.11 hrs HW=48.79' (Free Discharge)

-1=Orifice/Grate (Orifice Controls 0.21 cfs @ 1.83 fps)

-2=Exfiltration (Exfiltration Controls 0.01 cfs)

\/aluma

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=48.00' (Free Discharge)
—3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 9P: Roof Drip Edge

Inflow Area =	0.009 ac, 85.37% lm	pervious, Inflow D	epth > 4.52"	for 10-yr storm event
Inflow =	0.05 cfs @ 12.09 hrs	s, Volume=	0.004 af	
Outflow =	0.05 cfs @ 12.10 hrs	s, Volume=	0.003 af, Att	en= 0%, Lag= 0.7 min
Discarded =	0.05 cfs @ 12.10 hrs	s, Volume=	0.003 af	_
Secondary =	0.00 cfs @ 5.00 hrs	s, Volume=	0.000 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.63' @ 12.10 hrs Surf.Area= 60 sf Storage= 15 cf

Plug-Flow detention time= 40.2 min calculated for 0.003 af (96% of inflow) Center-of-Mass det. time= 25.6 min (774.8 - 749.2)

Augil Ctorosa Ctorosa Description

volume	Invert	Ava	II.Storage	Storage Descrip	tion	
#1	50.00'		78 cf	Custom Stage	Data (Prismatic)L	isted below
Elevatio		urf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
50.0	00	60	0.0	0	0	
52.0	00	60	40.0	48	48	
52.5	50	60	100.0	30	78	
Device #1 #2 #3	Routing Discarded Discarded Secondary	50 50	0.50' 6.0" 0.00' 1.00 0.00' 110	0' long x 3.0' bro	on over Surface a	sted Rectangular Weir
			2.50 Coe	3.00 3.50 4.00	4.50 2.58 2.68 2.67	2.65 2.64 2.64 2.68 2.68

Type III 24-hr 10-yr storm Rainfall=5.60"

Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC Printed 11/2/2018

Page 52

Discarded OutFlow Max=0.05 cfs @ 12.10 hrs HW=50.63' (Free Discharge)

1=Orifice/Grate (Orifice Controls 0.05 cfs @ 1.21 fps)
2=Exfiltration (Exfiltration Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=50.00' (Free Discharge)
—3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Type III 24-hr 25-yr storm Rainfall=7.10" Printed 11/2/2018

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 53

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1.1S: (new Subcat) Flow Length	Runoff Area=3,988 sf 25.45% Impervious Runoff Depth>3.44" =130' Slope=0.0750'/' Tc=6.0 min CN=70 Runoff=0.39 cfs 0.026 af
Subcatchment 1.2S: (new Subcat)	Runoff Area=5,446 sf 25.27% Impervious Runoff Depth>3.33" Flow Length=230' Tc=18.1 min CN=69 Runoff=0.37 cfs 0.035 af
Subcatchment 1.3S: Roof 1	Runoff Area=410 sf 85.37% Impervious Runoff Depth>5.90" Tc=6.0 min CN=93 Runoff=0.06 cfs 0.005 af
Subcatchment 2.1S: (new Subcat)	Runoff Area=8,038 sf 36.17% Impervious Runoff Depth>3.75" Flow Length=95' Tc=6.0 min CN=73 Runoff=0.85 cfs 0.058 af
Subcatchment 2.2S: (new Subcat)	Runoff Area=10,052 sf 9.19% Impervious Runoff Depth>2.54" Flow Length=200' Tc=12.3 min CN=61 Runoff=0.59 cfs 0.049 af
Subcatchment 2.3S: (new Subcat)	Runoff Area=3,804 sf 34.23% Impervious Runoff Depth>3.65" Flow Length=65' Tc=6.0 min CN=72 Runoff=0.39 cfs 0.027 af
Subcatchment 2.4S: (new Subcat)	Runoff Area=19,654 sf 0.74% Impervious Runoff Depth>1.55" Flow Length=130' Tc=7.5 min CN=50 Runoff=0.74 cfs 0.058 af
Subcatchment 2.5S: (new Subcat)	Runoff Area=43,461 sf 0.00% Impervious Runoff Depth>1.89" Flow Length=320' Tc=17.7 min CN=54 Runoff=1.59 cfs 0.157 af
Subcatchment 2.6S: Roof 1	Runoff Area=1,740 sf 87.36% Impervious Runoff Depth>5.90" Tc=6.0 min CN=93 Runoff=0.26 cfs 0.020 af
Subcatchment 2.7S: Roof 2	Runoff Area=2,000 sf 86.00% Impervious Runoff Depth>5.90" Tc=6.0 min CN=93 Runoff=0.30 cfs 0.023 af
Reach 1R: (new Reach) n=0.022	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af L=100.0' S=0.1020 '/' Capacity=6.31 cfs Outflow=0.00 cfs 0.000 af
Reach 2R: POA #2 n=0.0	Avg. Flow Depth=0.13' Max Vel=4.17 fps Inflow=0.39 cfs 0.026 af 122 L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=0.39 cfs 0.026 af
Reach 3R: Roadside ditch	Avg. Flow Depth=0.32' Max Vel=2.52 fps Inflow=0.85 cfs 0.058 af L=160.0' S=0.0125 '/' Capacity=2.21 cfs Outflow=0.81 cfs 0.058 af
Reach 4R: Swale	Avg. Flow Depth=0.31' Max Vel=5.57 fps Inflow=1.78 cfs 0.136 af L=85.0' S=0.0624 '/' Capacity=4.93 cfs Outflow=1.82 cfs 0.136 af
Reach 5R: POA #1	Avg. Flow Depth=0.33' Max Vel=7.30 fps Inflow=2.62 cfs 0.270 af 0.22 L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=2.62 cfs 0.270 af
Reach 8R: (new Reach)	Avg. Flow Depth=0.35' Max Vel=3.75 fps Inflow=1.45 cfs 0.121 af L=80.0' S=0.0250 '/' Capacity=3.12 cfs Outflow=1.44 cfs 0.121 af

4916	post	-REVIE	SD 1	02018
------	------	--------	------	-------

Type III 24-hr 25-yr storm Rainfall=7.10"

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 11/2/2018

Page 54

Pond 1P: G.U.S.F. #1	Pea	k Elev=50.10'	Storage=901 cf	Inflow=0.37 cfs	0.035 af
	Di			2 22 21 2	0010 6

Discarded=0.03 cfs 0.018 af Primary=0.00 cfs 0.000 af Outflow=0.03 cfs 0.018 af

Pond 2P: G.U.S.F. #2 Peak Elev=47.62' Storage=1,123 cf Inflow=1.83 cfs 0.170 af

Discarded=0.02 cfs 0.014 af Primary=1.78 cfs 0.136 af Outflow=1.80 cfs 0.150 af

Pond 3P: G.U.S.F.#3

Peak Elev=48.01' Storage=174 cf Inflow=0.39 cfs 0.027 af

Discarded=0.01 cfs 0.005 af Primary=0.37 cfs 0.018 af Outflow=0.38 cfs 0.023 af

Pond 5P: G.U.S.F.#4 Peak Elev=50.28' Storage=1,122 cf Inflow=1.59 cfs 0.157 af Discarded=0.03 cfs 0.018 af Primary=1.45 cfs 0.121 af Outflow=1.48 cfs 0.139 af

Pond 6P: Roof Drip Edge Peak Elev=50.64' Storage=56 cf Inflow=0.26 cfs 0.020 af Discarded=0.26 cfs 0.019 af Secondary=0.00 cfs 0.000 af Outflow=0.26 cfs 0.019 af

Pond 7P: Roof Drip Edge Peak Elev=48.84' Storage=94 cf Inflow=0.30 cfs 0.023 af

Discarded=0.29 cfs 0.022 af Secondary=0.00 cfs 0.000 af Outflow=0.29 cfs 0.022 af

Pond 9P: Roof Drip Edge Peak Elev=50.64' Storage=15 cf Inflow=0.06 cfs 0.005 af

Discarded=0.06 cfs 0.004 af Secondary=0.00 cfs 0.000 af Outflow=0.06 cfs 0.004 af

Total Runoff Area = 2.263 ac Runoff Volume = 0.456 af Average Runoff Depth = 2.42" 88.58% Pervious = 2.005 ac 11.42% Impervious = 0.258 ac

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 55

Summary for Subcatchment 1.1S: (new Subcat)

Runoff = 0.39 cfs @ 12.09 hrs, Volume= 0.026 af, Depth> 3.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

