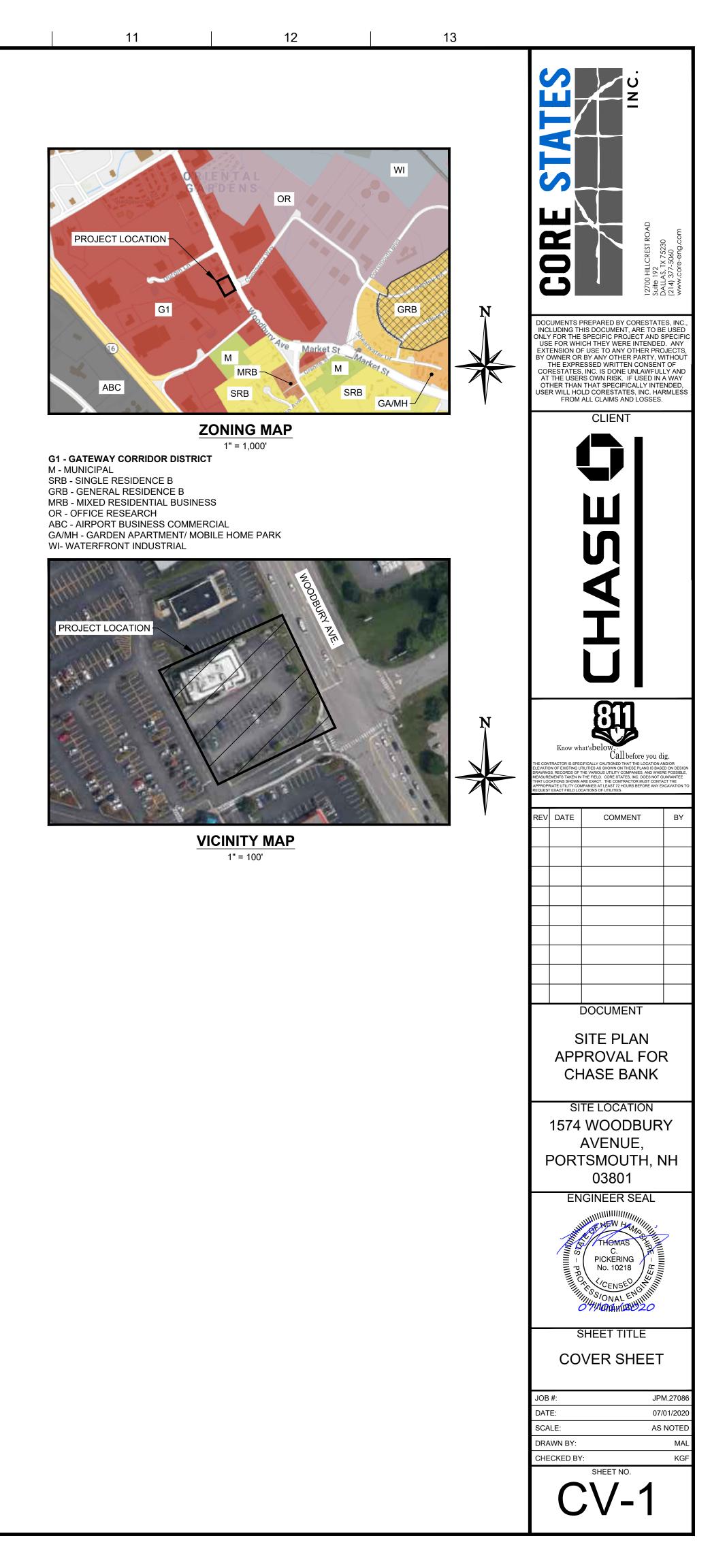
| APPLICANT | J.P. MORGAN CHASE BANK 1450 BRICKELL AVENUE 3RD FLOOR | | | | | _ |
|--|---|---|--|---|----------------|------|
| | MIAMI, FL 33131 CONTACT: CHRIS FOIT (786) 473-1769 | SIT. | E PLAN | | Ο\/Α | |
| OWNER | RICHARD FUSEGNI 201 KEARSARGE WAY PORTSMOUTH, NH 03801 | | | | | |
| CIVIL ENGINEER | CONTACT: SCOTT MITCHELL (603) 475-377 CORE STATES INC. | | | OR | | |
| GIVIL ENGINEER | 9 GALEN STREET, SUITE 117 WATERTOWN, MA 02472 CONTACT: ALAN D. ROSCOE, P.E. (857) 500-4702 | | | UN | | |
| ARCHITECT | CORE STATES INC. 201 S. MAPLE AVE AMBLER, PA 19002 CONTACT: KEN MACKENZIE (267) 464-8048 | | | | | |
| SURVEYOR | ALLEN & MAJOR ASSOCIATES, INC. 400 HARVEY ROAD MANCHESTER, NH 03103 CONTACT: JAMES P. SMITH NH LLS (603) 627-5500 | | | | | |
| GOVERNING A | GENCIES CONTACTS: | | | | | |
| PLANNING | PLANNING DEPARTMENT 1 JUNKINS AVENUE, 3RD FLOOR PORTSMOUTH, NH 03801 CONTACT: JULIET WALKER, PLANNING DIRECTOR (603) 610-7216 | | | | | |
| BUILDING | INSPECTION DEPARTMENT 1 JUNKINS AVENUE PORTSMOUTH, NH 03801 CONTACT: ROBERT MARSILIA, CHIEF BUILDING INSPECTOR (603) 610-7243 | | PROPOSED | CHASE B | SANK | |
| FIRE AUTHORITY | FIRE DEPARTMENT 170 COURT STREET PORTSMOUTH, NH 03801 CONTACT: TODD GERMAIN, INTERIM FIRE CHIEF (603) 427-1515 | | 1574 WOOE | DBURY AVEN | JUE | |
| UTILITY CONT | ACTS: | | CITY OF F | PORTSMOUT | ⁻ H | |
| | | | | | | |
| GAS | UNITIL 6 LIBERTY LANE WEST HAMPTON, NH 03842 | | ACCESSORS | 5 MAP 238. LC | JI 17 | |
| GAS ELECTRIC AND FIRE ALARM | 6 LIBERTY LANE WEST HAMPTON, NH 03842 (866) 933-3820 EVERSOURCE 55 BEARFOOT ROAD NORTHBOROUGH, MA 01532 | | ACCESSORS SHAM COUNT | • | | E |
| | 6 LIBERTY LANE WEST HAMPTON, NH 03842 (866) 933-3820 EVERSOURCE 55 BEARFOOT ROAD | | | • | | E |
| ELECTRIC AND FIRE ALARM | 6 LIBERTY LANE WEST HAMPTON, NH 03842 (866) 933-3820 EVERSOURCE 55 BEARFOOT ROAD NORTHBOROUGH, MA 01532 (800) 322-3223 PORTSMOUTH PUBLIC WORKS DEPARTMENT 680 PEVERLY HILL ROAD PORTSMOUTH, NH 03801 | ROCKING | SHAM COUNT | Y, NEW HAN | IPSHIRI | |
| ELECTRIC AND FIRE ALARM | 6 LIBERTY LANE WEST HAMPTON, NH 03842 (866) 933-3820 EVERSOURCE 55 BEARFOOT ROAD NORTHBOROUGH, MA 01532 (800) 322-3223 PORTSMOUTH PUBLIC WORKS DEPARTMENT 680 PEVERLY HILL ROAD PORTSMOUTH, NH 03801 (603) 427-1530 PORTSMOUTH PUBLIC WORKS DEPARTMENT 680 PEVERLY HILL ROAD PORTSMOUTH, NH 03801 (603) 427-1530 VERIZON 185 FRANKLIN STREET BOSTON, MA 02107 | | SHAM COUNT SHEET IN ER DESCRIPTION CIVIL PLANS | Y, NEW HAW | | 1BER |
| ELECTRIC AND FIRE ALARM WATER SEWER | 6 LIBERTY LANE WEST HAMPTON, NH 03842 (866) 933-3820 EVERSOURCE 55 BEARFOOT ROAD NORTHBOROUGH, MA 01532 (800) 322-3223 PORTSMOUTH PUBLIC WORKS DEPARTMENT 680 PEVERLY HILL ROAD PORTSMOUTH, NH 03801 (603) 427-1530 PORTSMOUTH PUBLIC WORKS DEPARTMENT 680 PEVERLY HILL ROAD PORTSMOUTH, NH 03801 (603) 427-1530 VERIZON 185 FRANKLIN STREET | ROCKING | SHAM COUNT | Y, NEW HAN | | 1BER |
| ELECTRIC AND FIRE ALARM WATER SEWER TELEPHONE | 6 LIBERTY LANE WEST HAMPTON, NH 03842 (866) 933-3820 EVERSOURCE 55 BEARFOOT ROAD NORTHBOROUGH, MA 01532 (800) 322-3223 PORTSMOUTH PUBLIC WORKS DEPARTMENT 680 PEVERLY HILL ROAD PORTSMOUTH, NH 03801 (603) 427-1530 PORTSMOUTH PUBLIC WORKS DEPARTMENT 680 PEVERLY HILL ROAD PORTSMOUTH, NH 03801 (603) 427-1530 VERIZON 185 FRANKLIN STREET BOSTON, MA 02107 (800) 870-9999 COMCAST 179 W MAIN STREET AYER, MA 01432 | ROCKING PAGE NUMB CV-1 CV-2 C-1 C-2 | SHAN COUNT SHEET IN ER DESCRIPTION CIVIL PLANS COVER SHEET GENERAL NOTES DEMOLITION PLAN SITE PLAN | Y, NEW HAW NDEX PREPARED BY CORE STATES CORE STATES CORE STATES CORE STATES CORE STATES | | 1BER |
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| STAN | DARD ABBREVIATIONS | GE | NERAL SITE NOTES: |
|-----------------|---|------|---|
| AC | ACRES | | ALL CONSTRUCTION MATERIALS AND TECHNIQUES OF INSTALLATION SHALL MEET |
| ADA ARCH | AMERICANS WITH DISABILITY ACT | | PERFORMANCE VALUES OF THE MATERIALS SPECIFIED AND COMPLY WITH ALL |
| BC | BOTTOM OF CURB | _ | AUTHORITY HAVING JURISDICTION REGULATIONS AND CODES AND O.S.H.A. STANDARDS. |
| BF | BASEMENT FLOOR | 2. | THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ENSURING THAT THIS PROJECT IS CONSTRUCTED IN ACCORDANCE WITH THESE DOCUMENTS AND IN |
| BK BL | BLOCK BASELINE | | COMPLIANCE WITH CODES INDICATED HEREIN. THE QUALITY OF WORKMANSHIP AND |
| BLD | BUILDING | | INSTALLATION OF MATERIALS SPECIFIED BY THE ARCHITECT/ENGINEER ARE THE RESPONSIBILITY OF THE CONTRACTOR. THE ARCHITECT/ENGINEER WILL NOT BE HELD |
| BOL | BOLLARD | | RESPONSIBLE FOR ANY SUBSTANDARD OR INSUFFICIENT WORKMANSHIP, MATERIALS, OR SERVICES PROVIDED IN THE EXECUTION OF ANY PHASE OF CONSTRUCTION OF |
| BM | BENCH MARK | _ | THIS PROJECT. |
| BRL CF | BUILDING RESTRICTION LINE | 3. | ALL MATERIALS ARE TO BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S REQUIREMENTS. THE GENERAL CONTRACTOR SHALL ENSURE THAT ALL |
| CL | CENTERLINE | | MANUFACTURER'S WARRANTIES WILL BE HONORED. |
| CMP | CORRUGATED METAL PIPE | 4. | ALL CONDITIONS SHOWN TO BE "EXISTING" SHALL BE VERIFIED IN THE FIELD BY THE GENERAL CONTRACTOR PRIOR TO START OF CONSTRUCTION. ANY DISCREPANCIES |
| CONN | CONNECTION | | SHALL BE NOTED AND SUBMITTED TO THE OWNER AND THE ARCHITECT/ENGINEER FOR REVIEW. CHANGES TO THE ORIGINAL DESIGN OF THE PROJECT DUE TO EXISTING |
| CONC | CONCRETE CORRUGATED PLASTIC PIPE | | SITE CONDITIONS MUST BE APPROVED BY BOTH THE OWNER AND THE |
| CY | CUBIC YARDS | 5. | ARCHITECT/ENGINEER PRIOR TO MAKING ANY CHANGES. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR HAVING A THOROUGH |
| DEC | DECORATIVE | | KNOWLEDGE OF EXISTING FIELD CONDITIONS AND OF ALL DRAWINGS AND SPECIFICATIONS RELATED TO THEIR FIELD. THE FAILURE TO ACQUAINT THEMSELVES |
| DEP | DEPRESSED DUCTILE IRON PIPE | | WITH THIS PROJECT AND ONES FIELD OF SERVICE SHALL NOT RELIEVE THEM OF ANY |
| DOM | DOMESTIC | | RESPONSIBILITY FOR PERFORMING THEIR WORK PROPERLY. NO ADDITIONAL COMPENSATION SHALL BE ALLOWED DUE TO THE GENERAL CONTRACTOR'S FAILURE |
| ELEC | ELECTRIC | | TO CONVEY THE NECESSARY KNOWLEDGE TO FAMILIARIZE WORKERS AND SUBCONTRACTORS WITH THIS PROJECT. |
| ELEV | ELEVATION | 6. | THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ENSURING THE SAFETY OF |
| EP ES | EDGE OF PAVEMENT EDGE OF SHOULDER | | ALL PERSONS ON THE JOB SITE AT ALL TIMES INCLUDING (BUT NOT LIMITED TO) SUBCONTRACTORS, FACILITY EMPLOYEES, VENDORS, DESIGN STAFF |
