

GENERAL NOTES

1. DESIGN LOADING: HL-93

2. DESIGN METHOD: LOAD AND RESISTANCE FACTOR DESIGN METHOD (LRFD)

3. SPECIFICATIONS: AA

4. FOUNDATION DATA:

AASHTO LRFD 2012 AS AMENDED.

NHDOT 2010 STANDARD SPECIFICATIONS AS AMENDED.

ABUTMENTS — CASED DRILLED SHAFTS WITH ROCK SOCKETS PIERS — CASED DRILLED SHAFTS WITH ROCK SOCKETS

5. REINFORCING STEEL: AASHTO M 31 (ASTM A 615) GRADE 60.

DECK, CURBS, SIDEWALKS, ÁBUTMENTS, WINGWALLS, DRILLED SHAFTS,

APPROACH SLABS, AND PIER CAP REINFORCING STEEL SHALL BE EPOXY

6. STRUCTURAL STEEL: AASHTO M 270, GRADE 50 (ASTM A709, GRADE 50),

PAINTED. EXCEPT AS NOTED.

7. CONCRETE: — DECK SLAB, BRUSH CURB, AND SIDEWALK:

4000 PSI, ITEM 520.70026, CONCRETE BRIDGE DECK (PANEL OPTION)

(QC/QA) (F)

-ABUTMENT CAPS AND BACKWALLS, WINGWALLS, RETAINING WALL STEMS, AND PIER CAPS: 4000 PSI, ITEM 520.0102, CONCRETE CLASS AA (QC/QA) (F)

-RETAINING WALL FOOTINGS: 3000 PSI, ITEM 520.213, CONCRETE CLASS B,

FOOTINGS (ON SOIL) (F)

-APPROACH SLABS: 4000 PSI, ITEM 520.0302 CONCRETE CLASS AA,

APPROACH SLABS (QC/QA) (F)

-DRILLED SHAFTS AND ROCK SOCKETS: 5000 PSI, ITEM 509.2 DRILLED

SHAFT, CLASS AAA (MODIFIED)

- 8. SEISMIC: ZONE 1 As=Fpga X PGA=1.6 x 0.098=0.157g SITE CLASS "D"
- 9. ALL EXISTING BRONZE DISCS REPRESENTING STATE BENCHMARKS OR SURVEY TRIANGULATION POINTS MUST NOT BE DISTURBED. WHEN THE WORK CALLED FOR INVOLVES DISTURBING A BRONZE DISC, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SUFFICIENTLY IN ADVANCE OF THE WORK TO PERMIT THE STATE TO TEMPORARILY RELOCATE THE AFFECTED MARKER.
- 10. MAINTENANCE OF TRAFFIC: BRIDGE CLOSED DURING CONSTRUCTION. SEE ROADWAY PLANS FOR DETOUR OF SAGAMORE AVENUE.
- 11. FOR SURVEY LAYOUT SEE BRIDGE SHEET 06.
- 12. THE CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS AND ELEVATIONS OF THE EXISTING STRUCTURES AND SHALL BE PREPARED TO MAKE ANY ADJUSTMENTS REQUIRED TO PROPERLY COMPLETE THE CONSTRUCTION OF PROPOSED STRUCTURES.
- 13. THE CONTRACTOR SHALL TAKE ALL NECESSARY MEASURES TO INSURE THAT DEBRIS DOES NOT FALL INTO THE WATERWAY BELOW THE EXISTING STRUCTURE. ALL COSTS SHALL BE PAID UNDER ITEM 502 AND SHALL INCLUDE ERECTION, MAINTENANCE AND REMOVAL OF TEMPORARY STRUCTURES OR OTHER SUCH METHODS AS APPROVED.
- 14. NO SCAFFOLDS SHALL BE ERECTED OR OPERATIONS CONDUCTED IN THE WATERWAY, UNLESS APPROVED BY THE CONTRACT ADMINISTRATOR.
- 15. THE CONTRACTOR HAS THE OPTION TO USE PRECAST PRESTRESSED DECK PANEL STAY-IN-PLACE FORMS.

HYDRAULIC DATA

- 1. DRAINAGE AREA: 3.0 SQ. MILES
- 2. DESIGN FLOOD: Q100 = 6,017 CFS
- 3. DESIGN VELOCITY: 1.7 FPS
- 4. DESIGN FLOOD (SURGE) HEIGHT: ELEVATION 08.87 (NAVD88)
- 5. BRIDGE WATERWAY OPENING: 5,330 SQ. FT. BELOW Q100 ELEVATION

CONSTRUCTION STAGING NOTES

- 1. THE CONTRACTOR SHALL SUBMIT A REMOVAL PLAN AND CONSTRUCTION PLAN SHOWING MEANS AND METHODS OF REMOVING THE EXISTING BRIDGE AND SUBSTRUCTURE, CONSTRUCTING THE NEW BRIDGE AND SUBSTRUCTURE, AND SHALL SHOW ANY TEMPORARY STRUCTURES AND ACCESS.
- 2. SEE PERMITS FOR FURTHER REQUIREMENTS INCLUDING PUBLIC ACCESS TO THE NAVIGATIONAL CHANNEL.

BRIDGE REMOVAL NOTES

- 1. THE CONTRACTOR'S METHOD FOR REMOVAL OF THE EXISTING BRIDGE SHALL BE SUBMITTED FOR DOCUMENTATION IN ACCORDANCE TO 105.02, PRIOR TO THE COMMENCEMENT OF ANY REMOVAL OPERATIONS.
- 2. ITEM 502, REMOVAL OF EXISTING BRIDGE STRUCTURE, SHALL INCLUDE THE FOLLOWING: THE EXISTING BRIDGE SUPERSTRUCTURE SHALL BE REMOVED IN ITS ENTIRETY. THE EXISTING ABUTMENTS AND WINGWALLS SHALL BE REMOVED TO EL. 3.3. THE EXISTING PIERS SHALL BE REMOVED TO EL. -11.5.
- 3. THE CONTRACTOR IS ADVISED THAT EXISTING AERIAL POWER, CABLE AND TELEPHONE LINES WILL REMAIN IN PLACE UNTIL EACH UTILITY HAS FINISHED THEIR RELOCATION EFFORTS. THESE ITEMS SHALL BE BID ACCORDINGLY. SEE THE UTILITIES SECTION OF THE PROSECUTION OF WORK FOR ADDITIONAL INFORMATION.
- 4. PLANS OF THE EXISTING BRIDGE STRUCTURE MAY BE OBTAINED FROM THE CITY OF PORTSMOUTH.
- 5. THE CONTRACTOR SHALL MEET OSHA REQUIREMENTS FOR WORKER PROTECTION FOR WORKING WITH MEMBERS WITH LEAD BASED PAINT.
- 6. THE EXISTING BRIDGE HAS BEEN POSTED FOR A LOAD LIMIT OF 6 TONS BASED ON THE CONDITION OF SOME OF THE EXISTING FLOOR BEAMS. THE CONTRACTOR SHALL VERIFY THE CONDITION OF FLOOR BEAMS THAT WILL BE LOADED BY THE CONSTRUCTION OPERATIONS AND DETERMINE THAT THE PROPOSED CONSTRUCTION LOADING DOES NOT EXCEED THE BEAM CAPACITIES.
- 7. THE CONTRACTOR SHALL NOT PLACE EQUIPMENT OR MATERIALS ON THE EXISTING BRIDGE IN A MANNER THAT COULD OVERLOAD THE EXISTING BRIDGE COMPONENTS (DECK, FLOOR BEAMS, GIRDERS ETC.) AND SHALL CONSIDER THE DETERIORATED CONDITION OF STRUCTURAL ELEMENTS AS NEEDED. IF THE CONTRACTOR'S WORK PLAN INVOLVES PLACEMENT OF EQUIPMENT ON THE BRIDGE, SUCH AS A SMALL CRANE, TO FACILITATE COMPLETION OF THE WORK, THE CONTRACTOR SHALL PLAN HIS WORK IN ADVANCE AND SHALL PROVIDE LAYOUT DRAWINGS OF PROPOSED EQUIPMENT AND LOAD DISTRIBUTION METHODS WITH STRUCTURAL CALCULATIONS PREPARED UNDER THE SUPERVISION OF A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NEW HAMPSHIRE. THE PLAN AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND SHALL BE STAMPED BY THE CONTRACTOR'S PROFESSIONAL ENGINEER.
- 8. EXCAVATION FOR REMOVAL OF EXISTING ABUTMENTS SHALL BE BACKFILLED WITH SUITABLE MATERIAL MEETING THE REQUIREMENTS OF SECTION 209, GRANULAR BACKFILL (BRIDGE), TO THE SUBGRADE LINES SHOWN ON THE PLANS. COST FOR EXCAVATION AND BACKFILL SHALL BE SUBSIDIARY TO ITEM 502. PLACE BACKFILL WHILE TIDE LEVELS ARE LOWER THAN BACKFILL LEVEL.

CONSTRUCTION ACCESS NOTES

- 1. ACCESS FOR BRIDGE CONSTRUCTION MAY BE OPEN STRUCTURES (TEMPORARY TRESTLES OR WORK PLATFORMS) OR BARGES. TEMPORARY STONE FILL CAUSEWAYS MAY NOT BE USED.
- 2. THE LOCATION OF THE TEMPORARY TRESTLES SHOWN ON THE PLANS IS APPROXIMATE. ACTUAL TRESTLE LOCATIONS, IF USED, SHALL BE DETERMINED BY THE CONTRACTOR.
- 3. ALL COSTS FOR THE DESIGN, CONSTRUCTION MAINTENANCE, AND REMOVAL OF TEMPORARY ACCESS SHALL BE INCLUDE IN ITEM 500.02, ACCESS FOR BRIDGE CONSTRUCTION. SEE SECTION 500 SPECIAL PROVISIONS FOR ADDITIONAL INFORMATION.
- 4. THE CONTRACTOR SHALL COMPLY WITH ALL PERMIT AND ENVIRONMENTAL DOCUMENT REQUIREMENTS.

BORING NOTES

- 1. BORINGS INDICATED THUS WERE MADE IN MAY OF 2010. FIGURES IN THE "BLOWS PER" COLUMN INDICATE THE NUMBER OF BLOWS REQUIRED TO DRIVE A 2" STANDARD SPLIT SPOON SAMPLER 6", USING A 140 LB. WEIGHT FALLING 30 INCHES.
- 2. BORINGS ARE FOR DESIGN PURPOSES SHOWING CONDITIONS AT BORING POINTS ONLY, AND DO NOT NECESSARILY INDICATE MATERIAL TO BE ENCOUNTERED DURING CONSTRUCTION.
- 3. THE SOILS REPORT IS AVAILABLE AT CITY OF PORTSMOUTH. SEE PROSECUTION OF WORK FOR MORE INFORMATION.
- 4. ROCK CORES WERE MADE USING AN NX 1 7/8" I.D. CORE BARREL.
- 5. THE WATER LEVELS INDICATED WERE MEASURED AT TIME OF EXPLORATION. WATER LEVELS ENCOUNTERED DURING CONSTRUCTION MAY VARY CONSIDERABLY DUE TO PREVAILING CLIMATE, RAINFALL OR OTHER FACTORS.
- 6. THE SURFACE ELEVATION ON EACH BORING LOG IS THE ELEVATION OF THE EXISTING GROUND AT THE TIME THE BORING WAS TAKEN.

FAY. SPOFFORD & THORNDIKE.

- BEDFORD, NH -

DRILLED SHAFT NOTES

- 1. SEE SPECIAL PROVISIONS SECTION 509 FOR CONSTRUCTION REQUIREMENTS RELATED TO THE INSTALLATION OF THE DRILLED SHAFTS.
- 2. THE CONTRACTOR SHALL SUBMIT AN INSTALLATION PLAN AS NOTED IN THE SPECIAL PROVISIONS.
- 3. THE MINIMUM ROCK SOCKET LENGTH MEASURED FROM THE BOTTOM OF THE PERMANENT STEEL CASING/TOP OF ROCK SOCKET SHALL BE AS INDICATED ON THE SUMMARY TABLE ON BRIDGE SHEET 12. LONGER ROCK SOCKET LENGTHS SHALL BE AS DIRECTED. THE TOP OF ROCK SOCKET ELEVATIONS ARE ESTIMATES ONLY. THE TOP OF ROCK SOCKET AND ROCK SOCKET LENGTHS AT EACH DRILLED SHAFT LOCATION SHALL BE APPROVED BY THE ENGINEER.
- 4. CONCRETE FOR DRILLED SHAFTS SHALL BE CLASS AAA MODIFIED. SEE THE SECTION 509 SPECIAL PROVISIONS FOR ADDITIONAL INFORMATION.
- 5. CONSTRUCTION PHASE TEST BORINGS ARE REQUIRED AT ABUTMENTS A AND B AND PIERS 1 AND 2. SEE SPECIAL PROVISIONS FOR SECTION 210. THE TEST BORING WORK AT AN INDIVIDUAL PIER SHALL BE COMPLETED A MINIMUM OF 30 DAYS PRIOR TO INITIATING DRILLED SHAFT WORK AT THAT PIER. THE CONTRACTOR SHALL NOTIFY THE ENGINEER A MINIMUM 21 DAYS IN ADVANCE OF THE TEST BORING WORK, SO THAT ARRANGEMENTS FOR INSPECTION BY THE ENGINEER CAN BE MADE.
- 6. CROSSHOLE SONIC (CSL) TESTING WILL BE CONDUCTED BY THE CONTRACTOR PER ITEM 509.5.
- 7. DRILLING OF OBSTRUCTIONS AS DEFINED IN SECTION 509 AND ANY BEDROCK ABOVE THE FINAL TIP ELEVATIONS OF THE PERMANENT CASING SHALL BE PAID UNDER ITEM 509.3. DRILLING OF THE ROCK SOCKET BELOW THE TIP OF THE PERMANENT CASING SHALL BE PAID UNDER ITEM 509.4. REFER TO THE 509 SPECIAL PROVISIONS.
- 8. THE FOUNDATION CONTRACTOR SHALL MEET THE PREQUALIFICATION REQUIREMENTS AS COVERED IN SPECIAL PROVISION 509.
- 9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING SUPPORT OF REINFORCING CAGES DURING FABRICATION, ERECTION AND PLACEMENT OF CONCRETE. PLANS SHOWING REINFORCING CAGE SUPPORT METHODS OF ERECTION AND CENTERING DEVICES SHALL BE SUBMITTED TO THE BRIDGE FOUNDATION ENGINEER FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.
- 10. ANY CONSTRUCTION JOINT NOT SHOWN ON THE PLANS WILL REQUIRE THE APPROVAL OF THE BRIDGE FOUNDATION ENGINEER PRIOR TO CONSTRUCTION.
- 11. 2 INCH I.D. PVC SCHEDULE 40 ELECTRICAL CONDUIT SHALL BE PLACED AS SHOWN IN THE SHAFT SECTIONS FROM 6 INCH ABOVE THE SHAFT BOTTOM TO 12 INCH ABOVE THE TOP OF DRILLED SHAFT CONSTRUCTION JOINT OR 12 INCH ABOVE BOTTOM OF ABUTMENT BEAMS WITH GLUED CAP BOTTOM AND THREADED CAP TOP END. CONDUIT SHALL BE SECURELY FASTENED TO ALTERNATE TIES, TYPICAL. THE CONTRACTOR SHALL ENSURE THAT THE CONDUIT IS TIED IN A PLUMB AND STRAIGHT POSITION TO THE REINFORCING CAGE SO THAT THE ULTRASONIC PROBE MAY BE LOWERED FREELY TO THE BOTTOM OF THE SHAFTS. ALL SHAFTS SHALL HAVE THE CONDUIT INSTALLED AND SHALL BE INCLUDED IN ITEM 509.5. ALL PVC CONDUITS SHALL BE FILLED WITH WATER IMMEDIATELY AFTER CONCRETE PLACEMENT. ADDITIONAL INTEGRITY TESTING REQUIRED DUE TO THE CONTRACTOR NOT FILLING PVC CONDUITS AS SPECIFIED. RESULTING IN INCONCLUSIVE READINGS, OR DUE TO BROKEN OR BLOCKED PVC TUBES WILL BE AT THE CONTRACTOR'S COST.
- 12. PILE INTEGRITY TESTING OF THE DRILLED SHAFTS WILL BE PERFORMED WITHIN 24 HOURS AFTER THE PLACEMENT OF THE CONCRETE. THE CROSSHOLE SONIC TEST WILL BE CONDUCTED THROUGH ALL OF THE 2 INCH PVC CONDUITS. SHOULD THE PILE INTEGRITY TESTING INDICATE A LOSS OF STRUCTURAL INTEGRITY OF THE DRILLED SHAFT DUE TO SLOUGHING OF THE SOIL INTO THE SPECIFIED DIAMETER OF THE SHAFT OR LACK OF CONSOLIDATION OF THE CONCRETE, THE SHAFT SHALL BE CONSIDERED DEFECTIVE. THE CONTRACTOR SHALL PROPOSE REMEDIAL MEASURES TO CORRECT SUCH DEFECTIVE SHAFTS, INCLUDING IF NECESSARY, CONSTRUCTION OF ADDITIONAL SHAFTS ADJACENT TO THE DEFECTIVE SHAFT WITH A TRANSFER BEAM CAST OVER THE SHAFTS BELOW GRADE. ALL ADDITIONAL SHAFTS. STRUCTURAL CONCRETE, AND LABOR SHALL BE AT NO ADDITIONAL COST TO THE CITY AND NO EXTENSION OF THE CONTRACT TIME WILL BE ALLOWED.
- 13. REINFORCEMENT FOR DRILLED SHAFTS IS QUANTIFIED IN ITEM NO. 509.6 DRILLED SHAFT REINFORCING BARS EPOXY COATED, (CONTRACTOR DETAILED) AND PAID FOR SEPARATELY. THE CONTRACTOR SHALL ANTICIPATE THE LENGTHENING OR SHORTENING OF DRILLED SHAFT REINFORCEMENT TO ACCOMMODATE SHAFT AND SOCKET FIELD CONDITIONS. SEE SHEET 13 FOR DRILLED SHAFT REINFORCEMENT AND DETAILS.
- 14. THE CONTRACTOR SHALL ANTICIPATE OBSTRUCTIONS IN THE FOUNDATION SOILS, AND SHALL PROPOSE ADEQUATE DRILLING EQUIPMENT.
- 15. PERMANENT DRILLED SHAFT CASINGS AT THE PIERS SHALL BE COATED WITH COAL TAR EPOXY POLYAMIDE (BLACK) SUITABLE FOR WATERFRONT STRUCTURES AND SUBMITTED PER 105.02. COATING SHALL BE APPLIED TO THE CUTOFF TOP SURFACE OF THE CASING, AND THE OUTSIDE OF THE CASING TO A DEPTH OF 10 FEET BELOW MUDLINE. SURFACES TO RECIEVE COATING SHALL BE BLAST CLEANED TO SSPC-10, AND PRIMED WITH A ZINC-RICH PRIMER WITHIN FOUR HOURS OF BLAST CLEANING. APPLY TWO COATS OF COAL TAR EPOXY POLYAMIDE TO A MINIMUM DRY THICKNESS OF 8 MILS PER COAT. PRIOR TO INSTALLATION, TEST FOR HOLIDAYS IN TOTAL COATING SYSTEM, USING A HOLIDAY DETECTOR OF LESS THAN 90 VOLTS. REPAIR ANY HOLIDAY AREAS AND RETEST REPAIRED AREAS. MEASURE DRY FILM THICKNESS OF REPAIRED AREAS PER ASTM D7091 AND ASTM E376. COST OF COATING AND TESTING SHALL BE SUBSIDIARY TO ITEM 509.2.

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FILE NAME: R:\VQ-064 Sagamore Creek\CAD Drawings\05 Structural\02 BRIDGE NOTES - SHEET 1 OF 3.dwg PLOTTED: Wednesday, July 24, 2013 - 2:09pm USER: Der

ABUTMENT, RET. WALL, AND WINGWALL NOTES

- 1. ITEM 534.3, WATER REPELLENT (SILANE-SILOXANE) (F), SHALL BE APPLIED TO ALL EXPOSED CONCRETE SURFACES OF RETAINING WALLS ABUTMENTS, WINGWALLS, BACKWALLS, AND BRIDGE SEATS TO 1'-0" BELOW FILL LINES.
- 2. ITEM 538.2, BARRIER MEMBRANE, VERTICAL SURFACES (F), 2' WIDE, SHALL BE PLACED OVER THE BEARING SEAT CONSTRUCTION JOINT, 1'-0" ABOVE AND BELOW THE JOINT.
- 3. CONCRETE CLASS AA (QC/QA) (F), ITEM 520.0102, SHALL BE FORMED INTO 12"X24" BLOCKS ON APPROACH SLAB SEATS TO SUPPORT THE APPROACH CURBS. (QC/QA TESTING REQUIREMENT WAIVED)
- 4. ITEM 585.21, STONE FILL, CLASS B (BRIDGE), SHALL BE 2'-0" THICK, UNLESS OTHERWISE NOTED.
- 5. BLOCKOUTS SHALL BE PROVIDED IN THE ABUTMENT BACKWALLS, BETWEEN GIRDERS 4 AND 5 TO ALLOW FOR THE INSTALLATION OF NEW WATER LINE. SEE BRIDGE SHEET 41 FOR DETAILS.
- 6. PROTRUDING BOULDERS OR COBBLES ENCOUNTERED AT THE FINAL EXCAVATION DEPTH SHALL BE REMOVED OR SPLIT TO PROVIDE A LEVEL BEARING SURFACE AND BACKFILLED.
- 7. ABUTMENT CAP SHALL BE CONSTRUCTED ON A 1'-0" THICK LAYER OF STRUCTURAL FILL.
- 8. TEMPORARY FILL PLACED WITHIN THE CREEK WILL NOT BE ALLOWED.
- 9. ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED 3/4".
- 10. ALL CONCRETE IN THE STUB ABUTMENTS, INCLUDING BACKWALLS, SHALL BE ITEM 520.0102, CONCRETE CLASS AA, (QC/QA)(F).
- 11. ANCHOR BOLTS SHALL BE SET BY TEMPLATE PRIOR TO PLACING ABUTMENT CONCRETE. FOR ANCHOR BOLT DETAILS, SEE BRIDGE SHEET 24.
- 12. CONSTRUCT BACKWALL AND WINGWALLS ABOVE APPROACH SLAB SEAT AFTER STRAIGHT GRANITE (APPROACH) CURBING HAS BEEN SECURELY BLOCKED AND SET TO FINISHED GRADE.
- 13. ALL REINFORCING IN THE ABUTMENTS SHALL BE EPOXY COATED AND SHALL BE PAID AS ITEM 544.31, REINFORCING STEEL, EPOXY COATED (CONTRACTOR DETAILED).
- 14. ALL REINFORCING SHALL BE A MINIMUM OF 2 1/2" FROM CONCRETE SURFACES, UNLESS NOTED OTHERWISE.
- 15. EXPOSED WINGWALL AND RETAINING WALL VERTICAL SURFACES SHALL HAVE A FORM LINER AS SHOWN ON THE PLANS. THE FORM LINER SHALL BE ASHLAR STONE P/C 30664, SYMONS DURA-TEX, AS MANUFACTURED BY SYMONS CORPORATION, 200 E. TOUHY AVENUE, DES PLAINES, IL. 60018 (TEL: 1-800-733-7654) OR ASHLAR STONE NO. 330 MULTI-CAST, AS MANUFACTURED BY GREEN STREAK. 3400 TREE COURT INDUSTRIAL BOULEVARD, ST. LOUIS, MO. 63122 (TEL: 1-800-325-9504) OR AN APPROVED EQUAL. THE COST OF THE FORM LINER SHALL BE INCLUDED IN ITEM 520.99.
- 16. BACKFILL FRONT FACES OF ABUTMENT CAPS PRIOR TO BACK FACES. BACKFILL UP TO ABUTMENT BACKWALL JOINT, AND MEASURE DISTANCES BETWEEN ABUTMENT BEARINGS PRIOR TO SETTING THE BEARINGS. TO CHECK THAT THE ABUTMENTS HAVE NOT SHIFTED DURING BACKFILLING. NOTIFY THE ENGINEER IF THE ABUTMENTS ARE NOT SEPARATED BY THE DIMENSIONS ON THE PLANS.

PIER NOTES

- 1. SEE BRIDGE SHEET 25 FOR BEARING DETAILS, ANCHOR BOLT DETAILS, AND ANCHOR BOLT LAYOUT.
- 2. ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED 3/4".
- 3. PIER CAP AND COLUMN TIE REINFORCEMENT SHALL BE 2 1/2" FROM CONCRETE SURFACES, UNLESS OTHERWISE NOTED. PIER CAP REINFORCEMENT SHALL BE ADJUSTED TO AVOID ANCHOR BOLTS.
- 4. COAT THE ENTIRE PIER SURFACES INCLUDING THE TOP OF SHAFTS, ENTIRE CAP, BEARING SEATS, AND PEDESTALS. WITH ITEM 534.3. WATER REPELLENT (SILANE-SILOXANE).

ELASTOMERIC BEARING ASSEMBLY NOTES

1. BEARING ASSEMBLIES, INCLUDING ELASTOMERIC BEARING PADS, SOLE PLATES, MASONRY PLATES, ANCHOR BOLTS, NUTS AND WASHERS, SHALL BE PAID AS ELASTOMERIC BEARING ASSEMBLIES (F), ITEM 548.21. DESIGN LOADS: (METHOD A AASHTO, LRFD 14.7.6)

> **ABUTMENT** 340 KIPS MAXIMUM DEAD LOAD 130 KIPS 130 KIPS MAXIMUM LIVE LOAD 70 KIPS

- 2. ELASTOMERIC BEARING PADS SHALL BE VIRGIN NATURAL RUBBER, HARDNESS (SHORE "A" DUROMETER) OF 50, GRADE 3. SHEAR MODULUS RANGE 95 PSI TO 130 PSI.
- 3. ANCHOR RODS SHALL BE FABRICATED IN ACCORDANCE WITH SECTION 550.2.5. ANCHOR BOLTS, NUTS 5. AND WASHERS SHALL BE GALVANIZED AFTER FABRICATION AND CONFORM TO AASHTO M232 ASTM A153.
- 4. STEEL PLATES SHALL CONFORM TO AASHTO M 270 GRADE 50 (ASTM A709 GRADE 50). THE STEEL REINFORCING PLATES SHALL CONFORM TO AASHTO M 270 GRADE 50 (ASTM A709 GRADE 50).
- 5. SOLE PLATES & MASONRY PLATE SHALL BE VULCANIZED TO THE ELASTOMER. ALL SURFACES THAT ARE TO BE BONDED TO THE ELASTOMER SHALL BE BLAST CLEAN AS SPECIFIED IN SSPC-SP 10.
- 6. SOLE AND MASONRY PLATES SHALL BE BLAST CLEANED (SSPC-SP 10) AFTER THE VULCANIZING PROCEDURE PRIOR TO PAINTING BEARING ASSEMBLIES. AFTER WELDING TO THE GIRDER FLANGE. CLEAN AND APPLY FINISH COATS TO THE SOLE PLATES.
- 7. BEARINGS SHALL BE INSTALLED AT TEMPERATURES BETWEEN 20°F AND 70°F. INSTALLATION TEMPERATURES OUTSIDE THIS RANGE WILL REQUIRE ADJUSTMENT.
- 8. THE MANUFACTURER SHALL CLEARLY MARK THE FRONT OF THE BEARINGS TO ENSURE PROPER ORIENTATION IN THE FIELD.
- 9. STEEL REINFORCING FOR ELASTOMERIC BEARING PADS SHALL CONFORM TO SECTION 548.2.3
- 10. THE CONTINUOUS WELD CONNECTING THE BOTTOM FLANGE OF GIRDERS TO THE TOP OF THE SOLE PLATES SHALL BE ALLOWED TO COOL AFTER EACH PASS. THE TEMPERATURE OF THE STEEL ADJACENT TO THE ELASTOMER SHALL NOT EXCEED 200°F (TEMPERATURE SHALL BE CONTROLLED BY WELDING PROCEDURES AND TEMPERATURE INDICATING CRAYON, OR OTHER DEVICES APPROVED BY THE ENGINEER). ALL PLATES SHALL BE FLAT AND TRUE AFTER WELDING.
- 11. THE TOP OF ALL SOLE PLATES SHALL BE BEVELED TO MATCH THE APPROXIMATE ROADWAY GRADE IS AS SHOWN. BEVELED WASHERS SHALL BE REQUIRED BENEATH THE NUTS.

STRUCTURAL STEEL NOTES

- 1. STRUCTURAL STEEL SHALL CONFORM TO AASHTO M 270, GRADE 50 (ASTM A709, GRADE 50) PAINTED (EXCEPT AS NOTED). ALL STRUCTURAL STEEL SHALL BE PAID UNDER ITEM 550.1, STRUCTURAL STEEL (F).
- 2. ALL WELDING AND THE PREPARATION AND ASSEMBLY OF MATERIAL FOR WELDING SHALL CONFORM TO THE NHDOT STANDARD SPECIFICATIONS, THE BRIDGE WELDING CODE (AASHTO/AWS D1.5) AND ALL INTERIM REVISIONS.
- 3. THE LOCATION OF SHOP SPLICES SHALL BE APPROVED BY THE ENGINEER. WEB SPLICES SHALL BE LOCATED A MINIMUM OF 9" FROM WELDED FLANGE SPLICES AND A MINIMUM OF 6" FROM TRANSVERSE STIFFENERS OR CONNECTION PLATES.
- 4. ALL BOLTED FIELD CONNECTIONS SHALL BE MADE WITH 7/8" DIA. HIGH STRENGTH BOLTS AASHTO M164 (ASTM A325) TYPE 1 PLACED IN 15/16" DIA. HOLES. BOLTS IN PAINTED AREAS SHALL BE ASTM A325 TYPE 1 GALVANIZED.
- 5. DIRECT TENSION INDICATOR WASHERS SHALL BE INSTALLED WITH HIGH STRENGTH BOLTS.
- 6. HOLES FOR FIELD SPLICES SHALL BE SHOP DRILLED WHILE GIRDERS ARE ASSEMBLED TO FIT BEARING ELEVATIONS.
- 7. TOP FLANGE BOLTS OF THE FIELD SPLICE SHALL BE INSTALLED WITH THE BOLT HEAD ON THE TOP SPLICE PLATE TO AVOID CONFLICTS IF PRECAST CONCRETE DECK PANELS ARE USED.
- 8. GIRDERS SHALL BE CAMBERED FOR THE FULL DEAD LOAD DEFLECTION AND THE EFFECT OF VERTICAL CURVATURE. SEE BRIDGE SHEET 27 FOR CAMBER TABLE.
- 9. SHOP OR FIELD WELDING OF ATTACHMENTS TO, OR PLACEMENT OF HOLES IN ANY EXPOSED PORTION OF THE PLATE GIRDERS FOR CONSTRUCTION PURPOSES, IS NOT PERMITTED. SHOP OR FIELD ATTACHMENTS TO THE TOP FLANGE FOR CONSTRUCTION PURPOSES MUST BE APPROVED BY THE ENGINEER.
- 10. CROSS FRAMES SHALL BE SHOP WELDED WITH 5/16" FILLET WELDS, UNLESS NOTED OTHERWISE. THE GRAVITY AXES OF CROSS FRAME MEMBERS SHOULD INTERSECT AS NEARLY AS PRACTICAL AT THE CENTERLINE OF THE GIRDER.
- 11. BEARING STIFFENERS AND GIRDER ENDS SHALL BE VERTICAL UNDER FULL DEAD LOAD DEFLECTION.
- 12. GIRDERS AND CROSS FRAMES SHALL BE FABRICATED SO THAT GIRDER WEBS ARE PLUMB UNDER FULL DEAD LOAD DEFLECTION.
- 13. SCREED RAIL SUPPORTS REQUIRED FOR THE PLACEMENT OF THE DECK CONCRETE SHALL BE LOCATED AT THE CENTERLINE OF THE GIRDER.
- 14. ALL SHEAR CONNECTORS SHALL BE FIELD WELDED TO THE TOP FLANGE WITH AUTOMATICALLY TIMED STUD WELDING EQUIPMENT. SHEAR CONNECTORS AT FIELD SPLICE LOCATIONS SHALL BE ARRANGED TO CLEAR FASTENERS AND SHALL BE WELDED TO THE SPLICE PLATES. THE TOTAL NUMBER OF SHEAR CONNECTORS IN A GIVEN LENGTH SHALL NOT BE REDUCED.
- 15. STEEL ERECTION SHALL NOT BE PERMITTED UNTIL THE ABUTMENTS HAVE BEEN BACKFILLED TO THE LEVEL OF THE APPROACH SLAB.
- 16. PRIOR TO HANDLING THE STRUCTURAL STEEL, THE CONTRACTOR SHALL SUBMIT DETAILED HANDLING AND ERECTION PLANS IN ACCORDANCE WITH SECTION 550.
- 17. TEMPORARY SHORING TOWERS SHALL NOT BE REMOVED UNTIL ALL STRUCTURAL STEEL IS ERECTED, AND ALL SPLICES AND CROSS FRAME CONNECTIONS ARE FULLY TIGHTENED. ALL TEMPORARY SHORING TOWERS SHALL BE REMOVED PRIOR TO CONSTRUCTING THE DECK.
- 18. ALL STEEL ERECTION COSTS ARE INCLUDED IN ITEM 550.1.

- 19. THE ENGINEER WILL INSPECT THE SHOP FABRICATION OF THE STRUCTURAL STEEL.
- 20. NOTCH TOUGHNESS REQUIREMENTS OF NHDOT STANDARD SPECIFICATIONS SHALL APPLY TO THE WEB AND FLANGES OF GIRDERS AND SPLICE PLATES.
- 21. THE STRUCTURAL STEEL FABRICATOR SHALL ARRANGE FOR NON-DESTRUCTIVE TESTING OF THE WELDS. ALL COSTS TO BE INCLUDED IN ITEM 550.1.
- 22. THE PAINT FINISH COLOR WILL BE DETERMINED BY THE CITY AT A LATER TIME.

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			BRI	IDGE	NOTE	S - SH	IEET	2 OF 3		BRIDGE SHEET
			ВҮ	DATE		BY	DATE	REVISIONS AFTER PROF	POSAL DATE	03 of 41
	SHEET SCALE	DESIGNED	TD	5/13	CHECKED	MAB	5/13			FILE NUMBER
	AS NOTED	DRAWN	FLC	5/13	CHECKED	TD	5/13			
FAY, SPOFFORD & THORN	IDIKE, INC	TRACED		-	CHECKED			FEDERAL PROJECT NO.	SHEET NO.	TOTAL SHEETS
- BEDFORD, NH -	, _ , , , , , , , , , , , , , , , , , ,	QUANTITIES	TD	6/13	CHECKED	МАВ	6/13	X-A000(417)	17	91
				•						

daid jot bribon Noigo - Siner S Ol Stang Fronte. Wednesday, saiy 24, 2013 - 11.11am osen. Coman

COFFERDAM NOTES

- 1. COFFERDAM LIMITS SHOWN ON THE PLANS ARE APPROXIMATE. THE CONTRACTOR SHALL DETERMINE THE COFFERDAM LIMITS REQUIRED TO SUPPORT EXISTING EMBANKMENTS AND PROPOSED EXCAVATION.
- 2. ALL COSTS FOR DESIGN, INSTALLATION AND REMOVAL OF COFFERDAMS SHALL BE INCLUDED IN ITEMS 503.201.
- 3. THE CONTRACTOR SHALL SUBMIT THE COFFERDAM DESIGN AND PROPOSED METHOD OF CONSTRUCTION TO THE ENGINEER IN ACCORDANCE WITH SECTION 105.02 OF THE NHDOT STANDARD SPECIFICATIONS. COFFERDAM SUBMITTALS SHALL BE DESIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NEW HAMPSHIRE.