_	Α	rea (sf)	CN [Description					
20.000		1,015	98 F	Paved parki	ing, HSG B				
		2,373	61 >	75% Grass	s cover, Go	ood, HSG B			
_		600	55 V	Voods, Go	od, HSG B				
		3,988	70 V	Veighted A	/eighted Average				
		2,973	7	4.55% Per	vious Area				
		1,015	2	25.45% Imp	ervious Are	ea			
	Tc	Length	Slope		Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	0.5	130	0.0750	4.11 Shallow Concentrated Flow,					
_						Grassed Waterway Kv= 15.0 fps			
	0.5	130	Total,	Increased to minimum Tc = 6.0 min					

Summary for Subcatchment 1.2S: (new Subcat)

Runoff = 0.37 cfs @ 12.26 hrs, Volume= 0.035 af, Depth> 3.33"

	Α	rea (sf)	CN I	Description				
		150	98	Roofs, HSG B				
		1,226		Paved park				
		2,570				ood, HSG B		
		1,500	55	Woods, Go	od, HSG B			
		5,446		Weighted A	_			
		4,070		74.73% Per				
		1,376		25.27% Imp	pervious Are	ea		
	-	1	01	\	0	December		
	Tc	Length	Slope		Capacity	Description		
,	(min)	(feet)	(ft/ft)		(cfs)			
	15.2	100	0.0400	0.11		Sheet Flow,		
		400	0.0450	0.04		Woods: Light underbrush n= 0.400 P2= 3.69"		
	2.7	100	0.0150	0.61		Shallow Concentrated Flow,		
	0.0	20	0.0000	0.40		Woodland Kv= 5.0 fps		
	0.2	30	0.0200	2.12		Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps		
						Grasseu Waterway NV- 15.0 Ips		
	18 1	230	Total					

Type III 24-hr 25-yr storm Rainfall=7.10"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 56

Summary for Subcatchment 1.3S: Roof 1

Runoff = 0.06 cfs @ 12.09 hrs, Volume=

0.005 af, Depth> 5.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

	Α	rea (sf)	CN	Description	Description					
		350	98	Roofs, HSG	B					
		60	61	>75% Gras	75% Grass cover, Good, HSG B					
		410	93	Weighted A	/eighted Average					
		60		14.63% Pervious Area						
		350		85.37% Imp	ervious Ar	ea				
	Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description				
_	6.0					Direct Entry,				

Summary for Subcatchment 2.1S: (new Subcat)

Runoff = 0.85 cfs @ 12.09 hrs, Volume=

0.058 af, Depth> 3.75"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

A	rea (sf)	CN I	Description			
	2,907	98	Paved park	ing, HSG B		
	3,899	61	>75% Grass	s cover, Go	ood, HSG B	
	1,232	55 \	Noods, Go	od, HSG B		
	8,038	73 \	Veighted Average			
	5,131	(3.83% Per	vious Area		
	2,907	;	36.17% Imp	ervious Ar	ea	
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
5.3	60	0.2000	0.19		Sheet Flow,	
					Woods: Light underbrush n= 0.400 P2= 3.69"	
0.2	35	0.0600	3.67		Shallow Concentrated Flow,	
					Grassed Waterway Kv= 15.0 fps	
5.5	95	Total,	Increased t	o minimum	Tc = 6.0 min	

Summary for Subcatchment 2.2S: (new Subcat)

Runoff = 0.59 cfs @ 12.18 hrs, Volume= 0.049 af, Depth> 2.54"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 57

	Α	rea (sf)	CN E	escription		
		150	98 F	Roofs, HSG	ВВ	
		774	98 F	Paved park	ing, HSG B	
		6,280	61 >	75% Grass	s cover, Go	ood, HSG B
		2,848	48 E	Brush, Goo	d, HSG B	
		10,052	61 V	Veighted A	verage	
		9,128	9	0.81% Per	vious Area	
		924	ę	.19% Impe	ervious Area	a
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	11.5	100	0.0800	0.15		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.69"
	0.8	100	0.0200	2.12		Shallow Concentrated Flow,
						Grassed Waterway Kv= 15.0 fps
	12.3	200	Total	•		

Summary for Subcatchment 2.3S: (new Subcat)

Runoff = 0.39 cfs @ 12.09 hrs, Volume=

0.027 af, Depth> 3.65"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

	A	rea (sf)	CN I	Description					
		150	98	Roofs, HSG	Roofs, HSG B				
		1,152	98	Paved park	ing, HSG B				
		1,622	61 :	>75% Gras	s cover, Go	ood, HSG B			
		880	55 \	Noods, Go	od, HSG B				
		3,804	72	Neighted A	verage				
		2,502	(55.77% Per	vious Area				
		1,302	;	34.23% Imp	pervious Are	ea			
	Tc	Length	Slope		Capacity	Description			
-	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	4.3	40	0.1500	0.16		Sheet Flow,			
						Woods: Light underbrush n= 0.400 P2= 3.69"			
	0.1	25	0.1800	6.36		Shallow Concentrated Flow,			
						Grassed Waterway Kv= 15.0 fps			

4.4 65 Total, Increased to minimum Tc = 6.0 min

Summary for Subcatchment 2.4S: (new Subcat)

Runoff = 0.74 cfs @ 12.12 hrs, Volume=

0.058 af, Depth> 1.55"

Type III 24-hr 25-yr storm Rainfall=7.10"

Prepared by Altus Engineering, Inc.
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 11/2/2018 Page 58

A	rea (sf)	CN E	escription		
	145	98 F	Paved park	ng, HSG B	3
	1,100				ood, HSG B
	17,252	48 E	Brush, Goo	d, HSG B	
	1,157	55 V	Voods, Go	od, HSG B	
3)	19,654	50 V	Veighted A	verage	
	19,509	9	9.26% Per	vious Area	f and the second se
	145	C	.74% Impe	rvious Are	a
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.4	50	0.0875	0.13		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
1.1	80	0.0625	1.25		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
7.5	130	Total			

Summary for Subcatchment 2.5S: (new Subcat)

Runoff = 1.59 cfs @ 12.27 hrs, Volume=

0.157 af, Depth> 1.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

	Α	rea (sf)	CN I	Description						
		2,480				ood, HSG B				
		5,450		Brush, Goo	•					
_		35,531	55	Noods, Go	od, HSG B					
		43,461	54 \	Neighted A	verage					
		43,461		100.00% Pe		a				

Tc Length Slope Velocity Capacity Description						Description				
	(min)	(feet)	(ft/ft)		(cfs)	The state of the s				
-	15.2	100	0.0400	0.11		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 3.69"				
	1.2	50	0.0200	0.71		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	1.3	170	0.0200	2.12		Shallow Concentrated Flow,				
	1.0		5.5200	2.12		Grassed Waterway Kv= 15.0 fps				
-	17.7	320	Total		ALL MANAGEMENT OF THE PARTY OF					

Summary for Subcatchment 2.6S: Roof 1

Runoff = 0.26 cfs @ 12.09 hrs, Volume=

0.020 af, Depth> 5.90"

Type III 24-hr 25-yr storm Rainfall=7.10"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 59

	Α	rea (sf)	CN	Description						
100		1,520	98	Roofs, HSG	B					
		220	61	>75% Gras	75% Grass cover, Good, HSG B					
		1,740	93	Weighted A	/eighted Average					
		220		12.64% Per	vious Area					
		1,520		87.36% Imp	pervious Ar	ea				
	т.	l amada	01		0					
	Tc	Length	Slop		Capacity	Description				
-	(min)	(feet)	(ft/fi) (ft/sec)	(cfs)	to the second se				
	6.0					Direct Entry.				