| EW | END OF WALL | | PROFESSIONALS AND INSPECTION PERSONNEL. |
| EX | EXISTING | 7. | THE GENERAL CONTRACTOR SHALL PROVIDE DUMPSTERS, PORTABLE TOILETS AND TEMPORARY POWER FOR UNRESTRICTED PROJECT RELATED USE BY OTHERS FOR |
| FES | | | THE DURATION OF THE PROJECT. |
| FF FH | FINISH FLOOR ELEVATION | . 8. | THE GENERAL CONTRACTOR SHALL COORDINATE PROJECT PHASING AND STORAGE OF MATERIALS WITH THE OWNER AND EROSION AND SEDIMENT CONTROL |
| FG | FINISHED GRADE | | OF MATERIALS WITH THE OWNER AND EROSION AND SEDIMENT CONTROL AUTHORITY. |
| G | GRADE | 9. | THE GENERAL CONTRACTOR IS RESPONSIBLE FOR THE RECEIVING, UNLOADING, STORING AND PROTECTION OF MATERIALS AND EQUIPMENT SUPPLIED BY THE |
| GF | | | OWNER UNTIL IT HAS BEEN INSTALLED AND ACCEPTED BY THE OWNER. |
| GH GL | GRADE HIGH SIDE OF WALL GRADE LOW SIDE OF WALL | 10. | THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR KEEPING THE AREA CLEAN |
| GRT | GRATE | | AND FREE OF DEBRIS AT ALL TIMES DURING CONSTRUCTION. THE GENERAL CONTRACTOR SHALL POWER WASH THE ENTIRE CONSTRUCTION AREA PRIOR TO |
| GV | GATE VALVE | | TURNOVER TO THE OWNER. THE GENERAL CONTRACTOR SHALL FIELD VERIFY THE EXACT LOCATION OF ALL |
| HDPE | HIGH DENSITY POLYETHYLENE PIPE | 11. | PUBLIC AND PRIVATE UTILITIES, INCLUDING IRRIGATION, SPECIFIC TO THIS PROJECT |
| HOR | HIGH POINT | | PRIOR TO THE START OF ANY DEMOLITION OR CONSTRUCTION. SHOULD ANY UTILITY REQUIRE RELOCATION, CONTRACTOR SHALL COORDINATE WITH THE OWNER AND |
| HW | HEADWALL | | THE ARCHITECT/ENGINEER. |
| | | 12. | SAWCUT AND REMOVE PORTIONS OF EXISTING PAVING ONLY AS REQUIRED TO INSTALL NEW UTILITIES OR TO CONSTRUCT PROPOSED FACILITIES PER THIS PLAN. |
| LF | INVERT | | REPLACE PORTIONS REMOVED TO MATCH EXISTING FLUSH AND SMOOTH. |
| LOC | LIMITS OF CLEARING | 13. | IF REQUESTED BY THE OWNER OR AUTHORITY HAVING JURISDICTION, CONTRACTOR TO PROVIDE TEMPORARY CONSTRUCTION FENCING, PER AUTHORITY HAVING |
| LOD | LIMITS OF DISTURBANCE | | JURISDICTION REQUIREMENTS OR A MINIMUM 6 FOOT HIGH, AROUND ENTIRE AREA OF CONSTRUCTION OR PER THE CLIENTS STANDARDS. FIELD VERIFY EXACT LOCATION |
| LOS | LINE OF SIGHT | | AND SPECIFICATIONS OF FENCE WITH THE OWNER PRIOR TO START OF |
| LP LS | LOW POINT | | CONSTRUCTION. REMOVE FENCING AT COMPLETION OF PROJECT AND PATCH PAVING AS REQUIRED AT FENCE POST HOLES. |
| MAX | MAXIMUM | 14. | ALL DIMENSIONS ARE TO GROUND LEVEL IMPROVEMENTS (FACE OF CURB, CONCRETE SLAB, ETC. UNILESS NOTED OTHERWISE) |
| ME | MATCH EXISTING | 15. | SLAB, ETC UNLESS NOTED OTHERWISE). CONTRACTOR IS RESPONSIBLE FOR PROTECTION OF ALL PROPERTY CORNERS. |
| MIN | MINIMUM | 16. | CONTRACTOR SHALL MATCH PROPOSED CURB AND GUTTER, CONCRETE, AND |
| MH | MANHOLE MECHANICAL JOINT | | PAVEMENT TO EXISTING IN GRADE AND ALIGNMENT. |
| OC | ON CENTER | 17. | CONSTRUCTION SHALL COMPLY WITH ALL AUTHORITY HAVING JURISDICTION CODES AND BE CONSTRUCTED TO SAME. |
| PC | | 18. | CONTRACTOR IS RESPONSIBLE FOR REPAIRING THE DAMAGE DONE TO ANY EXISTING |
| PCCR PI | POINT OF COMPOUND CURVATURE, CURB RETUR | IN | ITEM TO REMAIN DURING CONSTRUCTION, SUCH AS, BUT NOT LIMITED TO, DRAINAGE, UTILITIES, PAVEMENT, STRIPING, CURB, ETC. REPAIRS SHALL BE EQUAL TO, OR |
| POG | POINT OF GRADE | | BETTER THAN, EXISTING CONDITIONS. CONTRACTOR IS RESPONSIBLE TO DOCUMENT ALL EXISTING DAMAGE AND NOTIFY OWNER PRIOR TO CONSTRUCTION START. |
| POI | POINT OF INTEREST | | CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ANY DAMAGE INCURRED TO |
| PROP | | | ADJACENT PROPERTIES DURING THE CONSTRUCTION PHASES OF THIS PROJECT. CONTRACTOR SHALL REFER TO THE ARCHITECTURAL PLANS FOR EXACT LOCATIONS |
| PT | | | AND DIMENSIONS OF BUILDING APPURTENANCES, STAIRS, RAMPS, SLOPE PAVING, |
| PTCR PVC | | | SIDEWALKS, PRECISE BUILDING DIMENSIONS AND EXACT BUILDING UTILITY ENTRANCE LOCATIONS. |
| PVC PVI | | 20. | ALL DISTURBED AREAS ARE TO RECEIVE A MINIMUM OF FOUR INCHES OF TOPSOIL, |
| PVT | POINT OF VERTICAL INTERSECTION POINT OF VERTICAL TANGENCY | | UNLESS OTHERWISE NOTED IN THESE PLANS, SEED OR SOD, MULCH AND WATER UNTIL A HEALTHY STAND OF GRASS IS ESTABLISHED. |
| R | POINT OF VERTICAL TANGENCY RADIUS | 21. | EXISTING STRUCTURES WITHIN CONSTRUCTION LIMITS ARE TO BE MAINTAINED, |
| RCP | REINFORCED CONCRETE PIPE | | ABANDONED, REMOVED OR RELOCATED AS NECESSARY. ALL COST SHALL BE INCLUDED IN BASE BID. |
| RCPR | REINFORCED CONCRETE WITH RUBBER GASKET | 22. | CONTRACTOR SHALL BE RESPONSIBLE FOR ALL RELOCATIONS, (UNLESS OTHERWISE |
| RET-WALL R/W | RETAINING WALL RIGHT OF WAY | | NOTED ON PLANS) INCLUDING BUT NOT LIMITED TO, ALL UTILITIES, STORM DRAINAGE, SIGNS, TRAFFIC SIGNALS & POLES, ETC. AS REQUIRED. ALL WORK SHALL BE IN |
| S | SLOPE | | ACCORDANCE WITH AUTHORITY HAVING JURISDICTION REQUIREMENTS AND PROJECT SITE WORK SPECIFICATIONS AND SHALL BE APPROVED BY SUCH. ALL COST SHALL BE |
| SAN | SANITARY SEWER | | INCLUDED IN BASE BID. |
| SF STA | SQUARE FEET | 23. | THE SITE WORK FOR THIS PROJECT SHALL MEET OR EXCEED THE SPECIFICATIONS IN THE CONTRACT DOCUMENTS AND THE OWNER/ DEVELOPER SITE WORK |
| STA | STATION STORM | _ | SPECIFICATIONS. |
| TBR | TO BE REMOVED | 24. | ANY DISCREPANCIES ON THE DRAWINGS SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE OWNER AND ENGINEER OF RECORD BEFORE COMMENCING |
| TBRL | TO BE RELOCATED | | WORK. NO FIELD CHANGES OR DEVIATIONS FROM THE DESIGN ARE TO BE MADE |
| TC TEL | TOP OF CURB | 25. | WITHOUT PRIOR APPROVAL. |
| TP | TELEPHONE TREE PROTECTION | 20. | THE PROJECT, ALL CONSTRUCTION AND STOCKPILED VEGETATIVE DEBRIS AND FILL |
| TW | TOP OF WALL | | SHALL BE REMOVED FROM THE SITE AND THE SITE SHALL BE STABILIZED PER THE PERMIT FOR STORM WATER DISCHARGE FROM LARGE AND SMALL CONSTRUCTION |
| TYP | TYPICAL | | ACTIVITIES (NPDES). |
| UG | | 26. | THESE PLANS ARE INTENDED TO AND SHALL COMPLY WITH AMERICANS WITH DISABILITIES ACT. |
| UP | UTILITY POLE WATER LINE | 27. | CONTRACTOR IS RESPONSIBLE FOR PERMITTING, INSTALLATION AND MAINTENANCE |
| W/M | WATER METER | | OF ALL MAINTENANCE OF TRAFFIC OPERATIONS DURING CONSTRUCTION. MAINTENANCE OF TRAFFIC SHALL CONFORM TO AUTHORITY HAVING JURISDICTION |
| ± | PLUS OR MINUS | | STANDARDS. |
| 。 Ø | DEGREE | 28. | ALL DESIGN AND CONSTRUCTION MUST CONFORM TO THE MINIMUM STANDARDS SET DOWN IN THE AUTHORITY HAVING JURISDICTION DEVELOPMENT CODE, ZONING, |
| Ś | DIAMETER | | AND/OR RELATED ORDINANCES, AND MINIMUM TESTING FREQUENCY REQUIREMENTS. |
| # | NUMBER | 1 | |