DECK REINFORCEMENT NOTES

- 1. ALL REINFORCING IN THE BRIDGE DECK AND BRUSH CURBS SHALL BE EPOXY COATED AND SHALL BE PAID AS ITEM 544.31, REINFORCING STEEL, EPOXY COATED (CONTRACTOR DETAILED).
- 2. ALL REINFORCING SHALL BE 2 1/2" FROM CONCRETE SURFACES, UNLESS OTHERWISE NOTED.

DECK SLAB ELEVATION NOTES

- 1. AFTER THE STEEL GIRDERS ARE ERECTED, ELEVATIONS ON THE TOP FLANGE OF THE GIRDERS ARE TO BE OBTAINED AT THE POINTS INDICATED IN THE TABLE. THE DIFFERENCE BETWEEN THE ELEVATIONS OBTAINED AND THOSE SHOWN IN THE TABLE IS THE ACTUAL BLOCKING DISTANCE FROM THE TOP OF THE GIRDER TO THE BOTTOM OF THE DECK SLAB AT THE CENTERLINE OF THE GIRDER. SEE ELEVATION TABLE AND HAUNCH DETAIL ON BRIDGE SHEET 32.
- 2. ELEVATIONS SHOWN IN THE TABLE ARE BOTTOM OF SLAB ELEVATIONS ADJUSTED FOR TOTAL DEAD LOAD DEFLECTION, LESS THE DEFLECTION DUE TO GIRDER WEIGHT.
- 3. THE BRIDGE DECK CONCRETE SHALL REMAIN PLASTIC THROUGHOUT EACH POUR. THE DECK PLACEMENT SHALL PROCEED UP-GRADE. FOR DECK POUR SEQUENCE, SEE BRIDGE SHEET 32.

APPROACH SLAB NOTES

- 1. CONCRETE FOR THE APPROACH SLABS SHALL BE ITEM 520.0302, CONCRETE CLASS AA, APPROACH SLABS (QC/QA) (PRECAST OPTION) (F). CONCRETE COVER FOR REINFORCING STEEL SHALL NOT BE MEASURED.
- 2. SYNTHETIC FIBER REINFORCEMENT SHALL BE ADDED TO THE CONCRETE FOR THE APPROACH SLABS AND SHALL BE PAID UNDER ITEM 544.7, SYNTHETIC FIBER REINFORCEMENT (F).
- 3. REINFORCEMENT IN THE APPROACH SLABS SHALL BE EPOXY COATED, AND PAID UNDER ITEM 544.31, REINFORCING STEEL EPOXY COATED (CONTRACTOR DETAILED).
- 4. FILL SPACES BETWEEN THE APPROACH CURBS AND APPROACH SLABS WITH ITEM 520.0302 WITH ALL QC/QA REQUIREMENTS WAIVED.
- 5. APPROACH SLABS SHALL BE PLACED AFTER THE CONCRETE DECK HAS BEEN CONSTRUCTED.

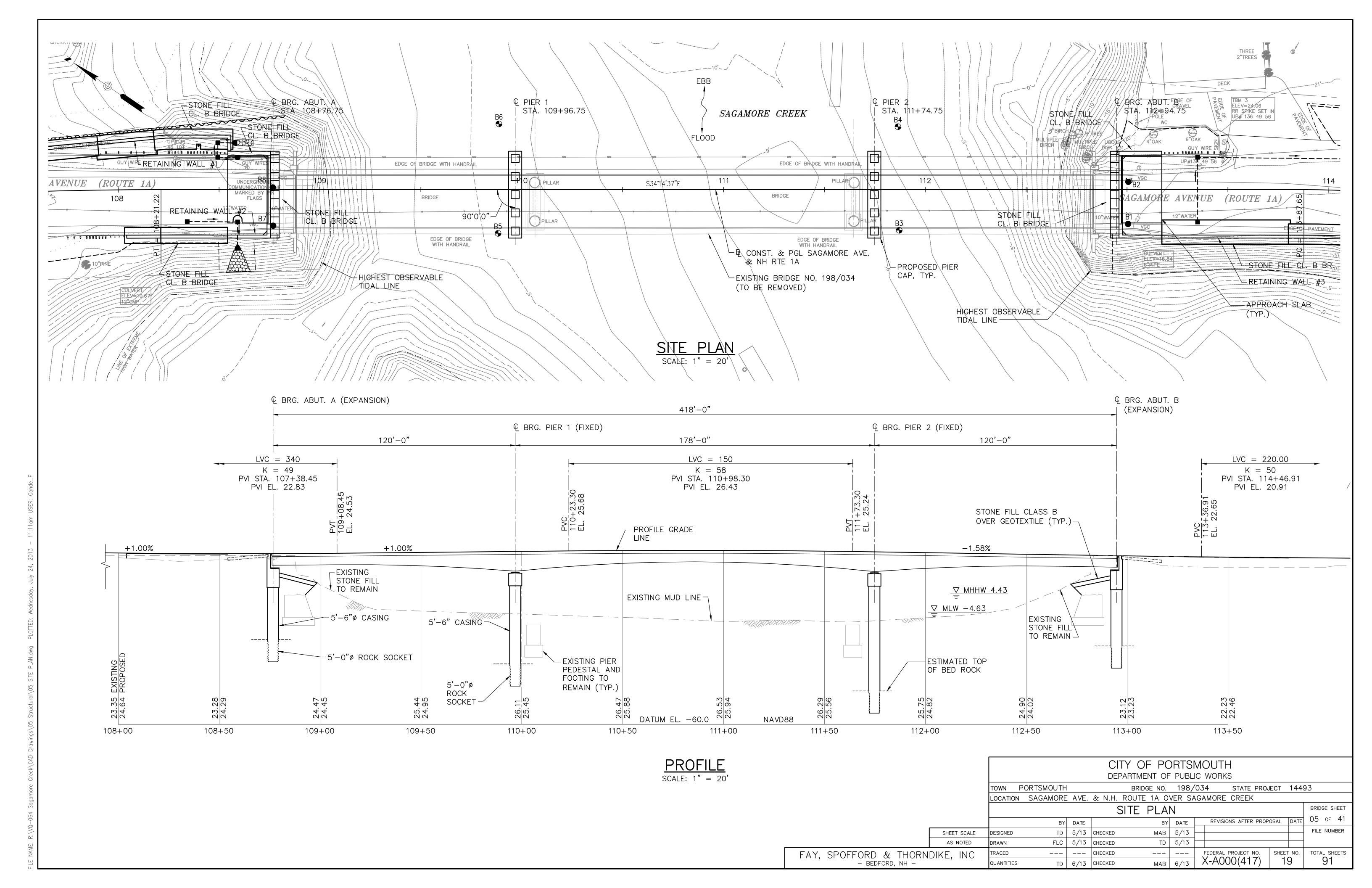
EXPANSION JOINT NOTES

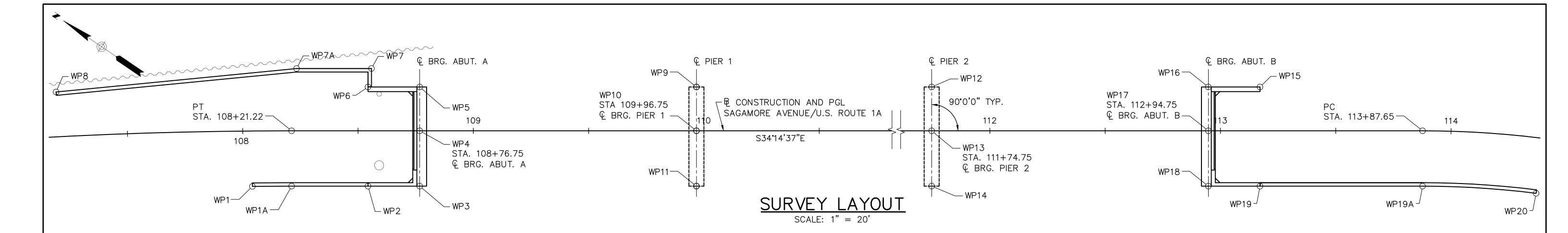
- 1. EXPANSION JOINT STEEL SHALL CONFORM TO AASHTO M 270, GRADE 50 (ASTM A 709, GR 50) GALVANIZED, EXCEPT AS OTHERWISE ALLOWED. THE ENTIRE ASSEMBLY, INCLUDING THE ELASTOMERIC SEAL, SHALL BE PAID FOR AS ITEM 561.110, PREFABRICATED EXPANSION JOINT, TYPE A (F).
- 2. SPLICES FOR EXPANSION JOINT STEEL SHALL DEVELOP FULL STRENGTH.
- 3. THE EXPANSION JOINT SHALL BE PRESET TO THE TEMPERATURE ANTICIPATED AT THE TIME OF INSTALLATION. FINAL SETTING IN THE FIELD SHALL BE DETERMINED BY THE ENGINEER (SEE TEMPERATURE ADJUSTMENT TABLE ON BRIDGE SHEET 37).
- 4. ELASTOMERIC STRIP SEALS SHALL BE FURNISHED IN ONE CONTINUOUS LENGTH. NO SPLICE WILL BE ALLOWED.
- 5. JOINT SUPPORT PLATES SHALL BE SHOP WELDED TO THE EXPANSION JOINT STEEL AND SHALL BE VERTICAL AFTER THE JOINT ASSEMBLY HAS BEEN ADJUSTED FOR ROADWAY CROSS—SLOPE AND PROFILE GRADE.
- 6. THE EXPANSION JOINT ASSEMBLY SHALL BE INSTALLED ONLY AFTER THE ABUTMENT HAS BEEN BACKFILLED TO WITHIN 3'-0" OF FINISHED GRADE.
- 7. IMMEDIATELY AFTER THE JOINT HAS BEEN SECURED TO THE STRUCTURAL STEEL AND BACKWALL, REMOVE SHIPPING DEVICES AND GRIND SMOOTH ANY WELDS ON EXPOSED SURFACES. REPAIR ANY DAMAGED GALVANIZED SURFACES.
- 8. PROTECT TOP OF EXPANSION JOINT DURING PLACEMENT OF CONCRETE AND BITUMINOUS PAVEMENT.
- 9. THE STRIP SEAL SYSTEM SHALL HAVE A MINIMUM RANGE OF MOVEMENT OF 4".
- 10. ELEVATIONS SHOWN ARE AT TOP OF ANGLES, WHICH ARE SET 1/8" LOWER THAN PROPOSED FINISHED ROADWAY GRADE.

MISCELLANEOUS NOTES

1. A NEW WATER LINE SHALL BE INSTALLED AT THE APPROXIMATE LOCATION SHOWN ON THE SITE PLAN.

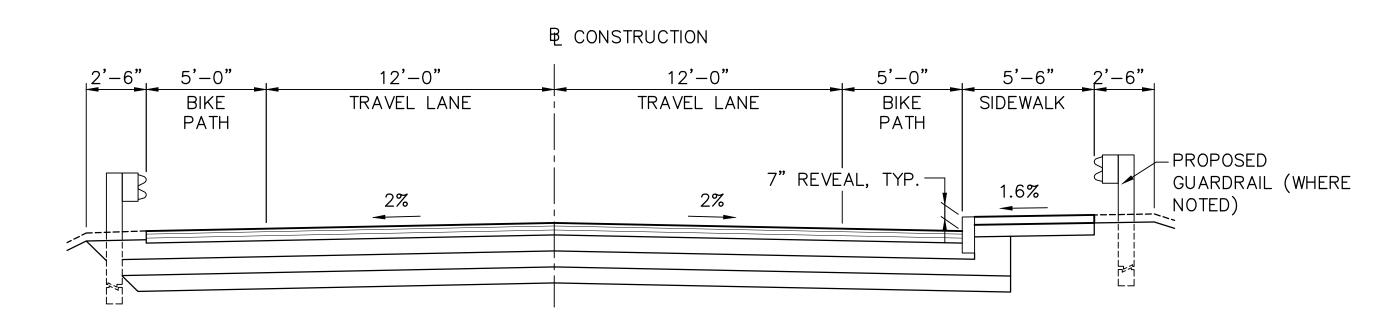
						TY OF PC PARTMENT OF								
		TOWN												
		LOCATIO	OCATION SAGAMORE AVE. & N.H. ROUTE 1A OVER SAGAMORE CREEK											
				BRI	IDGE	NOTES -	· SH	EET 3 OF 3			BRIDGE SHEET			
			BY	DATE		BY	DATE	REVISIONS AFTER PROF	POSAL D	ATE	04 of 41			
	SHEET SCALE	DESIGNED	TD	5/13	CHECKED	MAB	5/13				FILE NUMBER			
	AS NOTED	DRAWN	FLC	5/13	CHECKED	TD	5/13							
FAY, SPOFFORD & THORN	DIKE. INC	TRACED			CHECKED			FEDERAL PROJECT NO.	SHEET N	10.	TOTAL SHEETS			
- BEDFORD, NH -	•	QUANTITIES TD 6/13 CHECKED MAB 6/13 X-A000(417) 18 91												





ITEM NUMBER	DESCRIPTION	QUANTITY	UNIT
209.201	GRANULAR BACKFILL (BRIDGE) (F)	1,323	CY
210.6	MOBILIZATION AND DEMOBILIZATION FOR TEST BORING DRILLING EQUIPMENT	1	U
210.61	ADVANCING CASED BORING HOLE	200	LF
210.62	ADVANCING BORING HOLE BY DIAMOND CORE DRILLING	108	LF
403.911	HOT BITUMINOUS BRIDGE PAVEMENT, 1" BASE COURSE (F)	83	Т
500.02	ACCESS FOR BRIDGE CONSTRUCTION	1	U
502	REMOVAL OF EXISTING BRIDGE STRUCTURE	1	U
503.201	COFFERDAMS	1	U
504.1	COMMON BRIDGE EXCAVATION (F)	2,500	CY
508	STRUCTURAL FILL	200	CY
509.1	MOBILIZATION & DEMOBILIZATION OF DRILLED SHAFT DRILLING EQUIP.	1	U
509.2	DRILLED SHAFT	430	LF
509.3	OBSTRUCTION REMOVAL	100	LF
509.4	ROCK SOCKET EXCAVATION	120	LF
509.5	CROSSHOLE SONIC LOGGING (CSL) TESTS	30	EA
509.6	DRILLED SHAFT REINFORCING STEEL, EPOXY COATED (CONTRACTOR DETAILED)	130,000	LB
520.0102	CONCRETE CLASS AA, (QC/QA) (F)	658	CY
520.0302	CONCRETE CLASS AA, APPROACH SLABS (QC/QA) (F)	62	CY
520.213	CONCRETE CLASS B, FOOTINGS (ON SOIL) (F)	181	CY
520.70026	CONCRETE BRIDGE DECK (QC/QA) (PANEL OPTION) (F)	680	CY
520.99	FORM LINER FOR CONCRETE SURFACES	3,000	SF
534.3	WATER REPELLENT (SILANE-SILOXANE)	120	GAL
538.2	BARRIER MEMBRANE, PEEL AND STICK — VERTICAL SURFACES (F)	27	SY
538.5	BARRIER MEMBRANE, WELDED BY TORCH (F)	22	SY
538.6	BARRIER MEMBRANE, WELDED BY TORCH MACHINE METHOD (F)	1,586	SY
541.1	PVC WATERSTOPS, NH TYPE 1 (F)	125	LF
541.4	PVC WATERSTOPS, NH TYPE 4 (F)	62	LF
544.3	REINFORCING STEEL (CONTRACTOR DETAILED)	58,000	LB
544.31	REINFORCING STEEL, EPOXY COATED (CONTRACTOR DETAILED)	326,100	LB
544.7	SYNTHETIC FIBER REINFORCEMENT	434	LB
547.1	SHEAR CONNECTORS (F)	5,055	EA
548.21	ELASTOMERIC BEARING ASSEMBLIES (F)	20	EA
550.1	STRUCTURAL STEEL (F)	686,200	LB
556.201	CONTAINMENT AND ENVIRONMENTAL PROTECTION	1	U
556.301	WORKER PROTECTION	1 1	U
556.401	WASTE MANAGEMENT	1 1	U
561.11	PREFABRICATED EXPANSION JOINT, TYPE A (F)	86	LF
562.1	SILICONE JOINT SEALANT (F)	62	LF
563.24	BRIDGE RAIL T4	1,112	LF
565.242	BRIDGE APPROACH RAIL, T4 (STEEL POSTS) (F)	3	U
585.21	STONE FILL, CLASS B (BRIDGE)	600	CY
593.411	GEOTEXTILE PERM. EROSION CONTROL, CLASS 1, NON-WOVEN	900	SY

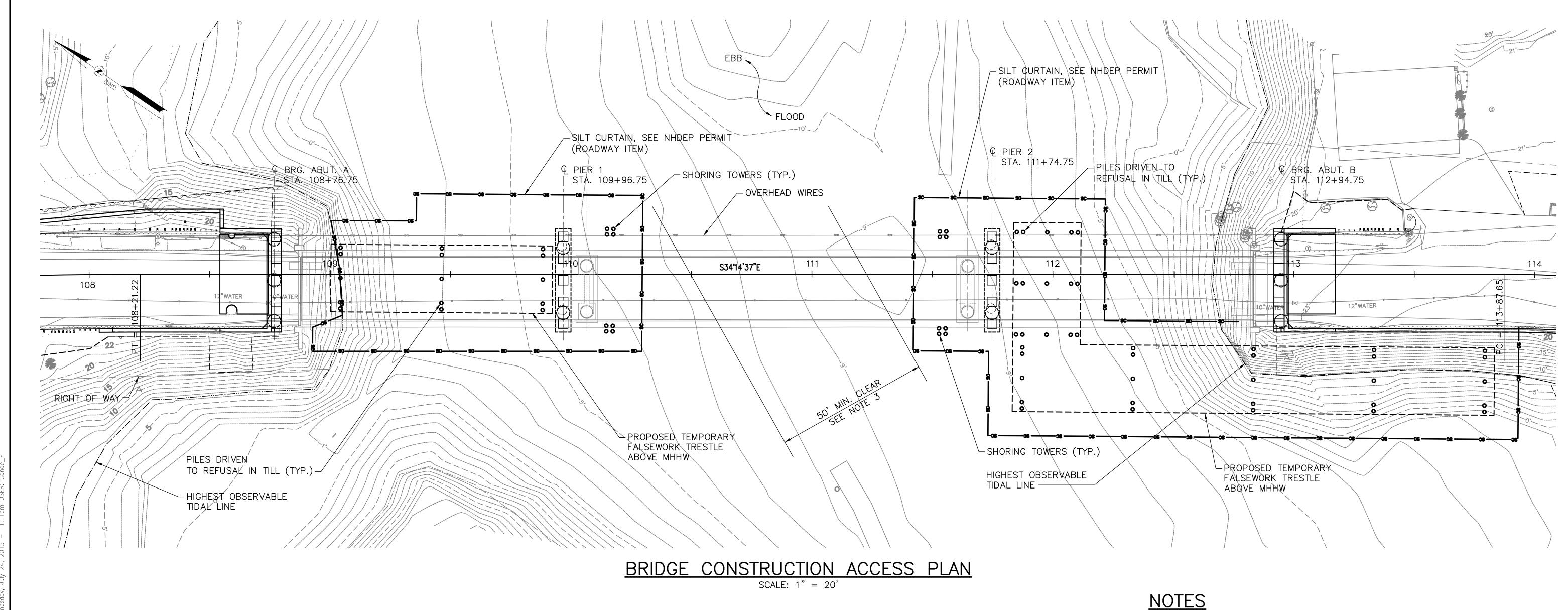
WORKING F	POINT COOR	RDINATES				
WORKING POINT NO.	NORTHING	EASTING				
WP1	203736.1398	1229331.5543				
WP1A	203722.1135	1229341.1908				
WP2	203694.7451	1229359.8207				
WP3	203676.2120	1229372.4364				
WP4	203689.7171	1229392.2761				
WP5	203700.4086	1229407.9825				
WP6	203718.9418	1229395.3668				
WP7	203722.2035	1229402.8241				
WP7A	203749.0644	1229384.5395				
WP8	203829.5504	1229317.4668				
WP9	203601.2103	1229475.5080				
WP10	203590.5187	1229459.8015				
WP11	203577.0137	1229439.9619				
WP12	203454.0661	1229575.6707				
WP13	203443.3745	1229559.9643				
WP14	203429.8694	1229540.1246				
WP15	203336.3346	1229655.8119				
WP16	203354.8677	1229643.1962				
WP17	203344.1762	1229627.4897				
WP18	203330.6711	1229607.6501				
WP19	203312.1379	1229620.2658				
WP19A	203253.8748	1229659.9262				
WP20	203211.4487	1229685.0897				



TYPICAL SAGAMORE AVENUE APPROACH SECTION SCALE 1/4" = 1'-0"

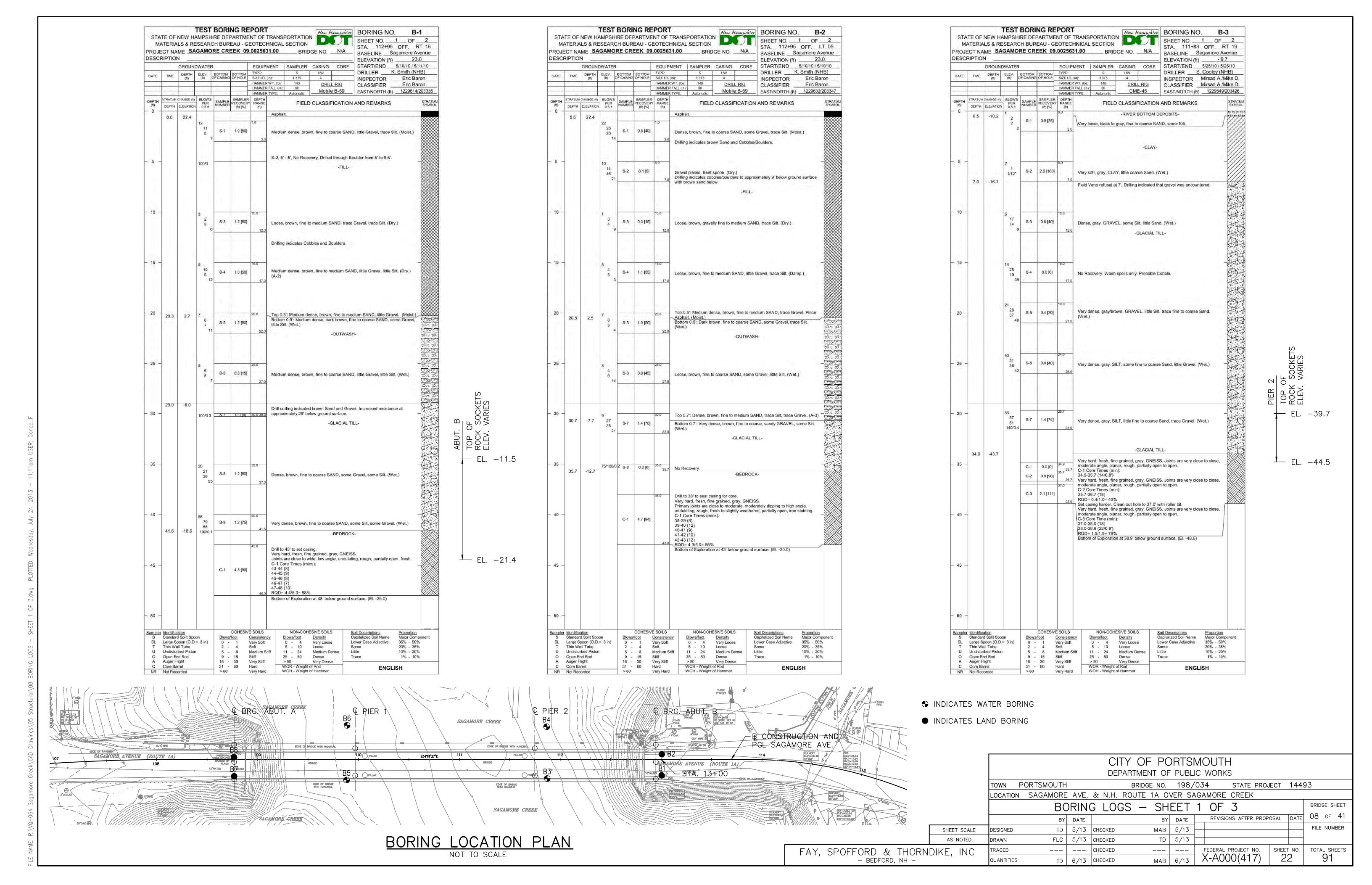
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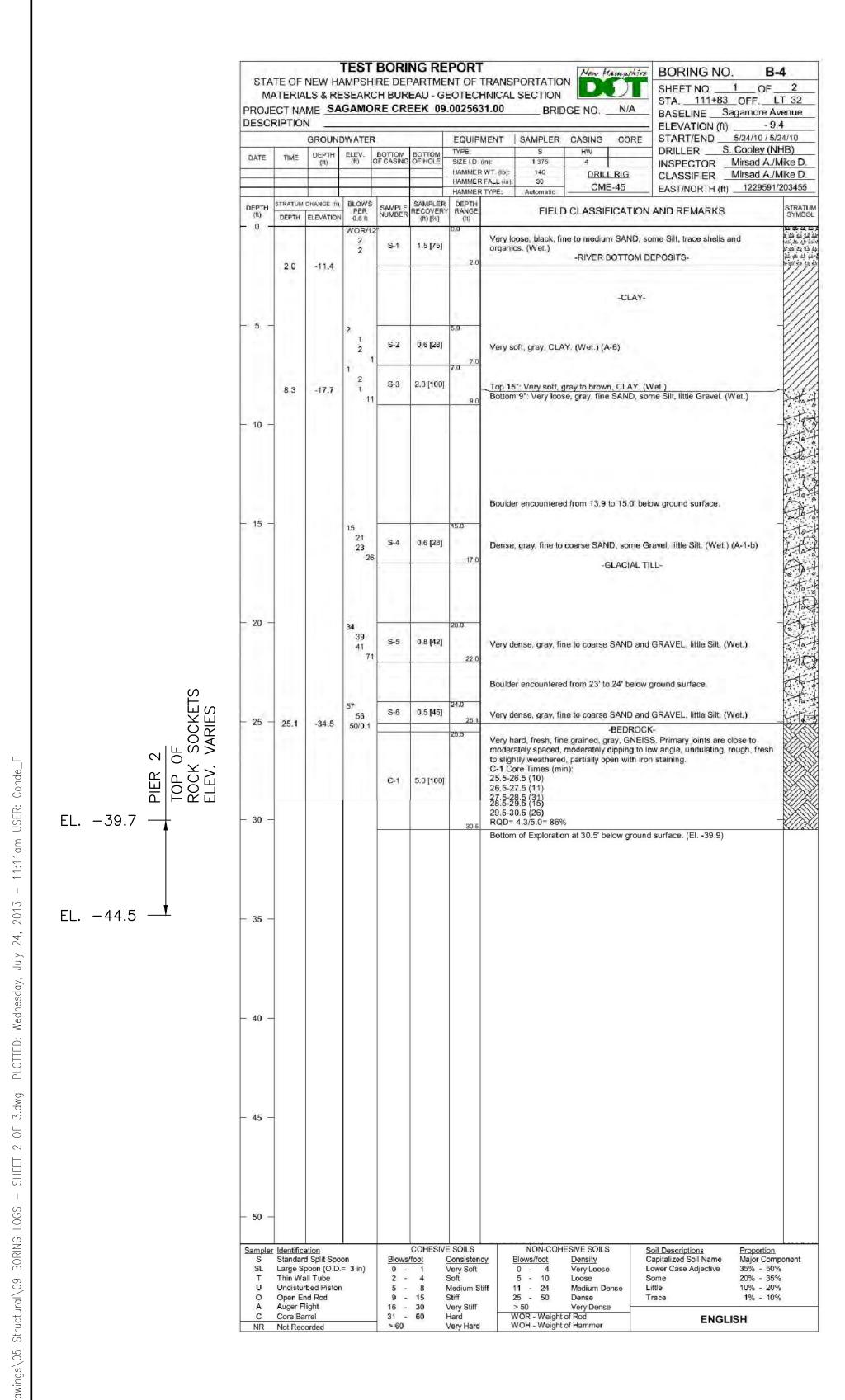
					_	Y OF PC ARTMENT OF				
		TOWN	PORTSMOUTH			BRIDGE NO.	198/	034 STATE PRO	JECT 1449	93
		LOCATIO	N SAGAMORE	AVE.	& N.H.	ROUTE 1A O	VER SA	GAMORE CREEK		
			BRIDGE	QUA	ITIT <i>NA</i>	ES AND	SUR	VEY LAYOUT		BRIDGE SHEET
			BY	DATE		BY	DATE	REVISIONS AFTER PROF	OSAL DATE	06 of 41
	SHEET SCALE	DESIGNED	TD	5/13	CHECKED	MAB	5/13			FILE NUMBER
	AS NOTED	DRAWN	FLC	5/13	CHECKED	TD	5/13			
FAY, SPOFFORD & THORN	DIKE. INC	TRACED			CHECKED			FEDERAL PROJECT NO.	SHEET NO.	TOTAL SHEETS
- BEDFORD, NH -	, <u>_</u>	QUANTITIES	S TD	6/13	CHECKED	MAB	6/13	X-A000(417)	20	91



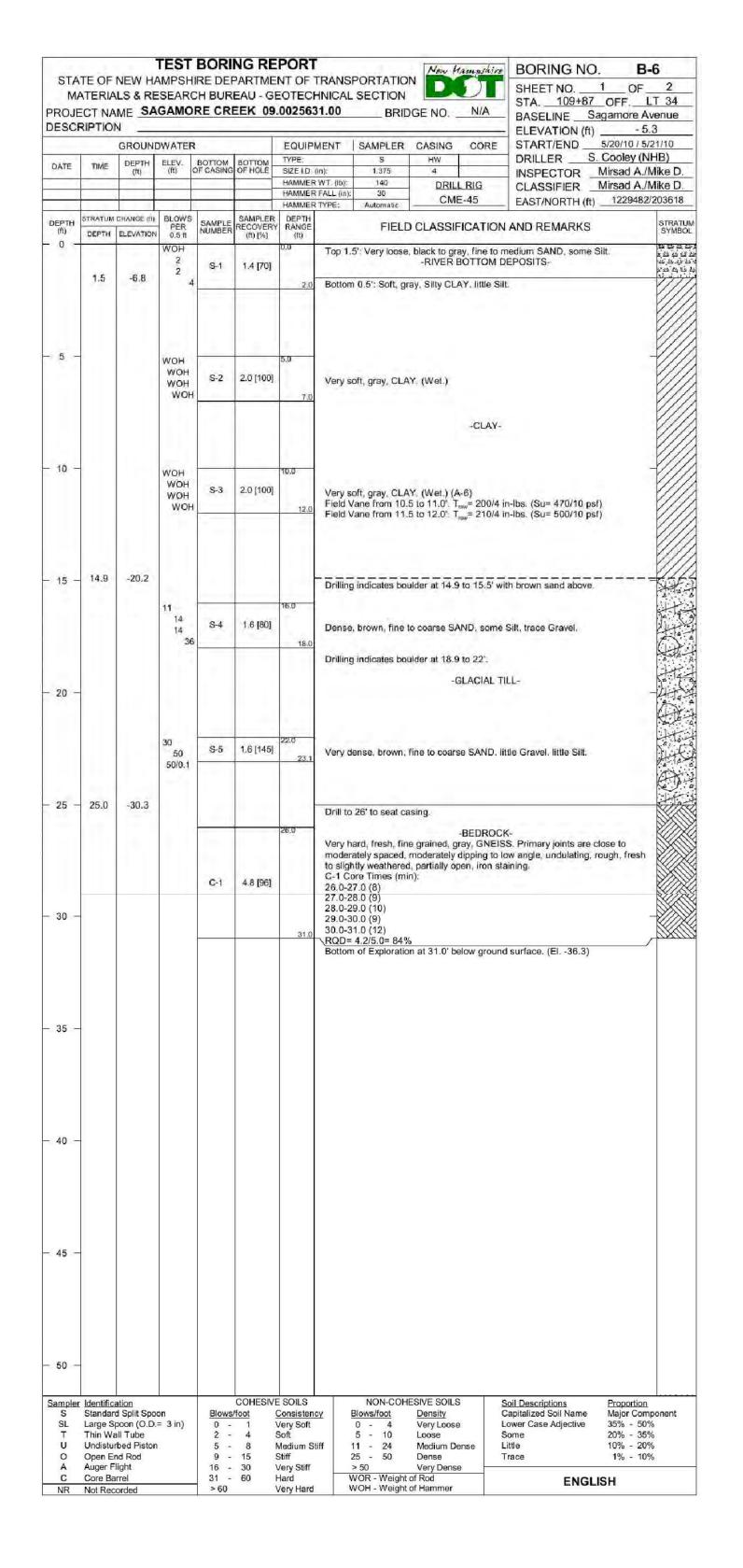
- 1. INFORMATION SHOWN FOR IN-WATER ACCESS WAS ASSUMED FOR PERMITTING PURPOSES.
- CONTRACTOR MAY CHANGE THE ACCESS CONFIGURATION SHOWN, BUT MUST SUBMIT PROPOSED CHANGES TO THE PERMITTING AGENCIES FOR APPROVAL PRIOR TO DISTURBING ANY PROTECTED RESOURCE.
- 3. MAINTAIN MINIMUM 50 FEET HORIZONTAL CLEARANCE NORMAL TO CHANNEL DURING CONSTRUCTION. SEE COAST GUARD PERMIT FOR ADDITIONAL REQUIREMENTS.
- 4. PROVIDE 10 FEET MINIMUM VERTICAL CLEARANCE TO BOTTOM OF TRESTLE FRAMING TO MHHW.
- 5. PLANS FOR TEMPORARY WORKS WITHIN THE LIMITS OF MHHW MUST BE SUBMITTED BY THE CONTRACTOR TO THE U.S. COAST GUARD DISTRICT COMMANDER AND BE APPROVED PRIOR TO THE START OF CONSTRUCTION.
- 6. TRESTLE WORK SHOWN PAID UNDER ITEM 500.02, ACCESS FOR BRIDGE CONSTRUCTION.