Summary for Subcatchment 2.7S: Roof 2

Runoff

0.30 cfs @ 12.09 hrs, Volume=

0.023 af, Depth> 5.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

A	rea (sf)	CN	Description							
	1,720	98	Roofs, HSG	Roofs, HSG B						
	280	61	>75% Gras	s cover, Go	ood, HSG B					
	2,000	93	Weighted Average							
	280		14.00% Pervious Area							
	1,720		86.00% Impervious Area							
т.	مالم مرا	Olam	Malaski	0	Describettes					
Tc (min)	Length	Slope		Capacity						
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Reach 1R: (new Reach)

Inflow Area = 0.125 ac, 25.27% Impervious, Inflow Depth = 0.00" for 25-yr storm event

Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.31 cfs

 $2.00' \times 0.50'$ deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 100.0' Slope= 0.1020 '/' Inlet Invert= 50.20', Outlet Invert= 40.00'

Type III 24-hr 25-yr storm Rainfall=7.10"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 60



Summary for Reach 2R: POA #2

Inflow Area = 0.217 ac, 25.34% Impervious, Inflow Depth > 1.46" for 25-yr storm event

Inflow 0.39 cfs @ 12.09 hrs, Volume= 0.026 af

0.39 cfs @ 12.09 hrs, Volume= Outflow 0.026 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.17 fps, Min. Travel Time= 0.0 min Avg. Velocity = 1.59 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.09 hrs Average Depth at Peak Storage= 0.13' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/'

Inlet Invert= 40.00', Outlet Invert= 39.90'



Summary for Reach 3R: Roadside ditch

Inflow Area = 0.185 ac, 36.17% Impervious, Inflow Depth > 3.75" for 25-yr storm event

0.85 cfs @ 12.09 hrs, Volume= 0.81 cfs @ 12.12 hrs, Volume= Inflow 0.058 af

Outflow 0.058 af, Atten= 5%, Lag= 1.8 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.52 fps, Min. Travel Time= 1.1 min Avg. Velocity = 0.95 fps, Avg. Travel Time= 2.8 min

Peak Storage= 54 cf @ 12.11 hrs Average Depth at Peak Storage= 0.32' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 2.21 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 160.0' Slope= 0.0125 '/' Inlet Invert= 42.00', Outlet Invert= 40.00'

Type III 24-hr 25-yr storm Rainfall=7.10"

Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC Printed 11/2/2018

Page 61



Summary for Reach 4R: Swale

Inflow Area = 1.228 ac, 1.73% Impervious, Inflow Depth > 1.33" for 25-yr storm event

1.78 cfs @ 12.42 hrs, Volume= Inflow 0.136 af

1.82 cfs @ 12.42 hrs, Volume= Outflow 0.136 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

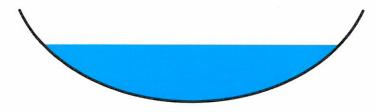
Max. Velocity= 5.57 fps, Min. Travel Time= 0.3 min Avg. Velocity = 2.59 fps, Avg. Travel Time= 0.5 min

Peak Storage= 28 cf @ 12.41 hrs Average Depth at Peak Storage= 0.31'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 4.93 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 85.0' Slope= 0.0624 '/'

Inlet Invert= 47.30', Outlet Invert= 42.00'



Summary for Reach 5R: POA #1

Inflow Area = 1.952 ac, 6.21% Impervious, Inflow Depth > 1.66" for 25-yr storm event

Inflow 2.62 cfs @ 12.41 hrs, Volume= 0.270 af

2.62 cfs @ 12.41 hrs, Volume= Outflow 0.270 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 7.30 fps, Min. Travel Time= 0.0 min Avg. Velocity = 3.01 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.41 hrs Average Depth at Peak Storage= 0.33'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/'

Inlet Invert= 40.00', Outlet Invert= 39.90'

Type III 24-hr 25-yr storm Rainfall=7.10"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 62



Summary for Reach 8R: (new Reach)

Inflow Area = 0.998 ac, 0.00% Impervious, Inflow Depth > 1.46" for 25-yr storm event

1.45 cfs @ 12.36 hrs, Volume= Inflow 0.121 af

1.44 cfs @ 12.37 hrs, Volume= Outflow 0.121 af, Atten= 1%, Lag= 0.8 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.75 fps, Min. Travel Time= 0.4 min Avg. Velocity = 1.82 fps, Avg. Travel Time= 0.7 min

Peak Storage= 31 cf @ 12.37 hrs Average Depth at Peak Storage= 0.35' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 3.12 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 80.0' Slope= 0.0250 '/' Inlet Invert= 50.00', Outlet Invert= 48.00'



Summary for Pond 1P: G.U.S.F. #1

Inflow Area = 0.125 ac, 25.27% Impervious, Inflow Depth > 3.33" for 25-yr storm event

0.37 cfs @ 12.26 hrs, Volume= 0.03 cfs @ 15.15 hrs, Volume= Inflow 0.035 af

Outflow 0.018 af, Atten= 93%, Lag= 173.4 min

Discarded = 0.03 cfs @ 15.15 hrs, Volume= 0.018 af Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.10' @ 15.15 hrs Surf.Area= 1,180 sf Storage= 901 cf

Plug-Flow detention time= 216.2 min calculated for 0.018 af (53% of inflow) Center-of-Mass det. time= 136.7 min (941.7 - 805.0)

Volume	Invert	Avail.Storage	Storage Description
#1	48.50'	1,552 cf	Custom Stage Data (Prismatic)Listed below

Type III 24-hr 25-yr storm Rainfall=7.10"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 63

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
48.50	300	0.0	0	0
49.00	300	40.0	60	60
50.00	1,115	100.0	708	768
50.60	1,500	100.0	785	1,552

Device	Routing	Invert	Outlet Devices
#1	Discarded	48.50'	1.000 in/hr Exfiltration over Surface area
#2	Primary	50.20'	4.0' long x 8.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64
			2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74

Discarded OutFlow Max=0.03 cfs @ 15.15 hrs HW=50.10' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=48.50' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 2P: G.U.S.F. #2

Inflow Area	=	1.228 ac,	1.73% Impervious, Ir	nflow Depth > 1.6	66" for 25-yr storm event
Inflow =	=	1.83 cfs @	12.36 hrs, Volume=	0.170 af	•
Outflow =	=	1.80 cfs @	12.42 hrs, Volume=	0.150 af,	Atten= 2%, Lag= 3.6 min
Discarded =	=	0.02 cfs @	12.42 hrs, Volume=	0.014 af	
Primary =	=	1.78 cfs @	12.42 hrs, Volume=	0.136 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 47.62' @ 12.42 hrs Surf.Area= 946 sf Storage= 1,123 cf

Plug-Flow detention time= 50.4 min calculated for 0.150 af (88% of inflow) Center-of-Mass det. time= 16.6 min (846.0 - 829.5)

Volume	Invert	Avai	I.Storag	je Storage Descr	iption	
#1	45.00		1,236	cf Custom Stage	e Data (Prismatio	:)Listed below
Elevatio	17.7	urf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
45.0	00	500	0.0	0	0	
46.5	50	500	40.0	300	300	
47.0	00	697	100.0	299	599	
47.7	' 5	1,000	100.0	636	1,236	
Device	Routing	In	vert O	outlet Devices		
#1	Discarded	45	.00' 1	.000 in/hr Exfiltrat	tion over Surface	area
#2	Primary	47	.30' 4	.0' long x 8.0' bre	adth Broad-Cres	ted Rectangular Weir
	-		Н	lead (feet) 0.20 0	.40 0.60 0.80 1.	00 1.20 1.40 1.60 1.80 2.00
			2	.50 3.00 3.50 4.0	0 4.50 5.00 5.5	0
			С	oef. (English) 2.43	3 2.54 2.70 2.69	2.68 2.68 2.66 2.64 2.64
			2	.64 2.65 2.65 2.6	66 2.66 2.68 2.7	0 2.74

Invert

Volume

Type III 24-hr 25-yr storm Rainfall=7.10"

Prepared by Altus Engineering, Inc.
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 11/2/2018 Page 64

Discarded OutFlow Max=0.02 cfs @ 12.42 hrs HW=47.61' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=1.74 cfs @ 12.42 hrs HW=47.61' (Free Discharge)
—2=Broad-Crested Rectangular Weir (Weir Controls 1.74 cfs @ 1.39 fps)

Summary for Pond 3P: G.U.S.F. #3

Inflow Area = 0.087 ac, 34.23% Impervious, Inflow Depth > 3.65" for 25-yr storm event 0.39 cfs @ 12.09 hrs, Volume= 0.027 af 0.023 af, Atten= 4%, Lag= 1.1 min 0.01 cfs @ 12.11 hrs, Volume= 0.005 af 0.37 cfs @ 12.11 hrs, Volume= 0.018 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 48.01' @ 12.11 hrs Surf.Area= 305 sf Storage= 174 cf

Plug-Flow detention time= 56.6 min calculated for 0.023 af (87% of inflow) Center-of-Mass det. time= 18.0 min (808.2 - 790.3)