| 6 | 7 | 8 | 9 | 10 |
|---|---|---|---|----|
| | | | | |

30. SAFETY NOTICE TO CONTRACTOR: IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR SHALL BE SOLELY AND COMPLETELY RESPONSIBLE FOR CONDITIONS OF THE JOB SITE, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS. ANY CONSTRUCTION OBSERVATION BY THE ENGINEER OF THE CONTRACTOR'S PERFORMANCE IS NOT INTENDED TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES ON OR NEAR THE CONSTRUCTION SITE.

SOIL EROSION AND SEDIMENT CONTROL NOTES:

REVISIONS

- ALL APPLICABLE EROSION AND SEDIMENT CONTROL PRACTICES SHALL BE IN PLACE PRIOR TO ANY GRADING OPERATION AND/OR INSTALLATION OF PROPOSED STRUCTURES OR UTILITIES.
- SOIL EROSION AND SEDIMENT CONTROL PRACTICES ON THIS PLAN SHALL BE CONSTRUCTED IN ACCORDANCE WITH ALL AUTHORITY HAVING JURISDICTION STANDARDS FOR SOIL EROSION AND SEDIMENT CONTROL.
- APPLICABLE EROSION AND SEDIMENT CONTROL PRACTICES SHALL BE LEFT IN PLACE UNTIL CONSTRUCTION IS COMPLETED AND/OR THE AREA IS STABILIZED.
- THE CONTRACTOR SHALL PERFORM ALL WORK, FURNISH ALL MATERIALS AND INSTALL ALL MEASURES REQUIRED TO REASONABLY CONTROL SOIL EROSION RESULTING FROM CONSTRUCTION OPERATIONS AND PREVENT EXCESSIVE FLOW OF SEDIMENT FROM THE CONSTRUCTION SITE.
- ANY DISTURBED AREA THAT IS TO BE LEFT EXPOSED FOR MORE THAN 14 DAYS, UNLESS OTHERWISE NOTED IN THE PLANS, AND NOT SUBJECT TO CONSTRUCTION TRAFFIC SHALL IMMEDIATELY RECEIVE A TEMPORARY SEEDING AND FERTILIZATION IN ACCORDANCE WITH ALL AUTHORITY HAVING JURISDICTION STANDARDS. IF THE SEASON PROHIBITS TEMPORARY SEEDING, THE DISTURBED AREAS WILL BE MULCHED WITH SALT HAY OR EQUIVALENT AND ANCHORED.
- ALL SEDIMENTATION STRUCTURES WILL BE INSPECTED AND MAINTAINED ON A REGULAR BASIS AND AFTER EVERY STORM EVENT.
- 7. A CRUSHED STONE TIRE CLEANING PAD WILL BE INSTALLED WHEREVER A CONSTRUCTION ACCESS EXISTS. THE STABILIZED PAD WILL BE INSTALLED ACCORDING TO THE STANDARD FOR STABILIZED CONSTRUCTION ACCESS.
- ALL CATCH BASIN INLETS WILL BE PROTECTED ACCORDING TO THE CERTIFIED PLAN. ALL STORM DRAINAGE OUTLETS WILL BE STABILIZED, AS REQUIRED, BEFORE THE DISCHARGE POINTS BECOME OPERATIONAL.
- 10. OFFSITE SEDIMENT DISTURBANCE MAY REQUIRE ADDITIONAL CONTROL MEASURES TO BE DETERMINED BY THE EROSION CONTROL INSPECTOR.
- 11. A COPY OF THE CERTIFIED SOIL EROSION AND SEDIMENT CONTROL PLAN MUST BE MAINTAINED ON THE PROJECT SITE DURING CONSTRUCTION.
- 12. THE AUTHORITY HAVING JURISDICTION SHALL BE NOTIFIED PER AUTHORITY HAVING JURISDICTION REQUIREMENTS PRIOR TO ANY LAND DISTURBANCE.
- 13. ANY CONVEYANCE OF THIS PROJECT PRIOR TO ITS COMPLETION WILL TRANSFER FULL RESPONSIBILITY FOR COMPLIANCE WITH THE CERTIFIED PLAN TO ANY SUBSEQUENT OWNERS.
- 14. MAXIMUM SIDE SLOPES OF ALL EXPOSED SURFACES SHALL NOT BE CONSTRUCTED STEEPER THAN 3:1 UNLESS OTHERWISE APPROVED BY THE AUTHORITY HAVING JURISDICTION.
- 15. ADJOINING PROPERTIES SHALL BE PROTECTED FROM EXCAVATION AND FILLING OPERATIONS ON THE PROPOSED SITE.
- 16. USE STAGED CONSTRUCTION METHODS TO MINIMIZE EXPOSED SURFACES, WHERE APPLICABLE.
- 17. ALL VEGETATIVE MATERIAL SHALL BE SELECTED IN ACCORDANCE WITH AMERICAN STANDARDS FOR NURSERY STOCK OF THE AMERICAN ASSOCIATION OF THE NURSERYMAN.
- 18. NATURAL VEGETATION AND SPECIES SHALL BE RETAINED WHERE SPECIFIED ON THE LANDSCAPING PLAN.
- THE SOIL EROSION INSPECTOR MAY REQUIRE ADDITIONAL SOIL EROSION MEASURES TO BE INSTALLED, AS DIRECTED BY THE INSPECTOR.