						TY OF PC PARTMENT OF				
		TOWN	PORTSMOUTH			BRIDGE NO.	198/	034 STATE PRO	JECT 1449	93
		LOCATION	I SAGAMORE	AVE.	& N.H.	ROUTE 1A O	VER SA	GAMORE CREEK		
			BRIDG	E C	ONST	RUCTION	ACC	CESS PLAN		BRIDGE SHEET
			ВҮ	DATE		ВҮ	DATE	REVISIONS AFTER PROP	OSAL DATE	07 of 41
	SHEET SCALE	DESIGNED	TD	5/13	CHECKED	MAB	5/13			FILE NUMBER
	AS NOTED	DRAWN	FLC	5/13	CHECKED	TD	5/13			
FAY, SPOFFORD & THORN	DIKE, INC	TRACED			CHECKED			FEDERAL PROJECT NO.	SHEET NO.	TOTAL SHEETS
- BEDFORD, NH -	,	QUANTITIES	TD	6/13	CHECKED	MAB	6/13	X-A000(417)	21	91





M/ PROJE	ATERIA	NEW HALS & RE	AMPSH SEAR	IIRE DEI CH BUR		NT OF T	TRANS	PORTATION SECTION BRID		N/A	SHEET NO. 1 OF STA. 109+87 OFF. BASELINE Sagamore	Avenue
JESUR	KIPTIO	GROUNI	OWATE	R		EQUIPM	MENT	SAMPLER	CASING	CORE	ELEVATION (ft)	5.3
DATE	TIME	DEPTH	ELEV.	воттом	воттом	TYPE:		S	HW		DRILLER S. Cooley (I	38 5 31 V 77 77 1
2.0.0	2.00	(ft)	(ft)	OF CASING	OF HOLE	SIZE I.D. (1.375	DRILL R	liG.	INSPECTOR Mirsad A. CLASSIFIER Mirsad A.	
						HAMMER HAMMER		30 Automatic	CME-4	0.70	OLI IOGII ILIT	7/203588
EPTH	STRATUM	CHANGE (R)	BLOWS	SAMPLE	SAMPLER	DEPTH	HFE,		La razzona			STRATUM
(ft)	DEPTH	ELEVATION	PER 0.5 ft	NUMBER	REGOVERY (ft) [%]	RANGE (ft)		FIELD	CLASSIFIC	ATION	AND REMARKS	SYMBOL
0 -			WOH/1	8" 5-1	0.0 [0]	0.0	M- D-	Calmian				1111
					0.0 [0]	2.0	No Re	covery.				1///
						2,0			7	-CLAY-		1///
												1///
												1///
						,						1///
5 -			4	1100	1417	5.0						1///
			2	S-2	1.5 [75]		Very s	oft, gray to bro	wn, Clayey SI	LT. (Wet.)	1///
			1	3		7.0						11/1
						1111						1///
												1///
												1///
10 -		-	2			10,0	Top 0.	9'; Gray to bro	wn, Silty CLAY	, trace fir	ne Sand. (Wet.)	-////
	10.9	-17.2	10 12	S-3	1.7 [85]		Bottom	1 1' Medium	dense brown	fine to c	coarse SAND, some Gravel, little	Ki d
			2	Ö		12.0	Silt. (V	Vet.)				4 4
								/ane from 10.5 /ane refusal a		620/200	in-lbs. (Su=1470/470 psf)	10-14
								Acceptage A				4
15 -			28			15.0						- 1
			19	S-4	0.8 [40]	,,,,,	67.					0 4
			17		0.0 [40]	17.0	Dense	, brown, fine t	o coarse Grave	ally SAND), little Silt. (Wet.)	1
						17:0			-GLA	ACIAL TIL	i.	4
												1
) leto
20	20.0	20.0										ar.
20 -	20.0	-26.3							-Bi	EDROCK	ž.	
						21.0					S. Primary joints are close to	
					100.11		to sligh	ntly weathered	partially open		w angle, undulating, rough, fresh ining.	
				C-1	4.1 [103]			ore Times (mir 2.0 (19)	1):			
				C 52	30.0		22.0-2	3.0 (10)				
							23.0-2	4.0 (8) 5.0 (10)				
25 -				C-2	0.7 [70]	25.0 25.0	RQD=	3.1/4.0= 78%		GNEICO	S. Primary joints are close to	
				0.2	5.7 [60]	26.0	moder	ately spaced,	moderately dip	ping to lo	w angle, undulating, rough, fresh	3//
							C-2 Co	ntly weathered ore Times (mir	, partially open			
							25.0-2	6.0 (11) 0.7/1.0= 70%	**			
		1								w ground	surface. (El32.3)	t l
ampler	Identific	ation			COHESIVE	SOILS		NON-COHE	SIVE SOILS	Sc	oil Descriptions Proportion	1
S SL	Standar	d Split Spo poon (O.D.		Blows 0 -		Consistence Very Soft	Y !	Blows/foot 0 - 4	Density Very Loose	Ca	apitalized Soil Name Major Cor ower Case Adjective 35% - 50	nponent
T	Thin Wa	all Tube		2 -	4 3	Soft		5 - 10	Loose	So	ome 20% - 35	5%
0	Undistu Open E	rbed Piston nd Rod	1	5 -		Medium St Stiff	V	11 - 24 25 - 50	Medium Dens Dense		ttle 10% - 20 race 1% - 10	
Α	Auger F	light		16 -	30	Very Stiff	1113	> 50 VOR - Weight	Very Dense	1		A. S. C.
C NR	Core Ba			31 - > 60		Hard Very Hard		VOR - Weight			ENGLISH	



NOTE: SEE SHEET 8 FOR BORING LOCATION PLAN.

− EL. −28.3

-30.1

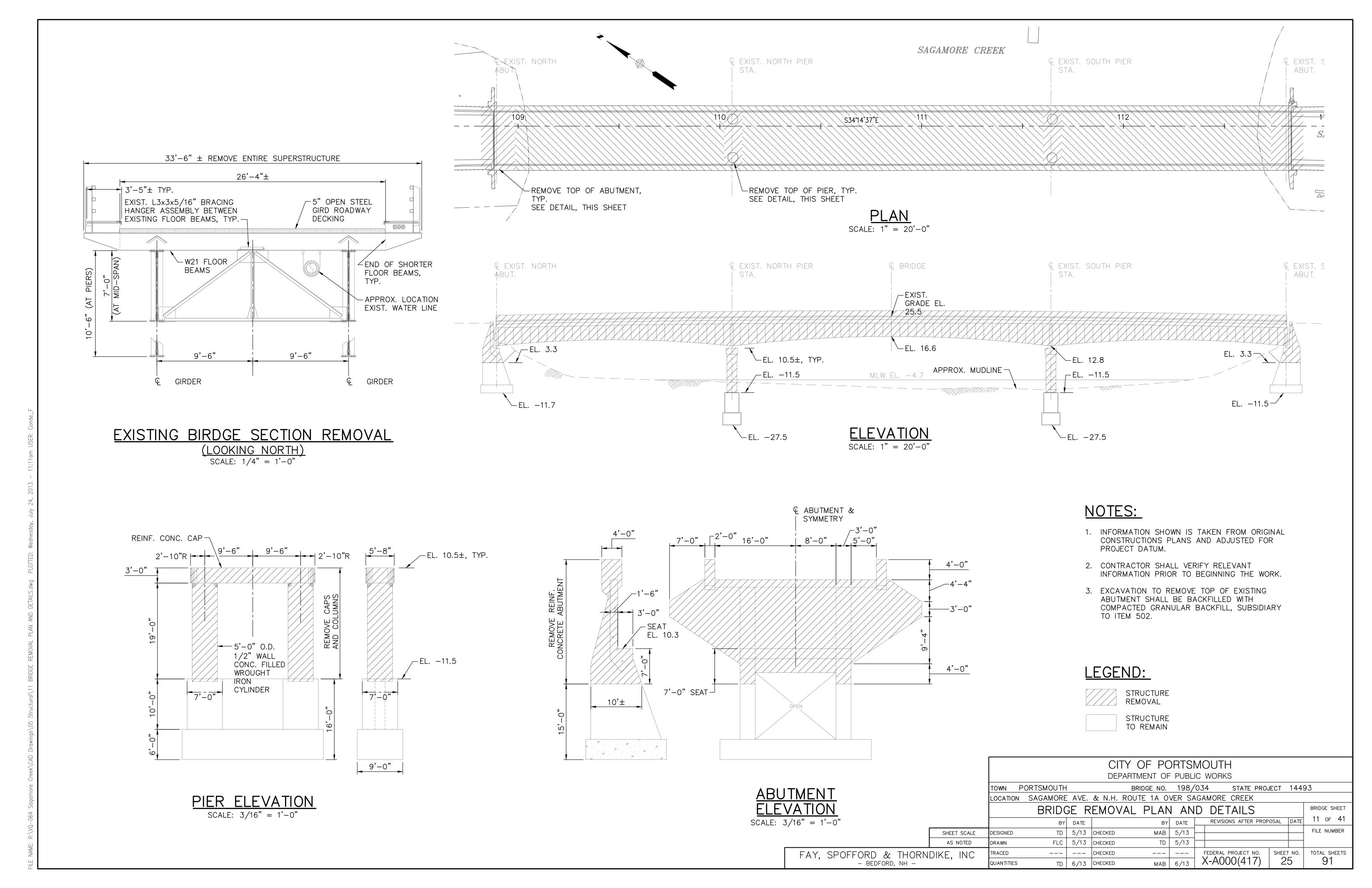
SHEET 8 FOR BORING LOCATION PLA	N.				_	OF PC	_				
		TOWN P	ORTSMOUTH			BRIDGE NO.	198/	034 STATE PRO	JECT 14	493	
		LOCATION	SAGAMORE	AVE.	& N.H. RC	OUTE 1A O	VER SA	GAMORE CREEK			
			BORING LOGS - SHEET 2 OF 3								
			BY	DATE		ВҮ	DATE	REVISIONS AFTER PROF	OSAL DA	-E 09 of 41	
	SHEET SCALE	DESIGNED	TD	5/13	CHECKED	MAB	5/13			FILE NUMBER	
	AS NOTED	DRAWN	FLC	5/13	CHECKED	TD	5/13				
FAY, SPOFFORD & THORN	DIKE. INC	TRACED			CHECKED			FEDERAL PROJECT NO.	SHEET NO		
- BEDFORD, NH -	,_	QUANTITIES	TD	6/13	CHECKED	МАВ	6/13	X-A000(417)	23	91	

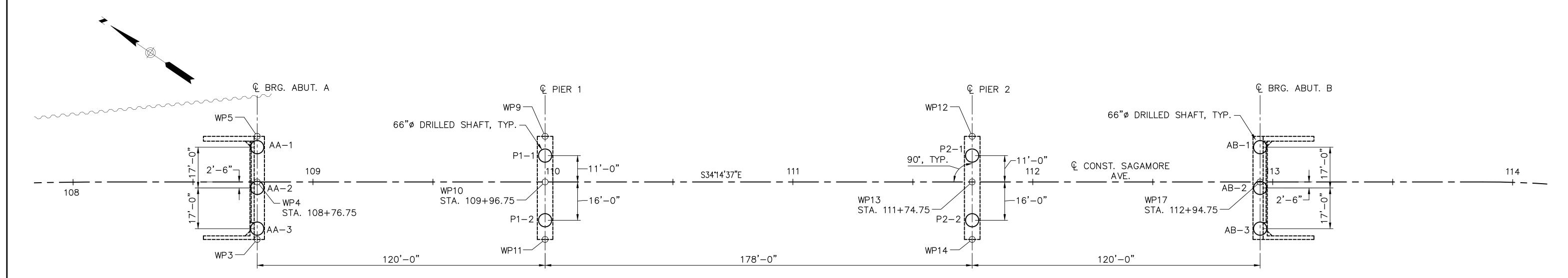
DATE	TIME	GROUNI DEPTH (ft)		BOTTOM OF CASING	BOTTOM OF HOLE		S	
DEPTH .		CHANGE (ff)	PER	SAMPLE NUMBER	SAMPLER RECOVERY	HAMMER DEPTH RANGE		
(ft) 0		ELEVATION	0.5 ft	NUMBER	(ft) [%]	(ft)	Asphalt	
	0.9	22.1	15 8	S-1	1.2 [60]	1.0	Medium dense, brown, fine to coarse SAND, some Gravel, trace Silt. (Dry.)	
	3.0	20.0	12		722 [99]	3.0	-FILL-	
5 -			9 8	S-2	0.2 [15]	5.0	Gray/brown, GRAVEL, little Sand, trace Silt, angular. (Dry.)	
			100/0.3			6.3	Drill cuttings indicate presence of brown sand.	
							-ROCK FILL-	
10 -								
10			12 7 18	S-3	0.3 [15]	10.0	Gray/brown, GRAVEL, trace Sand, angular. Tip plugged with piece of gravel.	
			31			12.0	(Moist.)	
							Drill cutting indicate presence of brown sand.	
15 -								
	15.7	7.3	7 7			16.0	Drilled to 16' to penetrate boulder/cobble.	
			7 6	S-4	1.2 [60]	18.0	Medium dense, brown, fine to coarse SAND, some Gravel, little Silt. (Moist.) (A-1-b)	
20 -			8 14	17.4		20.0	Dense, brown, fine to coarse SAND, some Gravel, little Silt, mottled. (Wet.)	
			20	S-5	0.4 [20]	22.0	.Fu L	
							ν	
25 -			7 10	S-6	0.8 [40]	25.0	Medium dense, brown/gray, Sandy SILT, little Gravel. Sulfur odor. (Wet.)	
			11			27.0	$\circ \circ $	
	28.0	-5.0					L	
30 -			24		П	30.0	Change in drilling resistance and wash color. ABCT ABCT ABCT ABCT ABCT ABCT ABCT ABC	
			22 28	S-7	0.2 [10]	2200303	CLACIAL TILL	-8.0
			51			32.0		
35 —								
			43 53	S-8	1.2 [60]	36.0	Drill to 36' to penetrate cobble/boulder. Very dense, brown/gray, fine to coarse SAND and GRAVEL, little Silt.	
			50 78			38.0	(A-1-b)	
name to 11	39.5	-16.5				1-		
40 —					li .	40.0	Very hard, slightly weathered, fine grained, gray, GNEISS. Joints are close to very close, moderately dipping to high angle, undulating, rough, slightly weathered, partially open, iron staining.	
				C-1	2.5 [50]		Core Times (mins): 40-41 (7)	-19.2
					2.5 [50]		41-42 (6.5) 42-43 (8) 43-44 (5)	
45 —						45.0 45.0	44-45 (6) RQD= 0/5.0= 0% -BEDROCK-	
							Very hard fresh fine grained gray CNEISS Primary lainte are year close to	
				C-2	5.0 [100]		Very hard, fresh, fine grained, gray, GNEISS. Primary joints are very close to moderate, low angle, undulating to stepped, rough, fresh to slightly weathered, partially open, iron staining.	
							Secondary joints are similar but high angle. RQD= 1.4/5.0 =28%	
50 —						50.0	Bottom of Exploration at 50' below ground surface. (El27.0)	
ampler	Identifica	ition I Split Spo		Blows	COHESIVE	SOILS Consistence	NON-COHESIVE SOILS Soil Descriptions Proportion acy Blows/foot Density Capitalized Soil Name Major Component	

DESC	RIPTIO	GROUNE	WATER			EQUIP	MENT I	SAMPLER	CASING	CORE	BASELINE Sagamore Av ELEVATION (ft) 23.0 START/END /
DATE	TIME	DEPTH (#)	ELEV.	BOTTOM OF CASING	BOTTOM OF HOLE	TYPE: SIZE LD: HAMMER	(in): : WT. (ib):	S 1.375 140	HW 4	L RIG	DRILLER K. Smith (NH INSPECTOR Eric Bar CLASSIFIER Eric Bar
W	STRATUM	CHANGE (ff)	BLOWS	V 3 3 1 1	SAMPLER	HAMMER HAMMER DEPTH	FALL (in): TYPE:	30 Automatic	Mobil	e B-59	EAST/NORTH (ft)1229393/2
DEPTH (ft) - 0 -		ELEVATION	PER 0.5 ft	SAMPLE NUMBER	DECOMERS	RANGE (ft)	Asphalt	- 2757	CLASSI	FICATION	AND REMARKS
	8.0	22.2	17	-		1.0	Top 0.8	: Dense, bro			and, trace Silt. SAND, little Silt, little Gravel.
			18 11 16	S-1	1,6 [80]	3.0	(Moist.)		Di Gwii, iirie		ozare, mae one, mae oraver.
										-FILL-	
- 5 -	5.0	18.0	12	30	255 726	5.0					
			36 67	S-2	0.1 [5]	7,0		RAVEL piec indicates col			
- 10 -			A.								
10			49 23 17	S-3	0.1 [5]	10.0	Gray, G	RAVEL piec	es, angular		
			17	-		12.0				-ROCK FILL	
- 15 -	_		7 8			15.0					
			14	S-4	0.2 [10]	17.0	Medium Gravel i	dense, brov n tip. (Moist.	vn, fine to r .)	nedium SAN	D, some Gravel, trace Silt.
							Drilling i	indicates bro	own sand at	approximate	ely 18' below ground surface.
200											
- 20 -			5 6 8	S-5	0.3 [15]	20.0	Medium	dense, brov	wn/gray, Gr	avelly SAND	, little Silt. (Wet.)
			4			22.0					
								tings show o round surfac		ark brown, fi	ne Sand at approximately 23'
- 25 -	25.3	-2.3	10			25.0				, some Grav	
	2.72		8 17 13	S-6	1.3 [65]	27,0	Silt. (We				rse SAND, some Gravel, trace
									9	GLACIAL TII	LLS
**											
- 30 -			8 12 100/0.3	S-7	1.2 [92]	30.0	Brown,	fine to coars		ome Gravel, GLACIAL TII	
			.500.0						7	111	100 K
	33.5	-10.5		r	44	34.0					S. Joint is low angle, undulating,
- 35 -				C-1	1.0 [100]	35.0 35.0	Core Tir 34-35 (9			staining.	
				C-2	4.0 [100]		Very ha		e grained, g		S. Primary joints are close to
				5-2			slightly s similar,	weathered, t moderate, h	ight to parti igh angle to	ally open, iro	undulating, rough, fresh to n staining. Secondary joints are
gro-zav					N .	39.0	72.000 P. C.	2.6/4.0= 65% of Exploration	97 WASSESS OF THE	ow ground s	urface. (El16.0)
- 40 -											
- 45 -											
- 50 -											
Sample: S	r Identifica Standar	ation d Split Spo	on	Blows	COHESIVE	SOILS Consisten	cy BI	NON-COH ows/foot	ESIVE SOIL Density		oil Descriptions Proportion apitalized Soil Name Major Comp

NOTE: SEE SHE

SHEET 8 FOR BORING LOCATION	PLAN.					TY OF POPARTMENT OF				
		TOWN F	ORTSMOUTH			BRIDGE NO.	198/	034 STATE PRO	JECT 144	93
		LOCATION	SAGAMORE	AVE.	& N.H.	ROUTE 1A O	VER SA	AGAMORE CREEK		
			ВС		BRIDGE SHEET					
			BY	DATE		BY	DATE	REVISIONS AFTER PROF	POSAL DATE	10 of 41
[SHEET SCALE	DESIGNED	TD	5/13	CHECKED	MAB	5/13			FILE NUMBER
	AS NOTED	DRAWN	FLC	5/13	CHECKED	TD	5/13			-
AY, SPOFFORD & THORNI	DIKE. INC	TRACED			CHECKED			FEDERAL PROJECT NO.	SHEET NO.	TOTAL SHEETS
- BEDFORD, NH -	•	QUANTITIES	TD	6/13	CHECKED	МАВ	6/13	X-A000(417)	24	91

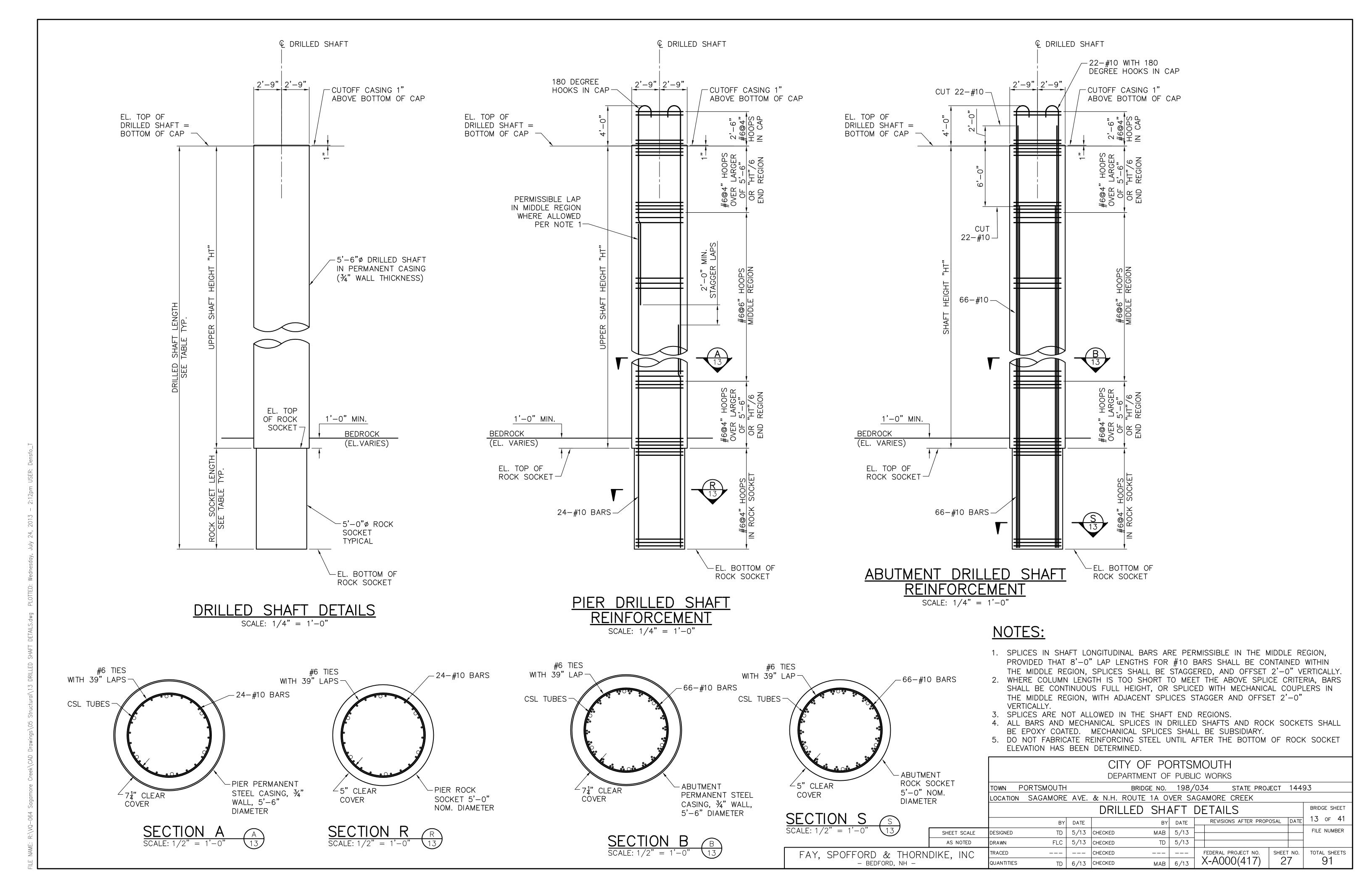


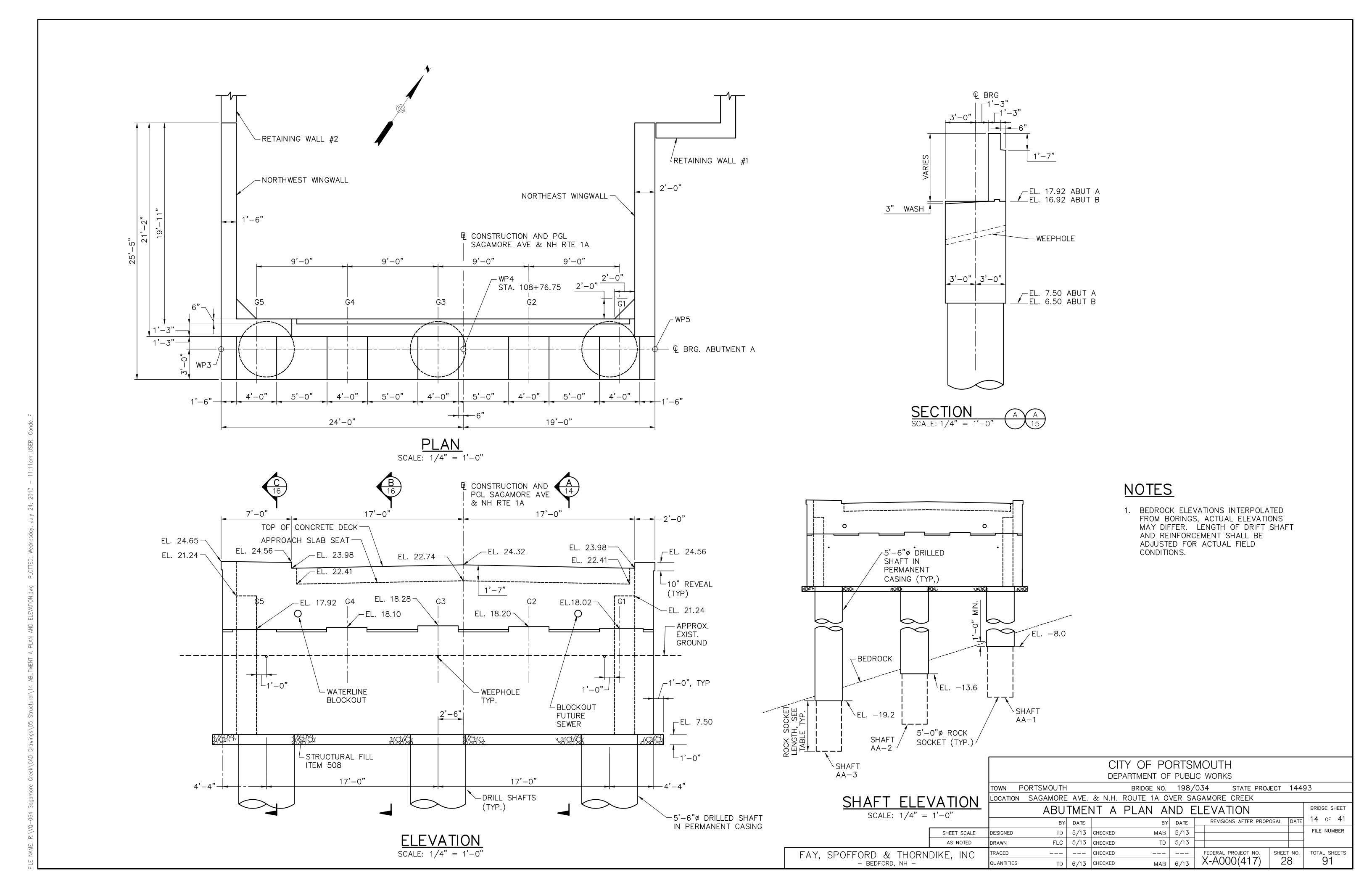


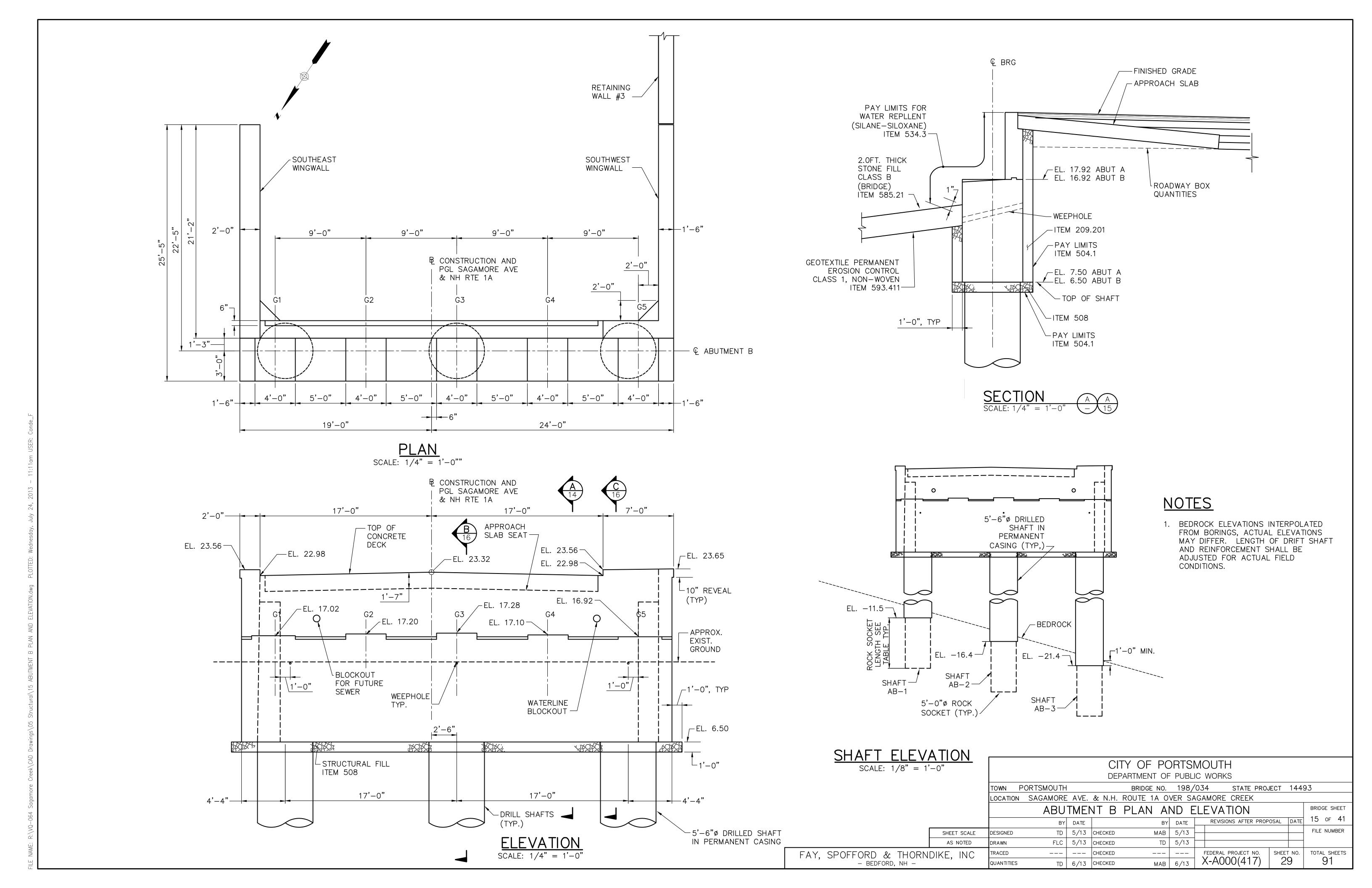
DRILLED SHAFT LAYOUT PLAN SCALE: 1" = 20'

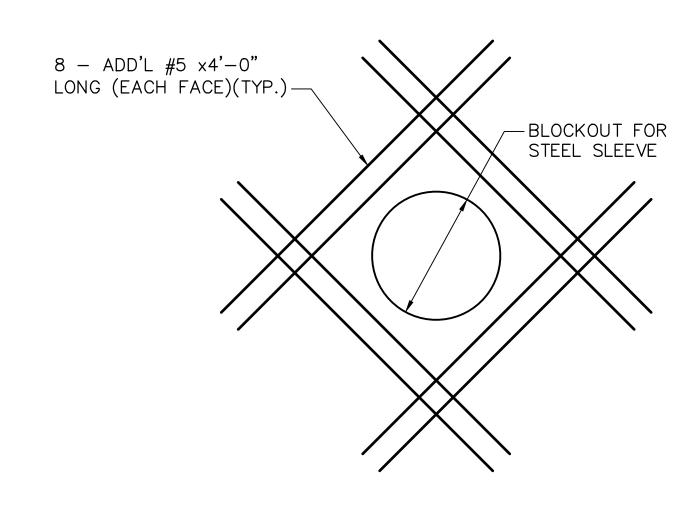
	DRILLED SHAFT SUMMARY TABLE														
SHAFT	SHAFT DIA. (IN)	ROCK SOCKET DIA. (IN)	TOP OF SHAFT ELEV. (FT)	EST. TOP OF ROCK SOCKET ELEV. (FT)	BOTTOM OF ROCK SOCKET ELEV. (FT)	EST. ROCK SOCKET LENGTH (FT)	EST. UPPER SHAFT LENGTH (FT)	DRILLED SHAFT LENGTH (FT)	VERTICAL BARS, NO. AND SIZE	SERVICE I AXIAL LOAD (K)	STRENGTH I AXIAL LOAD (K)				
AA-1	66	60	7.50	-8.00	-20.00	12.0	15.50	27.50	66-#10	700	920				
AA-2	66	60	7.50	-13.60	-25.60	12.0	21.10	33.10	66-#10	700	920				
AA-3	66	60	7.50	-19.20	-31.20	12.0	26.70	38.70	66-#10	700	920				
P1-1	66	60	8.41	-30.10	-42.10	12.0	38.51	50.51	24-#10	1700	2230				
P1-2	66	60	8.41	-28.50	-40.50	12.0	36.91	48.91	24-#10	1700	2230				
P2-1	66	60	8.21	-39.70	-51.70	12.0	47.91	59.91	24-#10	1700	2230				
P2-2	66	60	8.21	-44.50	-56.50	12.0	52.71	64.71	24-#10	1700	2230				
AB-1	66	60	6.5	-11.50	-23.50	12.0	18.00	30.00	66-#10	700	920				
AB-2	66	60	6.5	-16.40	-28.50	12.0	22.90	34.90	66-#10	700	920				
AB-3	66	60	6.5	-21.40	-33.40	12.0	27.90	39.90	66-#10	700	920				

					CIT	Y OF PC	PRTSI	MOUTH						
					DEP	ARTMENT OF	PUBLI	C WORKS						
		TOWN	PORTSMOUTH			BRIDGE NO.	198/	034 STATE PRO	JECT 144	93				
		LOCATION	ATION SAGAMORE AVE. & N.H. ROUTE 1A OVER SAGAMORE CREEK											
				DR	RILLED	SHAFT	LAY(DUT		BRIDGE SHEET				
			ВҮ	DATE		BY	DATE	REVISIONS AFTER PROP	OSAL DATE	12 of 41				
	SHEET SCALE	DESIGNED	TD	5/13	CHECKED	MAB	5/13			FILE NUMBER				
	AS NOTED	DRAWN	FLC	5/13	CHECKED	TD	5/13							
FAY, SPOFFORD & THORN	DIKE. INC	TRACED			CHECKED			FEDERAL PROJECT NO.	SHEET NO.	TOTAL SHEETS				
- BEDFORD, NH -	<u>_</u> ,	QUANTITIES	S TD	6/13	CHECKED	МАВ	6/13	X-A000(417)	26	91				