Avail.Storage Storage Description

#1	46.00'		347 cf	Custom Stage	Data (Prismatic)Listed below
Elevatio		ırf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
46.0	00	110 110	0.0 40.0	0	0 66	
48.0 48.5	00	302 410	100.0 100.0	103 178	169 347	
Device	Routing	In	vert Ou	tlet Devices		
#1 #2	Discarded Primary		'.90' 4.0 He 2.5	' long x 8.0' bread (feet) 0.20 0.0 3.00 3.50 4.0	40 0.60 0.80 1.0 0 4.50 5.00 5.50	ted Rectangular Weir 00 1.20 1.40 1.60 1.80 2.00

Discarded OutFlow Max=0.01 cfs @ 12.11 hrs HW=48.01' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.36 cfs @ 12.11 hrs HW=48.01' (Free Discharge)
—2=Broad-Crested Rectangular Weir (Weir Controls 0.36 cfs @ 0.81 fps)

Summary for Pond 5P: G.U.S.F. #4

2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 65

Inflow Area =	0.998 ac,	0.00% Impervious, Inflow D	epth > 1.89"	for 25-yr storm event
Inflow =	1.59 cfs @	12.27 hrs, Volume=	0.157 af	
Outflow =	1.48 cfs @	12.36 hrs, Volume=	0.139 af, Att	en= 7%, Lag= 5.3 min
Discarded =	0.03 cfs @	12.36 hrs, Volume=	0.018 af	
Primary =	1.45 cfs @	12.36 hrs, Volume=	0.121 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.28' @ 12.36 hrs Surf.Area= 1,285 sf Storage= 1,122 cf

Plug-Flow detention time= 53.2 min calculated for 0.139 af (88% of inflow) Center-of-Mass det. time= 18.5 min (849.9 - 831.4)

Volume	Invert	t Ava	il.Storage	Storage Descri	ption		
#1	48.50	•	1,540 cf	Custom Stage	Data (Prismatic	Listed below	
				01	0 0		
Elevation	on S	urf.Area	Voids	Inc.Store	Cum.Store		
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)		
48.5	50	300	0.0	0	0		
49.0	00	300	40.0	60	60		
50.0	00	1,100	100.0	700	760		
50.6	80	1,500	100.0	780	1,540		
Device	Routing	In	vert Out	let Devices			
#1	Discarded	48	3.50' 1.0 0	00 in/hr Exfiltrat	ion over Surface	area	
#2	Primary	50	0.00' 4.0'	long x 8.0' bre	adth Broad-Cres	ted Rectangular Weir	
			Hea	ad (feet) 0.20 0.	40 0.60 0.80 1.0	00 1.20 1.40 1.60 1.80	2.00
			2.50	3.00 3.50 4.0	0 4.50 5.00 5.50	0	
			Coe	ef. (English) 2.43	3 2.54 2.70 2.69	2.68 2.68 2.66 2.64	2.64
					6 2.66 2.68 2.70		

Discarded OutFlow Max=0.03 cfs @ 12.36 hrs HW=50.28' (Free Discharge)
—1=Exfiltration (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=1.44 cfs @ 12.36 hrs HW=50.28' (Free Discharge) —2=Broad-Crested Rectangular Weir (Weir Controls 1.44 cfs @ 1.30 fps)

Summary for Pond 6P: Roof Drip Edge

Inflow Area =	0.040 ac, 87.36% Impervious, Inflow D	epth > 5.90" for 25-yr storm event
Inflow =	0.26 cfs @ 12.09 hrs, Volume=	0.020 af
Outflow =	0.26 cfs @ 12.10 hrs, Volume=	0.019 af, Atten= 0%, Lag= 0.6 min
Discarded =	0.26 cfs @ 12.10 hrs, Volume=	0.019 af
Secondary =	0.00 cfs @ 5.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.64' @ 12.10 hrs Surf.Area= 220 sf Storage= 56 cf

Plug-Flow detention time= 30.6 min calculated for 0.019 af (96% of inflow) Center-of-Mass det. time= 13.4 min (758.4 - 745.0)

Type III 24-hr 25-yr storm Rainfall=7.10"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 66

Volume	Invert	Avail.	.Storage	Storage Descript	ion	
#1	50.00'		286 cf	Custom Stage [)ata (Prismatic)	Listed below
Elevatio		urf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
50.0	00	220	0.0	0	0	
52.0	00	220	40.0	176	176	
52.5	50	220	100.0	110	286	
Device #1	Routing Discarded	Inv 50.8	50' 6.0"	et Devices Horiz. Orifice/Gr		
#2 #3	Discarded Secondary	50.0 52.0	00' 1.00 00' 110. Head 2.50 Coef	d (feet) 0.20 0.40 3.00 3.50	n over Surface eadth Broad-Cre 0 0.60 0.80 1.0	area ested Rectangular Weir 0 1.20 1.40 1.60 1.80 2.00 2.66 2.70 2.77 2.89 2.88

Discarded OutFlow Max=0.26 cfs @ 12.10 hrs HW=50.64' (Free Discharge)

-1=Orifice/Grate (Weir Controls 0.26 cfs @ 1.20 fps)

2=Exfiltration (Exfiltration Controls 0.01 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=50.00' (Free Discharge)
—3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 7P: Roof Drip Edge

Inflow Area =	0.046 ac, 86.00% Impervious, Inflow	Depth > 5.90" for 25-	yr storm event
Inflow =	0.30 cfs @ 12.09 hrs, Volume=	0.023 af	
Outflow =	0.29 cfs @ 12.11 hrs, Volume=	0.022 af, Atten= 3%,	Lag= 1.4 min
Discarded =	0.29 cfs @ 12.11 hrs, Volume=	0.022 af	
Secondary =	0.00 cfs @ 5.00 hrs, Volume=	0.000 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 48.84' @ 12.11 hrs Surf.Area= 280 sf Storage= 94 cf

Plug-Flow detention time= 35.4 min calculated for 0.022 af (96% of inflow) Center-of-Mass det. time= 18.1 min (763.2 - 745.0)

Volume	Invert A	/ail.Storage	Storage Descrip	tion	
#1	48.00'	364 cf	Custom Stage Data (Prismatic)Listed below		
Elevation (feet)	Surf.Are (sq-f		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
48.00	28	0.0	0	0	
50.00	28	0 40.0	224	224	
50.50	28	0 100.0	140	364	

Type III 24-hr 25-yr storm Rainfall=7.10"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 67

Device	Routing	Invert	Outlet Devices
#1	Discarded	48.50'	6.0" Vert. Orifice/Grate C= 0.600
#2	Discarded	48.00'	1.000 in/hr Exfiltration over Surface area
#3	Secondary	50.00'	140.0' long x 2.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50
			Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88
			2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.29 cfs @ 12.11 hrs HW=48.84' (Free Discharge)

-1=Orifice/Grate (Orifice Controls 0.28 cfs @ 1.98 fps)

-2=Exfiltration (Exfiltration Controls 0.01 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=48.00' (Free Discharge)
—3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 9P: Roof Drip Edge

Inflow Area =	0.009 ac, 85.37% Impervious, I	nflow Depth > 5.90"	for 25-yr storm event
Inflow =	0.06 cfs @ 12.09 hrs, Volume=	0.005 af	
Outflow =	0.06 cfs @ 12.10 hrs, Volume=	0.004 af, Att	en= 0%, Lag= 0.6 min
Discarded =	0.06 cfs @ 12.10 hrs, Volume=	0.004 af	
Secondary =	0.00 cfs @ 5.00 hrs, Volume=	0.000 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.64' @ 12.10 hrs Surf.Area= 60 sf Storage= 15 cf

Plug-Flow detention time= 34.4 min calculated for 0.004 af (96% of inflow) Center-of-Mass det. time= 17.5 min (762.5 - 745.0)

Volume	Invert	Avai	I.Storage	Storage Descrip	tion	
#1	50.00'		78 cf	Custom Stage I	Data (Prismatic)	Listed below
Elevatio		Area	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
50.0		60	0.0	0	0	
52.0	0	60	40.0	48	48	
52.5	0	60	100.0	30	78	
Device #1 #2 #3	Routing Discarded Discarded Secondary	50 50	.50' 6.0" .00' 1.00 .00' 110 . Hea 2.50	d (feet) 0.20 0.40 3.00 3.50 4.00	on over Surface eadth Broad-Cro 0 0.60 0.80 1.0 4.50	ested Rectangular Weir 00 1.20 1.40 1.60 1.80 2.00
				7. (English) 2.44 2 2.81 2.92 2.97		2.65 2.64 2.64 2.68 2.68