DEMOLITION NOTES:

- THE CONTRACTOR SHALL COMPLY WITH ALL FEDERAL, STATE AND LOCAL LAWS AND CODES AND OBTAIN ALL REQUIRED PERMITS FOR ANY CONSTRUCTION ACTIVITY. THE CONTRACTOR SHALL CONTACT 811 DIG SAFELY BEFORE PERFORMING ANY EXCAVATION WORK.
- 3. THE CONTRACTOR SHALL INSTALL ALL CONSTRUCTION FENCING AND EROSION AND SEDIMENT CONTROL DEVICES PRIOR TO THE START OF ANY DEMOLITION OR CONSTRUCTION ACTIVITY.
- ALL STRUCTURES, UTILITIES, SITE IMPROVEMENTS AND TREES DESIGNATED ON THE DRAWINGS OR DIRECTED BY THE ENGINEER TO REMAIN SHALL BE PROTECTED FROM DAMAGE BY ALL CONSTRUCTION OPERATIONS. THIS SHALL BE ACCOMPLISHED BY ERECTING BARRIERS, GUARDS AND ENCLOSURES AS SHOWN ON THE DRAWINGS OR OTHER APPROVED MEANS. PROTECTION SHALL BE MAINTAINED UNTIL ALL WORK IN THE VICINITY OF THE WORK BEING PROTECTED HAS BEEN COMPLETED.
- THE CONTRACTOR SHALL COMPLY WITH ALL DEMOLITION AND NEW CONSTRUCTION INSPECTIONS AS REQUIRED BY FEDERAL, STATE AND AUTHORITY HAVING JURISDICTION LAWS, REGULATIONS AND BUILDING CODES.
- THE CONTRACTOR IS RESPONSIBLE FOR THE DEMOLITION, REMOVAL, AND DISPOSAL (IN A LOCATION APPROVED BY ALL AUTHORITIES HAVING JURISDICTION) ALL STRUCTURES, PADS, WALLS, FLUMES, FOUNDATIONS, PARKING, DRIVES, DRAINAGE, STRUCTURES, UTILITIES, ETC., SUCH THAT THE IMPROVEMENTS SHOWN ON THE REMAINING PLANS CAN BE CONSTRUCTED. UTILITIES ARE TO BE REMOVED TO THE RIGHT-OF-WAY, UNLESS OTHERWISE NOTED.
- ALL FACILITIES TO BE REMOVED SHALL BE UNDERCUT TO SUITABLE MATERIAL AND BROUGHT TO GRADE WITH SUITABLE COMPACTED FILL MATERIAL PER THE CONTRACT DOCUMENTS.
- CONTRACTOR IS TO REMOVE AND DISPOSE OF ALL DEBRIS, RUBBISH, VEGETATION FROM CLEARING AND GRUBBING, AND OTHER MATERIALS RESULTING FROM PREVIOUS AND CURRENT DEMOLITION OPERATIONS. DISPOSAL WILL BE IN ACCORDANCE WITH ALL LOCAL, STATE AND/OR FEDERAL REGULATIONS GOVERNING SUCH OPERATIONS. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING FOR ALL PERMITS REQUIRED FOR DEMOLITION AND DISPOSAL.
- THE CONTRACTOR IS RESPONSIBLE FOR REMOVING ALL DEBRIS FROM THE SITE AND DISPOSING THE DEBRIS IN A LAWFUL MANNER. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL PERMITS REQUIRED FOR DEMOLITION AND DISPOSAL.
- 10. THE CONTRACTOR SHALL COORDINATE WITH RESPECTIVE UTILITY COMPANIES PRIOR TO THE REMOVAL AND/OR RELOCATION OF UTILITIES. THE CONTRACTOR SHALL COORDINATE WITH THE UTILITY COMPANY CONCERNING PORTIONS OF WORK WHICH MAY BE PERFORMED BY THE UTILITY COMPANY'S FORCES AND ANY FEES WHICH ARE TO BE PAID TO THE UTILITY COMPANY FOR THEIR SERVICES. THE CONTRACTOR IS RESPONSIBLE FOR PAYING ALL FEES AND CHARGES.
- 11. THE LOCATIONS OF ALL EXISTING UTILITIES SHOWN ON THIS PLAN HAVE BEEN DETERMINED FROM THE BEST INFORMATION AVAILABLE AND ARE GIVEN FOR THE CONVENIENCE OF THE CONTRACTOR. THE ENGINEER ASSUMES NO RESPONSIBILITY FOR THEIR ACCURACY. PRIOR TO THE START OF ANY DEMOLITION ACTIVITY, THE CONTRACTOR SHALL NOTIFY THE UTILITY COMPANIES FOR ONSITE LOCATIONS OF EXISTING UTILITIES.
- 12. ALL EXISTING SEWERS, PIPING AND UTILITIES SHOWN ARE NOT TO BE INTERPRETED

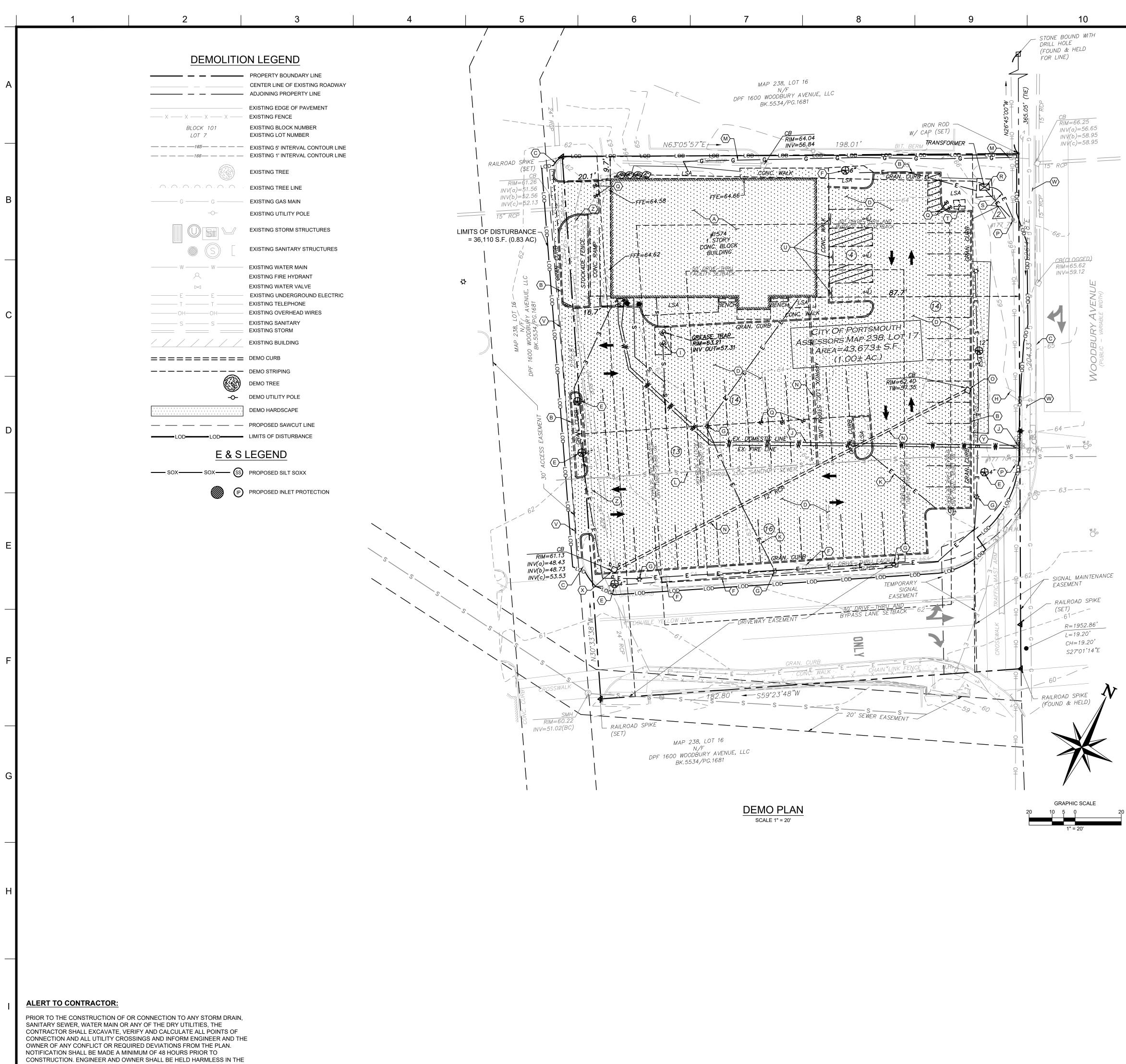
AS THE EXACT LOCATION, OR AS THE ONLY OBSTACLES THAT MAY OCCUR ON SITE. VERIFY EXISTING CONDITIONS AND PROCEED WITH CAUTION AROUND AN ANTICIPATED FEATURES. GIVE NOTICE TO ALL UTILITY COMPANIES REGARDING DESTRUCTION AND REMOVAL OF ALL SERVICE LINES AND CAP ALL LINES BEFO PROCEEDING WITH THE WORK. UTILITIES DETERMINED TO BE ABANDONED AND IN PLACE SHALL BE GROUTED IF UNDER BUILDING.