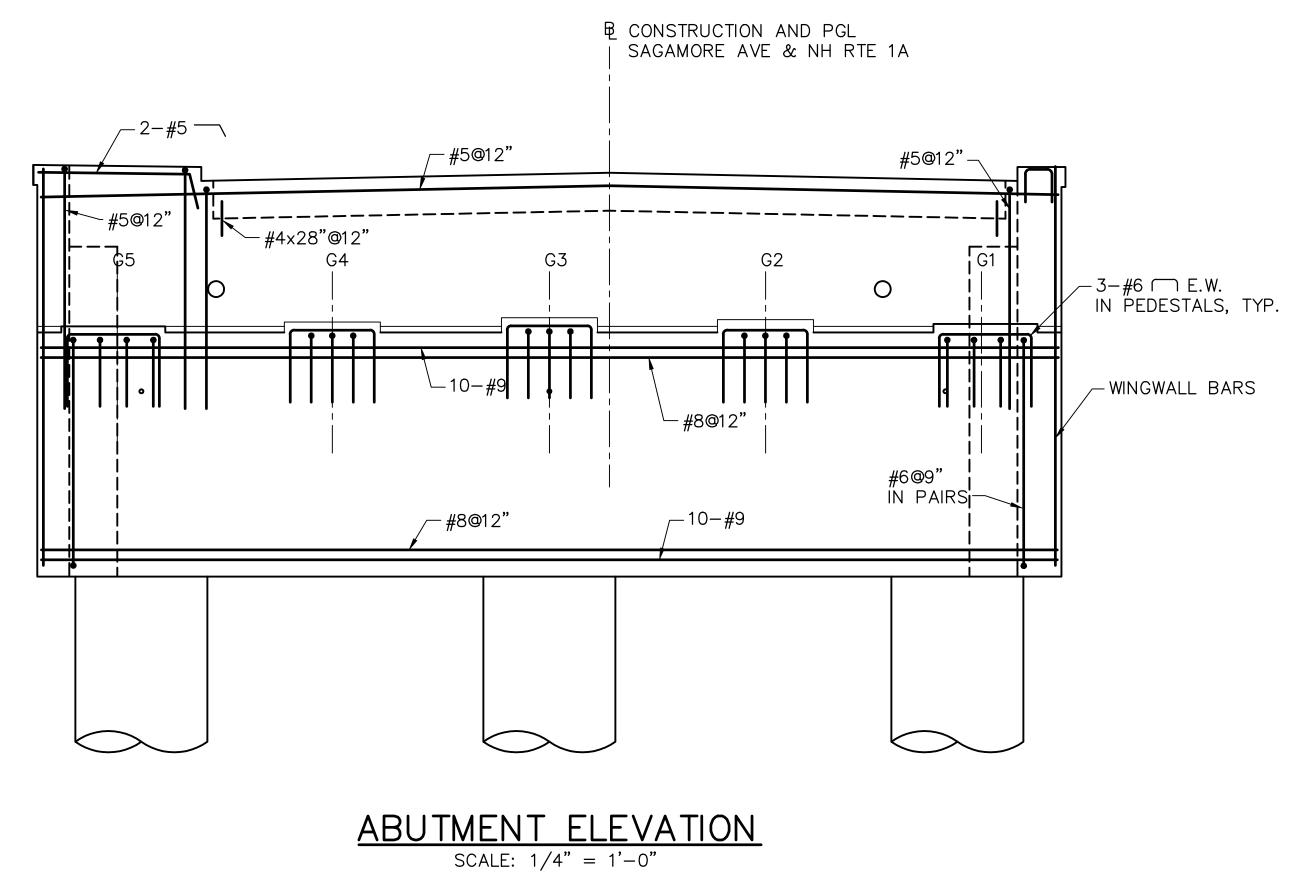








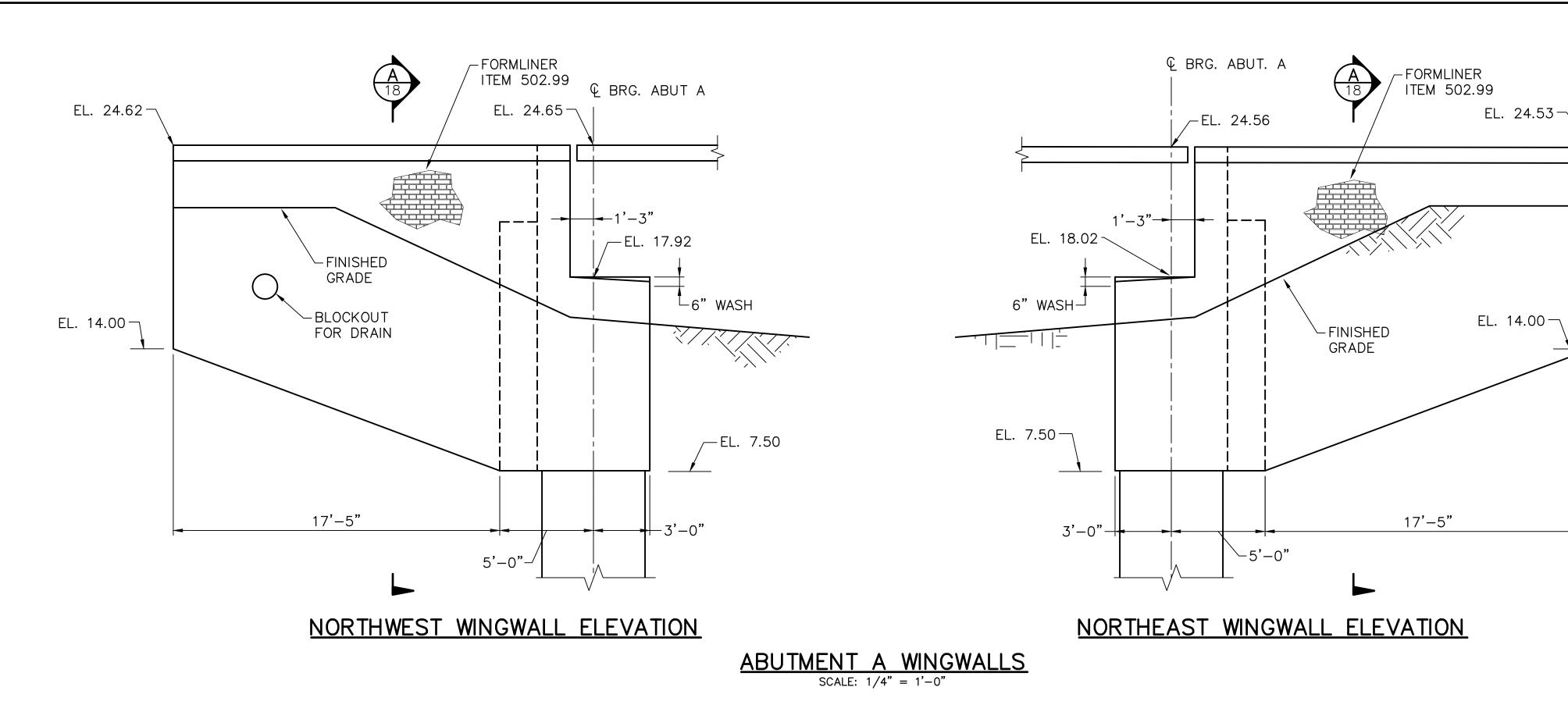
UTILITY BLOCK OUT DETAIL SCALE: 1" = 1'-0"

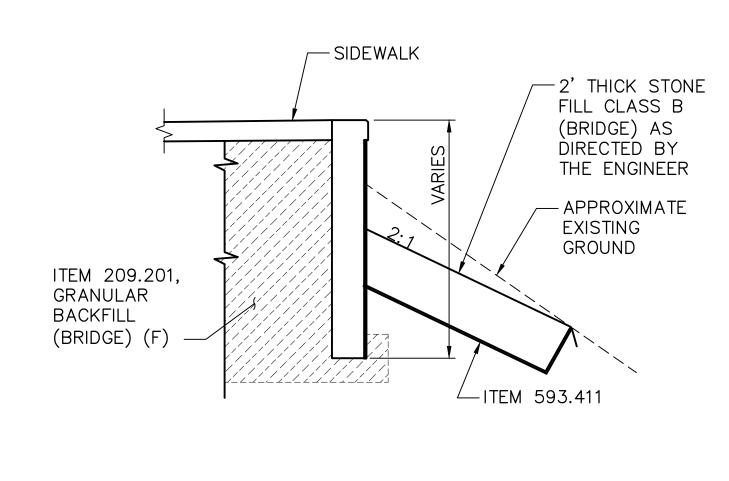


CITY OF PORTSMOUTH DEPARTMENT OF PUBLIC WORKS STATE PROJECT 14493 TOWN PORTSMOUTH BRIDGE NO. 198/034 LOCATION SAGAMORE AVE. & N.H. ROUTE 1A OVER SAGAMORE CREEK

ABUTMENT SECTIONS AND DETAILS BRIDGE SHEET 16 of 41 REVISIONS AFTER PROPOSAL DATE BY DATE BY DATE FILE NUMBER TD 5/13 CHECKED MAB 5/13 SHEET SCALE DESIGNED FLC 5/13 CHECKED TD 5/13 AS NOTED FEDERAL PROJECT NO. SHEET NO. TOTAL SHEETS X-A000(417) 30 91FAY, SPOFFORD & THORNDIKE, INC - BEDFORD, NH ---- CHECKED MAB 6/13

TD 6/13 CHECKED

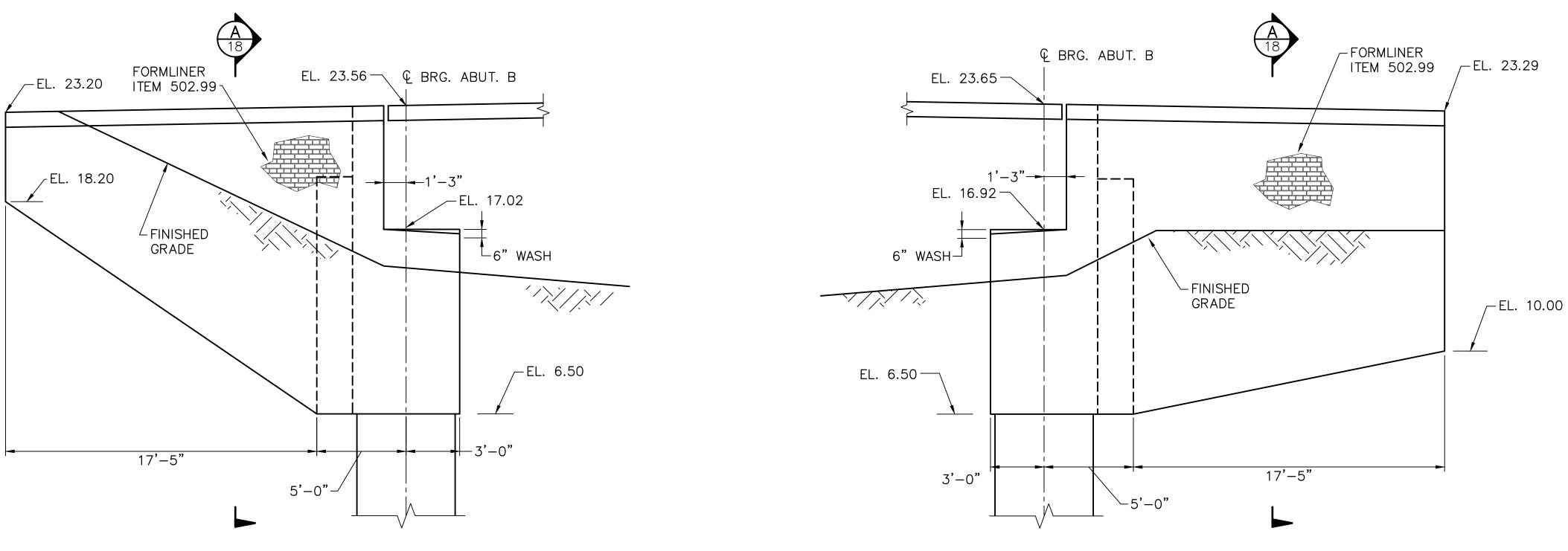




SECTION - WINGWALL

SCALE: 1/4" = 1'-0"

18



SOUTHWEST WINGWALL ELEVATION

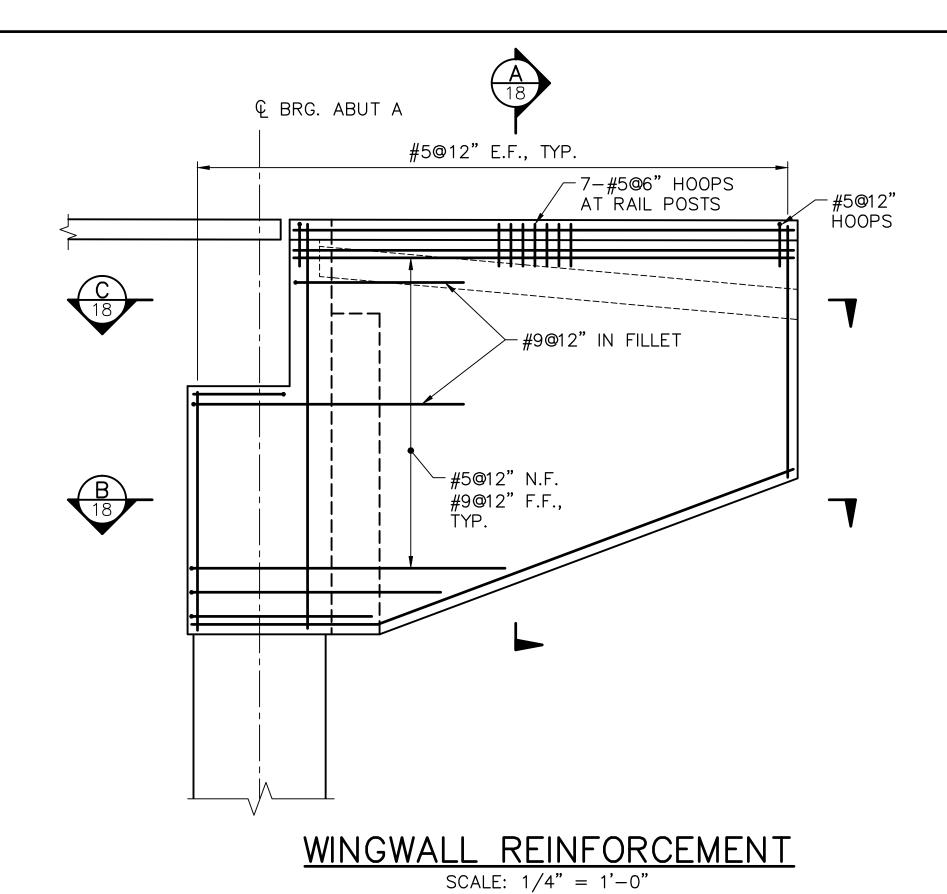
ABUTMENT B WINGWALLS

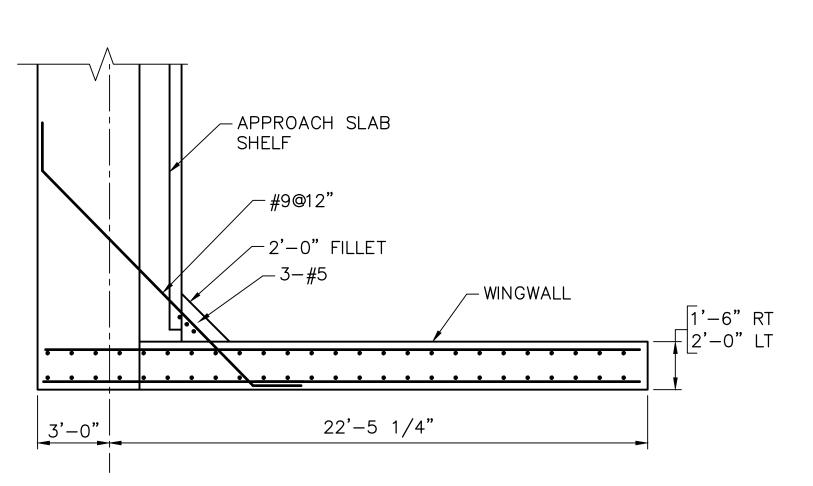
SCALE: 1/4" = 1'-0"

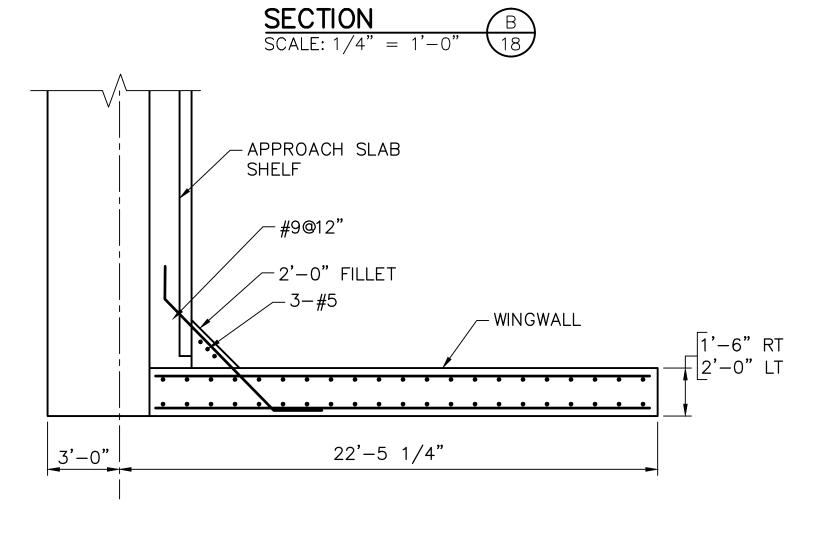
SOUTHEAST WINGWALL ELEVATION

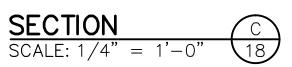
						Y OF PC ARTMENT OF								
		TOWN	PORTSMOUTH			BRIDGE NO.	198/	034 STATE PRO	JECT 1449	93				
		LOCATIO	ON SAGAMORE AVE. & N.H. ROUTE 1A OVER SAGAMORE CREEK											
			WINGWALL ELEVATIONS BRIDGE SHEET											
			BY	DATE		BY	DATE	REVISIONS AFTER PROI	POSAL DATE	17 of 41				
	SHEET SCALE	DESIGNED	TD	5/13	CHECKED	MAB	5/13			FILE NUMBER				
	AS NOTED	DRAWN	FLC	5/13	CHECKED	TD	5/13							
FAY, SPOFFORD & THORN	DIKE. INC	TRACED	CHECKED FEDERAL PROJECT NO. SHEET NO. TOTAL SHEETS											
- BEDFORD, NH -	_,	QUANTITIES	S TD	6/13	CHECKED	MAB	6/13	X-A000(417)	31	91				

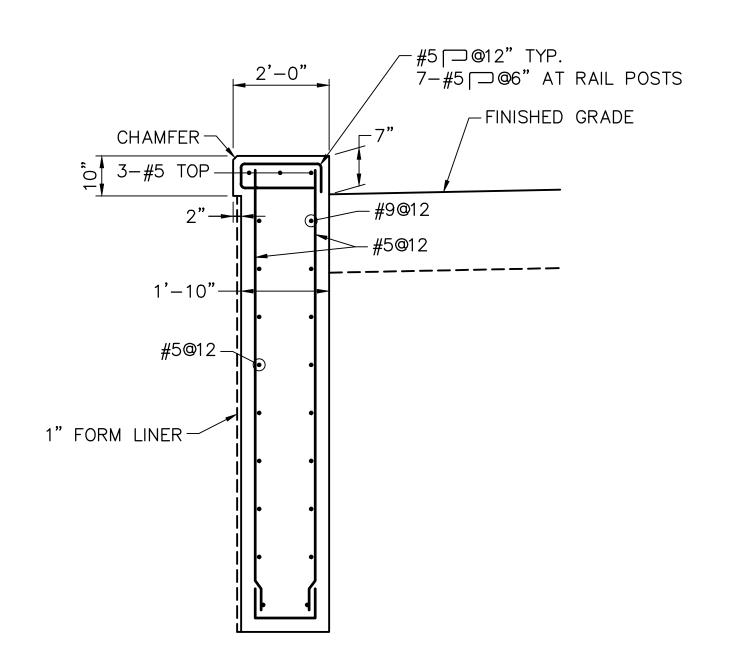
.ME: R:\VQ-064 Sagamore Creek\CAD Drawings\05 Structural\17 WINGWALL ELEVATIONS.dwg PLOTTED: Wednesday, July 24, 2013 - 11:12am USER: Conde_F





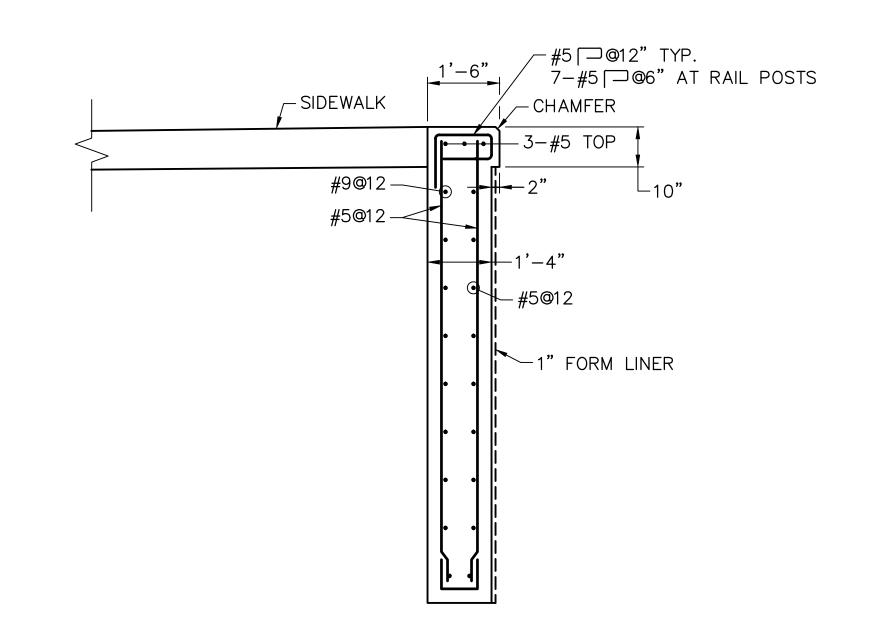






TYPICAL SECTION A
(AT LEFT WINGWALLS)

SCALE: ½" = 1'-0"



TYPICAL SECTION A
AT RIGHT WINGWALLS)

SCALE: ½" = 1'-0"

CITY OF PORTSMOUTH

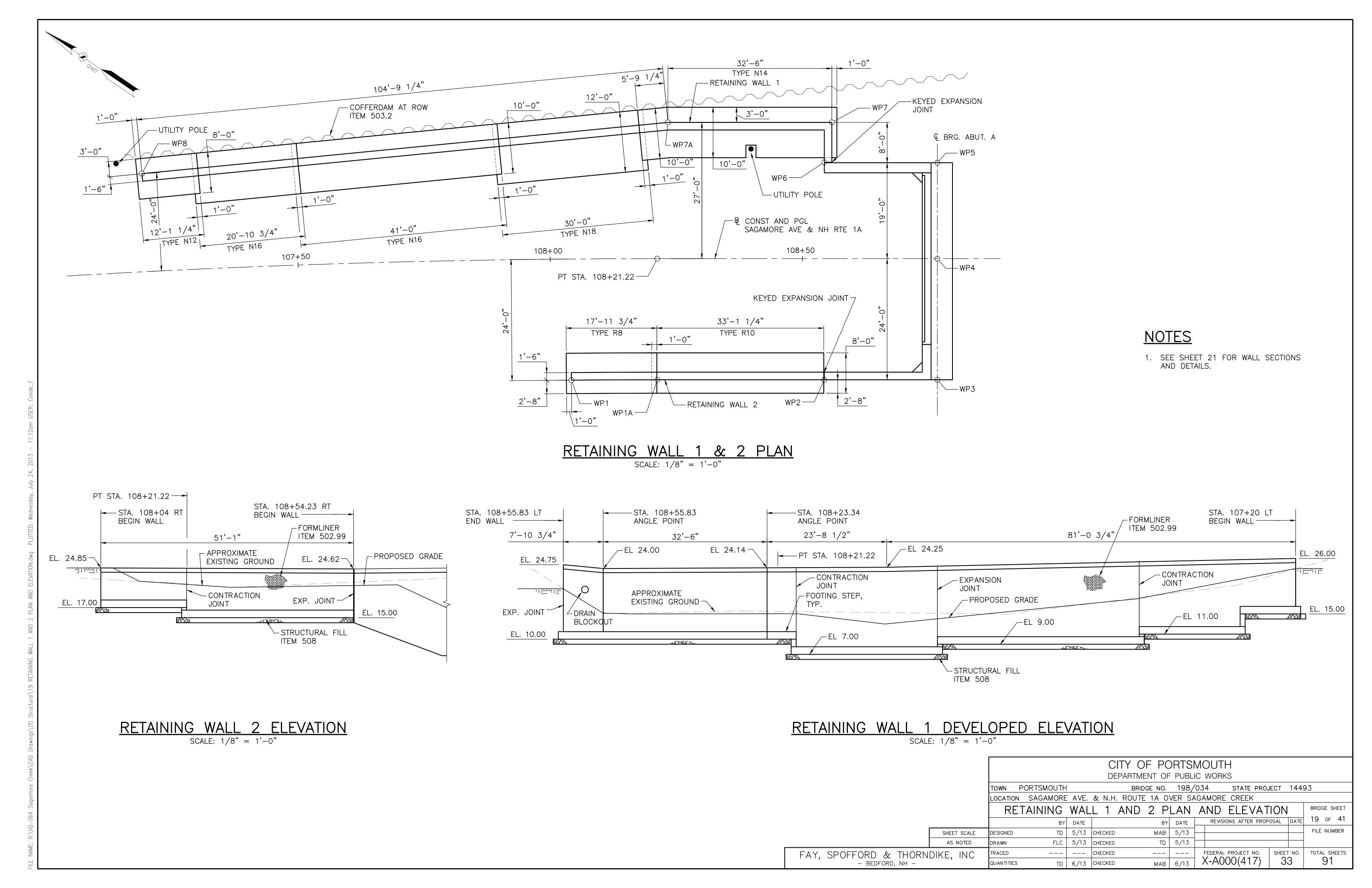
DEPARTMENT OF PUBLIC WORKS

TOWN PORTSMOUTH

BRIDGE NO. 198/034 STATE PROJECT 14493

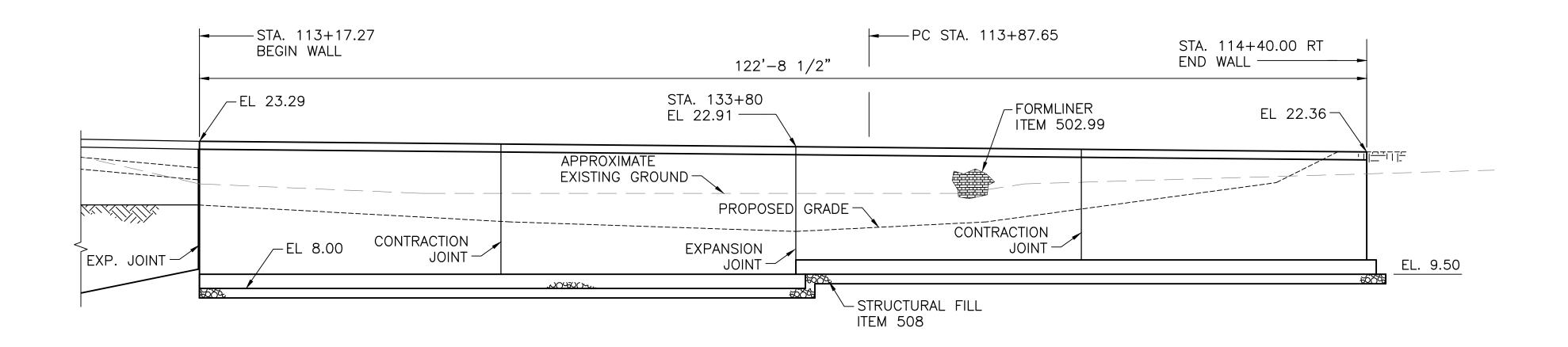
LOCATION SAGAMORE AVE. & N.H. ROUTE 1A OVER SAGAMORE CREEK

WINGWALL DETAILS BRIDGE SHEET REVISIONS AFTER PROPOSAL DATE 18 of 41 BY DATE BY DATE FILE NUMBER MAB 5/13 TD 5/13 CHECKED SHEET SCALE DESIGNED AS NOTED FLC 5/13 CHECKED TD 5/13 --- --- FEDERAL PROJECT NO. SHEET NO. TOTAL SHEETS Y-A000(417) 32 91 FAY, SPOFFORD & THORNDIKE, INC
- BEDFORD, NH ---- CHECKED TD 6/13 CHECKED



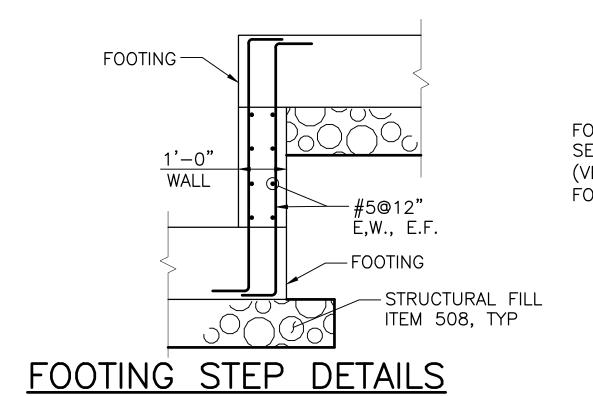
<u>NOTES</u>

 SEE SHEET 21 FOR WALL SECTIONS AND DETAILS.



RETAINING WALL 3 ELEVATION SCALE: 1/8" = 1'-0"

					_	Y OF PC artment of	_						
		TOWN	PORTSMOUTH		DEI 7	BRIDGE NO.	198/		JECT 144	-93			
		LOCATION	N SAGAMORE	AVE.	& N.H. F	ROUTE 1A OV	/ER SA	GAMORE CREEK					
			RETAINING WALL 3 PLAN AND ELEVATION BRIDGE SHEET										
			BY	DATE		ВҮ	DATE	REVISIONS AFTER PROP	OSAL DATE	20 of 41			
	SHEET SCALE	DESIGNED	TD	5/13	CHECKED	MAB	5/13			FILE NUMBER			
	AS NOTED	DRAWN	FLC	5/13	CHECKED	TD	5/13						
FAY, SPOFFORD & THORNI	DIKE. INC	TRACED			CHECKED			FEDERAL PROJECT NO.	SHEET NO.	TOTAL SHEETS			
- BEDFORD, NH -	•	QUANTITIES	TD	6/13	CHECKED	MAB	6/13	X-A000(417)	34	91			



VERTICAL SECTION ~ SCALE: NONE

40¢ LAP, TYP. FOR SIZE & REINF SEE SCHED OR DETAILS (VERT REINF NOT SHOWN FOR CLARITY) -CORNER

> CORNER REINFORCEMENT PLAN ~ SCALE: NONE

ONE CUBIC FOOT GRANULAR MATERIAL ENCLOSED IN FILTER FABRIC AT 2"Ø SLEEVE RETAINING WALL DESIGN DATA: MAX FACTORED BEARING RESISTANCE = 6,000 psf FACE OF WALL -2"ø PVC SLEEVE SLOPE 1/2" - PROVIDE PAIR OF #4 X 18" - TOP OF FINISHED GRADE

SCALE: NONE

FOOTING/SOIL FRICTION COEFFICIENT = 0.4FLAT BACKSLOPE, 2 FT. SURCHARGE ACTIVE EQUIVALENT FLUID PRESSURE = 36 pcf RAIL LOAD (WHERE PRESENT) = 10 KIPS
PASSIVE EQUIVALENT FLUID PRESSURE = 350 pcf SOIL UNIT WEIGHT = 125pcf

LIMITS OF ITEM 504.1 COMMON BRIDGE

1'-0" (TYP.)