Type III 24-hr 25-yr storm Rainfall=7.10"

Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC Printed 11/2/2018

Page 68

Discarded OutFlow Max=0.06 cfs @ 12.10 hrs HW=50.64' (Free Discharge)

1=Orifice/Grate (Orifice Controls 0.06 cfs @ 1.29 fps)
2=Exfiltration (Exfiltration Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=50.00' (Free Discharge)
—3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Type III 24-hr 50-yr storm Rainfall=8.50" Printed 11/2/2018

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 69

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1.1S: (new Subcatchment 1.1S: (new Subcatchment 1.1S)	at) Runoff Area=3,988 sf 25.45% Impervious Runoff Depth>4.57" Length=130' Slope=0.0750 '/' Tc=6.0 min CN=70 Runoff=0.51 cfs 0.035 af
Subcatchment 1.2S: (new Subcatchment 1.2S)	Runoff Area=5,446 sf 25.27% Impervious Runoff Depth>4.44" Flow Length=230' Tc=18.1 min CN=69 Runoff=0.49 cfs 0.046 af
Subcatchment 1.3S: Roof 1	Runoff Area=410 sf 85.37% Impervious Runoff Depth>7.19" Tc=6.0 min CN=93 Runoff=0.08 cfs 0.006 af
Subcatchment 2.1S: (new Subc	Runoff Area=8,038 sf 36.17% Impervious Runoff Depth>4.92" Flow Length=95' Tc=6.0 min CN=73 Runoff=1.11 cfs 0.076 af
Subcatchment 2.2S: (new Subc	Runoff Area=10,052 sf 9.19% Impervious Runoff Depth>3.53" Flow Length=200' Tc=12.3 min CN=61 Runoff=0.82 cfs 0.068 af
Subcatchment 2.3S: (new Subc	Runoff Area=3,804 sf 34.23% Impervious Runoff Depth>4.81" Flow Length=65' Tc=6.0 min CN=72 Runoff=0.51 cfs 0.035 af
Subcatchment 2.4S: (new Subc	Runoff Area=19,654 sf 0.74% Impervious Runoff Depth>2.33" Flow Length=130' Tc=7.5 min CN=50 Runoff=1.18 cfs 0.087 af
Subcatchment 2.5S: (new Subc	Runoff Area=43,461 sf 0.00% Impervious Runoff Depth>2.75" Flow Length=320' Tc=17.7 min CN=54 Runoff=2.39 cfs 0.228 af
Subcatchment 2.6S: Roof 1	Runoff Area=1,740 sf 87.36% Impervious Runoff Depth>7.19" Tc=6.0 min CN=93 Runoff=0.32 cfs 0.024 af
Subcatchment 2.7S: Roof 2	Runoff Area=2,000 sf 86.00% Impervious Runoff Depth>7.19" Tc=6.0 min CN=93 Runoff=0.37 cfs 0.028 af
Reach 1R: (new Reach)	Avg. Flow Depth=0.06' Max Vel=2.43 fps Inflow=0.06 cfs 0.006 af =0.022 L=100.0' S=0.1020 '/' Capacity=6.31 cfs Outflow=0.06 cfs 0.006 af
Reach 2R: POA #2	Avg. Flow Depth=0.15' Max Vel=4.53 fps Inflow=0.51 cfs 0.041 af n=0.022 L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=0.51 cfs 0.041 af
Reach 3R: Roadside ditch	Avg. Flow Depth=0.36' Max Vel=2.72 fps Inflow=1.11 cfs 0.076 af =0.022 L=160.0' S=0.0125 '/' Capacity=2.21 cfs Outflow=1.06 cfs 0.076 af
Reach 4R: Swale	Avg. Flow Depth=0.39' Max Vel=6.32 fps Inflow=2.88 cfs 0.224 af n=0.022 L=85.0' S=0.0624 '/' Capacity=4.93 cfs Outflow=2.85 cfs 0.224 af
Reach 5R: POA #1	Avg. Flow Depth=0.42' Max Vel=8.40 fps Inflow=4.26 cfs 0.413 af n=0.022 L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=4.26 cfs 0.413 af
Reach 8R: (new Reach)	Avg. Flow Depth=0.43' Max Vel=4.28 fps Inflow=2.29 cfs 0.191 af n=0.022 L=80.0' S=0.0250 '/' Capacity=3.12 cfs Outflow=2.28 cfs 0.191 af

4916	oost-REVIESD_102018
Prepa	ed by Altus Engineering, Inc.
and the second second second second	ADE 40.00.00 -/- 04000 @ 0040

Type III 24-hr 50-yr storm Rainfall=8.50"

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 70

Pond 1P: G.U.S.F.#1	Peak Elev=50.23' Storage=1,0	74 cf Inflow=0.49 cfs 0.046 af
	Discarded=0.03 cfs 0.020 af Primary=0.06 cfs 0.006	af Outflow=0.09 cfs 0.026 af

Pond 2P: G.U.S.F. #2	Peak Elev=47.73' Storage=1,218 cf Inflow=2.89 cfs 0.2	259 af
	Discarded=0.02 cfs 0.015 af Primary=2.88 cfs 0.224 af Outflow=2.90 cfs 0.2	39 af

Pond 3P: G.U.S.F. #3	Peak Elev=48.04' Storage=182 cf Inflow=0.51 cfs 0.035 at	f
	Discarded=0.01 cfs 0.005 at Primary=0.49 cfs 0.026 at Outflow=0.50 cfs 0.032 at	F

Pond 5P: G.U.S.F. #4		Peak E	Elev=50.37'	Storage=1,244 cf	Inflow=2.39 cfs	0.228 af
	Discarded=0.03 cfs	0.019 af	Primary=2	.29 cfs 0.191 af	Outflow=2.32 cfs	0.210 af

Pond 6P: Roof Drip Edge	Peak Elev=50.65'	Storage=58 cf	Inflow=0.32 cfs 0.024 af
Discarded=0.32 cfs 0.023	af Secondary=0.00	cfs 0.000 af	Outflow=0.32 cfs 0.023 af

Pond 7P: Roof Drip Edge	Peak Elev=48.89'	Storage=100 cf	Inflow=0.37 cfs 0.028 af
Discarded=0.35 cfs	0.026 af Secondary=0.00	cfs 0.000 af	Outflow=0.35 cfs 0.026 af

Pond 9P: Roof Drip Edge	Peak Elev=50.66' Storage=16 cf Infl	low=0.08 cfs 0.006 af
	f Secondary=0.00 cfs 0.000 af Outfl	low=0.08 cfs 0.005 af

Total Runoff Area = 2.263 ac Runoff Volume = 0.633 af Average Runoff Depth = 3.35" 88.58% Pervious = 2.005 ac 11.42% Impervious = 0.258 ac

Type III 24-hr 50-yr storm Rainfall=8.50"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 71

Summary for Subcatchment 1.1S: (new Subcat)

Runoff

=

0.51 cfs @ 12.09 hrs, Volume=

0.035 af, Depth> 4.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

_	Α	rea (sf)	CN [Description				
Men		1,015	98 F	Paved park	ing, HSG B			
		2,373	61 >	75% Gras	s cover, Go	ood, HSG B		
		600	55 V	Voods, Go	od, HSG B			
		3,988	70 V	Weighted Average				
		2,973	7	74.55% Pervious Area				
		1,015	2	25.45% Impervious Area				
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	0.5	130	0.0750	4.11		Shallow Concentrated Flow,		
						Grassed Waterway Kv= 15.0 fps		
	0.5	130	Total,	ncreased t	o minimum	Tc = 6.0 min		

Summary for Subcatchment 1.2S: (new Subcat)

Runoff

0.49 cfs @ 12.25 hrs, Volume=

0.046 af, Depth> 4.44"

1	Α	rea (sf)	CN	Description					
	8	150	98	Roofs, HSG B					
		1,226	98	Paved park	ing, HSG B	1			
		2,570	61	>75% Grass	s cover, Go	ood, HSG B			
_		1,500	55	Woods, Go	od, HSG B	10			
		5,446	69	Weighted A	verage				
		4,070		74.73% Per	vious Area				
		1,376	:	25.27% Imp	ervious Are	ea			
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	15.2	100	0.0400	0.11		Sheet Flow,			
						Woods: Light underbrush n= 0.400 P2= 3.69"			
	2.7	100	0.0150	0.61		Shallow Concentrated Flow,			
						Woodland Kv= 5.0 fps			
	0.2	30	0.0200	2.12		Shallow Concentrated Flow,			
						Grassed Waterway Kv= 15.0 fps			
	18 1	230	Total		1000				