- 13. ELECTRICAL, TELEPHONE, CABLE, WATER, FIBER OPTIC CABLE AND/OR GAS LIN NEEDING TO BE REMOVED OR RELOCATED SHALL BE COORDINATED WITH THE AFFECTED UTILITY COMPANY AND REMOVED TO THE PROPERTY LINE. ADEQUA TIME SHALL BE PROVIDED FOR RELOCATION AND CLOSE COORDINATION WITH UTILITY COMPANY IS NECESSARY TO PROVIDE A SMOOTH TRANSITION IN UTILITY SERVICE. CONTRACTOR SHALL PAY CLOSE ATTENTION TO EXISTING UTILITIES V ANY ROAD RIGHT OF WAY DURING CONSTRUCTION.
- 14. CONTRACTOR TO REPLACE ALL DEAD AND/OR DAMAGED SHRUBS IN KIND.

GENERAL UTILITY NOTES:

- 1. CONTRACTOR SHALL COORDINATE ANY DISRUPTIONS TO EXISTING UTILITY SEE WITH ADJACENT PROPERTY OWNERS.
- 2. ALL ELECTRIC, TELEPHONE AND GAS EXTENSIONS INCLUDING SERVICE LINES S BE CONSTRUCTED TO THE APPROPRIATE UTILITY COMPANY SPECIFICATIONS. UTILITY DISCONNECTIONS SHALL BE COORDINATED WITH THE DESIGNATED UTI COMPANIES.
- 3. CONSTRUCTION SHALL NOT START ON ANY PUBLIC UTILITY SYSTEM UNTIL WRI APPROVAL HAS BEEN RECEIVED BY THE ENGINEER FROM THE APPROPRIATE AUTHORITY HAVING JURISDICTION AND CONTRACTOR HAS BEEN NOTIFIED BY S ENGINEER.
- 4. PRIOR TO THE CONSTRUCTION OF OR CONNECTION TO ANY STORM DRAIN, SAM SEWER, WATER MAIN OR ANY DRY UTILITIES, THE CONTRACTOR SHALL EXCAVA VERIFY AND CALCULATE ALL POINTS OF CONNECTION AND ALL UTILITY CROSSI AND INFORM THE ENGINEER AND THE OWNER/DEVELOPER OF ANY CONFLICT OF REQUIRED DEVIATIONS FROM THE PLAN. NOTIFICATION SHALL BE MADE A MINI OF 72 HOURS PRIOR TO CONSTRUCTION, UNLESS OTHERWISE SPECIFIED IN PL. THE ENGINEER AND ITS OWNER SHALL BE HELD HARMLESS IN THE EVENT THAT CONTRACTOR FAILS TO MAKE SUCH NOTIFICATION.
- 5. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE SCHEDULE FOR INSTALLATION WITH THE UTILITY COMPANIES AND THE OWNER EXISTING UTILITIES DISRUPTED DURING PLACEMENT OF NEW UTILITIES SHALL REPAIRED AND OPERATING NORMALLY THE SAME DAY OF DISRUPTION. THE GE CONTRACTOR SHALL FIELD VERIFY THE EXACT LOCATION OF ALL EXISTING ITEL THAT WILL BE DISRUPTED DURING THE PLACEMENT OF NEW UTILITIES AND PRO THE OWNER A DETAILED PHASING SCHEDULE OUTLINING THE TIMELINE FOR INSTALLATION OF NEW UTILITIES INCLUDING THE PROPOSED TIMES THAT EXIST ITEMS WILL BE DISRUPTED. THE NEW UTILITIES TRENCH WIDTH AND DEPTH SHA MEET ALL LOCAL AND STATE REQUIREMENTS FOR THE DISPLACEMENT OF ALL UTILITIES. IF DIRECTIONAL BORING IS USED FOR INSTALLATION, THE ABOVE LIS ITEMS ARE STILL REQUIRED TO BE SUBMITTED TO THE OWNER.
- 6. ALL FILL MATERIAL IS TO BE IN PLACE, AND COMPACTED BEFORE INSTALLATION PROPOSED UTILITIES.
- CONTRACTOR SHALL NOTIFY THE UTILITY AUTHORITIES' INSPECTORS PER AUTHORITY HAVING JURISDICTION REQUIREMENTS BEFORE CONNECTING TO A EXISTING LINE AND FOLLOW ALL REQUIREMENTS AND SPECIFICATIONS.
- 8. UNDERGROUND UTILITY LINES SHALL BE INSTALLED, INSPECTED AND APPROVE BEFORE BACKFILLING.
- 9. ALL CONCRETE FOR ENCASEMENTS SHALL MEET THE AUTHORITY HAVING JURISDICTION REQUIREMENTS FOR ENCASEMENT.
- 10. DRAWINGS DO NOT PURPORT TO SHOW ALL EXISTING UTILITIES. CONTRACTOR VERIFY ALL UTILITIES PRIOR TO COMMENCEMENT OF CONSTRUCTION.
- 11. THE CONTRACTOR SHALL CONSTRUCT GRAVITY SEWER LATERALS, MANHOLES GRAVITY SEWER LINES AND DOMESTIC WATER AND FIRE PROTECTION SYSTEM SHOWN ON THESE PLANS. THE CONTRACTOR SHALL FURNISH ALL NECESSARY MATERIALS, EQUIPMENT, MACHINERY, TOOLS, MEANS OF TRANSPORTATION AN LABOR NECESSARY TO COMPLETE THE WORK IN FULL AND COMPLETE IN ACCORDANCE WITH THE SHOWN, DESCRIBED AND REASONABLY INTENDED REQUIREMENTS OF THE CONTRACT DOCUMENTS AND JURISDICTIONAL AGENCY REQUIREMENTS. IN THE EVENT THAT THE CONTRACT DOCUMENTS AND THE JURISDICTIONAL AGENCY REQUIREMENTS ARE NOT IN AGREEMENT, THE MOST STRINGENT SHALL GOVERN.
- 12. THE CONTRACTOR SHALL RESTORE ALL DISTURBED VEGETATION IN KIND, UNL SHOWN OTHERWISE.
- 13. DEFLECTION OF PIPE JOINTS AND CURVATURE OF PIPE SHALL NOT EXCEED THI MANUFACTURER'S SPECIFICATIONS. SECURELY CLOSE ALL OPEN ENDS OF PIPE FITTINGS WITH A WATERTIGHT PLUG WHEN WORK IS NOT IN PROGRESS. THE INTERIOR OF ALL PIPES SHALL BE CLEAN AND JOINT SURFACES WIPED CLEAN A DRY AFTER THE PIPE HAS BEEN LOWERED INTO THE TRENCH. VALVES SHALL BE PLUMB AND LOCATED ACCORDING TO THE PLANS.
- 14. ALL UTILITY AND STORM DRAIN TRENCHES LOCATED UNDER AREAS TO RECEIV PAVING SHALL BE COMPLETELY BACKFILLED AND COMPACTED IN ACCORDANC SPECIFICATIONS. IN THE EVENT THAT THE CONTRACT DOCUMENTS AND JURISDICTIONAL AGENCY REQUIREMENTS ARE NOT IN AGREEMENT, THE MOST STRINGENT SHALL GOVERN.
- 15. SHOP DRAWINGS FOR ALL MATERIALS AND APPURTENANCE SHALL BE SUBMITT AND APPROVED BY THE AUTHORITY HAVING JURISDICTION UTILITY DEPARTMENT CONTRACTOR TO COPY THE ENGINEER OF RECORD WITH APPROVED DRAWING REQUIRED. NO WORK IS TO BEGIN UNTIL SHOP DRAWINGS HAVE BEEN REVIEWE APPROVED AND RETURNED TO THE CONTRACTOR.
- 16. PER AUTHORITY HAVING JURISDICTION TIMING REQUIREMENTS, PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL NOTIFY THE AUTHORITY HAVING JURISDICTION AND THE UTILITY COMPANY AND SUPPLY THEM WITH ALL REQUIR SHOP DRAWINGS, THE CONTRACTOR'S NAME, STARTING DATE, PROJECTED SCHEDULE AND OTHER INFORMATION AS REQUIRED. THE AUTHORITY HAVING JURISDICTION ENGINEERING INSPECTION OFFICE SHOULD ALSO BE CONTACTE TIMING REQUIREMENTS PRIOR TO CONSTRUCTION TO ENSURE AVAILABILITY OF INSPECTION PERSONNEL. ANY WORK PREFORMED PRIOR TO NOTIFYING THE AUTHORITY HAVING JURISDICTION ENGINEERING INSPECTION OFFICE OR WITH DEPARTMENT INSPECTOR PRESENT MAY BE SUBJECT TO REMOVAL AND REPLACEMENT AT THE SOLE EXPENSE OF THE CONTRACTOR.
- 17. SANITARY SEWER, FORCE MAINS, SEWER LATERALS, AND STORM SEWERS SHO CROSS UNDER WATER MAINS AND/OR WATER SERVICES WHENEVER POSSIBLE SANITARY SEWERS, FORCE MAINS, SEWER LATERALS, AND STORM SEWERS CROSSING WATER MAINS SHALL BE LAID TO PROVIDE A MINIMUM VERTICAL DIS OF 18 INCHES BETWEEN THE BOTTOM OF THE UPPER PIPE AND THE TOP OF TH LOWER PIPE, UNLESS OTHERWISE SPECIFIED IN THE PLANS.
- 18. A MINIMUM HORIZONTAL DISTANCE OF 10 FEET SHOULD BE MAINTAINED BETW WATER LINES AND ANY TYPE OF SEWER LINES OR OTHER SOURCES OF CONTAMINATION, UNLESS OTHERWISE NOTED IN THE PLANS. WATER LINES AN SEWERS SHALL NOT BE LAID IN THE SAME TRENCH EXCEPT ON THE WRITTEN APPROVAL OF THE AUTHORITY HAVING JURISDICTION. WATER MAINS NECESS IN CLOSE PROXIMITY TO SEWERS MUST BE PLACED SO THAT THE BOTTOM OF WATER LINE WILL BE AT LEAST 18 INCHES ABOVE THE TOP OF THE SEWER LIN HIGHEST POINT, UNLESS OTHERWISE NOTED IN THE PLANS. IF THIS DISTANCE UNAVOIDABLY BE REDUCED, THE WATER LINE OR THE SEWER LINE MUST BE ENCASED IN WATERTIGHT PIPE WITH SEALED WATERTIGHT ENDS EXTENDING LEAST 10 FEET EITHER SIDE OF THE CROSSING, UNLESS OTHERWISE NOTED II PLANS. ANY JOINT IN THE ENCASEMENT PIPE IS TO BE MECHANICALLY RESTR THE ENCASEMENT PIPE MAY BE VENTED TO THE SURFACE IF CARRYING WATE SEWER UNDER PRESSURE. WHERE A WATER LINE MUST UNAVOIDABLY PASS BENEATH THE SEWER LINE, AT LEAST 18 INCHES OF SEPARATION MUST BE MAINTAINED BETWEEN THE OUTSIDE OF THE TWO PIPES IN ADDITION TO THE PRECEDING ENCASEMENT REQUIREMENT. UNLESS OTHERWISE NOTED IN THE