1'-0" -PROPOSED GRADE **EXCAVATION**

-PAY LIMITS FOR WATER REPELLENT

ÎTEM 534.3

(SILANE-SILOXANE)

— APPROXIMATE

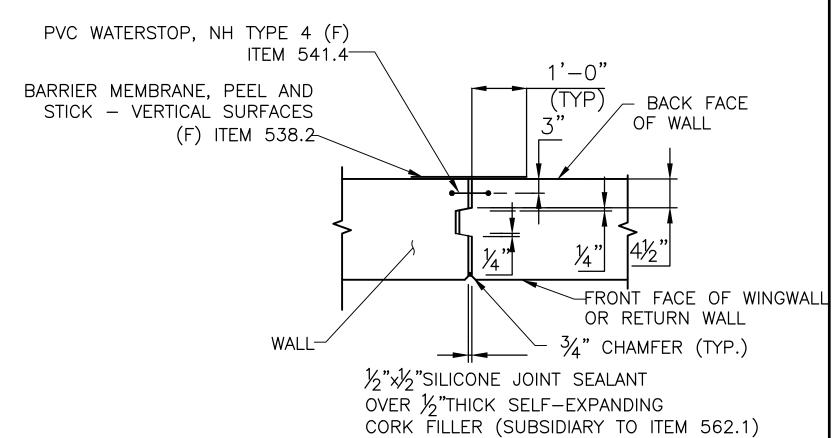
EXISTING GROUND

RETAINING WALL DATA

ITEM 505

-STRUCTURAL FILL

SCALE: 1/2" = 1'-0"



EXPANSION JOINT DETAILS SCALE: NONE

-BARRIER MEMBRANE, PEEL AND STICK -VERTICAL SURFACES (F) ITEM 538.2 BACK FACE OF WALL — -PVC WATERSTOP, NH TYPE 1 (F) ITEM 541.1 BACK FACE OF WALL FRONT FACE $\frac{3}{4}$ " CHAMFER (TYP.)_ OF WALL

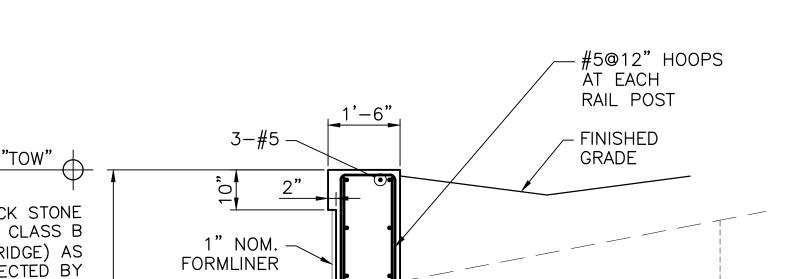
CONTRACTION JOINT DETAILS SCALE: NONE

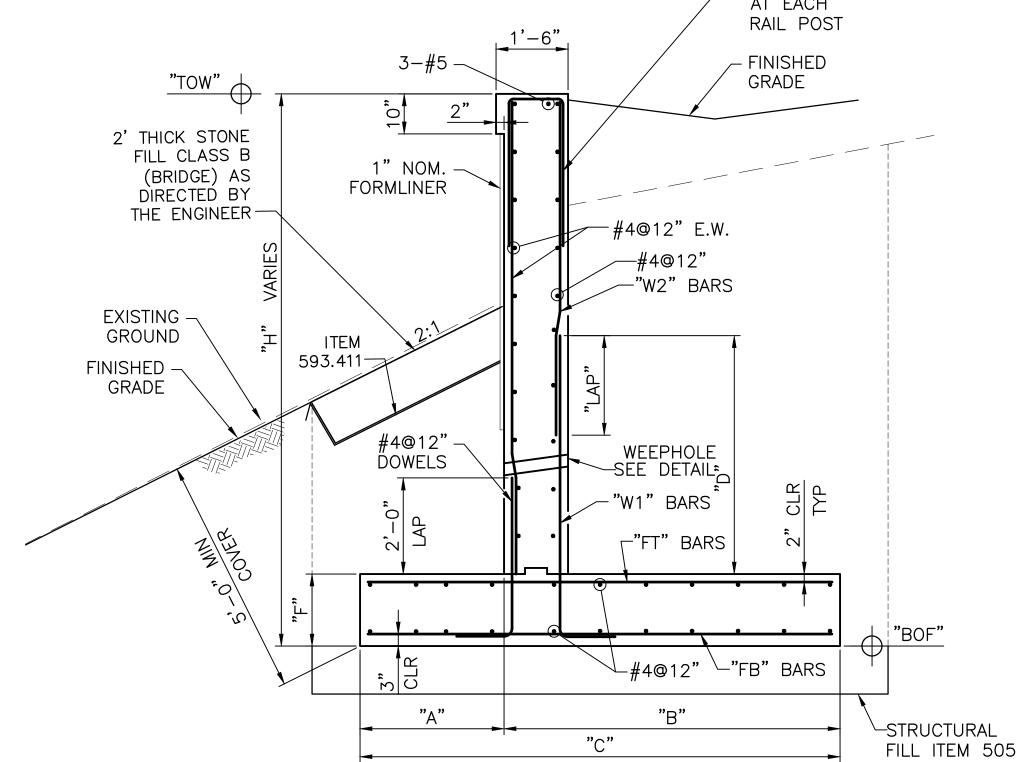
CITY OF PORTSMOUTH DEPARTMENT OF PUBLIC WORKS

STATE PROJECT 14493 TOWN PORTSMOUTH BRIDGE NO. 198/034 LOCATION SAGAMORE AVE. & N.H. ROUTE 1A OVER SAGAMORE CREEK

RETAINING WALL DETAILS AND REINFORCING BRIDGE SHEET 21 of 41 REVISIONS AFTER PROPOSAL DATE BY DATE BY DATE FILE NUMBER TD 5/13 CHECKED MAB 5/13 DESIGNED FLC 5/13 CHECKED TD 5/13 --- CHECKED

WEEPHOLE DETAILS





RETAINING WALL DATA (WALL #1, WITHOUT RAIL)

				`	.,				•						
WALL	"H"MAX	"A"	"B"	"C"	"F"	"W	1" BA	RS	"∖	V2" (BARS	"FT"	BARS	"FB"	BARS
TYPE	(FT.)	(FT.)	(FT.)	(FT.)	(FT.)	SIZE	SPA.	"D"	SIZE	SPA.	"LAP"	SIZE	SPA	SIZE	SPA
N12	12	3.00	5.00	8.00	1.50	#6	12"	6'-0"	#6	12"	2'-6"	#6	12"	#6	12"
N14	14	3.00	6.00	9.00	1.50	#7	12"	6'-0"	#6	12"	2'-6"	#6	12"	#6	12"
N16	16	3.00	7.00	10.00	1.50	#8	12"	6'-0"	#7	12"	3'-0"	#8	12"	#6	12"
N18	18	3.00	9.00	12.00	1.50	#8	9"	9'-0"	#8	12"	3'-6"	#9	12"	#6	12"

T4 STEEL BRIDGE RAIL 7-#5 HOOPS — @6" AT EACH RAIL POST SIDEWALK -NOM. **FORMLINER** #4@12" E.W.-#4@12**"**— "W2" BARS— **EXISTING** GROUND PROPOSED GRADE -2' THICK STONE FILL CLASS B WEEPHOLE SEE DETAIL-(BRIDGE) AS DOWELS DIRECTED BY THE ENGINEER "W1" BARS "FT" BARS ∠ITEM 593.411 "FB" BARS — "A"

RETAINING WALL DATA (WALLS #2 AND #3, WITH RAIL)

WALL	"H"MAX	"A"	"B"	"C"	"F"	" W					BARS		BARS	"FB"	BARS
TYPE	(FT.)	(FT.)	(FT.)	(FT.)	(FT.)	SIZE	SPA.	"D"	SIZE	SPA.	"LAP"	SIZE	SPA	SIZE	SPA
R8	8	2.67	5.33	8.00	1.50	#8	12"	6'-0"	#8	12"	3'-6"	#6	12"	#6	12"
R10	10	2.67	5.33	8.00	1.50	#8	12"	6'-0"	#8	12"	3'-6"	#6	12"	#6	12"
R12	12	3.00	6.00	9.00	1.50	#8	9"	6'-0"	#8	9"	3'-6"	#6	12"	#6	12"
R14	14	3.33	6.67	10.00	1.50	#8	9	6'-0"	#8	9"	3'-6"	#7	12"	#7	12"
R16	16	4.00	8.00	12.00	1.50	#8	°	6'-0"	#8	6"	3'-6"	#8	12"	#7	12"
R18	18	4.00	8.00	12.00	1.50	#9	6"	9'-0"	#8	6"	3'-6"	#9	12"	#7	12"

RETAINING WALL #1 DATA

SCALE: 1/2" = 1'-0"

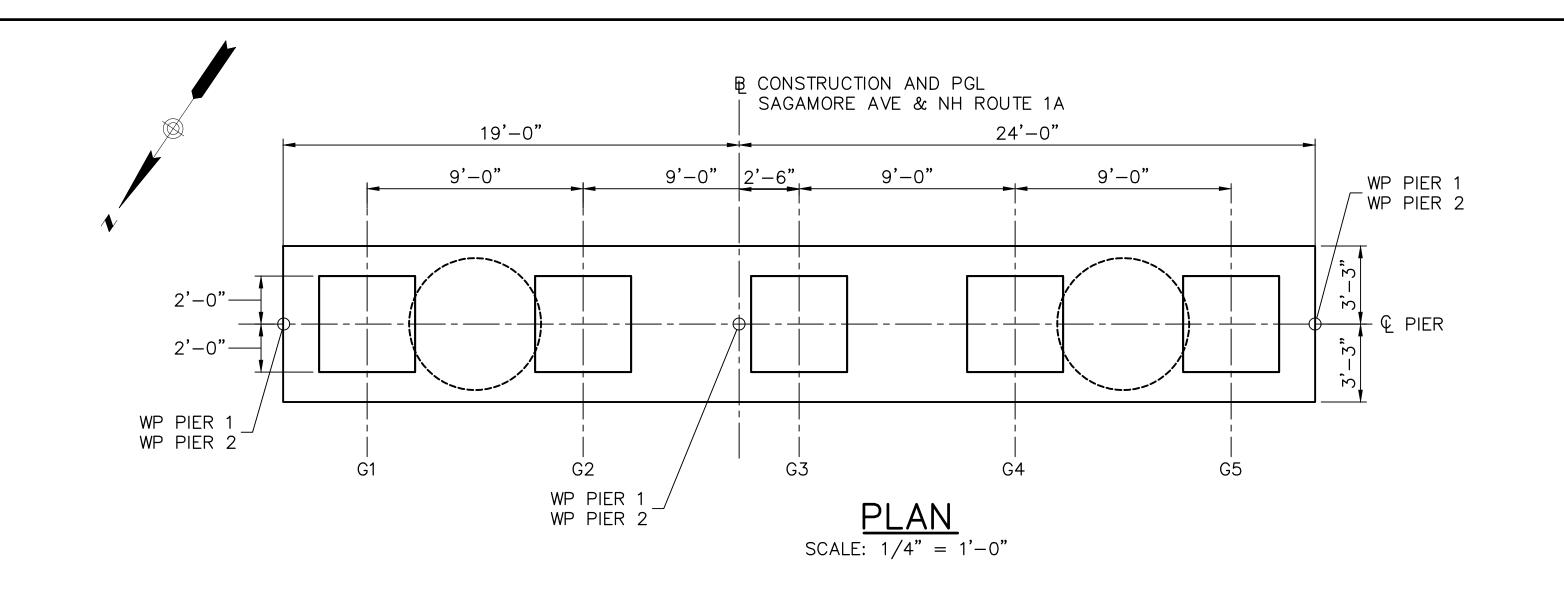
RETAINING WALLS #2 & #3 DATA SCALE: 1/2" = 1'-0"

FAY, SPOFFORD & THORNDIKE, INC

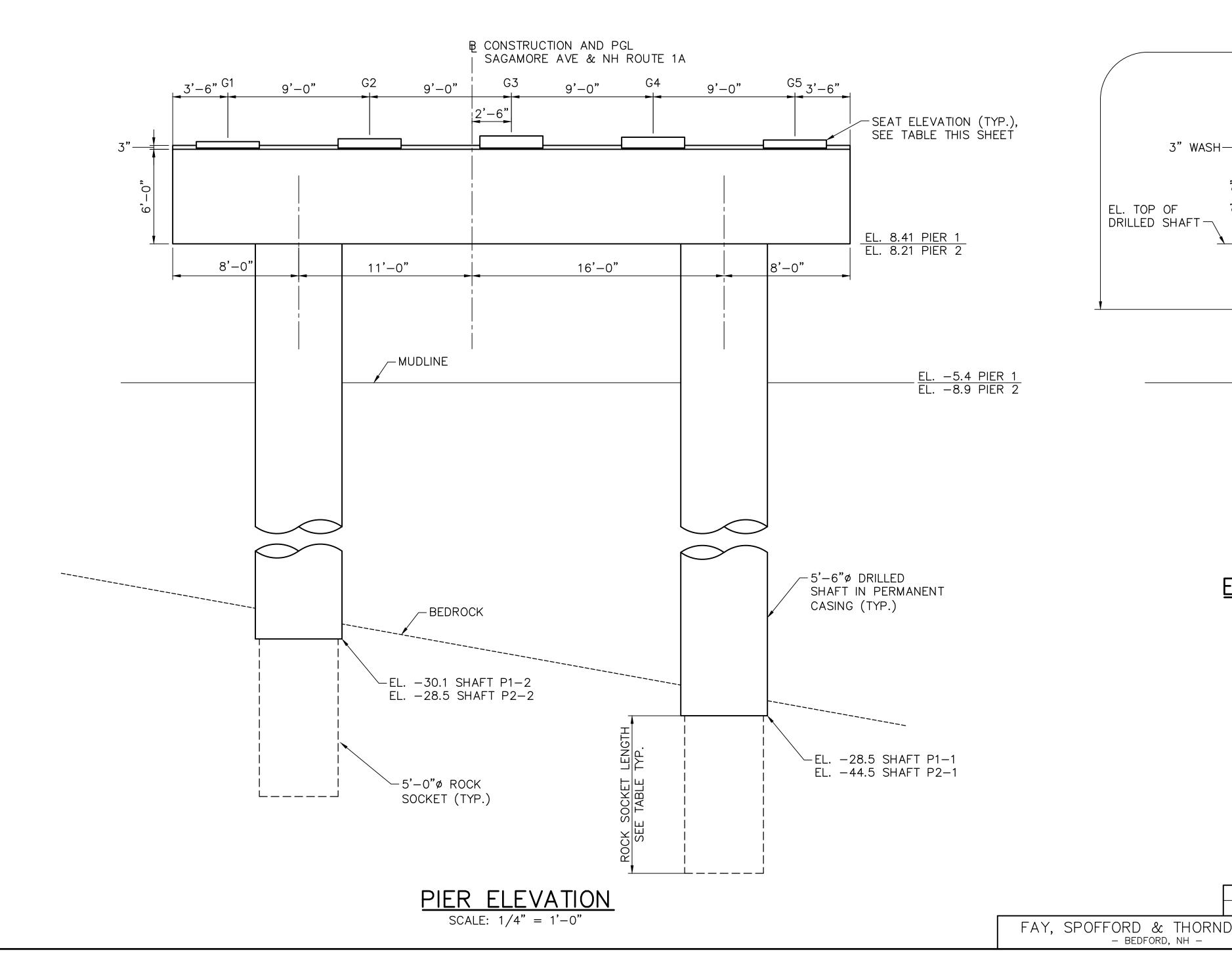
- BEDFORD, NH -

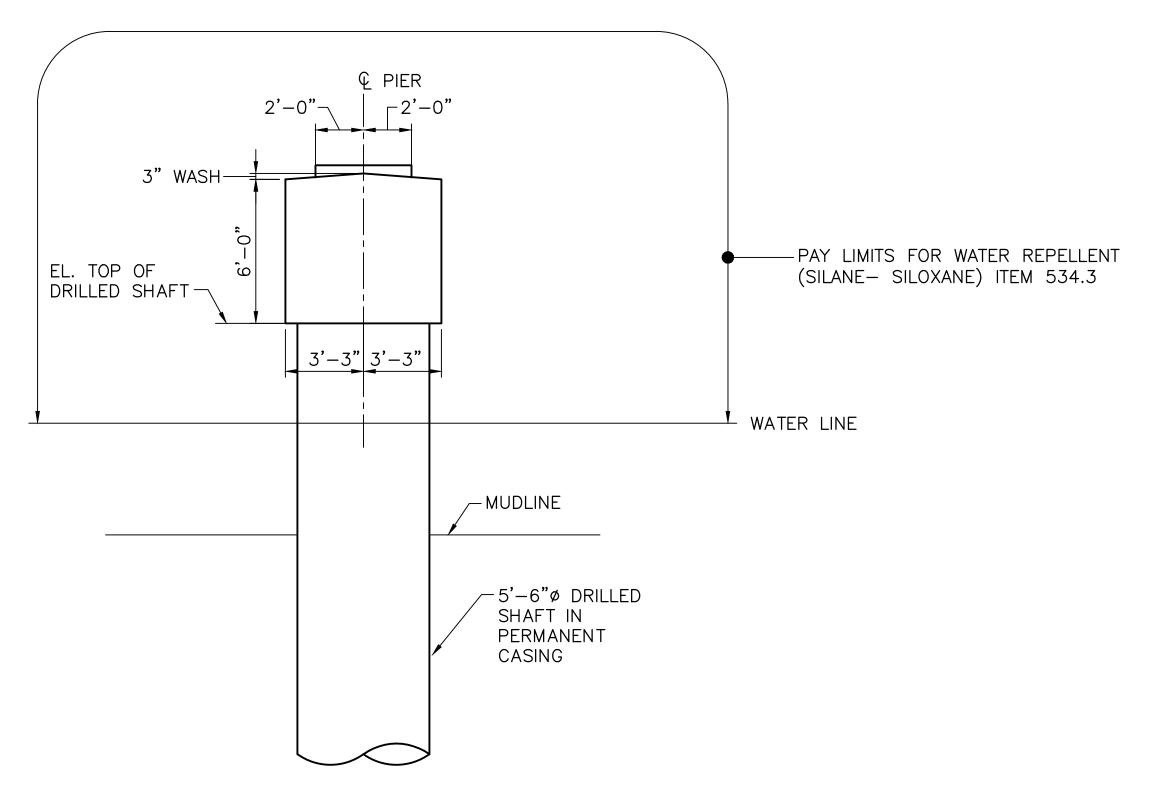
SHEET SCALE AS NOTED

TD 6/13 CHECKED



	SEAT ELEVATIONS												
GIRDER G1 G2 G3 G4 G5													
PIER 1 EL.	15.40	15.58	15.66	15.48	15.30								
PIER 2 EL. 15.20 15.38 15.46 15.28 15.10													





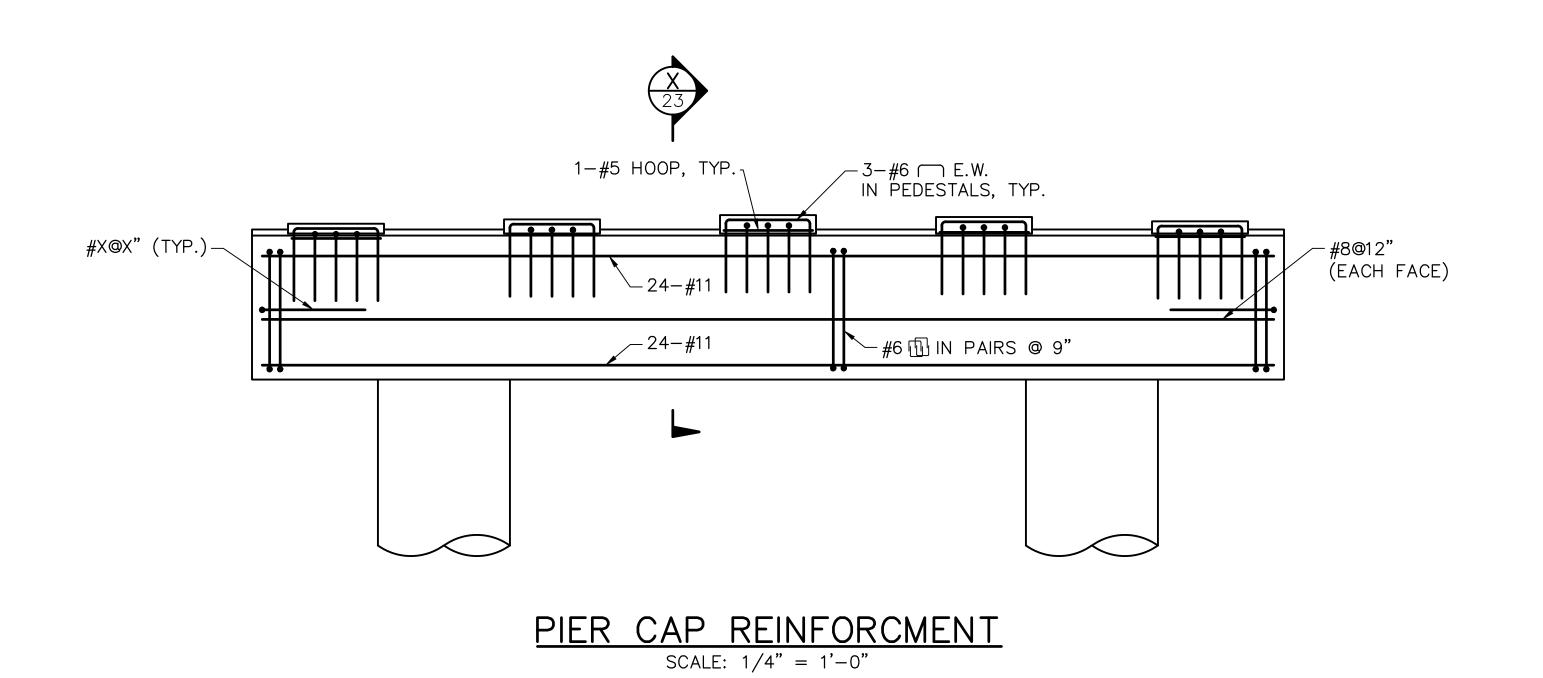
END ELEVATION SCALE: 1/4" = 1'-0"

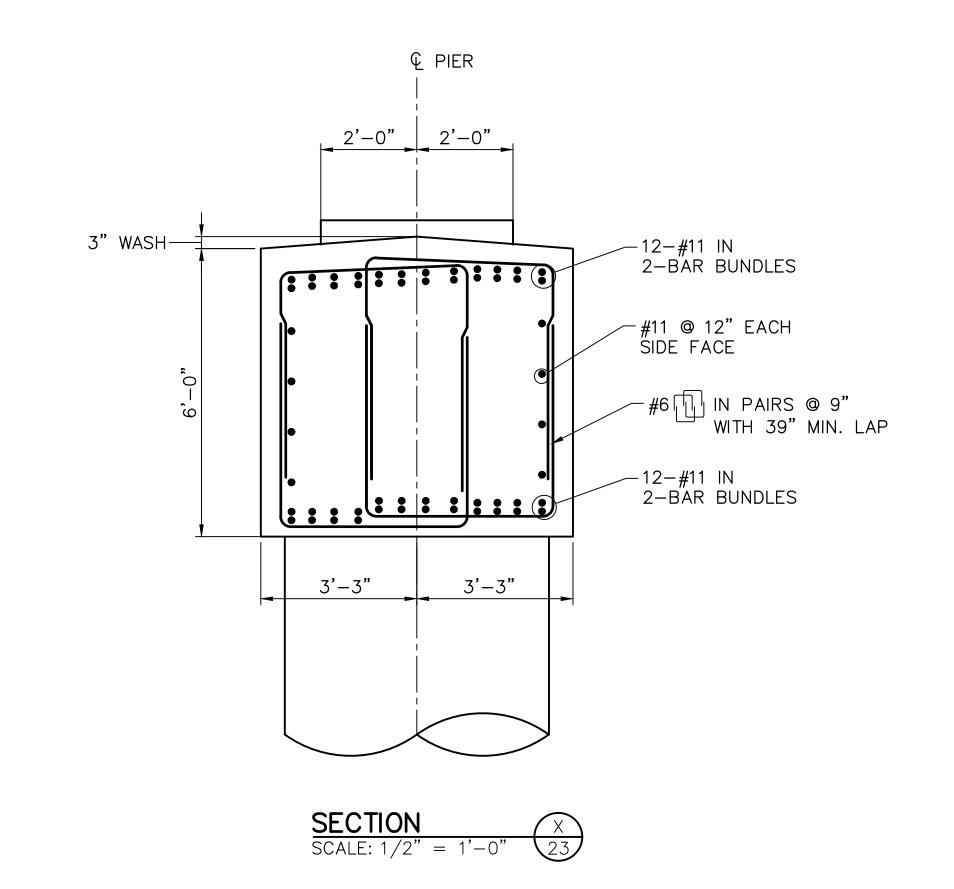
<u>NOTES</u>

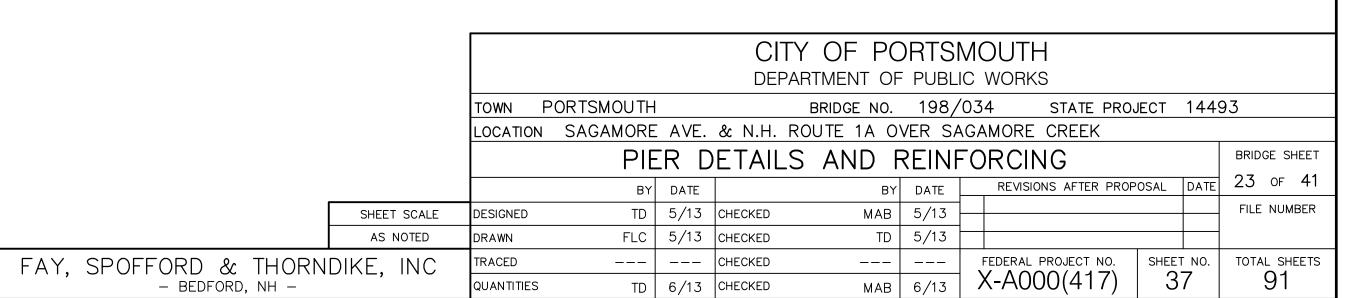
 BEDROCK ELEVATIONS INTERPOLATED FROM BORINGS, ACTUAL ELEVATIONS MAY DIFFER. LENGTH OF DRIFT SHAFT AND REINFORCEMENT SHALL BE ADJUSTED FOR ACTUAL FIELD CONDITIONS.

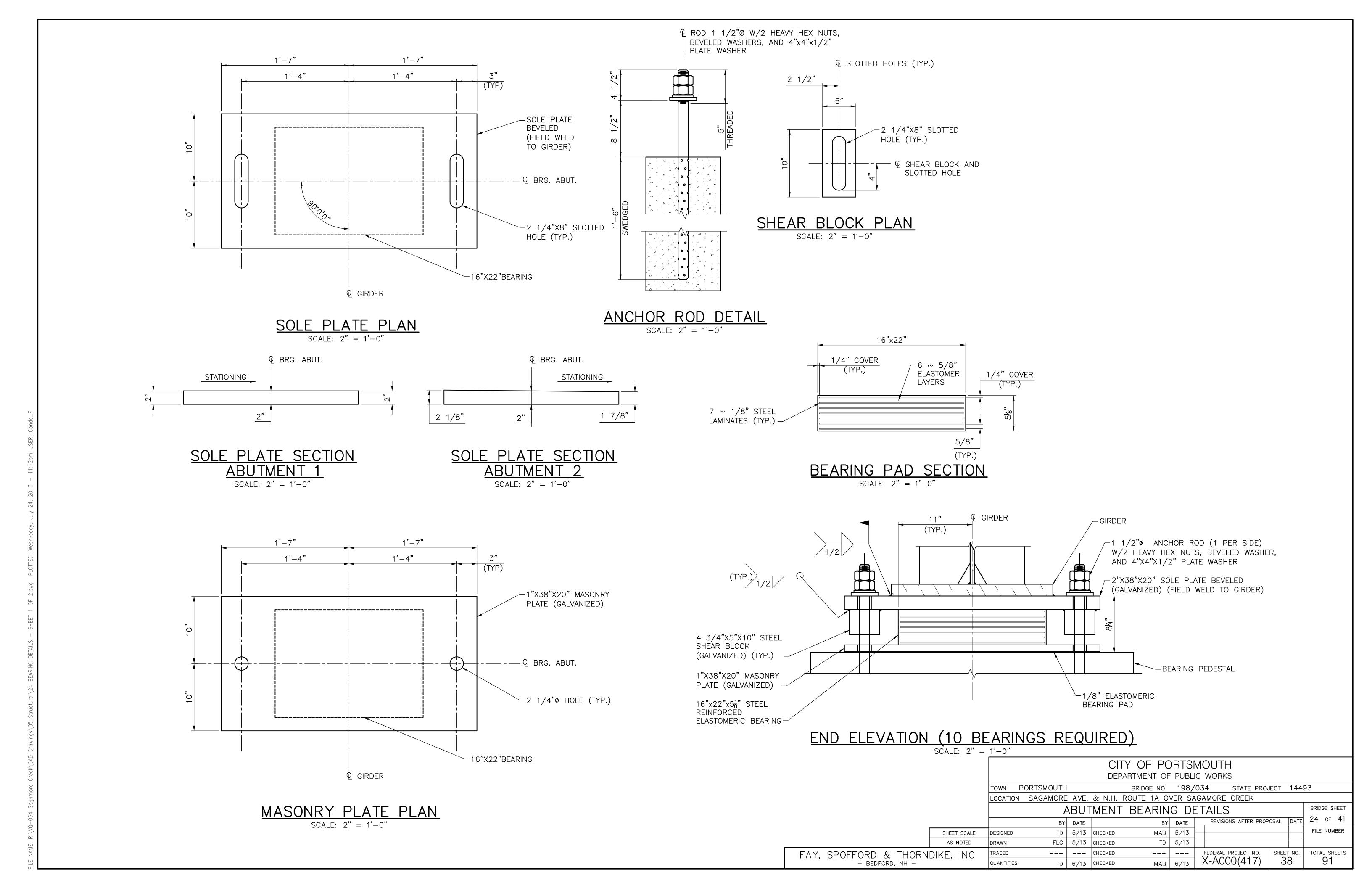
		CIT	Y OF PO	RTSMOUT	Н		
		DEPA	RTMENT OF	PUBLIC WORK	(S		
TOWN	PORTSMOUTH		BRIDGE NO.	198/034	STATE PROJECT	1449.	3
LOCATIO	N SAGAMORE	AVE. & N.H. R	OUTE 1A OV	ER SAGAMORE	CREEK		
	F	PIER PLAN	I AND E	LEVATION			BRIDGE SHEET

		LOCATION	SAGAMORE	AVE.	& N.H. RC	DUTE TA O	VER SA	GAMORE CREEK		
				PIER	PLAN	AND E	LEV	ATION		BRIDGE SHEET
			BY	DATE		BY	DATE	REVISIONS AFTER PROP	OSAL DATE	22 of 41
	SHEET SCALE	DESIGNED	TD	5/13	CHECKED	MAB	5/13			FILE NUMBER
	AS NOTED	DRAWN	FLC	5/13	CHECKED	TD	5/13			
]	DIKE, INC	TRACED			CHECKED			FEDERAL PROJECT NO.	SHEET NO.	TOTAL SHEETS
	•	QUANTITIES	TD	6/13	CHECKED	MAB	6/13	X-A000(417)	36	91

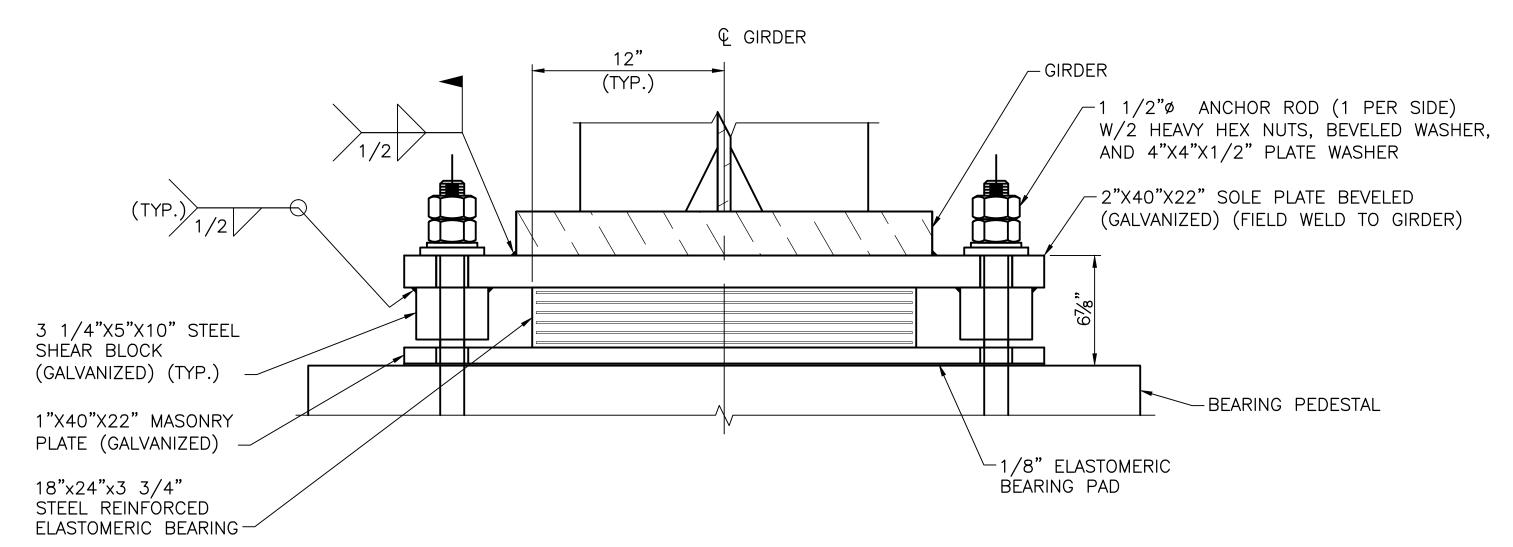




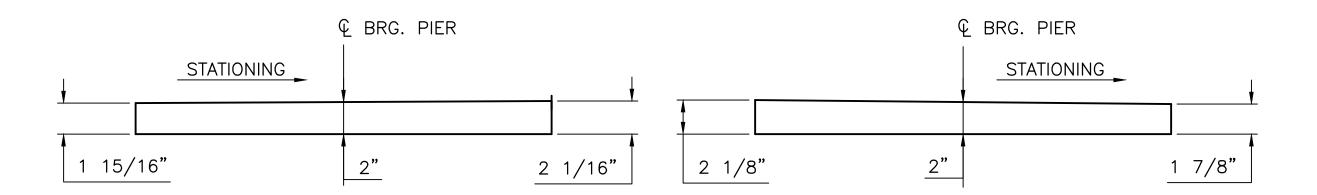






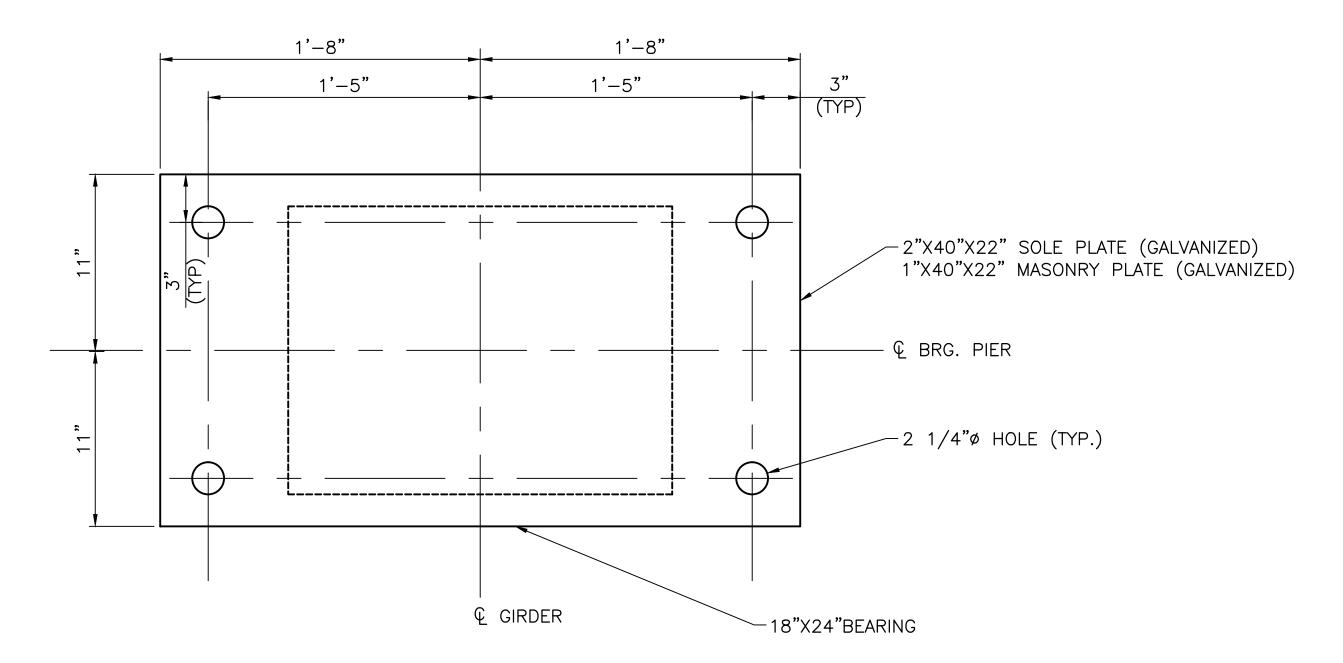


END ELEVATION (10 BEARINGS REQUIRED) SCALE: 2'' = 1'-0''



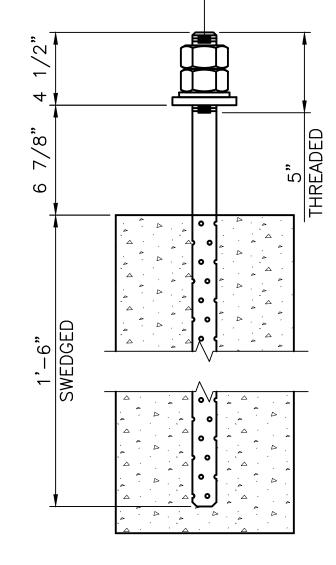
SOLE PLATE SECTION PIER 1 SCALE: 2" = 1'-0"

SOLE PLATE SECTION PIER 2 SCALE: 2" = 1'-0"

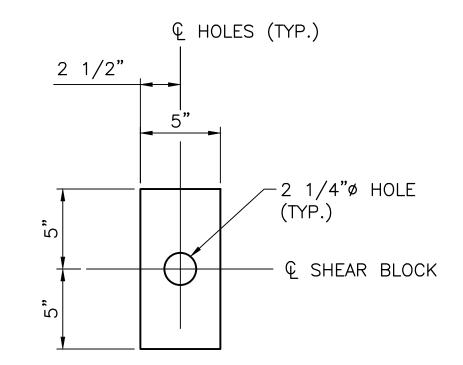


MASONRY AND SOLE PLATE PLAN SCALE: 2" = 1'-0"

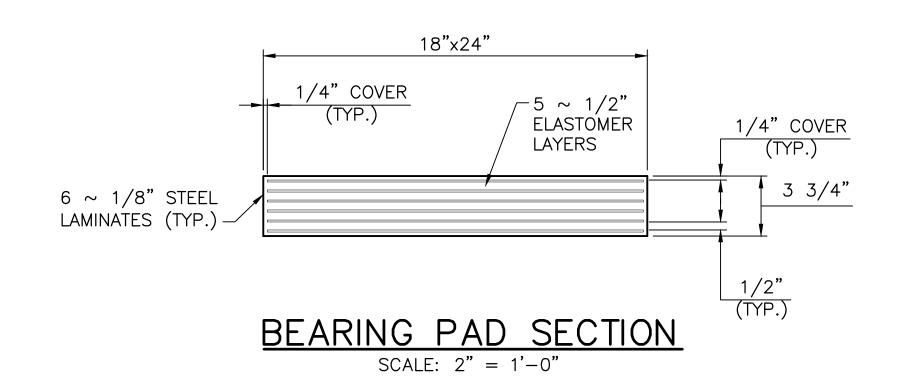
€ ROD 1 1/2"Ø W/2 HEAVY HEX NUTS, BEVELED WASHERS, AND 4"x4"x1/2"
PLATE WASHER