Type III 24-hr 50-yr storm Rainfall=8.50"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 72

Summary for Subcatchment 1.3S: Roof 1

Runoff =

0.08 cfs @ 12.09 hrs, Volume=

0.006 af, Depth> 7.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

	Α	rea (sf)	CN	Description						
		350	98	Roofs, HSG	Roofs, HSG B					
		60	61	>75% Gras	s cover, Go	ood, HSG B				
		410	93	Weighted A	Veighted Average					
		60		14.63% Per	14.63% Pervious Area					
		350		85.37% Impervious Area						
	Тс	Length	Slope	Velocity	Capacity	Description				
(r	nin)	(feet)	(ft/ft		(cfs)					
	6.0					Direct Entry,				

Summary for Subcatchment 2.1S: (new Subcat)

Runoff

1.11 cfs @ 12.09 hrs, Volume=

0.076 af, Depth> 4.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

	Α	rea (sf)	CN I	Description					
		2,907	98	Paved parking, HSG B					
		3,899	61	>75% Grass cover, Good, HSG B					
		1,232	55 \	Woods, Good, HSG B					
8377		8,038	73	Weighted Average					
		5,131	(63.83% Per	vious Area				
		2,907	;	36.17% Impervious Area					
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	5.3	60	0.2000	0.19		Sheet Flow,			
						Woods: Light underbrush n= 0.400 P2= 3.69"			
	0.2	35	0.0600	3.67		Shallow Concentrated Flow,			
				Grassed Waterway Kv= 15.0 fps					
	5.5	95	Total.	Increased to minimum Tc = 6.0 min					

Summary for Subcatchment 2.2S: (new Subcat)

Runoff = 0.82 cfs @ 12.18 hrs, Volume=

0.068 af, Depth> 3.53"

Type III 24-hr 50-yr storm Rainfall=8.50"

Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC Printed 11/2/2018

Page 73

	Α	rea (sf)	CN [Description						
		150	98 F	98 Roofs, HSG B						
		774	98 F	Paved park	ing, HSG B					
		6,280	61 >	75% Grass	s cover, Go	ood, HSG B				
-		2,848	48 E	Brush, Goo	d, HSG B					
10,052 61 Weighted Average										
		9,128	9	0.81% Per	vious Area					
		924	Ş	.19% Impe	ervious Area	a				
	Тс	Length	Slope		Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	***				
	11.5	100	0.0800	0.15		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 3.69"				
	0.8	100	0.0200	2.12		Shallow Concentrated Flow,				
						Grassed Waterway Kv= 15.0 fps				
	12.3	200	Total		=					

Summary for Subcatchment 2.3S: (new Subcat)

Runoff = 0.51 cfs @ 12.09 hrs, Volume=

0.035 af, Depth> 4.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

_	A	rea (sf)	CN	Description	4					
		150	98	Roofs, HSG						
		1,152	98	Paved parking, HSG B						
		1,622	61	>75% Grass	s cover, Go	ood, HSG B				
-		880	55	Woods, God	od, HSG B					
(3)		3,804	72	Weighted Average						
		2,502		65.77% Per	vious Area					
		1,302		34.23% Imp	ervious Ar	ea				
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
	4.3	40	0.1500	0.16		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 3.69"				
	0.1	25	0.1800	6.36		Shallow Concentrated Flow,				
	5-03E35265	100000		Wester on .		Grassed Waterway Kv= 15.0 fps				

4.4 65 Total, Increased to minimum Tc = 6.0 min

Summary for Subcatchment 2.4S: (new Subcat)

Runoff = 1.18 cfs @ 12.12 hrs, Volume=

0.087 af, Depth> 2.33"

Type III 24-hr 50-yr storm Rainfall=8.50"

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 11/2/2018 Page 74

				en e					
	A	rea (sf)	CN [Description					
		145	98 F	Paved parking, HSG B					
		1,100	61 >	>75% Grass cover, Good, HSG B					
		17,252		Brush, Good, HSG B					
		1,157		Voods, Go					
-		19,654 50 Weighted Average							
		19,509			vious Area				
		145	C	.74% Impe	rvious Area	a			
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
-	6.4	50	0.0875	0.13		Sheet Flow,			
			-,,-			Woods: Light underbrush n= 0.400 P2= 3.69"			
	1.1	80	0.0625	1.25		Shallow Concentrated Flow,			
				1.20		Woodland Kv= 5.0 fps			
-	7.5	130	Total						

Summary for Subcatchment 2.5S: (new Subcat)

Runoff = 2.39 cfs @ 12.26 hrs, Volume=

0.228 af, Depth> 2.75"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

91	Α	rea (sf)	CN [Description							
		2,480		75% Grass cover, Good, HSG B							
		5,450		Brush, Good, HSG B							
		35,531		Noods, Go							
		43,461		Weighted A							
		43,461		100.00% Pe	ervious Are	a					
Tc Length Slope Velocity Capacity Description						Description					
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description					
	15.2	100	0.0400	0.11	A STATE OF THE STA	Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 3.69"					
	1.2	50	0.0200	0.71		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
	1.3	170	0.0200	2.12		Shallow Concentrated Flow,					
						Grassed Waterway Kv= 15.0 fps					
	17.7	320	Total								

Summary for Subcatchment 2.6S: Roof 1

Runoff = 0.32 cfs @ 12.09 hrs, Volume=

0.024 af, Depth> 7.19"

Type III 24-hr 50-yr storm Rainfall=8.50"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 75

	Α	rea (sf)	CN	Description							
		1,520	98	Roofs, HSG	Roofs, HSG B						
		220	61	>75% Gras	>75% Grass cover, Good, HSG B						
1.5		1,740	93	Weighted A	Weighted Average						
		220		12.64% Per	vious Area						
		1,520		87.36% Imp	87.36% Impervious Area						
	То	Longth	Clan	o Valooitu	Conneitu	Description					
	Tc	Length	Slop		Capacity	Description					
-	(min)	(feet)	(ft/f	(ft/sec)	(cfs)						
	6.0					Direct Entry.					

Summary for Subcatchment 2.7S: Roof 2

Runoff

0.37 cfs @ 12.09 hrs, Volume=

0.028 af, Depth> 7.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

A	rea (sf)	CN	Description						
	1,720	98	Roofs, HSG B						
Maria	280	61	>75% Grass cover, Good, HSG B						
	2,000	93	Weighted Average						
	280		14.00% Pervious Area						
	1,720		86.00% Impervious Area						
_									
Tc	Length	Slope	,	Capacity					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Reach 1R: (new Reach)

0.125 ac, 25.27% Impervious, Inflow Depth = 0.57" for 50-yr storm event Inflow Area =

0.06 cfs @ 12.97 hrs, Volume= 0.006 af Inflow =

Outflow 0.06 cfs @ 13.00 hrs, Volume= 0.006 af, Atten= 1%, Lag= 1.5 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.43 fps, Min. Travel Time= 0.7 min

Avg. Velocity = 1.58 fps, Avg. Travel Time= 1.1 min

Peak Storage= 3 cf @ 12.99 hrs Average Depth at Peak Storage= 0.06' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.31 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 100.0' Slope= 0.1020 '/' Inlet Invert= 50.20', Outlet Invert= 40.00'

Type III 24-hr 50-yr storm Rainfall=8.50"

4916 post-REVIESD 102018

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 76

Printed 11/2/2018



Summary for Reach 2R: POA #2

Inflow Area = 0.217 ac, 25.34% Impervious, Inflow Depth > 2.26" for 50-yr storm event

0.51 cfs @ 12.09 hrs, Volume= Inflow 0.041 af

0.51 cfs @ 12.09 hrs, Volume= Outflow 0.041 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.53 fps. Min. Travel Time= 0.0 min Avg. Velocity = 1.76 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.09 hrs Average Depth at Peak Storage= 0.15' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/'

Inlet Invert= 40.00', Outlet Invert= 39.90'



Summary for Reach 3R: Roadside ditch

Inflow Area = 0.185 ac, 36.17% Impervious, Inflow Depth > 4.92" for 50-yr storm event