| | | 11 12 13 | |
|-----------------------------|------------|---|---|
| | | EXCEPTIONS TO THIS MUST BE APPROVED IN WRITING BY THE AUTHORITY HAVING | |
| ANY IG IORE | 19. | JURISDICTION. A MINIMUM HORIZONTAL DISTANCE OF 3 FEET, UNLESS OTHERWISE NOTED IN THE | U Z |
| ID LEFT | | PLANS, SHALL BE MAINTAINED BETWEEN WATER LINES AND OTHER UNDERGROUND OF A NONSANITARY NATURE (GAS, ELECTRIC, ETC.) EXCEPTIONS TO THIS MUST BE APPROVED IN WRITING BY THE AUTHORITY HAVING JURISDICTION. | |
| INES E | 20. | ALL DIP SHALL BE CLASS 50 OR HIGHER, DUCTILE IRON FITTINGS SHALL BE CLASS 350, UNLESS OTHERWISE NOTED IN THE PLANS. ADEQUATE PROTECTIVE MEASURES | |
| ATE I THE | | AGAINST CORROSION SHALL BE USED. | |
| .ITY S WITHIN | 21. | TREES SHALL BE PLACED SO AS TO AVOID BURIED UTILITIES. | |
| | 22. | ALL UTILITY MAIN LENGTHS SHOWN ARE APPROXIMATE. ALL MANHOLE TOP ELEVATIONS ARE APPROXIMATE. CONTRACTOR SHALL SET MANHOLE COVER LEVEL WITH FINISH PAVEMENT GRADES. | |
| | 23. 24. | PRESSURE PIPE TESTING SPECIFICATIONS SHALL REFERENCE THE AUTHORITY HAVING JURISDICTION AND/OR FIRE DEPARTMENT. CONTRACTOR IS RESPONSIBLE FOR ALL NECESSARY INSPECTIONS AND/OR | COBREST ROAD MILLCREST ROAD ALLAS, TX 75230 14) 377-5060 |
| SHALL | 21. | CERTIFICATIONS REQUIRED BY CODES AND/OR UTILITY SERVICE COMPANIES. THIS AND THE FINAL CONNECTIONS OF THE SERVICE SHALL BE COMPLETED 30 DAYS PRIOR TO POSSESSION. | 12700 HI Suite 19: DALLAS, (214) 377 |
| . ALL TILITY | 25. | REFER TO BUILDING PLANS FOR SITE ELECTRICAL PLAN. | DOCUMENTS PREPARED BY CORESTATES, IF |
| | 26. | ALL REINFORCED CONCRETE PIPE SHALL BE CLASS III UNLESS OTHERWISE NOTED AND INSTALLED IN ACCORDANCE WITH THE APPROPRIATE APPROVING AUTHORITIES | INCLUDING THIS DOCUMENT, ARE TO BE US ONLY FOR THE SPECIFIC PROJECT AND SPEC USE FOR WHICH THEY WERE INTENDED. AN |
| RITTEN ' SAID | | LATEST REQUIREMENTS AND SPECIFICATIONS OR AUTHORITY HAVING JURISDICTION SPECIFICATIONS, WHICHEVER IS MORE STRINGENT. | EXTENSION OF USE TO ANY OTHER PROJEC BY OWNER OR BY ANY OTHER PARTY, WITH THE EXPRESSED WRITTEN CONSENT OF CORESTATES, INC. IS DONE UNLAWFULLY A AT THE USERS OWN RISK. IF USED IN A WA |
| ANITARY /ATE, | GE | NERAL PAVING AND GRADING NOTES: | OTHER THAN THAT SPECIFICALLY INTENDE USER WILL HOLD CORESTATES, INC. HARMLE FROM ALL CLAIMS AND LOSSES. |
| SINGS OR NIMUM | 1. | ALL PAVING AND GRADING CONSTRUCTION MATERIALS AND METHODS SHALL MEET THE STANDARD SPECIFICATIONS AND REQUIREMENTS OF THE AUTHORITY HAVING JURISDICTION. | |
| LANS. AT THE | 2. | THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF VARIOUS UTILITY COMPANIES, AND WHERE POSSIBLE, MEASUREMENTS | |
| IE R. ANY | | TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR MUST CALL THE APPROPRIATE UTILITY COMPANIES AT | |
| L BE SENERAL | | LEAST 72 HOURS BEFORE ANY EXCAVATION TO REQUEST EXACT FIELD LOCATION OF UTILITIES, UNLESS OTHERWISE NOTED IN THE PLANS. IT SHALL BE THE | |
| EMS | | RESPONSIBILITY OF THE CONTRACTOR TO RELOCATE ALL EXISTING UTILITIES WHICH CONFLICT WITH THE PROPOSED IMPROVEMENTS SHOWN ON THE PLANS. | |
| | 3. | ALL CUT OR FILL SLOPES SHALL BE 3:1 OR FLATTER UNLESS OTHERWISE NOTED. | |
| STING HALL - ISTED | 4. | PRECAST STRUCTURES MAY BE USED AT CONTRACTOR'S OPTION AND SHALL MEET ALL AUTHORITY HAVING JURISDICTION REQUIREMENTS/SPECIFICATIONS AT A MINIMUM. | |
| ON OF | 5. | THE CONTRACTOR SHALL ADHERE TO ALL TERMS & CONDITIONS AS OUTLINED IN THE EPA OR APPLICABLE STATE GENERAL NPDES PERMIT FOR STORM WATER | |
| ANY | 6. | DISCHARGE ASSOCIATED WITH CONSTRUCTION ACTIVITIES. CONTRACTOR SHALL ADJUST AND/OR CUT EXISTING PAVEMENT AS NECESSARY TO ASSURE A SMOOTH FIT AND CONTINUOUS GRADE. | |
| | 7. | CONTRACTOR SHALL ASSURE POSITIVE DRAINAGE AWAY FROM BUILDINGS FOR ALL | |
| /ED | 8. | NATURAL AND PAVED AREAS. TOPOGRAPHIC INFORMATION IS TAKEN FROM A TOPOGRAPHIC SURVEY BY A | |
| R TO | | LICENSED PROFESSIONAL SURVEYOR AND MAPPER. IF THE CONTRACTOR DOES NOT ACCEPT EXISTING TOPOGRAPHY AS SHOWN ON THE PLANS, WITHOUT EXCEPTION, THEN THE CONTRACTOR SHALL SUPPLY, AT THEIR EXPENSE, A TOPOGRAPHIC | Know what's below. Call before you dig. The contractor is specifically cautioned that the location and/or |
| IX IO | 9. | SURVEY BY A REGISTERED LAND SURVEYOR TO THE OWNER FOR REVIEW. ALL UNSURFACED AREAS DISTURBED BY GRADING OPERATION SHALL RECEIVE A | ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON D DRAWINGS, RECORDS OF THE VARIOUS UTILITY COMPANIES, AND WHERE POSS MEASUREMENTS TAKEN IN THE FIELD. CORE STATES, INC. DOES NOT GUARAN THAT LOCATIONS SHOWN ARE EXACT. THE CONTRACTOR MUST CONTACT THE APPROPRIATE LITITY COMPANIES AT LFAST 27 HOURS REFORE ANY EXCAVAT |
| ES, M AS Y ND | 5. | MINIMUM OF 4 INCHES OF TOPSOIL, UNLESS OTHERWISE NOTED IN THE PLANS. CONTRACTOR SHALL APPLY STABILIZATION FABRIC TO ALL SLOPES 3H:1V OR STEEPER. CONTRACTOR SHALL STABILIZE DISTURBED AREAS IN ACCORDANCE WITH | REV DATE COMMENT E |
| CY | 10. | AUTHORITY HAVING JURISDICTION SPECIFICATIONS UNTIL A HEALTHY STAND OF VEGETATION IS OBTAINED. CONSTRUCTION SHALL COMPLY WITH ALL APPLICABLE AUTHORITY HAVING | |
| т | 4.4 | JURISDICTION CODES AND BE CONSTRUCTED TO SAME. | |
| LESS | 11. | ALL PAVING, CONSTRUCTION MATERIALS, AND WORKMANSHIP WITHIN RIGHT-OF-WAY SHALL BE IN ACCORDANCE WITH THE AUTHORITY HAVING JURISDICTION SPECIFICATIONS AND STANDARDS (LATEST EDITION) RESPECTIVELY. | |
| HE | 12. | ALL CONCRETE USED ON THE SITE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,500 PSI IN 28 DAYS, UNLESS OTHERWISE NOTED IN THE PLANS. ALL CONCRETE | |
| PE AND | | SIDEWALKS SHALL HAVE CONTROL JOINTS CUT ON 5-FOOT CENTERS AND EXPANSION JOINTS PLACED ON 60-FOOT CENTERS, CHANGES IN DIRECTION, AND ABUTTING | |
| I AND BE | | SEPARATE POURS. CONTRACTION JOINT SPACING SHALL MATCH WIDTH OF SIDEWALK AND EXPANSION JOINTS ARE REQUIRED AT A MAXIMUM OF 25 FEET, UNLESS | |
| VE | | OTHERWISE NOTED IN THE PLANS. THIS WOULD MEAN 24 FEET FOR A 6 INCH CURB. PAVEMENT JOINTS SHALL BE SPACED IN ACCORDANCE WITH THE PROJECT | |
| CE WITH | 40 | SPECIFICATIONS AND/OR DETAILS. | DOCOMENT |
| т | 13. | THE CONTRACTOR SHALL ENSURE THAT ALL PLANTING AREAS (INTERIOR ISLANDS, FOUNDATION PLANTING AREAS, ETC.) ARE NOT COMPACTED AND DO NOT CONTAIN | SITE PLAN |
| ITED TO ENT. | | LIMEROCK OR OTHER MATERIAL (CLAY, SUBGRADE MATERIAL, MARL, ETC.) WHICH MAY ADVERSELY AFFECT DRAINAGE OF GREEN AREAS. THE CONTRACTOR SHALL ALSO EXCAVATE AND REMOVE ALL UNDESIRABLE MATERIAL FROM ALL AREAS ON THE SITE TO BE PLANTED AND BACKFILL WITH CLEAN, FREE DRAINING TOPSOIL. | APPROVAL FOR CHASE BANK |
| IGS AS VED, | 14. | CONTRACTOR IS SPECIFICALLY CAUTIONED, DEPENDING ON THE TIME OF YEAR AND PROJECT LOCATION, AS DEWATERING MAY BY REQUIRED. | SITE LOCATION 1574 WOODBURY |
| IIRED | 15. | IF DEWATERING IS REQUIRED, THE CONTRACTOR SHALL OBTAIN ANY APPLICABLE REQUIRED PERMITS. THE CONTRACTOR IS TO COORDINATE WITH THE OWNER AND ARCHITECT/ENGINEER PRIOR TO EXCAVATION. | AVENUE, |
| ED PER | 16. | STRIP TOPSOIL AND ORGANIC MATTER AND PAVING MATERIAL FROM ALL AREAS TO BE IMPERVIOUS. TOPSOIL SHALL BE STOCKPILED ON SITE FOR REPLACEMENT ON SLOPES AND ALL OTHER GREEN AND LANDSCAPE AREAS. | PORTSMOUTH, NH 03801 |
| OF HOUT A | 17. | FIELD DENSITY TESTS SHALL BE TAKEN AT FREQUENCY AS REQUIRED IN THE SPECIFICATIONS OR AS REQUIRED BY THE AUTHORITY HAVING JURISDICTION | |
| IOULD | 18. | REGULATORY AGENCY, WHICHEVER IS MORE STRINGENT. CONTRACTOR SHALL ENSURE POSITIVE FLOW TO ALL INLETS WITHIN DRAINAGE | THOMAS |
| E. | 19. | BASINS TO PRECLUDE PONDED WATER. THE CONTRACTOR SHALL TAKE ALL MEASURES NECESSARY TO CONTROL SEDIMENT, | 三回、 No. 10218 / 先三 |
| ISTANCE HE | 101 | INCLUDING BUT NOT LIMITED TO THE INSTALLATION OF BARRIERS AT ALL LOCATIONS WHERE THE POSSIBILITY OF TRANSFERRING SUSPENDED SOLIDS INTO THE RECEIVING WATER BODY EXISTS DUE TO THE PROPOSED WORK. BARRIERS MUST BE | TOSIONAL ENGINI |
| VEEN | | MAINTAINED IN EFFECTIVE CONDITION AT ALL LOCATIONS UNTIL CONSTRUCTION IS COMPLETED. | SHEET TITLE |
| ND | | | GENERAL |
| SARILY | | | NOTES |
| THE | | | JOB #: JPM.27 |
| E MUST | | | DATE: 07/01/2 SCALE: NO SC/ |
| AT N THE | | | DRAWN BY: |
| AINED. ER OR | | | |
| | | | CV-2 |
| E PLANS. | | | |