ANCHOR ROD DETAIL SCALE: 2" = 1'-0"



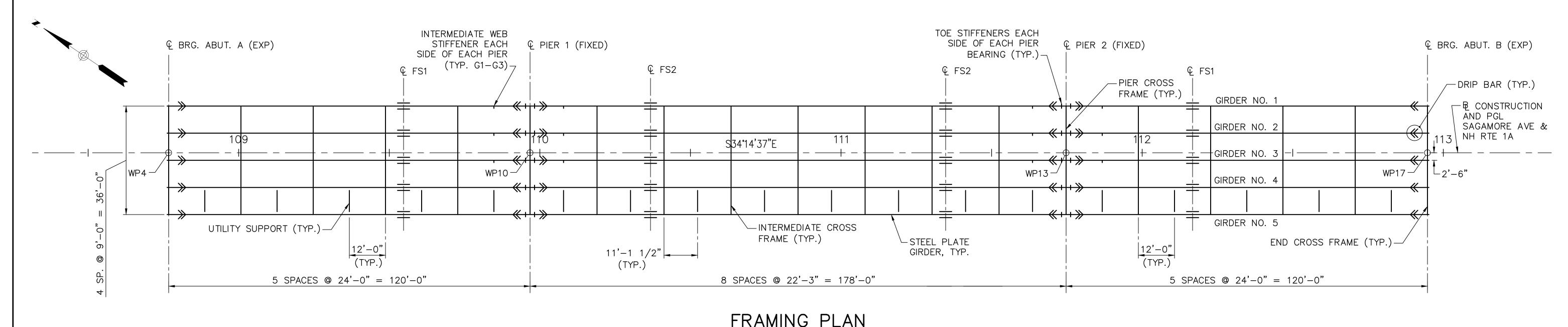
SHEAR BLOCK PLAN SCALE: 2" = 1'-0"



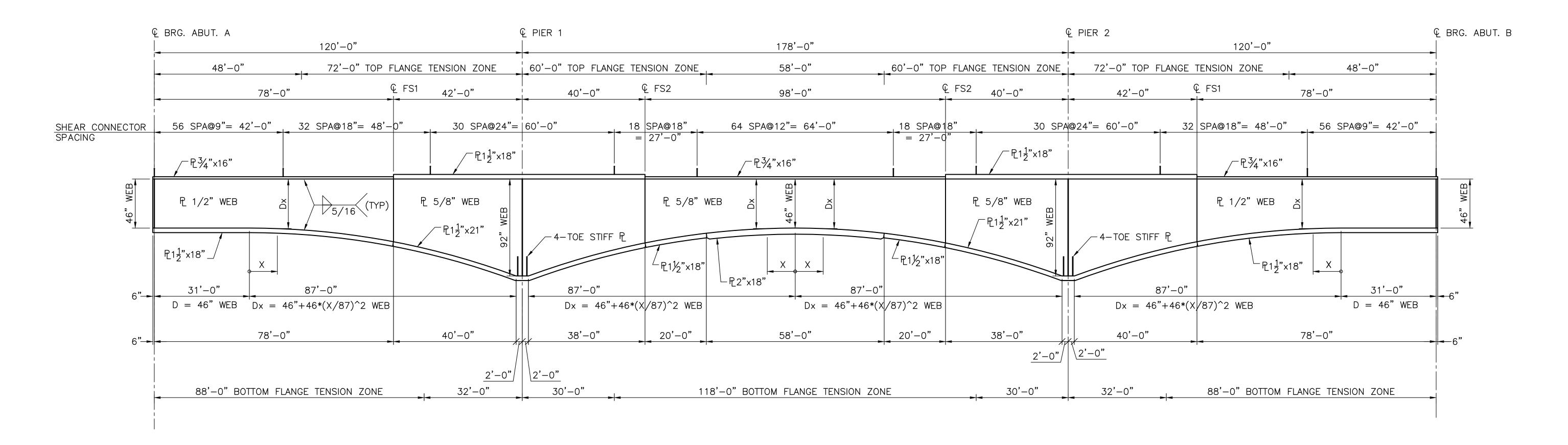
SHEET SCALE AS NOTED

FAY, SPOFFORD & THORNDIKE, INC
- BEDFORD, NH -

			Cl	TY OF PC)RTSI	MOUTH							
			DEF	PARTMENT OF	PUBL	IC WORKS							
TOWN	PORTSMOUTH			BRIDGE NO.	198/	034 STATE PROJECT 14493							
LOCATION	CATION SAGAMORE AVE. & N.H. ROUTE 1A OVER SAGAMORE CREEK												
	PIER BEARING DETAILS BRIDGE SHEET												
	BY	DATE		BY	DATE	REVISIONS AFTER PROPOSAL DATE 25 OF 41							
DESIGNED	TD	5/13	CHECKED	MAB	5/13	FILE NUMBER							
DRAWN	FLC	5/13	CHECKED	TD	5/13								
TRACED													
QUANTITIES	S TD	6/13	CHECKED	MAB	6/13	X-A000(417) 39 91							



FRAMING PLAN SCALE: 1/16" = 1'-0"



GIRDER ELEVATION NOT TO SCALE

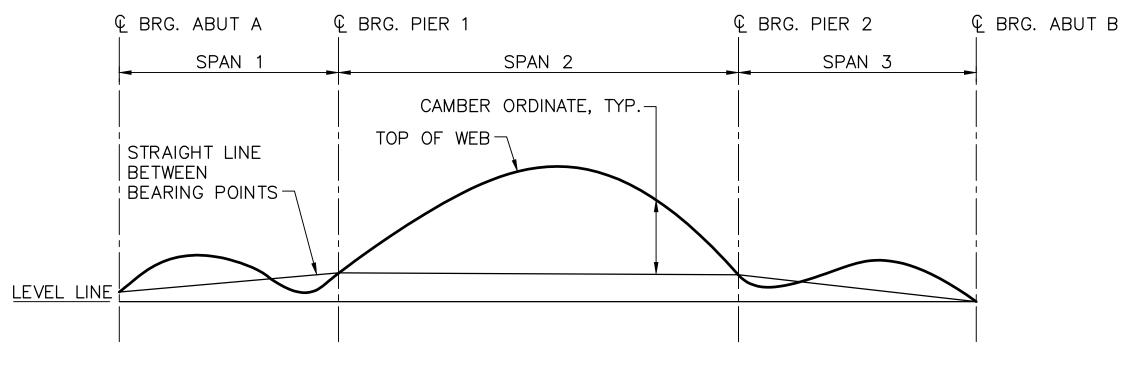
				OF PO								
	TOWN PORTSMOU	TH	[BRIDGE NO.	198/	034 STATE PRO	JECT 144	93				
	LOCATION SAGAMO	RE AVE.	& N.H. RO	UTE 1A OV	ER SA	GAMORE CREEK						
		FRAMING PLAN BRIDGE SHEET										
		BY DATE		BY	DATE	REVISIONS AFTER PROP	OSAL DATE	26 of 41				
SHEET SCALE	DESIGNED 7	D 5/13	CHECKED	MAB	5/13			FILE NUMBER				
AS NOTED	DRAWN FL	C 5/13	CHECKED	TD	5/13							
FAY, SPOFFORD & THORNDIKE, INC	TRACED		CHECKED			FEDERAL PROJECT NO.	SHEET NO.	TOTAL SHEETS				
· · · · · · · · · · · · · · · · · · ·	QUANTITIES 7	TITIES TD 6/13 CHECKED MAB 6/13 X-A000(417) 40 91										

						Span	1 Camber Table (lı	nches)				
	Point Along Span	CL. Brg. Abut. No. 1	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	CL. Brg. Pier No. 1
	Station	108+76.75	108+88.75	109+00.75	109+12.75	109+24.75	109+36.75	109+48.75	109+60.75	109+72.75	109+84.75	109+96.75
2	Girder DL Deflection	0.00	0.07	0.12	0.14	0.11	0.06	0.00	-0.05	-0.07	-0.05	0.00
o G5	Conc. Slab Deflection	0.00	0.53	0.93	1.11	1.05	0.81	0.47	0.16	-0.05	-0.11	0.00
7	Super. DL Deflection	0.00	0.14	0.25	0.30	0.29	0.23	0.14	0.04	-0.03	-0.05	0.00
e e	Total Deflection	0.00	0.74	1.30	1.54	1.45	1.10	0.61	0.16	-0.14	-0.21	0.00
2	VC Ordinate	0.00	-0.63	-0.91	-0.86	-0.73	-0.61	-0.49	-0.37	-0.24	-0.12	0.00
0	Total Camber	0.00	0.12	0.39	0.69	0.72	0.48	0.12	-0.21	-0.39	-0.33	0.00

						Span	2 Camber Table (li	nches)				
	Point Along Span	CL. Brg. Pier No. 1	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	CL. Brg. Pier No. 2
	Station	109+96.75	110+14.55	110+32.35	110+50.15	110+67.95	110+85.75	111+03.55	111+21.35	111+39.15	111+56.95	111+74.75
2	Girder DL Deflection	0.00	0.16	0.39	0.66	0.87	0.96	0.87	0.66	0.39	0.16	0.00
o G5	Conc. Slab Deflection	0.00	0.49	1.31	2.30	3.13	3.46	3.13	2.29	1.30	0.48	0.00
91.5	Super. DL Deflection	0.00	0.19	0.50	0.84	1.10	1.20	1.10	0.84	0.50	0.19	0.00
e e	Total Deflection	0.00	0.84	2.21	3.80	5.10	5.61	5.10	3.79	2.20	0.83	0.00
Gird	VC Ordinate	0.00	2.37	4.66	6.37	7.42	7.82	7.57	6.66	5.09	2.87	0.00
0	Total Camber	0.00	3.21	6.86	10.16	12.52	13.43	12.66	10.44	7.29	3.71	0.00

				Span 3 Camber Table (Inches)									
	Point Along Span	CL. Brg. Pier No. 2	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	CL. Brg. Abut. No. 2	
	Station	111+74.75	111+86.75	111+98.75	112+10.75	112+22.75	112+34.75	112+46.75	112+58.75	112+70.75	112+82.75	112+94.75	
2	Girder DL Deflection	0.00	-0.05	-0.07	-0.04	0.01	0.06	0.11	0.14	0.12	0.07	0.00	
Ö	Conc. Slab Deflection	0.00	-0.11	-0.04	0.18	0.49	0.81	1.05	1.11	0.93	0.53	0.00	
7.	Super. DL Deflection	0.00	-0.05	-0.03	0.04	0.14	0.23	0.29	0.30	0.25	0.14	0.00	
er C	Total Deflection	0.00	-0.20	-0.13	0.18	0.63	1.10	1.46	1.54	1.30	0.74	0.00	
<u>5</u>	VC Ordinate	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
9	Total Camber	0.00	-0.20	-0.13	0.18	0.63	1.10	1.46	1.54	1.30	0.74	0.00	

CAMBER TABLE



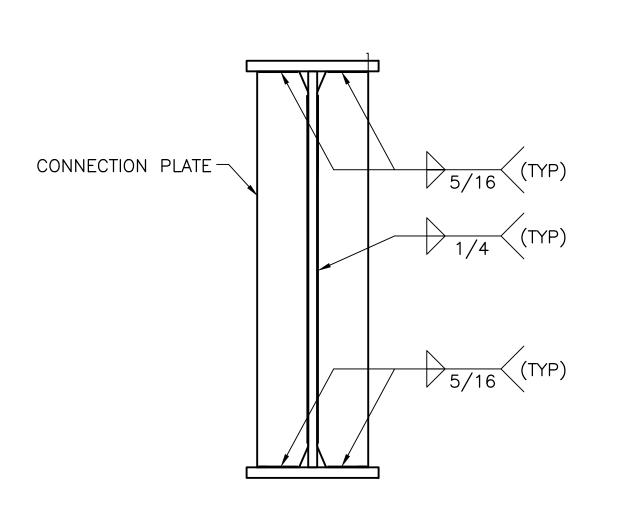
CAMBER DIAGRAM

NOT TO SCALE

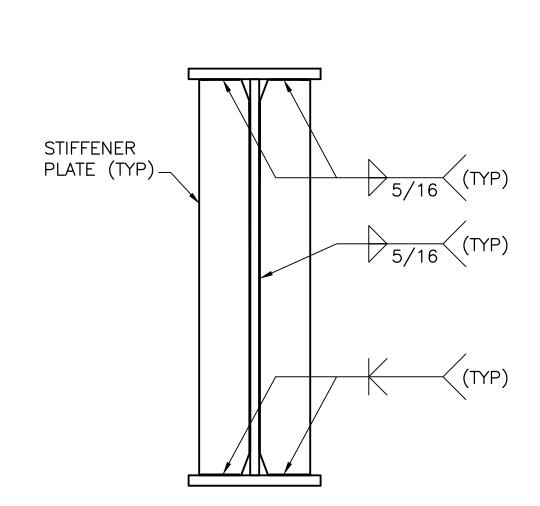
			CIT	Y OF PC	DRTSI	MOUTH						
			DEPA	ARTMENT OF	PUBL	C WORKS						
TOWN P	ORTSMOUTH			BRIDGE NO.	198/	034 s1	TATE PROJ	ECT	1449	93		
LOCATION	SAGAMORE	AVE.	& N.H. F	ROUTE 1A O	VER SA	GAMORE CF	REEK					
	GIRI	DER	DETAI	ILS - S	HEET	1 OF	4				SE SHE	
	BY	DATE		BY	DATE	REVISIONS	AFTER PROP	OSAL	DATE	27	of 4	⊦1 —
DESIGNED	TD	5/13	CHECKED	MAB	5/13					FILE	NUMBE	:R
DRAWN	FLC	5/13	CHECKED	TD	5/13							
TRACED			CHECKED			FEDERAL PRO		SHEET			SHEE	TS
QUANTITIES	TD	6/13	CHECKED	MAB	6/13	X-A000	(417)	4	1	(91	

SHEET SCALE
AS NOTED

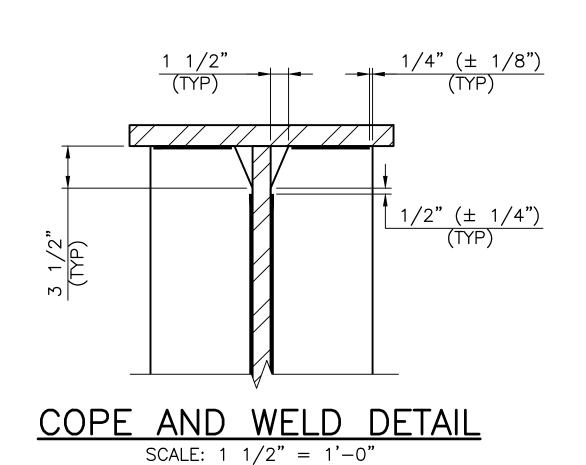
FAY, SPOFFORD & THORNDIKE, INC
- BEDFORD, NH -

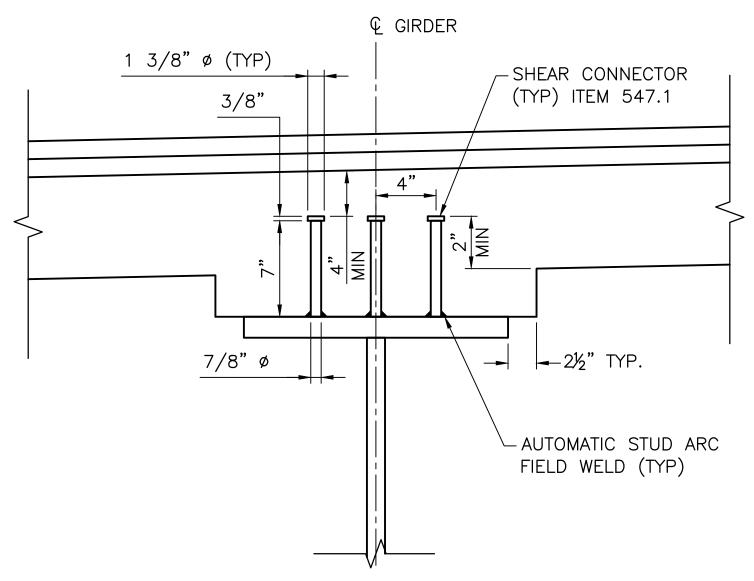


CONNECTION PLATE SCALE: 3/4" = 1'-0"

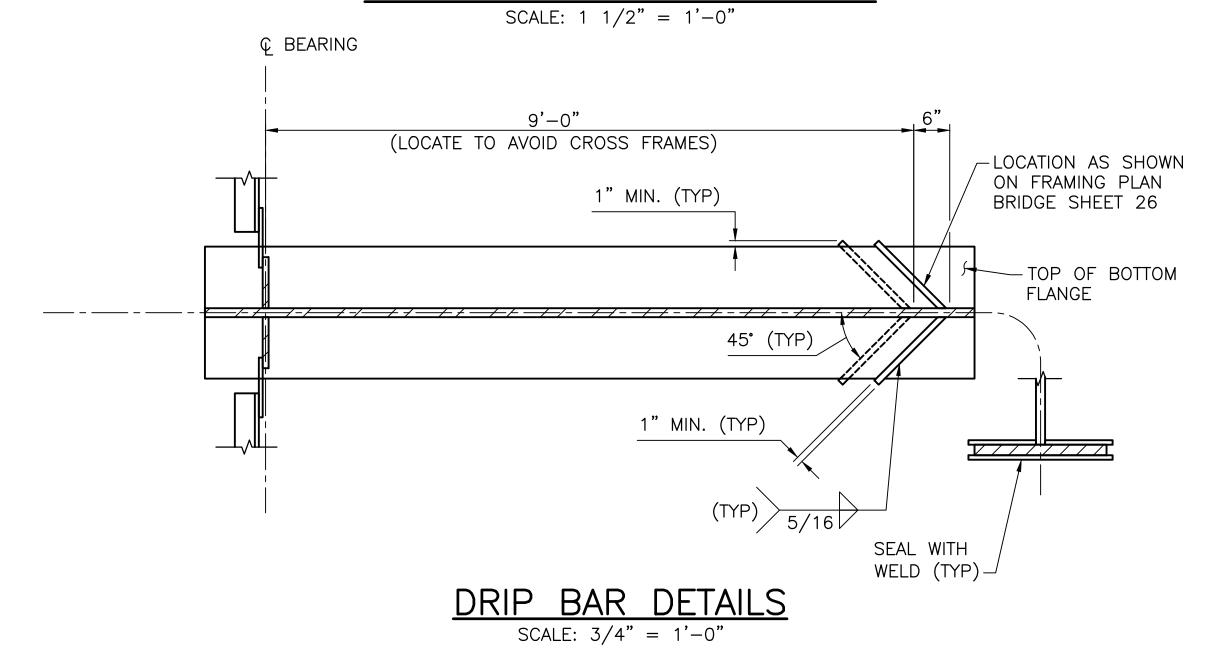


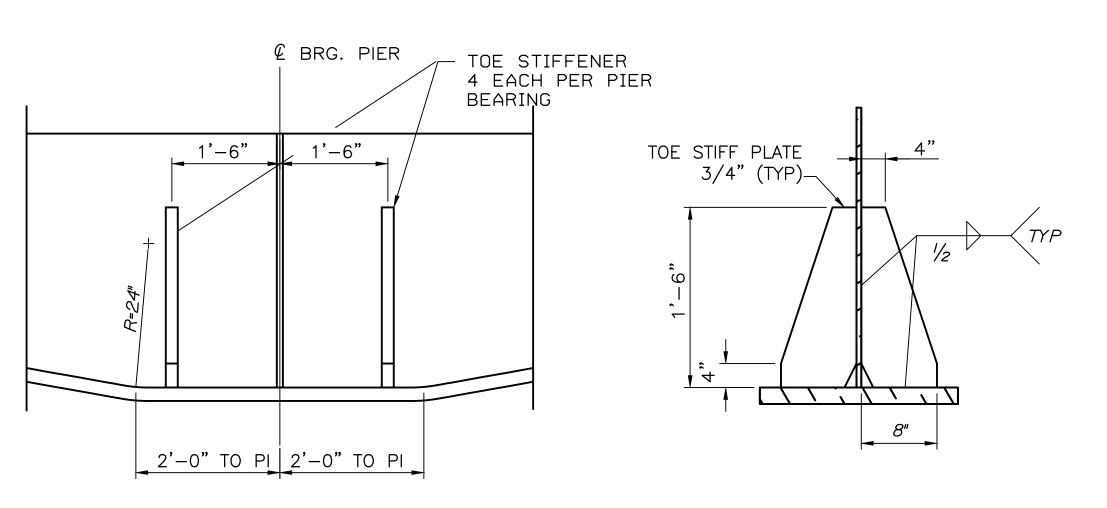
BEARING STIFFENERS SCALE: 3/4" = 1'-0"





SHEAR CONNECTOR DETAILS





TOE STIFFENER **ELEVATION** SCALE: 3/4" = 1'-0"

TOE STIFFENER <u>SECTION</u> SCALE: 3/4" = 1'-0"

FAY, SPOFFORD & THORNDIKE, INC
- BEDFORD, NH -

LONGITUDINAL
DIRECTION OF GIRDER € FLANGE SHOP SPLICE FLANGE THICKNESS TRANSITION DETAIL

NOT TO SCALE

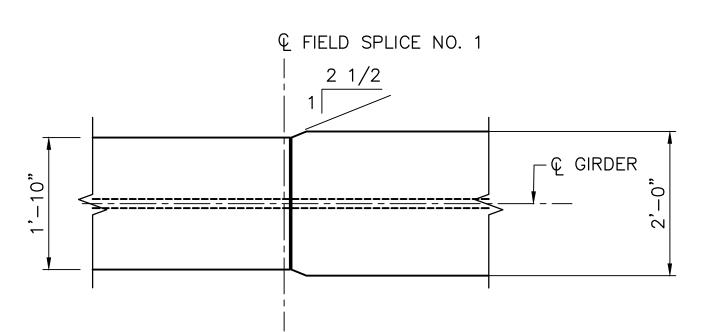
COMPLETE PENETRATION

GROOVE WELD TO BE GROUND FLUSH IN

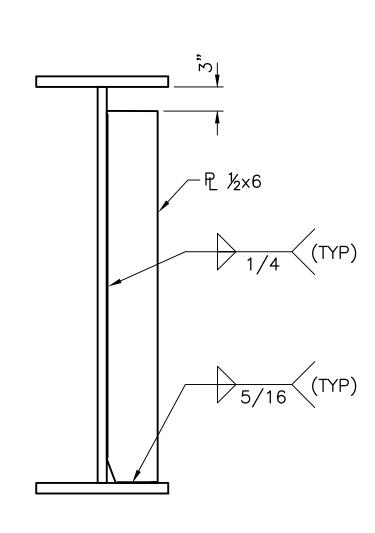
TAPER BEFORE

2 1/2

WELDING



FLANGE WIDTH TRANSITION DETAIL SCALE: 3/4" = 1'-0"

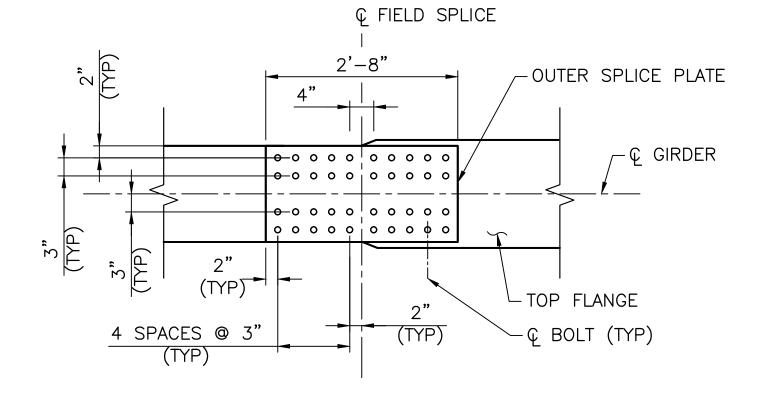


INTERMEDIATE STIFFENER SCALE: 3/4" = 1'-0"

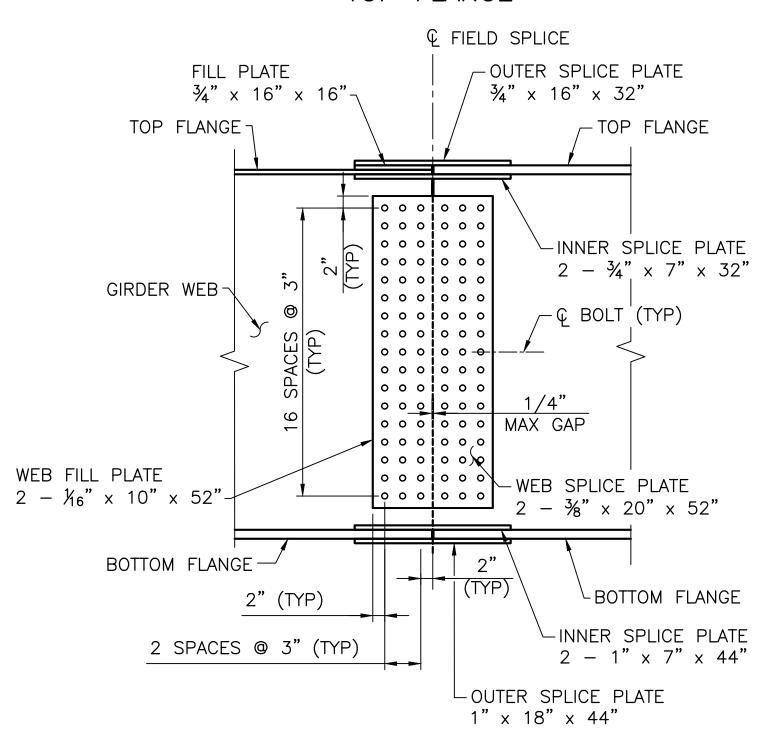
SHEET SCALE

AS NOTED

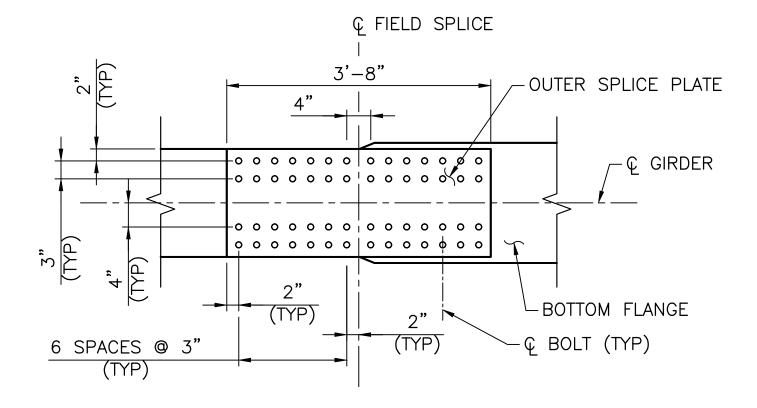
	CITY OF PORTSMOUTH												
	DEPARTMENT OF PUBLIC WORKS												
TOWN P	ORTSMOUTH			BRIDGE NO.	198/	/034 STATE PROJECT 14493							
LOCATION	SAGAMORE	AVE.	& N.H. I	ROUTE 1A O	VER SA	AGAMORE CREEK							
	GIRDER DETAILS — SHEET 2 OF 4 BRIDGE SHEET												
	BY	DATE		BY	DATE	REVISIONS AFTER PROPOSAL DATE 28 OF 47							
DESIGNED	TD	5/13	CHECKED	MAB	5/13	FILE NUMBER							
DRAWN	FLC	5/13	CHECKED	TD	5/13								
TRACED			CHECKED			FEDERAL PROJECT NO. SHEET NO. TOTAL SHEET							
QUANTITIES	TD	6/13	CHECKED	MAB	6/13	X-A000(417) 42 91							



TOP FLANGE

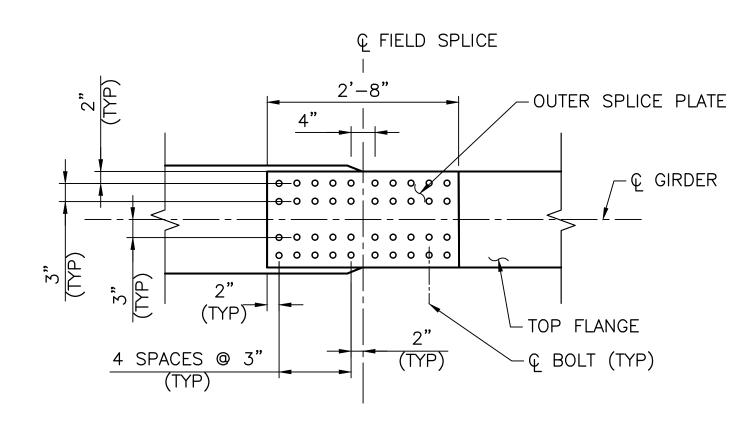


WEB SPLICE

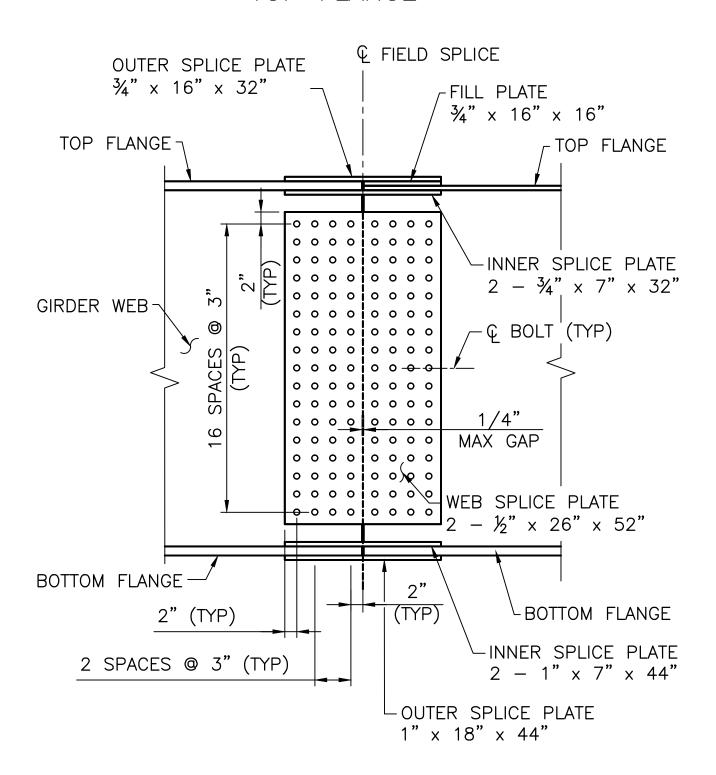


BOTTOM FLANGE

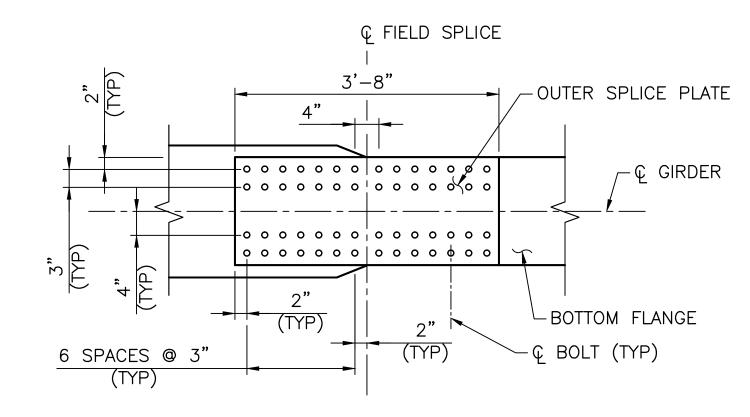
FIELD SPLICE NO. 1 SCALE: 3/4" = 1'-0"