Inflow 1.11 cfs @ 12.09 hrs, Volume= 0.076 af

1.06 cfs @ 12.12 hrs, Volume= Outflow 0.076 af, Atten= 5%, Lag= 1.6 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.72 fps, Min. Travel Time= 1.0 min Avg. Velocity = 1.01 fps, Avg. Travel Time= 2.7 min

Peak Storage= 65 cf @ 12.10 hrs Average Depth at Peak Storage= 0.36' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 2.21 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 160.0' Slope= 0.0125 '/'

Inlet Invert= 42.00', Outlet Invert= 40.00'

Type III 24-hr 50-yr storm Rainfall=8.50" Printed 11/2/2018

Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 77



Summary for Reach 4R: Swale

Inflow Area = 1.228 ac, 1.73% Impervious, Inflow Depth > 2.19" for 50-yr storm event

Inflow 2.88 cfs @ 12.32 hrs, Volume= 0.224 af

Outflow 2.85 cfs @ 12.33 hrs, Volume= 0.224 af, Atten= 1%, Lag= 0.6 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 6.32 fps, Min. Travel Time= 0.2 min Avg. Velocity = 2.97 fps, Avg. Travel Time= 0.5 min

Peak Storage= 39 cf @ 12.32 hrs Average Depth at Peak Storage= 0.39' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 4.93 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 85.0' Slope= 0.0624 '/' Inlet Invert= 47.30', Outlet Invert= 42.00'



Summary for Reach 5R: POA #1

1.952 ac, 6.21% Impervious, Inflow Depth > 2.54" for 50-yr storm event Inflow Area =

4.26 cfs @ 12.30 hrs, Volume= Inflow 0.413 af

Outflow 4.26 cfs @ 12.30 hrs, Volume= 0.413 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 8.40 fps, Min. Travel Time= 0.0 min

Avg. Velocity = 3.25 fps, Avg. Travel Time= 0.0 min

Peak Storage= 1 cf @ 12.30 hrs Average Depth at Peak Storage= 0.42'

Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight

Length= 1.0' Slope= 0.1000 '/'

Inlet Invert= 40.00', Outlet Invert= 39.90'

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 78



Summary for Reach 8R: (new Reach)

Inflow Area = 0.998 ac, 0.00% Impervious, Inflow Depth > 2.30" for 50-yr storm event

Inflow = 2.29 cfs @ 12.31 hrs, Volume= 0.191 af

Outflow = 2.28 cfs @ 12.32 hrs, Volume= 0.191 af, Atten= 1%, Lag= 0.7 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.28 fps, Min. Travel Time= 0.3 min Avg. Velocity = 2.05 fps, Avg. Travel Time= 0.6 min

Peak Storage= 43 cf @ 12.31 hrs Average Depth at Peak Storage= 0.43' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 3.12 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 80.0' Slope= 0.0250 '/' Inlet Invert= 50.00', Outlet Invert= 48.00'



Summary for Pond 1P: G.U.S.F. #1

Inflow Area =	0.125 ac, 25.27% Impervious, Inflow D	epth > 4.44" for 50-yr storm event
Inflow =	0.49 cfs @ 12.25 hrs, Volume=	0.046 af
Outflow =	0.09 cfs @ 12.97 hrs, Volume=	0.026 af, Atten= 81%, Lag= 43.1 min
Discarded =	0.03 cfs @ 12.97 hrs, Volume=	0.020 af
Primary =	0.06 cfs @ 12.97 hrs, Volume=	0.006 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.23' @ 12.97 hrs Surf.Area= 1,265 sf Storage= 1,074 cf

Plug-Flow detention time= 185.8 min calculated for 0.026 af (57% of inflow) Center-of-Mass det. time= 108.8 min (907.3 - 798.5)

Volume	Invert	Avail.Storage	Storage Description
#1	48.50'	1,552 cf	Custom Stage Data (Prismatic)Listed below

Type III 24-hr 50-yr storm Rainfall=8.50"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 79

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
48.50	300	0.0	0	0
49.00	300	40.0	60	60
50.00	1,115	100.0	708	768
50.60	1,500	100.0	785	1,552

Device	Routing	invert	Outlet Devices
#1	Discarded	48.50'	1.000 in/hr Exfiltration over Surface area
#2	Primary	50.20'	4.0' long x 8.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64
			2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74

Discarded OutFlow Max=0.03 cfs @ 12.97 hrs HW=50.23' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=0.06 cfs @ 12.97 hrs HW=50.23' (Free Discharge)
—2=Broad-Crested Rectangular Weir (Weir Controls 0.06 cfs @ 0.45 fps)

Summary for Pond 2P: G.U.S.F. #2

Inflow Area	=	1.228 ac,	1.73% Impervious, Inflow D	epth > 2.53"	for 50-yr storm event
Inflow	=	2.89 cfs @	12.29 hrs, Volume=	0.259 af	
Outflow	=	2.90 cfs @	12.32 hrs, Volume=	0.239 af, Att	ten= 0%, Lag= 1.5 min
Discarded	=	0.02 cfs @	12.32 hrs, Volume=	0.015 af	
Primary	=	2.88 cfs @	12.32 hrs, Volume=	0.224 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 47.73' @ 12.32 hrs Surf.Area= 991 sf Storage= 1,218 cf

Plug-Flow detention time= 35.4 min calculated for 0.238 af (92% of inflow) Center-of-Mass det. time= 11.2 min (833.0 - 821.9)

Volume	Inver	t Ava	il.Stora	age Storage Desc	ription	
#1	45.00)'	1,23	6 cf Custom Stag	e Data (Prismatio	c)Listed below
Elevatio		Surf.Area (sq-ft)	Voids (%		Cum.Store (cubic-feet)	
45.0	00	500	0.0	0 0	0	
46.5	50	500	40.0	300	300	
47.0	00	697	100.0	299	599	
47.7	' 5	1,000	100.0	636	1,236	
Device	Routing	In	vert	Outlet Devices		
#1	Discarded	45	5.00'	1.000 in/hr Exfiltra	tion over Surface	area
#2	Primary	47	'.30'	4.0' long x 8.0' bre	adth Broad-Cres	ted Rectangular Weir
				Head (feet) 0.20 0	.40 0.60 0.80 1.	00 1.20 1.40 1.60 1.80 2.00
				2.50 3.00 3.50 4.0	00 4.50 5.00 5.5	0
						2.68 2.68 2.66 2.64 2.64
				2.64 2.65 2.65 2.6	66 2.66 2.68 2.7	0 2.74

4916 post-REVIESD_102018Type III 24-hr 50-yr storm Rainfall=8.50"

Prepared by Altus Engineering, Inc.

Volume

Invert

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 80

Discarded OutFlow Max=0.02 cfs @ 12.32 hrs HW=47.73' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=2.84 cfs @ 12.32 hrs HW=47.73' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 2.84 cfs @ 1.67 fps)

Summary for Pond 3P: G.U.S.F. #3

Inflow Area = 0.087 ac, 34.23% Impervious, Inflow Depth > 4.81" for 50-yr storm event
Inflow = 0.51 cfs @ 12.09 hrs, Volume= 0.035 af
Outflow = 0.50 cfs @ 12.11 hrs, Volume= 0.032 af, Atten= 2%, Lag= 1.1 min
Discarded = 0.01 cfs @ 12.11 hrs, Volume= 0.005 af
Primary = 0.49 cfs @ 12.11 hrs, Volume= 0.026 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 48.04' @ 12.11 hrs Surf.Area= 310 sf Storage= 182 cf

Plug-Flow detention time= 47.4 min calculated for 0.031 af (90% of inflow) Center-of-Mass det. time= 15.9 min (799.8 - 783.9)

Avail Storage Storage Description

Volume	IIIVCI	t Ava	i.otorage	Oldrage Descrip	JUIOTT	
#1	46.00	'	347 cf	Custom Stage	Data (Prismatic)	Listed below
Elevatio (fee		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
46.0	0	110	0.0	0	0	
47.5	0	110	40.0	66	66	
48.0	0	302	100.0	103	169	
48.5	0	410	100.0	178	347	
Device	Routing	In	vert Ou	tlet Devices		
#1	Discarded	46	.00' 1.0	000 in/hr Exfiltrati	on over Surface	area
#2	Primary	47	'.90' 4.0)' long x 8.0' brea	dth Broad-Crest	ed Rectangular Weir
			He	ad (feet) 0.20 0.4	10 0.60 0.80 1.0	0 1.20 1.40 1.60 1.80 2.00
			2.5	3.00 3.50 4.00	4.50 5.00 5.50	
			Co	ef. (English) 2.43	2.54 2.70 2.69	2.68 2.68 2.66 2.64 2.64