EVENT THAT THE CONTRACTOR FAILS TO MAKE SUCH NOTIFICATION.

Plot Date/Time: Jul. 01, 20 - 10:20:18 Drawing: P:\J.P. Morgan Chase\Portsmouth, NH (1574 Woodbury Avenue) OVP# 38100P322370 - JPM.27086\CIVIL\Drawings\Presentation\JPM.27086-P-SITE-NEW.dwg ;C1-DEMC

13

○ DEMOLITION KEY NOTES:

- A. EXISTING BUILDING AND FOUNDATIONS TO BE REMOVED. CONTRACTOR TO BACKFILL AS NECESSARY AND PROVIDE COMPACTION PER GEO-TECHNICAL REPORT RECOMMENDATIONS.
- B. EXISTING CURB TO BE REMOVED.
- C. EXISTING CURB TO REMAIN. CONTRACTOR TO PROTECT IN PLACE. EXISTING HARDSCAPE TO BE REMOVED.

12

- EXISTING TREE TO BE REMOVED.
- EXISTING TREE TO REMAIN. CONTRACTOR TO PROTECT IN PLACE. G. EXISTING LIGHT POLE TO BE REMOVED.
- H. EXISTING SIGN TO REMAIN. CONTRACTOR TO PROTECT IN PLACE.
- EXISTING GREASE TRAP TO REMOVED. EXISTING DOMESTIC WATER LINE TO BE REMOVED AND CAPPED AT EXISTING WATER VALVE TO BE REUSED FOR PROPOSED WATER SERVICE. CONTRACTOR TO COORDINATE WITH UTILITY COMPANY AND LOCAL JURISDICTION PRIOR TO COMMENCING WORK.
- K. DE-ENERGIZE LIGHTING PRIOR TO COMMENCING WORK OF EXISTING LIGHT POLE ELECTRICAL LINES. LINES TO BE CUT, CAPPED AND ABANDONED IN PLACE. CONTRACTOR TO COORDINATE WITH UTILITY COMPANY PRIOR TO COMMENCING WORK. L. EXISTING SEWER TO BE CUT AND CAPPED AT EXISTING TAP. CONTRACTOR TO
- PROVIDE LOCATION, SIZE AND INVERT TO THE ENGINEER OF RECORD PRIOR TO COMMENCING OF PROPOSED WORK. M. EXISTING GAS SERVICE LINE TO BE CUT AND CAPPED AT EXISTING GAS VALVE.
- CONTRACTOR TO COORDINATE WITH UTILITY COMPANY PRIOR TO BEGINNING OF ANY WORK.
- N. EXISTING STORM LINE TO BE REMOVED. O. EXISTING CATCH BASIN TO BE REMOVED.
- EXISTING UTILITY POLE TO REMAIN AND TO BE PROTECTED IN PLACE.
- EXISTING BOLLARDS TO BE REMOVED. EXISTING TRANSFORMER AND CONCRETE PAD TO BE RELOCATED, REFER TO SITE PLANS FOR PROPOSED LOCATION. CONTRACTOR TO VERIFY IF THE EXISTING TRANSFORMER IS REQUIRED FROM THE UTILITY COMPANY AND ARCHITECTURAL DRAWINGS. IF A NEW SERVICE IS REQUIRED, CONTRACTOR TO COORDINATE WITH UTILITY COMPANY TO OBTAIN PROPER SERVICE. EXISTING ELECTRICAL CONDUITS AND METERS TO BE REMOVED AS NECESSARY
- EXISTING PYLON SIGN AND FOOTINGS TO BE REMOVED.
- U. EXISTING HANDICAP SIGNS TO BE REMOVED.
- V. PROPOSED SAWCUT LINE.
- W. EXISTING PUBLIC RIGHT-OF-WAY SIDEWALK TO REMAIN. CONTRACTOR TO PROTECT IN PLACE. X. EXISTING 24" RCP TO BE REMOVED FOR PROPOSED WATER QUALITY CDS UNIT BY CONTECH PER PROPOSED UTILITY DESIGN. CONTRACTOR TO
- REMOVE/REPLACE EXISTING PIPE AS NECESSARY TO INSTALL UNIT. ANY DAMAGED TO EXISTING STRUCTURES OR PIPES ARE TO BE REPLACED. Y. EXISTING FIRE LINE TO BE REMOVED AT EXISTING WATER VALVE FOR THE INSTALLATION OF PROPOSED FIRE HYDRANT. CONTRACTOR TO COORDINATE WITH AUTHORITATIVE JURISDICTION AND UTILITY COMPANY PRIOR TO
- COMMENCING WORK. Z. EXISTING ELECTRICAL CONDUIT TO REMAIN AND PROTECTED IN PLACE. CONTRACTOR TO VERIFY FINAL TERMINATION OF ELECTRICAL BEFORE BEGINNING OF WORK.