TOP FLANGE



WEB SPLICE



BOTTOM FLANGE

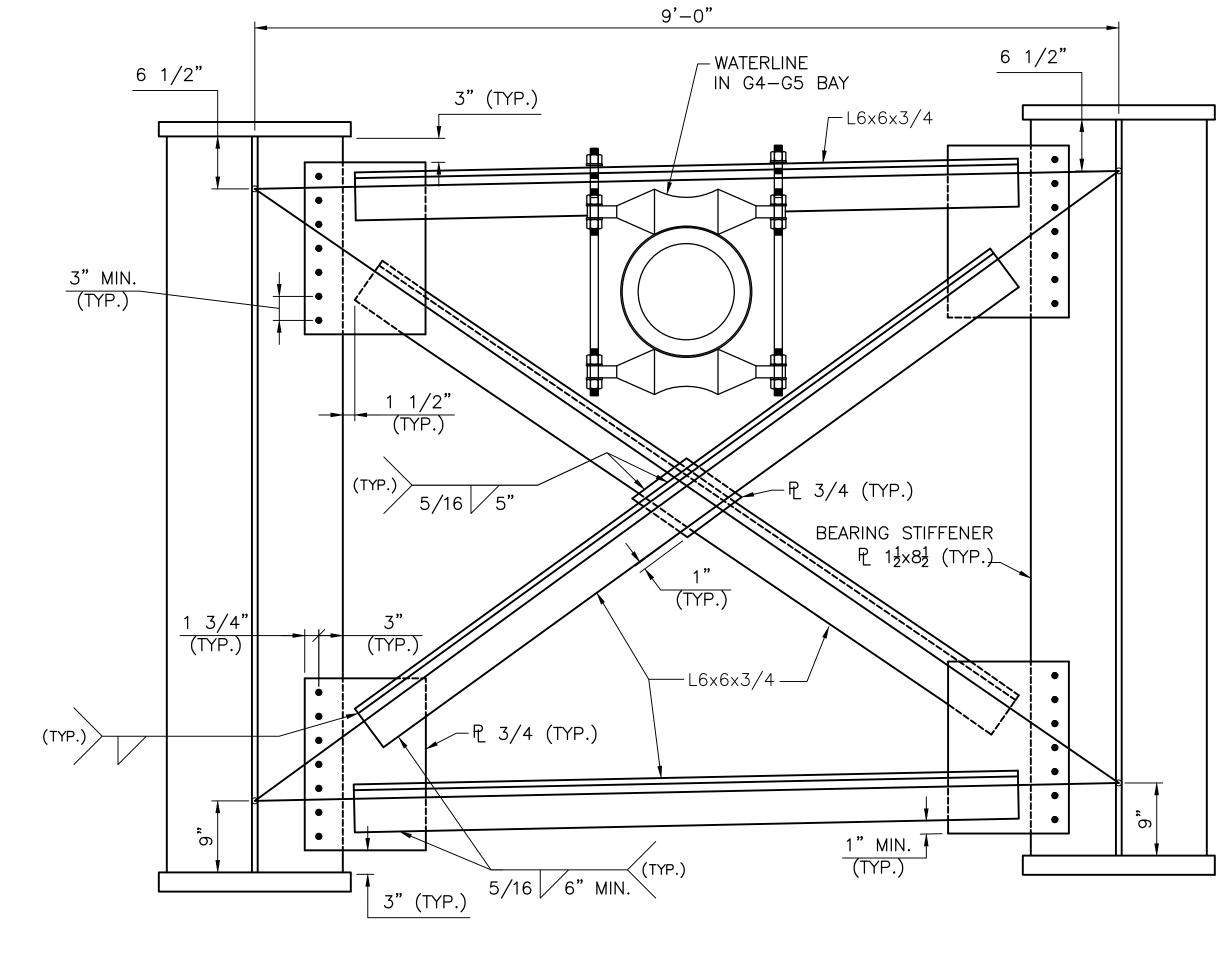
FIFID SPLICE NO 2

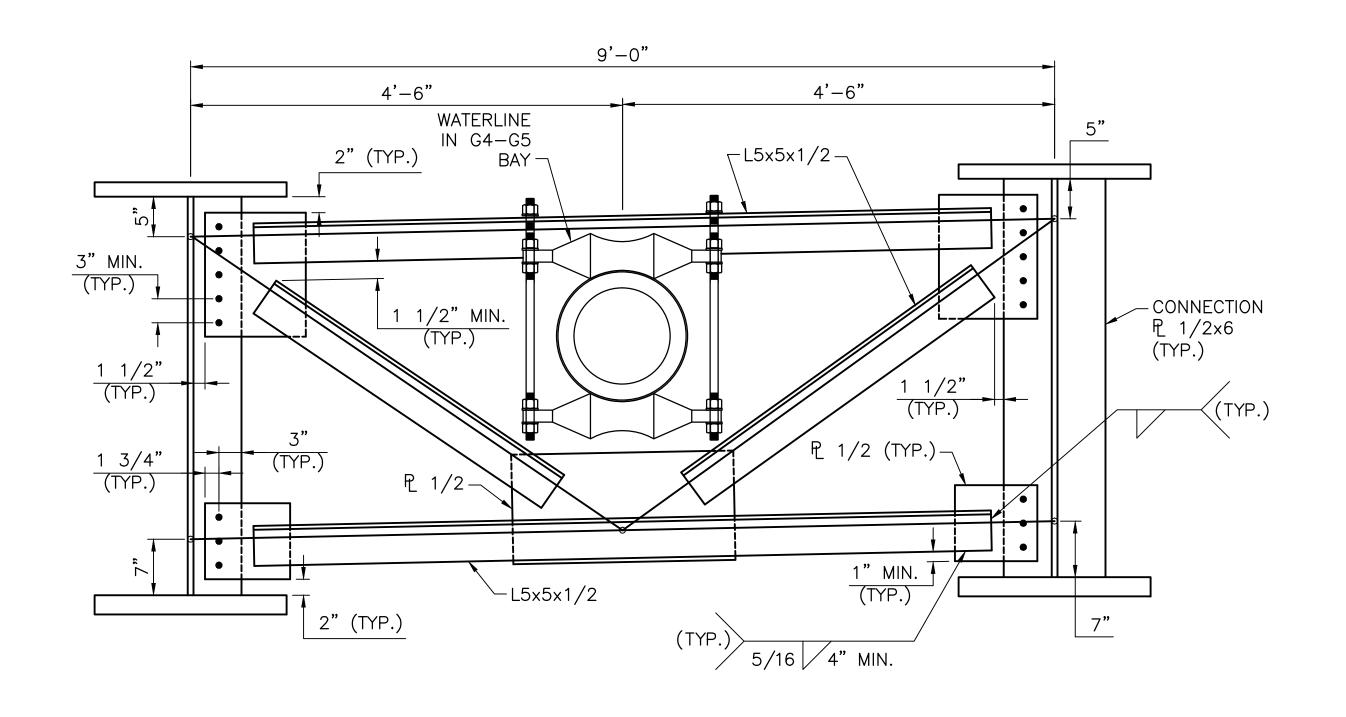
	<u> </u>	<u>. 190. Z</u>	
SCA	ALE: $3/4" =$	1'-0"	

	CITY OF PORTSMOUTH												
	DEPARTMENT OF PUBLIC WORKS												
TOWN P	ORTSMOUTH			BRIDGE NO.	198/	/034 STATE PROJECT 14493							
LOCATION	SAGAMORE	AVE.	& N.H.	ROUTE 1A O	VER SA	AGAMORE CREEK							
	GIRDER DETAILS — SHEET 3 OF 4 BRIDGE SHEET												
	BY	DATE		BY	DATE	REVISIONS AFTER PROPOSAL DATE 29 OF 41							
DESIGNED	TD	5/13	CHECKED	MAB	5/13	FILE NUMBER							
DRAWN	FLC	5/13	CHECKED	TD	5/13								
TRACED			CHECKED			FEDERAL PROJECT NO. SHEET NO. TOTAL SHEETS							
QUANTITIES	TD	6/13	CHECKED	MAB	6/13	X-A000(417) 43 91							

SHEET SCALE AS NOTED

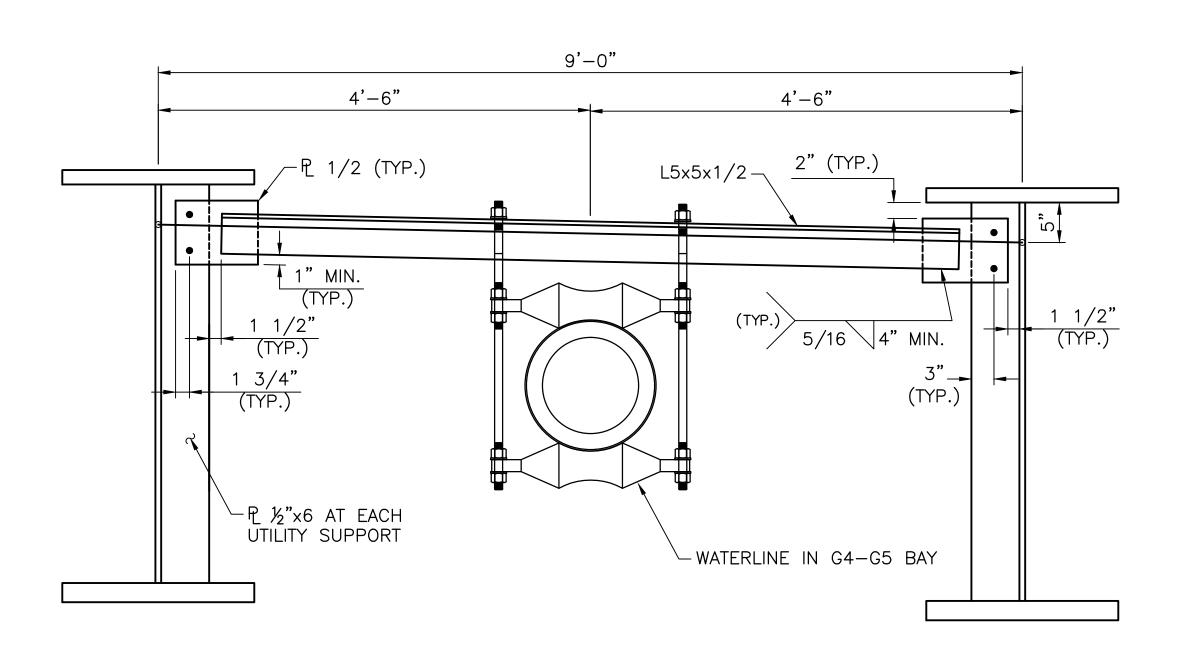
FAY, SPOFFORD & THORNDIKE, INC - BEDFORD, NH -

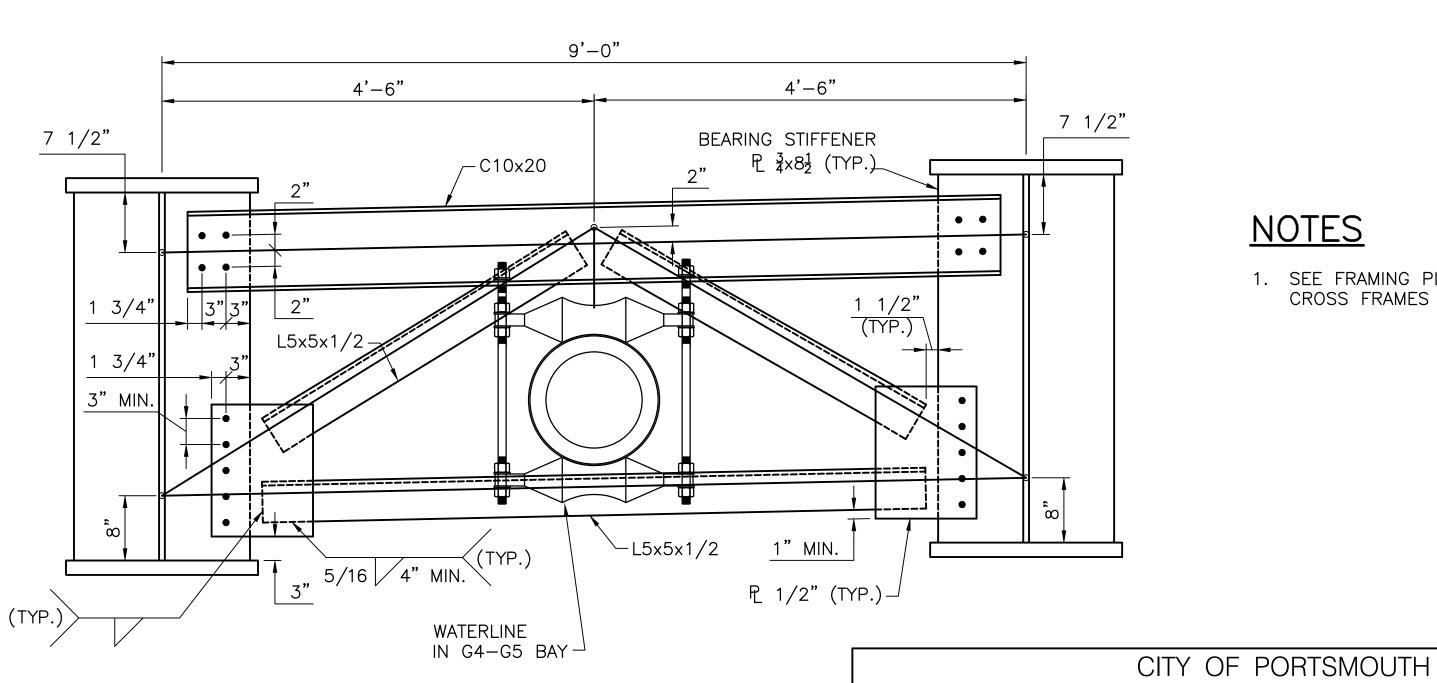




INTERMEDIATE CROSSFRAME DETAILS SCALE: 1" =1'-0"

PIER CROSSFRAME DETAILS SCALE: 1" =1'-0"





NOTES

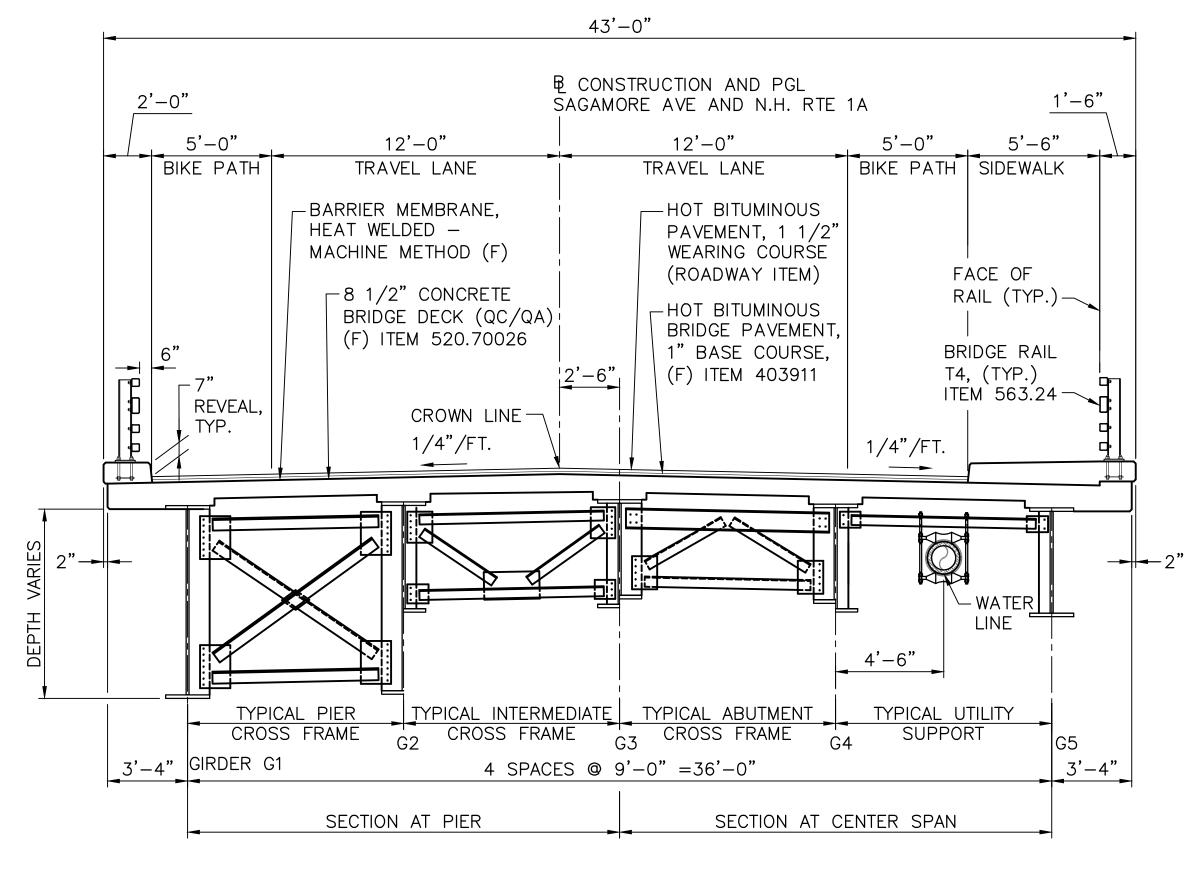
SEE FRAMING PLAN FOR LOCATIONS OF CROSS FRAMES AND UTILITY SUPPORTS.

UTILITY SUPPORT DETAILS SCALE: 1" =1'-0"

ABUTMENT CROSSFRAME DETAILS SCALE: 1" =1'-0"

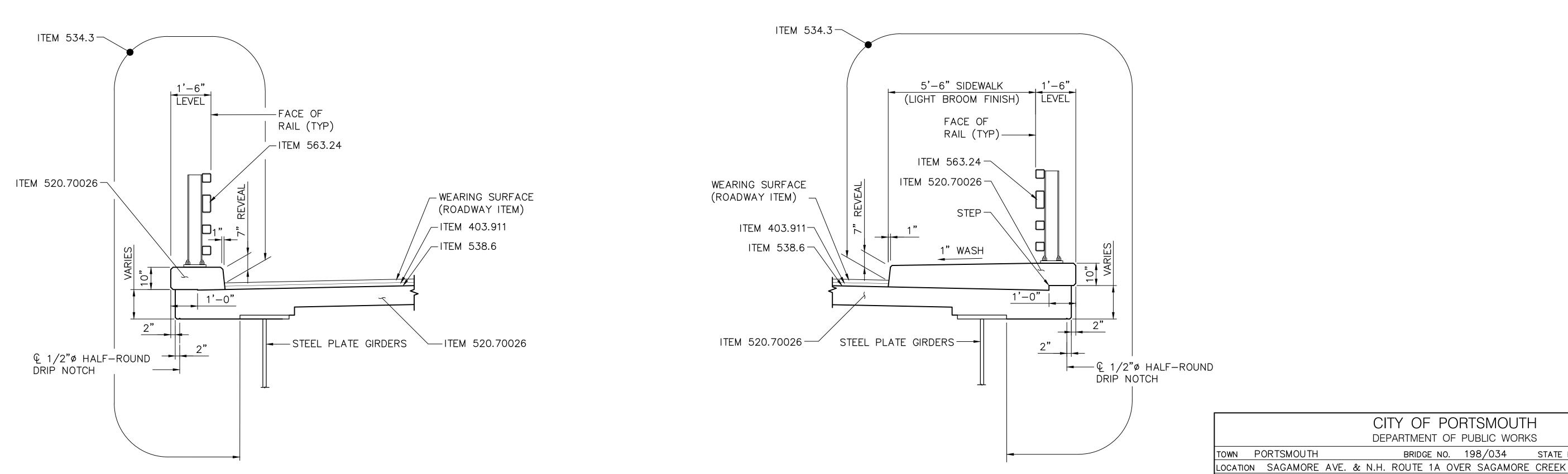
DEPARTMENT OF PUBLIC WORKS STATE PROJECT 14493 TOWN PORTSMOUTH BRIDGE NO. 198/034 LOCATION SAGAMORE AVE. & N.H. ROUTE 1A OVER SAGAMORE CREEK CROSSFRAME DETAILS BRIDGE SHEET 30 of 41 REVISIONS AFTER PROPOSAL DATE BY DATE BY DATE FILE NUMBER TD 5/13 CHECKED MAB 5/13 DESIGNED

SHEET SCALE FLC 5/13 CHECKED TD 5/13 AS NOTED FEDERAL PROJECT NO. SHEET NO. TOTAL SHEETS X-A000(417) 44 91FAY, SPOFFORD & THORNDIKE, INC - BEDFORD, NH ---- CHECKED MAB 6/13 TD 6/13 CHECKED



TYPICAL DECK SECTION

SCALE: 1/4" = 1'-0"



BRUSH CURB FASCIA DETAIL

SCALE: 1/2" = 1'-0"

SIDEWALK FASCIA DETAIL SCALE: 1/2" = 1'-0"

TYPICAL SECTION AND DECK SLAB DETAILS BRIDGE SHEET REVISIONS AFTER PROPOSAL DATE 31 of 41 BY DATE BY DATE FILE NUMBER TD 5/13 CHECKED MAB 5/13 SHEET SCALE DESIGNED FLC 5/13 CHECKED TD 5/13 AS NOTED FEDERAL PROJECT NO. SHEET NO. TOTAL SHEETS 91 --- CHECKED FAY, SPOFFORD & THORNDIKE, INC - BEDFORD, NH -TRACED MAB 6/13 TD 6/13 CHECKED

CITY OF PORTSMOUTH

DEPARTMENT OF PUBLIC WORKS

BRIDGE NO. 198/034

STATE PROJECT 14493

NOTES

1. SEE FRAMING PLAN FOR LOCATIONS OF CROSSFRAMES AND UTILITY SUPPORTS.

Point Along Span	CL Brg. Abut. No. 1	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	CL Brg. Pier No. 1
Station	108+76.75	108+88.75	109+00.75	109+12.75	109+24.75	109+36.75	109+48.75	109+60.75	109+72.75	109+84.75	109+96.75
Girder G1	23.08	23.19	23.32	23.45	23.57	23.66	23.75	23.83	23.93	24.04	24.18
Girder G2	23.26	23.37	23.50	23.63	23.75	23.84	23.93	24.01	24.11	24.22	24.36
Girder G3	23.34	23.45	23.58	23.71	23.83	23.92	24.01	24.09	24.19	24.30	24.44
Girder G4	23.16	23.27	23.40	23.53	23.65	23.74	23.83	23.91	24.01	24.12	24.26
Girder G5	22.98	23.09	23.22	23.35	23.47	23.56	23.65	23.73	23.83	23.94	24.08
Point Along Span	CL Brg. Pier No. 1	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	CL Brg. Pier No.

Point Along Span	CL Brg. Pier No. 1	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	CL Brg. Pier No. 2
Station	109+96.75	110+14.55	110+32.35	110+50.15	110+67.95	110+85.75	111+03.55	111+21.35	111+39.15	111+56.95	111+74.75
Girder G1	24.18	24.41	24.68	24.91	25.07	25.12	25.04	24.86	24.60	24.30	23.98
Girder G2	24.36	24.59	24.86	25.09	25.25	25.30	25.22	25.04	24.78	24.48	24.16
Girder G3	24.44	24.67	24.94	25.17	25.33	25.38	25.30	25.12	24.86	24.56	24.24
Girder G4	24.26	24.49	24.76	24.99	25.15	25.20	25.12	24.94	24.68	24.38	24.06
Girder G5	24.08	24.31	24.58	24.81	24.97	25.02	24.94	24.76	24.50	24.20	23.88

Point Along Span	CL Brg. Pier No. 2	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	CL Brg. Abut. No. 2
Station	111+74.75	111+86.75	111+98.75	112+10.75	112+22.75	112+34.75	112+46.75	112+58.75	112+70.75	112+82.75	112 +9 4.75
Girder G1	23.98	23.78	23.60	23.43	23.27	23.12	22.95	22.77	22.56	22.33	22.08
Girder G2	24.16	23.96	23.78	23.61	23.45	23.30	23.13	22.95	22.74	22.51	22.26
Girder G3	24.24	24.04	23.86	23.69	23.53	23.38	23.21	23.03	22.82	22.59	22.34
Girder G4	24.06	23.86	23.68	23.51	23.35	23.20	23.03	22.85	22.64	22.41	22.16
Girder C5	23.88	23.68	23.50	23.33	23.17	23.02	22.85	22.67	22.46	22.23	21.98

BOTTOM OF DECK SLAB ELEVATIONS

€ GIRDER ELEVATION IN TABLE GIVEN AT THIS POINT (BOTTOM OF SLAB) — -2 5/8" PAVEMENT, BARRIER MEMBRANE L8 1/2" DECK SLAB - $2\frac{1}{2}$ " TYP. 5" TYPICAL DISTANCE FROM TOP OF WEB TO -BLOCKING DISTANCE=5" BOTTOM OF DECK SLAB-LESS FLANGE THICKNESS

DECK SLAB ELEVATION NOTES

1. AFTER THE STRUCTURAL STEEL IS ERECTED BUT BEFORE THE DECK FORMS ARE BUILT, ELEVATIONS ON THE TOP FLANGES OF THE GIRDERS SHALL BE OBTAINED BY THE CONTRACTOR AT THE POINTS INDICATED IN THE TABLE. THE DIFFERENCE BETWEEN THE ELEVATIONS OBTAINED AND THOSE IN THE TABLE IS THE ACTUAL BLOCKING DISTANCE FROM THE TOP OF THE GIRDER TO THE BOTTOM OF THE DECK SLAB AT THE CENTERLINE OF THE GIRDER. SEE ELEVATION TABLE AND HAUNCH DETAIL THIS SHEET.

2. ELEVATIONS SHOWN IN THE TABLE ARE FINISHED BOTTOM OF SLAB ELEVATIONS ADJUSTED FOR TOTAL DEAD LOAD DEFLECTION, LESS THE DEFLECTION DUE TO GIRDER WEIGHT.

SHEET SCALE

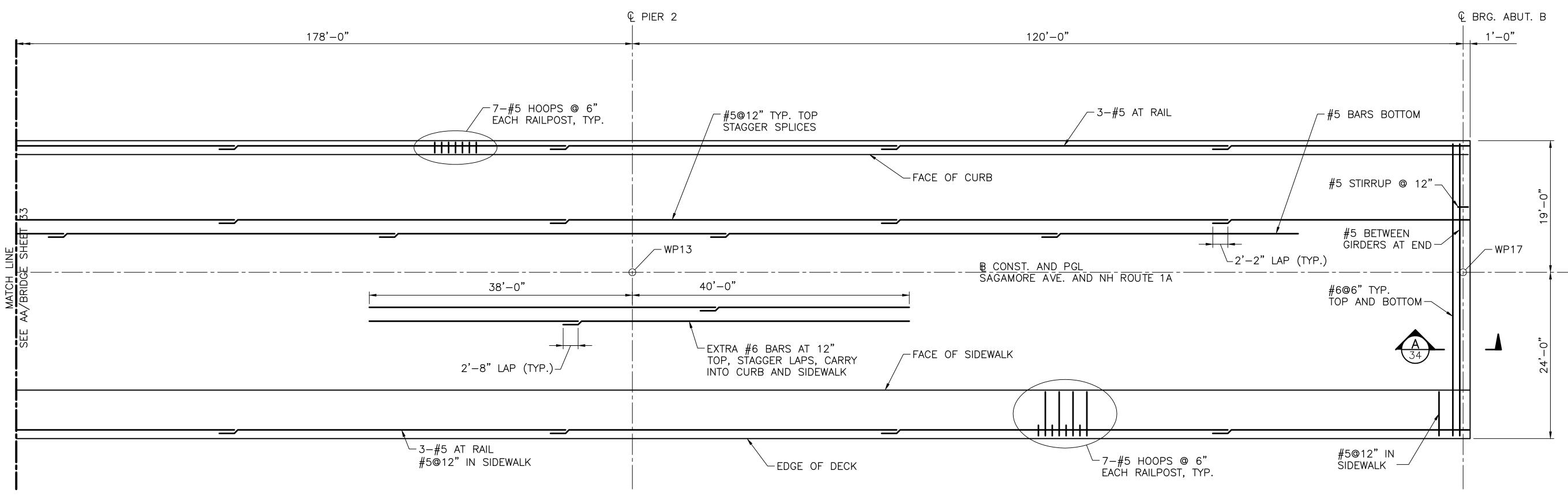
AS NOTED

FAY, SPOFFORD & THORNDIKE, INC - BEDFORD, NH -

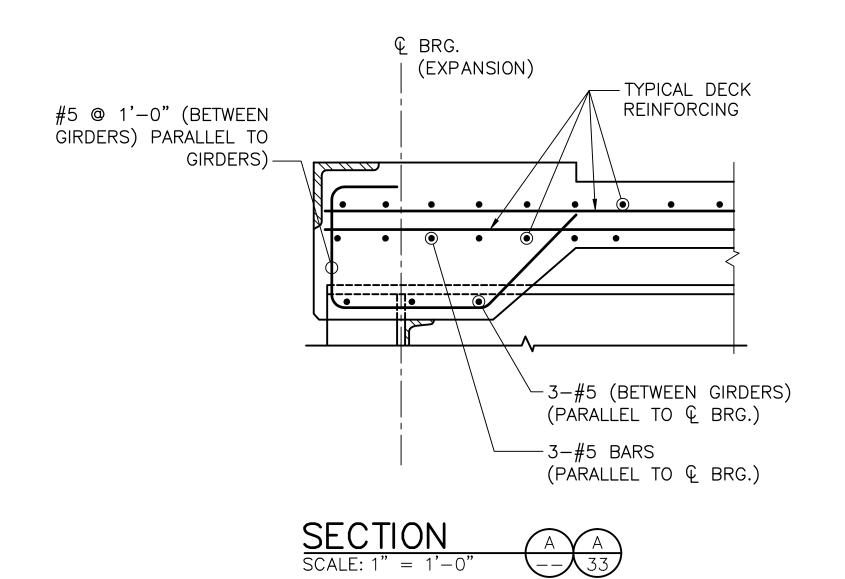
	CITY OF PORTSMOUTH												
	DEPARTMENT OF PUBLIC WORKS												
TOWN PORTSMOUTH	BRIDGE NO.	198/	034 STATE PROJE	ECT 144	93								
LOCATION SAGAMORE AVE	& N.H. ROUTE 1A O	VER SA	AGAMORE CREEK										
BOTTOM OF SLAB	ELEVATIONS A	ND D	ECK SLAB DET	AILS	BRIDGE SHEET								
BY DATE	BY	DATE	REVISIONS AFTER PROPO		32 of 41								
DESIGNED TD 5/13	CHECKED MAB	5/13	-		FILE NUMBER								
DRAWN FLC 5/13	CHECKED TD	5/13											
TRACED	CHECKED		FEDERAL PROJECT NO.	SHEET NO.	TOTAL SHEETS								
QUANTITIES TD 6/13	CHECKED MAB	6/13	X-A000(417)	46	91								

HAUNCH DETAIL SCALE: 3/4" = 1'-0"





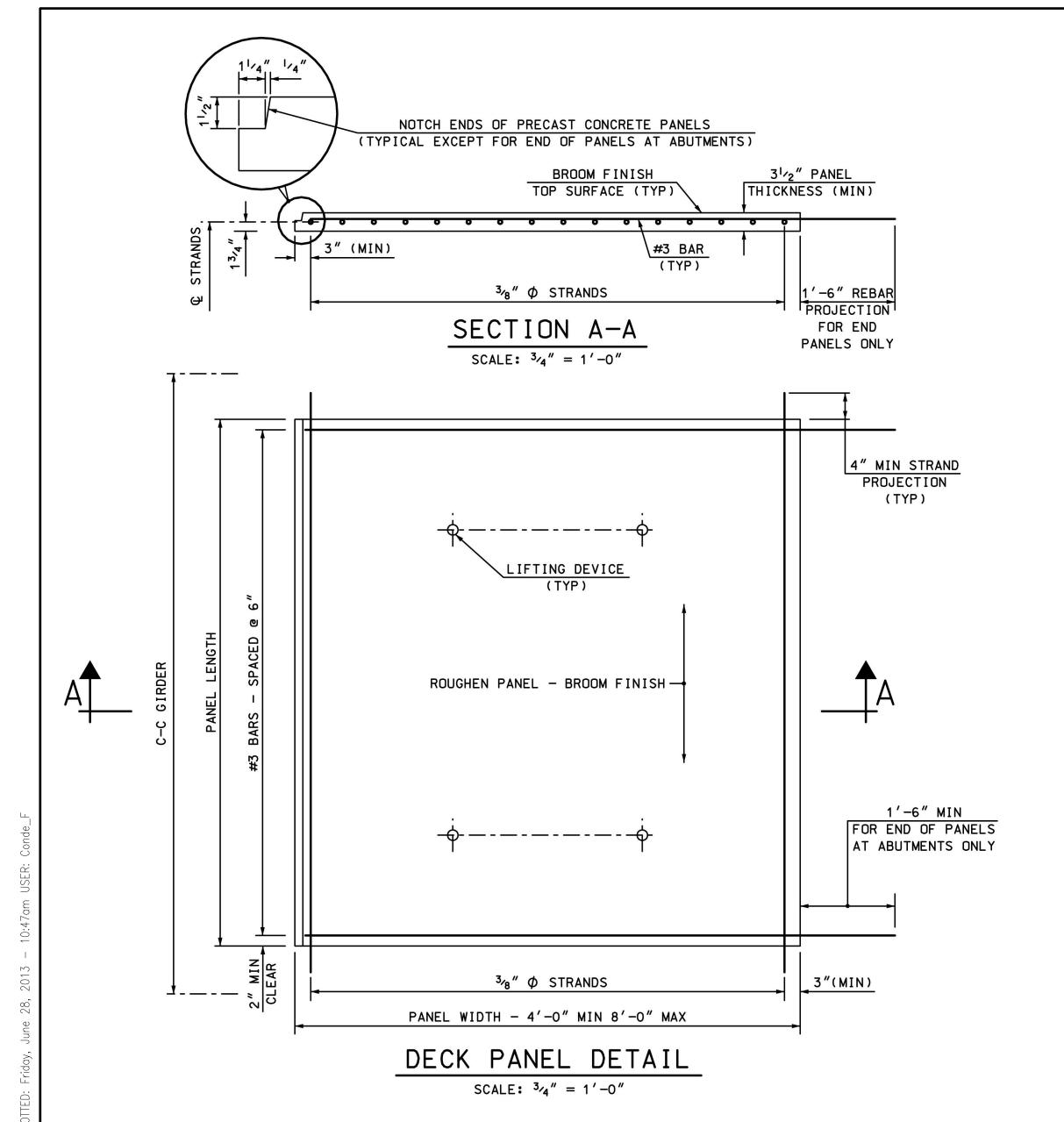
DECK REINFORCEMENT PLAN SCALE: 1/8" = 1'-0"

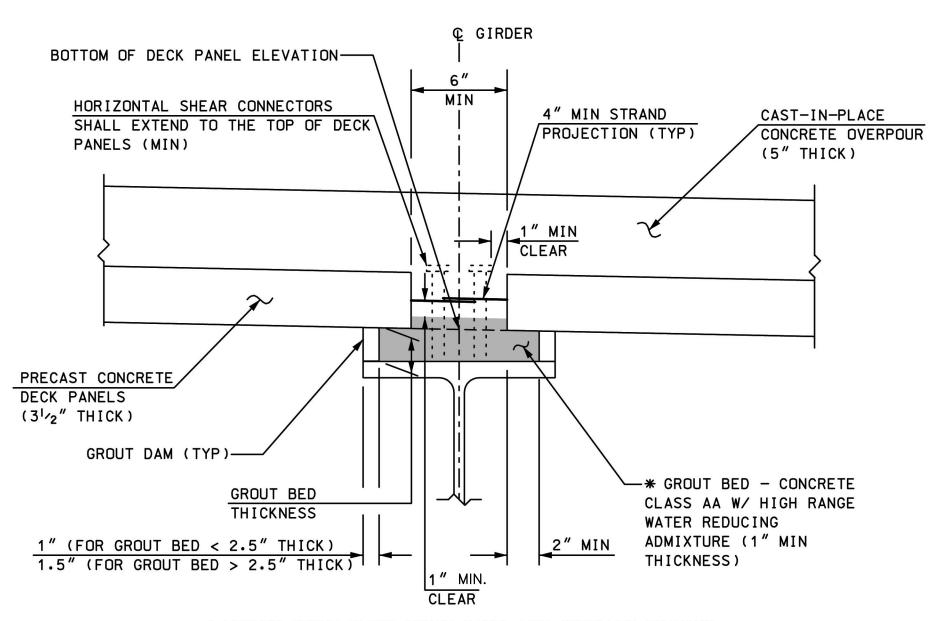


NOTES

1. SEE DECK SECTION FOR REINFORCING BAR SPACING.

			CITY OF PORTSMOUTH DEPARTMENT OF PUBLIC WORKS									
		TOWN	PORTSMOUTH			BRIDGE NO.	198/	034 STATE PRO	JECT 1449	93		
		LOCATIO	N SAGAMORE	AVE.	& N.H.	ROUTE 1A OV	/ER SA	AGAMORE CREEK				
			DECK PL	AN A	AND	SECTIONS	_	SHEET 2 OF	2	BRIDGE SHEET		
			BY	DATE		BY	DATE	REVISIONS AFTER PROP	OSAL DATE	34 of 41		
	SHEET SCALE	DESIGNED	TD	5/13	CHECKED	MAB	5/13			FILE NUMBER		
	AS NOTED	DRAWN	FLC	5/13	CHECKED	TD	5/13					
FAY, SPOFFORD & THORN	DIKE. INC	TRACED			CHECKED			FEDERAL PROJECT NO.	SHEET NO.	TOTAL SHEETS		
- BEDFORD, NH -	,	QUANTITIES	S TD	6/13	CHECKED	MAB	6/13	X-A000(417)	48	91		





* ENSURE GROUT FLOWS UNDER PANEL FOR COMPLETE BEARING

STEEL GIRDER HAUNCH DETAIL

SCALE: 2'' = 1'-0''

PRESTRESSED CONCRETE DECK PANEL NOTES

- (1) CONCRETE STRENGTH: f'c = 6,000 PSI MINIMUM AT 28 DAYS SEE TABLE A & B f'ci = 4,000 PSI MINIMUMDECK PANEL DESIGN
- (2) PRESTRESSING STRANDS SHALL BE 3/8 in. DIAMETER, GRADE 270 SEVEN WIRE LOW-RELAXATION TYPE, CONFORMING TO THE REQUIREMENTS OF ASTM A416. ALL STRANDS SHALL BE PULLED TO HAVE A NET TENSION OF 17.2 KIPS PER STRAND AFTER ALLOWING FOR CHUCK SLIPPAGE.
- (3) THE TOP SURFACE OF THE DECK PANELS SHALL BE BROOMED TO A SURFACE ROUGHNESS OF 0.06 in. BROOM THE SURFACE PARALLEL TO THE STRAND.
- (4) IF HIGH DENSITY EXPANDED POLYSTYRENE FOAM IS USED AS A TEMPORARY SUPPORT, IT SHALL BE CUT IN THE FIELD TO THE REQUIRED HEIGHT AND AFFIXED TO THE GIRDERS WITH AN APPROVED HIGH STRENGTH ADHESIVE.
- (5) PANEL LIFTING LOCATIONS SHOWN ARE ADVISORY ONLY. ACTUAL LIFTING LOCATIONS SHALL BE DETERMINED BY THE FABRICATOR AND INDICATED ON THE SHOP DRAWINGS.
- (6) CORROSION INHIBITOR (CALCIUM NITRITE) ADMIXTURE SHALL BE USED.
- (7) SEE STANDARD SPECIFICATIONS AND SPECIAL PROVISIONS FOR SECTIONS 520 AND 528 FOR ADDITIONAL INFORMATION.
- (8) IF LEVELING SCREWS ARE USED, THEY SHALL BE COMPLETELY REMOVED AFTER THE GROUTING OPERATIONS AND PRIOR TO DECK PLACEMENT. HOLES LEFT BY LEVELING SCREWS SHALL BE FILLED WITH AN APPROVED GROUT PRIOR TO DECK PLACEMENT.
- (9) TEMPORARY BRACING BETWEEN ENDS OF PANELS SHALL BE SUPPLIED AS REQUIRED TO PREVENT PANEL MOVEMENT TRANSVERSE TO THE GIRDERS.
- (10) THE FOLLOWING DECK PANEL DESIGN INFORMATION SHALL BE USED FOR THIS PROJECT: C-C GIRDER SPACING = 9'-0"

PANEL LENGTH PANEL THICKNESS = 3.5"

CONCRETE STRENGTHS f'ci = 6.000 PSIf'c = 4,000 PSI

STRAND SPACING = 6.0"

MULTI SPANS ONLY:

REINFORCEMENT REQUIREMENTS IN TOP MAT OVER PIER TO MEET LRFD 6.10.1.7 BAR SIZE = ALTERNATE #5 AND #6 BARS BAR SPACING = @ 6" SPACING

(11) REINFORCING IN PANELS SHALL BE BLACK BAR EXCEPT FOR END PANELS AT ABUTMENTS WHICH SHALL HAVE EPOXY COATED REBAR. CAST-IN-PLACE OVERPOUR SHALL HAVE EPOXY COATED REBAR AND FOLLOW LAYOUT OF TOP MAT OF STEEL SHOWN ON THE DECK REINFORCING SHEET.