Discarded OutFlow Max=0.01 cfs @ 12.11 hrs HW=48.03' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.48 cfs @ 12.11 hrs HW=48.04' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 0.48 cfs @ 0.89 fps)

Summary for Pond 5P: G.U.S.F. #4

2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74

Type III 24-hr 50-yr storm Rainfall=8.50"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 81

Inflow Area = 0.998 ac, 0.00% Impervious, Inflow Depth > 2.75" for 50-yr storm event Inflow 2.39 cfs @ 12.26 hrs, Volume= 0.228 af Outflow 2.32 cfs @ 12.31 hrs, Volume= 0.210 af, Atten= 3%, Lag= 2.8 min Discarded = 0.03 cfs @ 12.31 hrs, Volume= 0.019 af Primary = 2.29 cfs @ 12.31 hrs, Volume= 0.191 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs. dt= 0.05 hrs. Peak Elev= 50.37' @ 12.31 hrs Surf.Area= 1,348 sf Storage= 1,244 cf

Plug-Flow detention time= 39.4 min calculated for 0.209 af (92% of inflow) Center-of-Mass det. time= 13.7 min (836.7 - 823.1)

Volume	Inve	ert Ava	il.Storage	Storage Descr	iption	
#1	48.5	50'	1,540 ct	Custom Stage	e Data (Prismatio	:)Listed below
Elevatio		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
48.5	50	300	0.0	0	0	
49.0	00	300	40.0	60	60	
50.0	00	1,100	100.0	700	760	
50.6	60	1,500	100.0	780	1,540	
Device	Routing	In	vert Ou	tlet Devices		
#1	Discarde	ed 48	3.50' 1.0	00 in/hr Exfiltrat	ion over Surface	area
#2	Primary	50	0.00' 4.0	' long x 8.0' bre	adth Broad-Cres	ted Rectangular Weir
						00 1.20 1.40 1.60 1.80 2.00
			2.5	3.00 3.50 4.0	0 4.50 5.00 5.50	0
						2.68 2.68 2.66 2.64 2.64
			2.6	4 2.65 2.65 2.6	6 2.66 2.68 2.70	0 2.74

Discarded OutFlow Max=0.03 cfs @ 12.31 hrs HW=50.37' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=2.28 cfs @ 12.31 hrs HW=50.37' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 2.28 cfs @ 1.54 fps)

Summary for Pond 6P: Roof Drip Edge

Inflow Area =	0.040 ac, 87.36% Impervious, Inflow De	epth > 7.19" for 50-yr storm event
Inflow =	0.32 cfs @ 12.09 hrs, Volume=	0.024 af
Outflow =	0.32 cfs @ 12.10 hrs, Volume=	0.023 af, Atten= 0%, Lag= 0.6 min
Discarded =	0.32 cfs @ 12.10 hrs, Volume=	0.023 af
Secondary =	0.00 cfs @ 5.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.65' @ 12.10 hrs Surf.Area= 220 sf Storage= 58 cf

Plug-Flow detention time= 26.6 min calculated for 0.023 af (96% of inflow) Center-of-Mass det. time= 10.4 min (752.8 - 742.4)

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

A all Otamana Otamana Danadatian

Page 82

Volume	Invert	Avail.Sto	rage	Storage Descript	tion	
#1	50.00'	2	86 cf	Custom Stage I	Data (Prismatic)Li	sted below
Elevation (fee	et)		ds %)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
50.0	00	220 (0.0	0	0	
52.0	00	220 40	0.0	176	176	
52.	50	220 100	0.0	110	286	
#1 #2 #3	Routing Discarded Discarded Secondary	50.50' 50.00' 52.00'	6.0" Limi 1.00 110. Hea 2.50 Coe	0' long x 2.0' bred (feet) 0.20 0.40 3.00 3.50	low heads on over Surface at eadth Broad-Cres 0 0.60 0.80 1.00 2.61 2.61 2.60 2	rea Ited Rectangular Weir 1.20 1.40 1.60 1.80 2.00

Discarded OutFlow Max=0.31 cfs @ 12.10 hrs HW=50.65' (Free Discharge)

-1=Orifice/Grate (Weir Controls 0.31 cfs @ 1.28 fps)

-2=Exfiltration (Exfiltration Controls 0.01 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=50.00' (Free Discharge)
—3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 7P: Roof Drip Edge

Inflow Area =	0.046 ac, 86.00% Impervious, Inflo	w Depth > 7.19"	for 50-yr storm event
Inflow =	0.37 cfs @ 12.09 hrs, Volume=	0.028 af	
Outflow =	0.35 cfs @ 12.11 hrs, Volume=	0.026 af, Atte	en= 3%, Lag= 1.3 min
Discarded =	0.35 cfs @ 12.11 hrs, Volume=	0.026 af	
Secondary =	0.00 cfs @ 5.00 hrs, Volume=	0.000 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 48.89' @ 12.11 hrs Surf.Area= 280 sf Storage= 100 cf

Plug-Flow detention time= 31.6 min calculated for 0.026 af (96% of inflow) Center-of-Mass det. time= 14.4 min (756.8 - 742.4)

Volume	Invert	Ava	il.Storage	Storage Descrip	tion	
#1	48.00'		364 cf	Custom Stage	Data (Prismatic)Listed below	
Elevation (feet)		Area	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
48.00	1	280	0.0	0	0	
50.00		280	40.0	224	224	
50.50		280	100.0	140	364	

Type III 24-hr 50-yr storm Rainfall=8.50"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 83

Device	Routing	Invert	Outlet Devices
#1	Discarded	48.50'	6.0" Vert. Orifice/Grate C= 0.600
#2	Discarded	48.00'	1.000 in/hr Exfiltration over Surface area
#3	Secondary	50.00'	140.0' long x 2.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50
			Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88
			2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.35 cfs @ 12.11 hrs HW=48.88' (Free Discharge)

-1=Orifice/Grate (Orifice Controls 0.34 cfs @ 2.11 fps)

-2=Exfiltration (Exfiltration Controls 0.01 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=48.00' (Free Discharge)
—3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 9P: Roof Drip Edge

Inflow Area =	0.009 ac, 85.37% Impervious, Inflow De	epth > 7.19" for 50-yr storm event
Inflow =	0.08 cfs @ 12.09 hrs, Volume=	0.006 af
Outflow =	0.08 cfs @ 12.10 hrs, Volume=	0.005 af, Atten= 0%, Lag= 0.6 min
Discarded =	0.08 cfs @ 12.10 hrs, Volume=	0.005 af
Secondary =	0.00 cfs @ 5.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.66' @ 12.10 hrs Surf.Area= 60 sf Storage= 16 cf

Plug-Flow detention time= 30.4 min calculated for 0.005 af (96% of inflow) Center-of-Mass det. time= 13.3 min (755.7 - 742.4)

Volume	Invert	Ava	il.Storage	Storage Descri	iption	
#1	50.00'		78 cf	Custom Stage	e Data (Prismatio	:)Listed below
12	_					
Elevation	on Sur	f.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
50.0	00	60	0.0	0	0	
52.0	00	60	40.0	48	48	
52.5	50	60	100.0	30	78	
Device	Routing	In	vert Out	let Devices		
#1	Discarded	50	.50' 6.0	" Vert. Orifice/G	rate C= 0.600	
#2	Discarded	50	.00' 1.0	00 in/hr Exfiltrat	ion over Surface	area
#3	Secondary	52	2.00' 110	.0' long x 3.0' b	readth Broad-Cr	ested Rectangular Weir
			Hea	ad (feet) 0.20 0.	40 0.60 0.80 1.0	00 1.20 1.40 1.60 1.80 2.00
				0 3.00 3.50 4.0		
			Coe	ef. (English) 2.44	4 2.58 2.68 2.67	2.65 2.64 2.64 2.68 2.68
				2 2.81 2.92 2.9		

Type III 24-hr 50-yr storm Rainfall=8.50"

Prepared by Altus Engineering, Inc.

Printed 11/2/2018

HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Page 84

Discarded OutFlow Max=0.07 cfs @ 12.10 hrs HW=50.66' (Free Discharge)

1=Orifice/Grate (Orifice Controls 0.07 cfs @ 1.36 fps)
2=Exfiltration (Exfiltration Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=50.00' (Free Discharge)
—3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