- BEDFORD, NH -

TABLE A - DECK PANEL DESIGN

	(STEEL-GIRDER)												
C-C GIRDER	PANEL LENGTH	PANEL	f'ci	f'c	STRAND								
SPACING	TANLE LENGTH	THICKNESS	(PSI)	(PSI)	SPACING								
5'-6"	5′ - 0″	31/2"	4000	6000	8"								
6'-0"	5′ - 6″	3 ¹ /2"	4000	6000	8"								
6'-6"	6'-0"	3 ¹ /2"	4000	6000	8"								
7'-0"	6'-6"	31/2"	4000	6000	8"								
7′-6″	7'-0"	3 ¹ /2"	4000	6000	8"								
8'-0"	7′-6″	3 ¹ /2"	4000	6000	8"								
8'-6"	8'-0"	3 ¹ /2"	4000	6000	6"								
9'-0"	8'-6"	3 ¹ /2"	4000	6000	6"								
9'-6"	9'-0"	3 ¹ /2"	4000	6000	5 <i>"</i>								
10'-0"	9'-6"	31/2"	5000	6000	41/2"								

DESIGN CRITERIA:

- LIVE LOAD = HL-93

- ALLOWABLE TENSION IN CONCRETE = $0.19 \, \text{Vf'c}$ = 0.750 ksi (W/ f'ci = 4 ksi)- MAXIMUM INITIAL COMPRESSION

- C-I-P DECK THICKNESS = 5"

 PAVEMENT THICKNESS $= 2^{1/2}''$

- STEEL FLANGE WIDTH = 12"

- GROUT DAM WIDTH - GROUT BED THICKNESS

PANEL DESIGN NOTES:

- 1) IF LEVELING SCREWS ARE USED, THEIR LOCATIONS SHALL NOT INTERFERE WITH THE LOCATION
- 2) DECK SLAB THICKNESS IS THE SAME FOR PANEL OPTION OR CAST IN PLACE OPTION (8.5"), THEREFORE ADJUSTMENT IN THE BOTTOM SLAB ELEVATION TABLE IS NOT NEEDED, PER NOTE 2

DECK SLAB ELEVATION NOTES

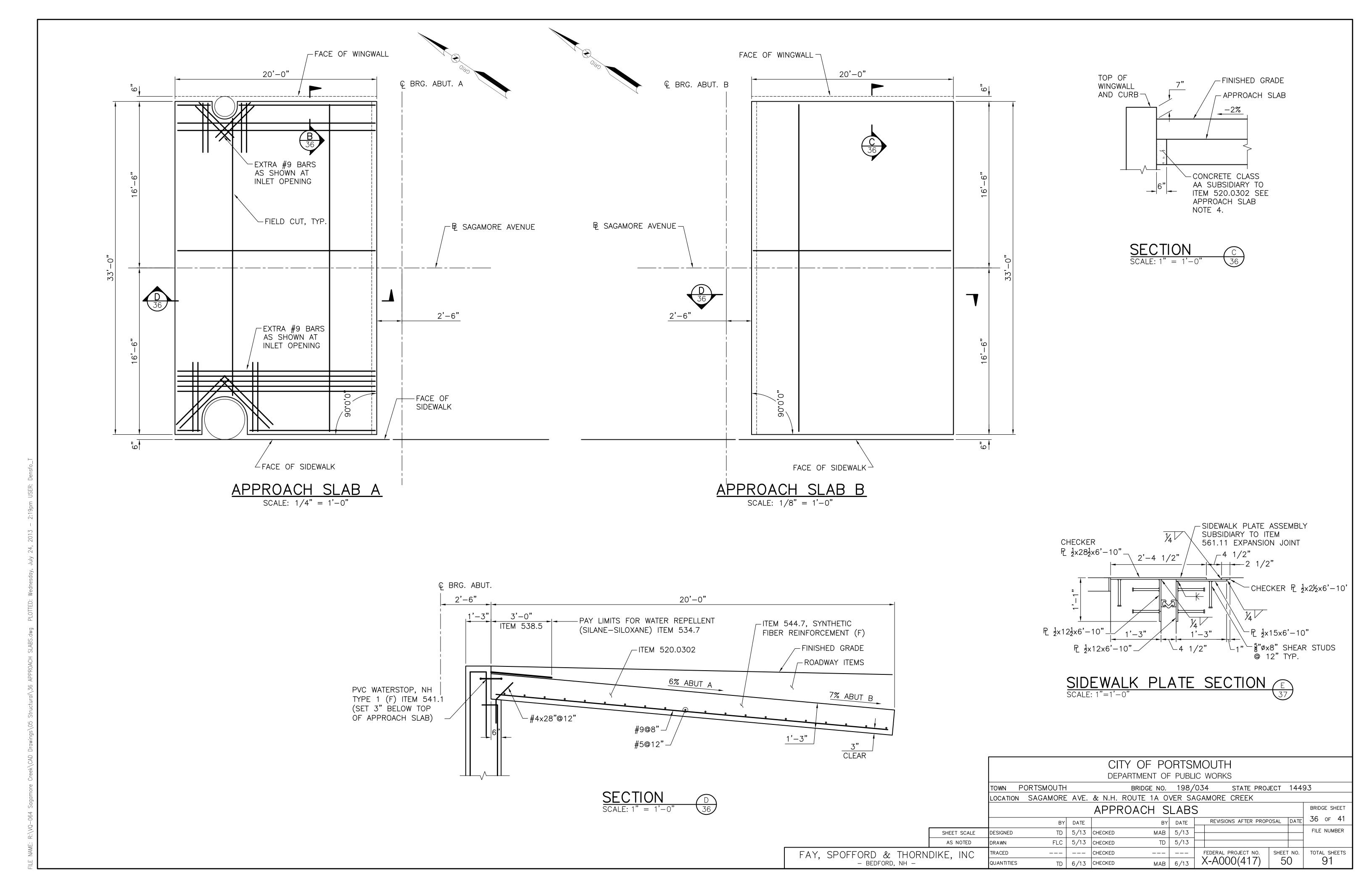
- 1) AFTER THE GIRDERS ARE ERECTED AND BEFORE PRECAST DECK PANELS ARE SET, ELEVATIONS ON THE TOP FLANGE OF THE GIRDERS ARE TO BE OBTAINED AT THE POINTS INDICATED IN "BOTTOM OF SLAB ELEVATION TABLE" DETAILED IN THE PLANS AND GIRDER HAUNCH DETAILS ON THIS SHEET.
- 2) THE BOTTOM OF SLAB ELEVATIONS SHALL BE ADJUSTED BY THE DIFFERENCE BETWEEN THE CAST-IN-PLACE DECK THICKNESS AND THE TOTAL COMPOSITE DECK THICKNESS.

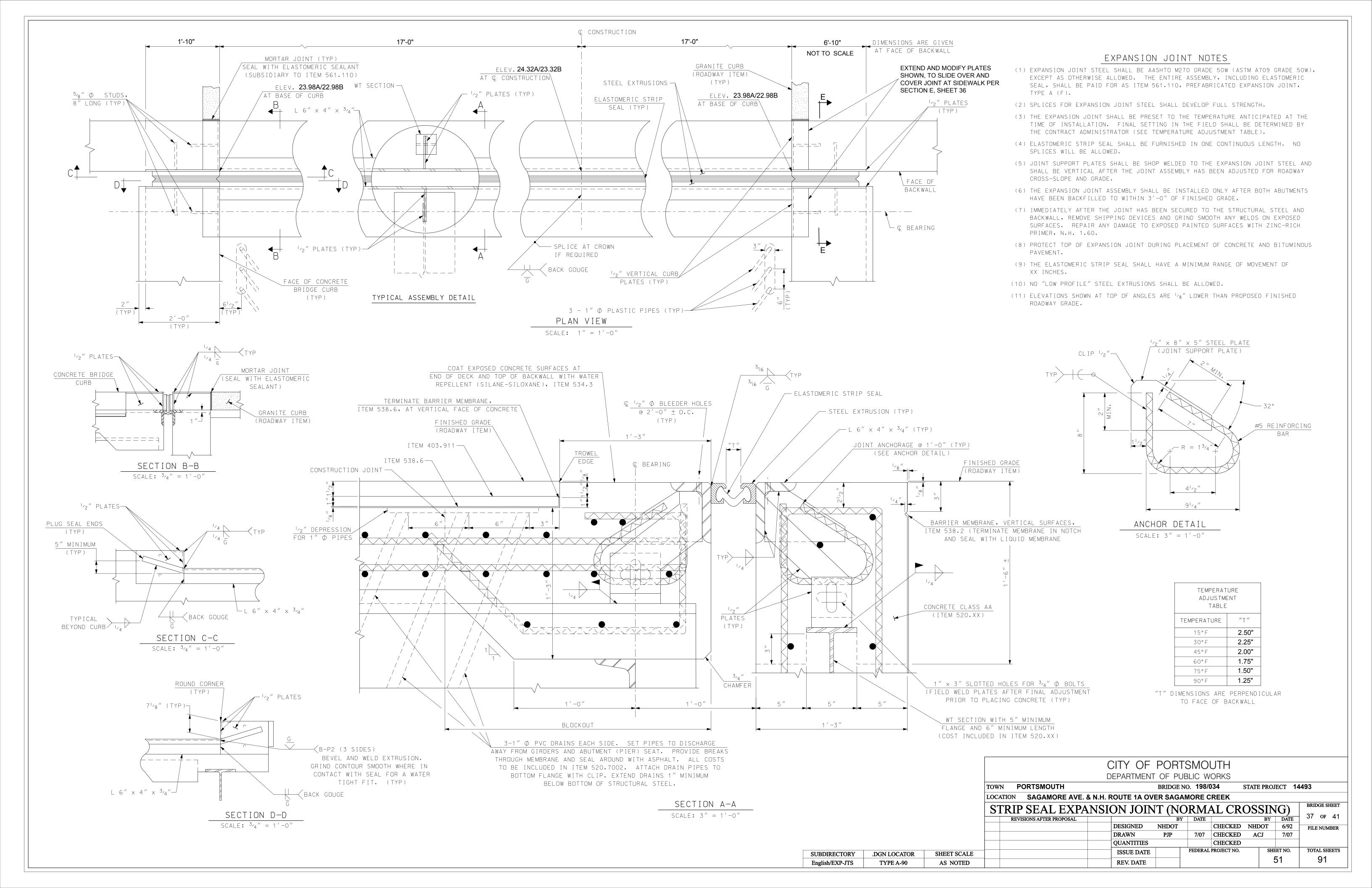
CITY OF PORTSMOUTH DEPARTMENT OF PUBLIC WORKS

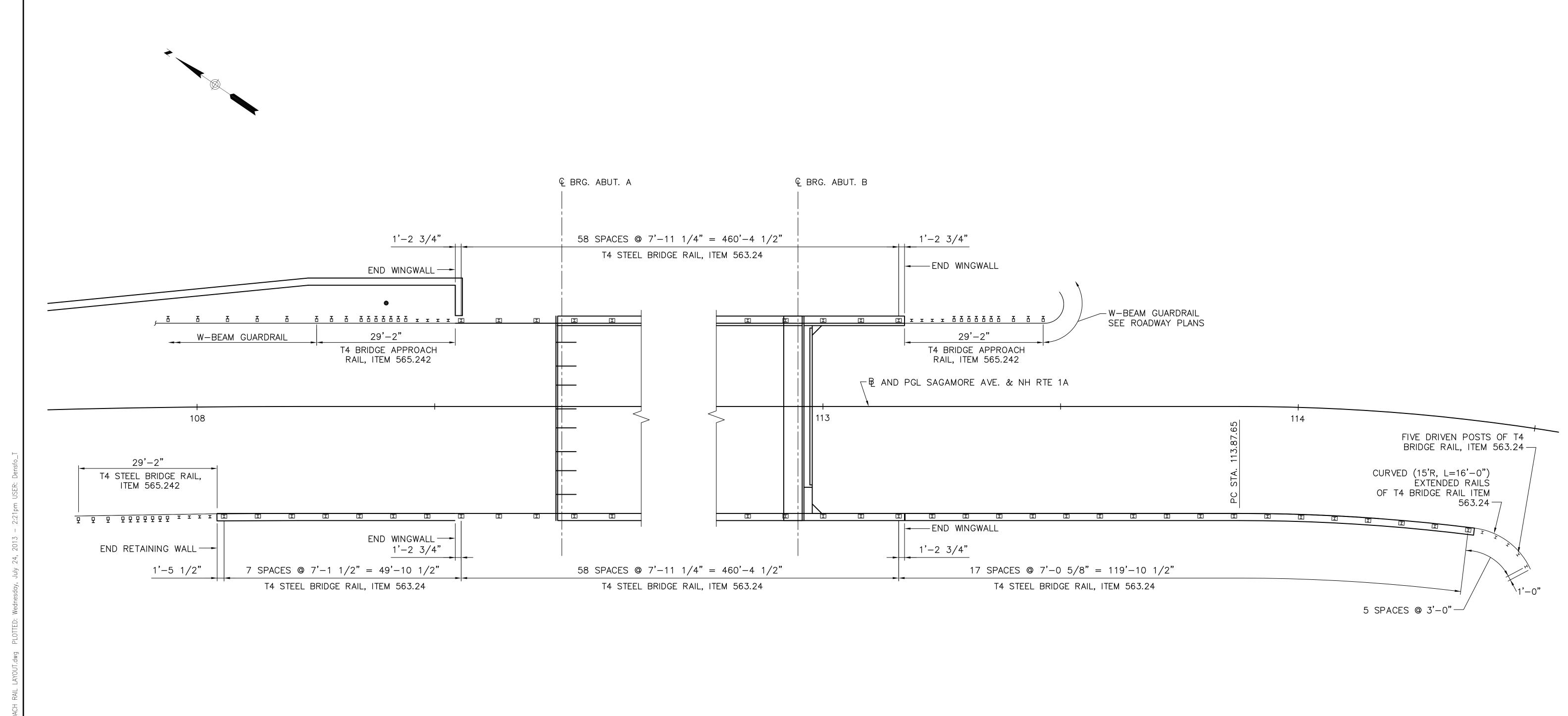
TOWN PORTSMOUTH STATE PROJECT 14493 BRIDGE NO. 198/034

LOCATION SAGAMORE AVE. & N.H. ROUTE 1A OVER SAGAMORE CREEK

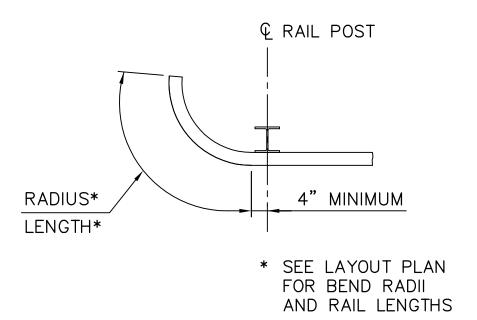
PRECAST CONCRETE DECK PANEL DETAILS BRIDGE SHEET 35 of 41 REVISIONS AFTER PROPOSAL DATE BY DATE BY DATE FILE NUMBER SHEET SCALE DESIGNED NHDOT 4/02 CHECKED NHDOT 4/02 NHDOT | 12/10 NHDOT | 12/10 | CHECKED AS NOTED DRAWN FAY, SPOFFORD & THORNDIKE, INC FEDERAL PROJECT NO. | SHEET NO. | TOTAL SHEETS TRACED --- | --- | CHECKED X-A000(417) --- CHECKED





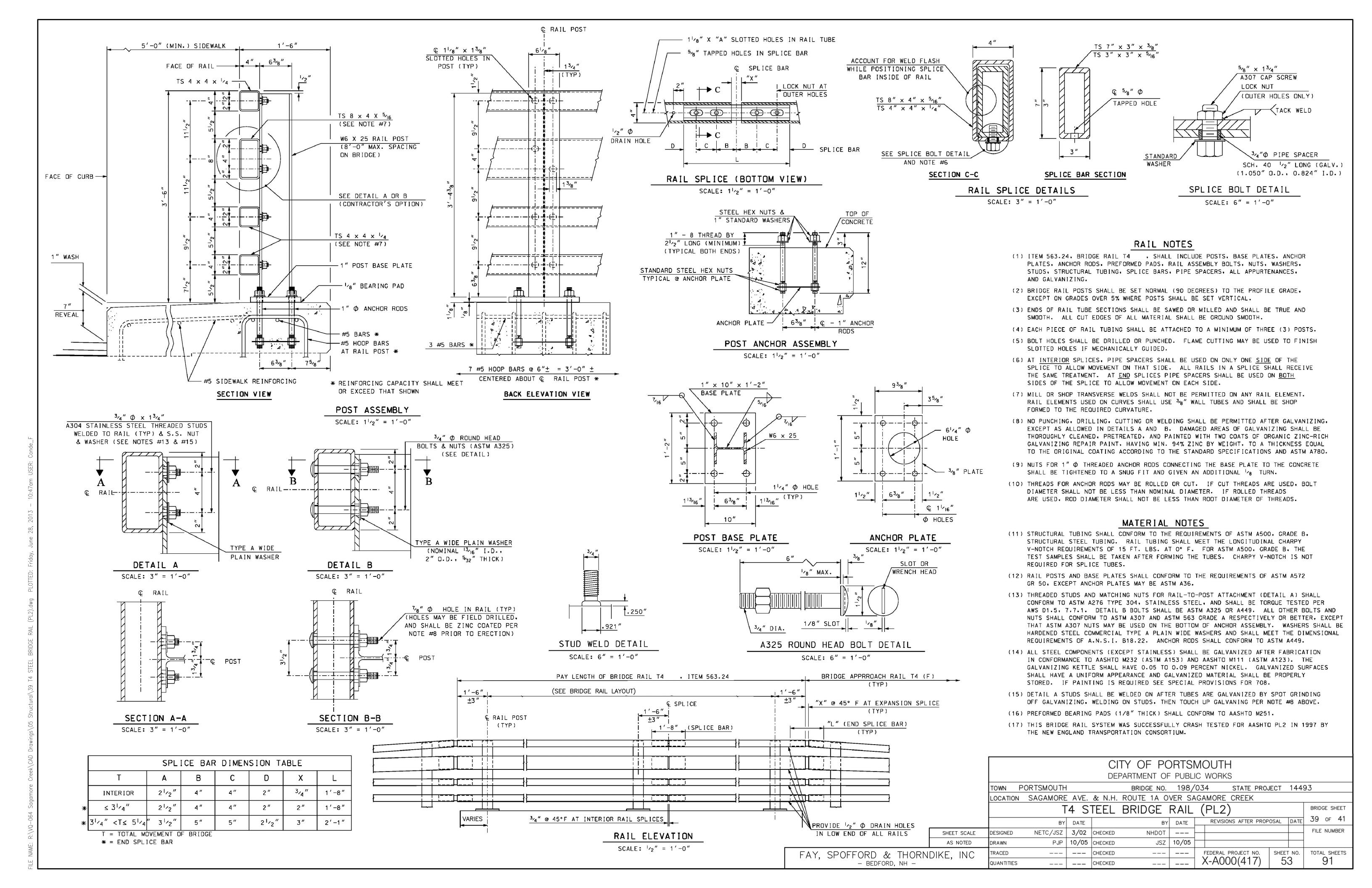


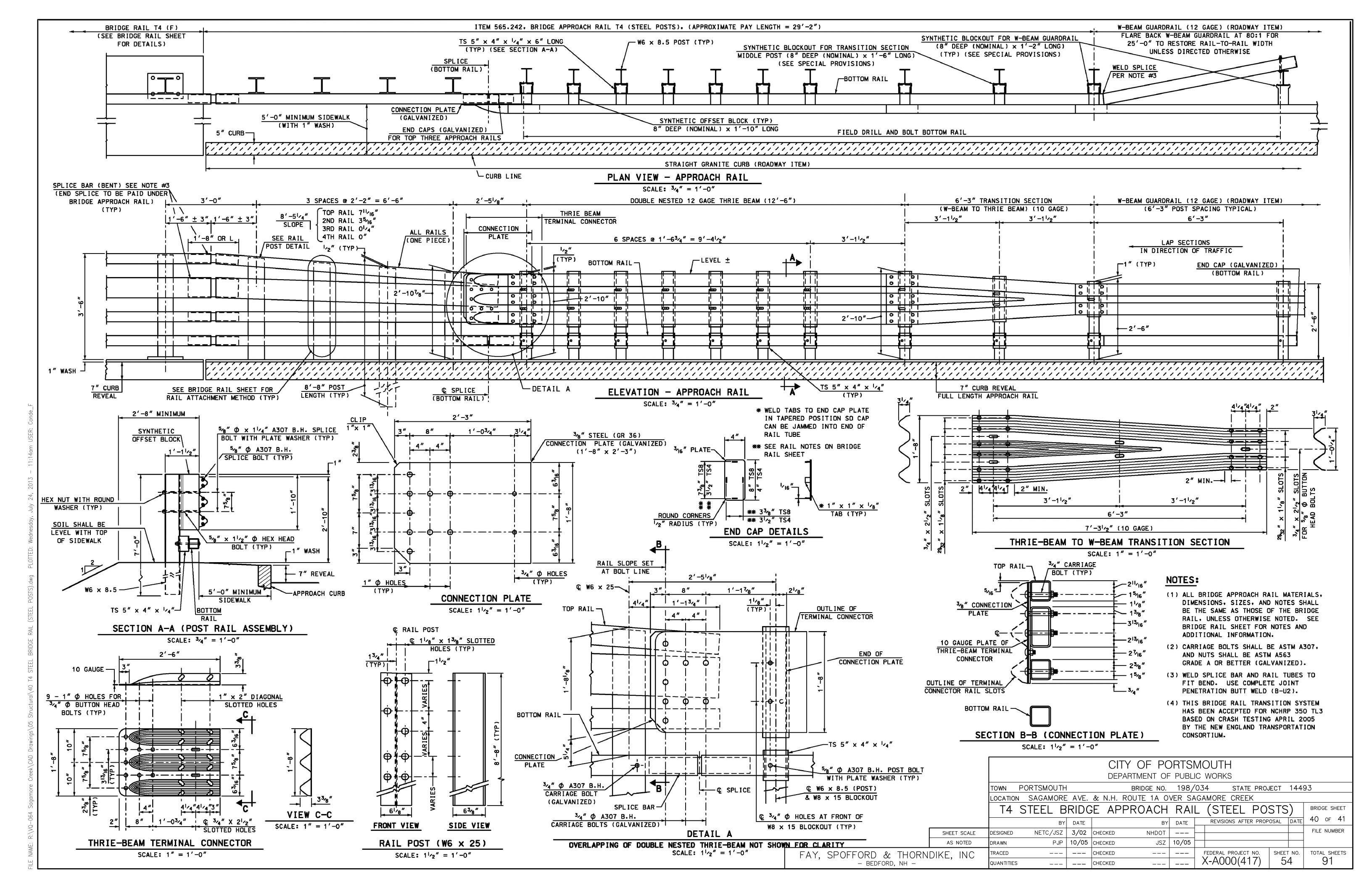


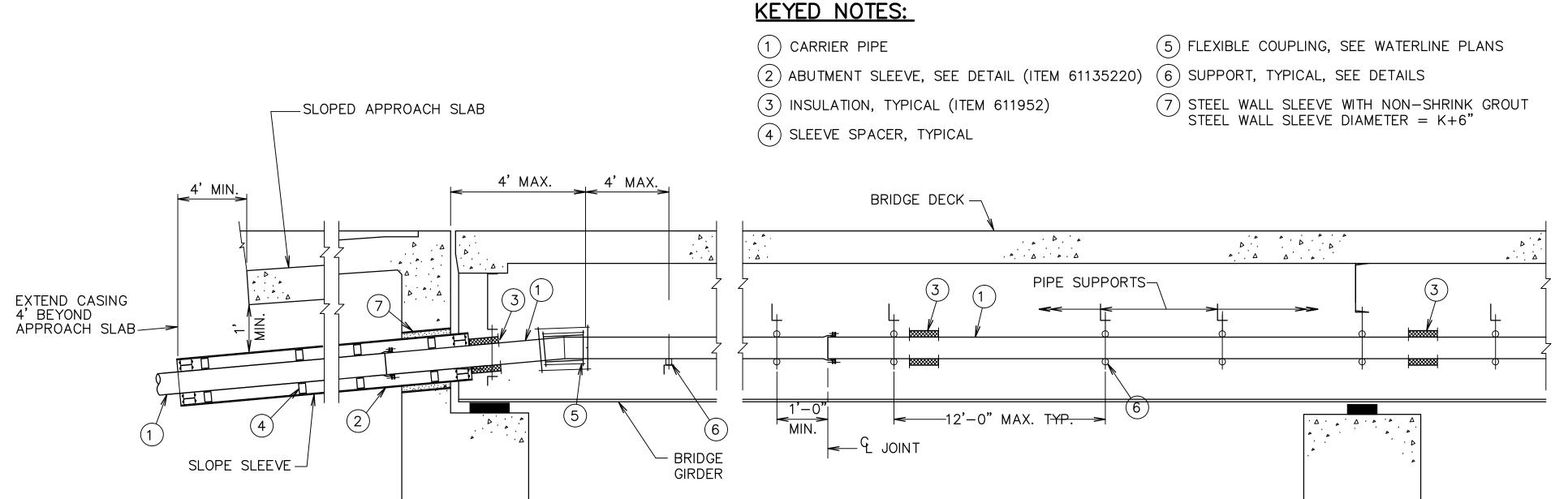


RAIL	BEND	DETAIL					
	NOT TO SC						

		CITY OF PORTSMOUTH DEPARTMENT OF PUBLIC WORKS								
		TOWN PORTSMOUTH BRIDGE NO. 198/034 STATE PROJECT 14493							93	
		LOCATION SAGAMORE AVE. & N.H. ROUTE 1A OVER SAGAMORE CREEK								
		BRIDGE AND APPROACH RAIL LAYOUT BRIDGE SHEE							BRIDGE SHEET	
			BY	DATE		BY	DATE	REVISIONS AFTER PROF	POSAL DATE	38 of 41
	SHEET SCALE	DESIGNED	TD	5/13	CHECKED	MAB	5/13			FILE NUMBER
	AS NOTED	DRAWN	FLC	5/13	CHECKED	TD	5/13			
FAY, SPOFFORD & THORN	DIKE. INC	TRACED			CHECKED			FEDERAL PROJECT NO.	SHEET NO.	TOTAL SHEETS
- BEDFORD, NH -		QUANTITIES	TD	6/13	CHECKED	MAB	6/13	X-A000(417)	52	91



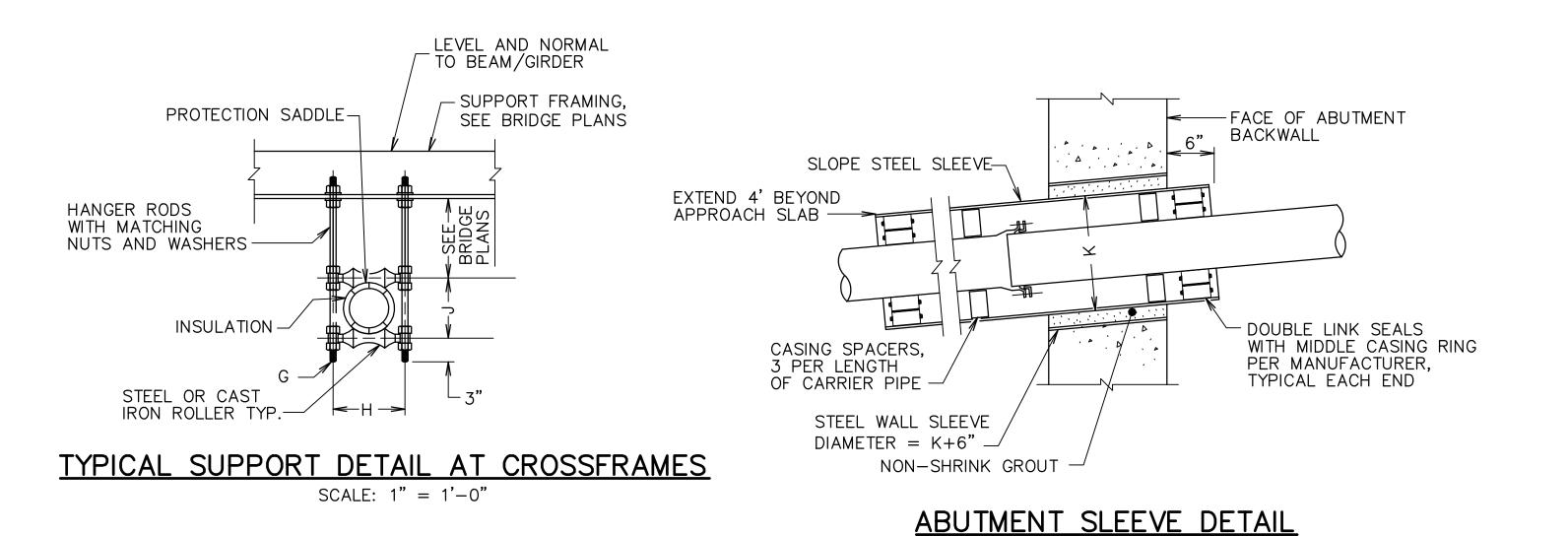




ABUTMENT

NOTES:

- 1. MATERIALS HANGER RODS - GALVANIZED ASTM A307 WITH MATCHING NUTS.
- 2. FOR SUPPORT FRAMING DETAILS SEE BRIDGE PLANS. STRUCTURAL STEEL FOR SUPPORTS SHALL BE THE SAME AS THAT FOR THE GIRDERS AND PAID UNDER ITEM 550.1. IF THE GIRDERS ARE PAINTED, THE ANGLES SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A123.
- 3. ABUTMENT-CASING SLEEVE AS DETAILED TO BE USED AT EACH ABUTMENT. PROVIDE DOUBLE LINK-SEAL AT EACH END OF EACH CASING SLEEVE.
- 4. EXPANSION JOINT DRESSER STYLE 63, TYPE 3, PAID AS ITEM 61106210
- 5. GALVANIZATION MISCELLANEOUS HARDWARE: RODS, NUTS, ETC. SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153. WHEN THE SUPPORTING ANGLES TO WHICH THE RODS ARE ATTACHED ARE WEATHERING STEEL, A NEOPRENE OR VINYL WASHER SHALL BE PLACED BETWEEN THE ANGLE SURFACE (ON BOTH SIDES) AND THE NUT/WASHER TO ISOLATE THE CONTACT BETWEEN THE TWO SURFACES.
- 6. PROVIDE 2" URETHANE WATERLINE INSULATION WITH 6 GAGE ALUMINUM JACKET THROUGHOUT.
- 7. PAYMENT WATER LINE (ITEM 61106210) SHALL INCLUDE FURNISHING AND INSTALLING DUCTILE IRON WATER MÁIN, EXPANSION JOINT, TESTING, DISINFECTING (WHEN REQUIRED), INSULATION AND COVER, HANGERS, ROLLERS, RODS, PROTECTION SADDLES, ABUTMENT SLEEVES, LINK SEALS, CASING UNDER APPROACH SLAB AND MISCELLANEOUS HARDWARE AS DETAILED ON THIS DRAWING.



<u>PIER</u>

SCALE: 1" = 1'-0"

DIMENSIONS								
PIPE Ø	G	Н	J	K				
6"	7/8"	1'-6"	1'-1"	1'-2"				
8"	7/8"	1'-7"	1'-3"	1'-4"				
10"	1"	1'-9"	1'-6"	1'-6"				
12"	1"	1'-11"	1'-8"	1'-8"				
14"	1"	2'-1"	1'-9"	1'-10"				
16"	1"	2'-3"	1'-11"	2'-0"				
18"	1-1/8"	2'-5"	2'-2"	2'-2"				
20"	1-1/4"	2'-8"	2'-4"	2'-4"				
24"	1-1/2"	2'-11"	2'-9"	2'-10"				
C - DIAMETER OF ROD								

G = DIAMETER OF ROD

WATERLINE SUPPORT DETAILS

WATERLINE ELEVATION

NOT TO SCALE

		CITY OF PORTSMOUTH								
		DEPARTMENT OF PUBLIC WORKS								
		TOWN PORTSMOUTH BRIDGE NO. 198/034 STATE PROJECT 14493							93	
		LOCATION SAGAMORE AVE. & N.H. ROUTE 1A OVER SAGAMORE CREEK								
		WATERLINE SUPPORT DETAILS BRIDGE SHEET								
			BY	DATE		BY	DATE	REVISIONS AFTER PROF	OSAL DATE	41 OF 41
	SHEET SCALE	DESIGNED	TD	5/13	CHECKED	MAB	5/13			FILE NUMBER
	AS NOTED	DRAWN	FLC	5/13	CHECKED	TD	5/13			
FAY, SPOFFORD & THORN	DIKE. INC	TRACED			CHECKED			FEDERAL PROJECT NO.	SHEET NO.	TOTAL SHEETS
- BEDFORD, NH -	· · · · · · · · · · · · · · · · · · ·	QUANTITIES	S TD	6/13	CHECKED	MAB	6/13	X-A000(417)	55	91