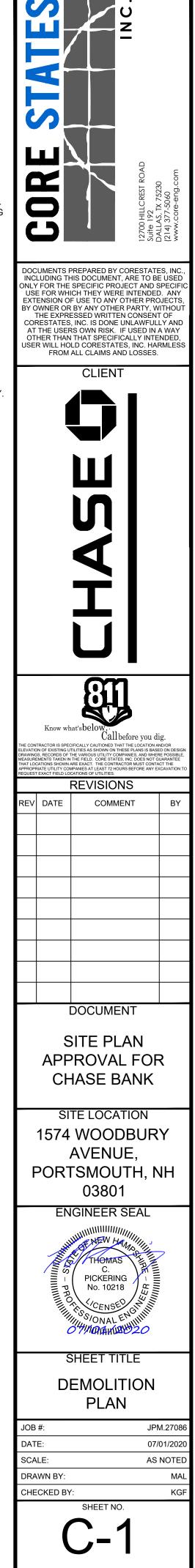
DEMOLITION NOTES:

- 1. THE TOTAL LIMITS OF DISTURBANCE FOR THIS PROJECT IS 36,110 S.F. (0.83 AC).
- 2. CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UNDERGROUND UTILITIES PRIOR TO CONSTRUCTION AND FOR CALLING THE APPROPRIATE ONE-CALL CENTER AT LEAST 72 HOURS IN ADVANCE OF ANY EXCAVATION.

MAINTENANCE NOTES:

ALL MEASURES STATED ON THIS PLAN SHALL BE MAINTAINED IN FULLY FUNCTIONAL CONDITION UNTIL NO LONGER REQUIRED FOR A COMPLETED PHASE OF WORK OR FINAL STABILIZATION OF THE SITE. ALL EROSION AND SEDIMENTATION CONTROL MEASURES SHALL BE CHECKED BY A QUALIFIED PERSON IN ACCORDANCE WITH THE CONTRACT DOCUMENTS OR THE APPLICABLE PERMIT, WHICHEVER IS MORE STRINGENT, AND REPAIRED IN ACCORDANCE WITH THE FOLLOWING:

- 1. INLET PROTECTION SHALL BE REPAIRED TO THEIR ORIGINAL CONDITION IF DAMAGED. SEDIMENT SHALL BE REMOVED FROM THE INLET PROTECTION WHEN CLOGGING BECOMES APPARENT.
- 2. SILT SOXX SHALL BE REPAIRED OR REPLACED TO THEIR ORIGINAL CONDITION IF DAMAGED.



| D |
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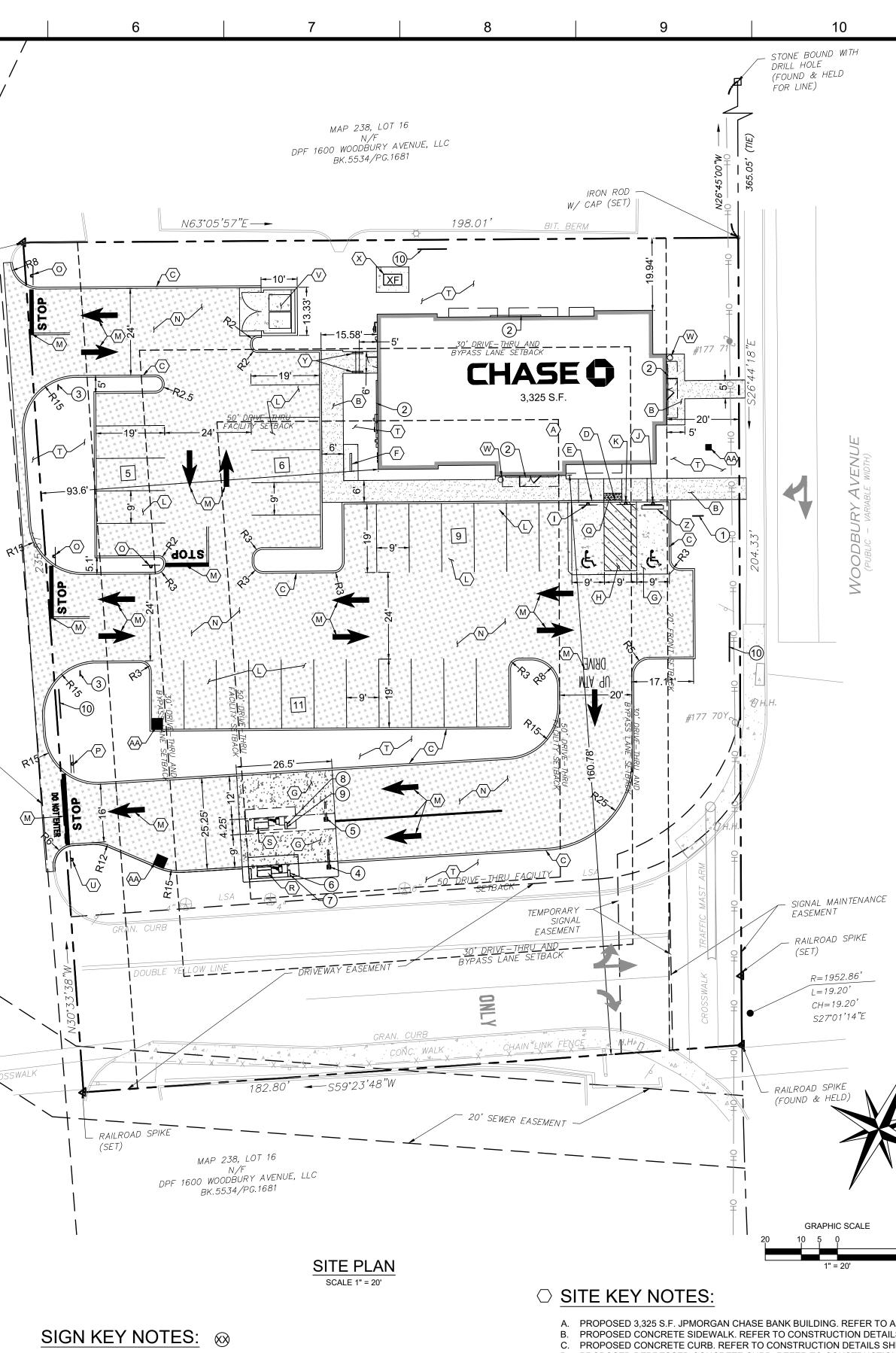
| SIGNA | GE TABLE (SIG | N DISTRICT 5) | |
|-----------------------------------|---|--|---|
| | TEMPORARY S | BIGNS | |
| PROVISION | REQUIRED | PROPOSED | COMMENT |
| MAXIMUM SIGN AREA | 64 SF | 60 SF | COMPLIANT |
| MAXIMUM SIGN HEIGHT | 12 FEET | <12 FT (MOUNTED ON CONSTRUCTION FENCE) | COMPLIANT |
| | FREESTANDING | G SIGN | |
| MAXIMUM ALLOWED PER LOT | 1 | 1 MONUMENT SIGN 1 ATM SIGN 1 FUTURE ATM SIGN | COMPLIANT VARIANCE VARIANCE |
| MAXIMUM SIGN AREA | 100 SF | 56.2 SF 9.5 SF 9.5 SF | COMPLIANT |
| MAXIMUM SETBACK FRONT LOT LINE | 10 FT | 10 FT | COMPLIANT |
| | WALL SIG | N | |
| MAXIMUM ALLOWED | ONE PER STREET FRONTAGE AND/OR AT MAIN ENTRANCE | 1 EAST ELEVATION (WOODBURY AVE) 1 SOUTH ELEVATION 1 NORTH ELEVATION 1 WEST ELEVATION | COMPLIANT - COMPLIANT VARIANCE VARIANCE |
| MAXIMUM SIGN AREA | 100 SF | 36.9 SF | COMPLIANT |
| 7 | TOTAL AGGREGA | TE SIGNS | |
| MAXIMUM AGGREGATE SIGN AREA | 1.5 SF PER LINEAR FEET OF BUILDING FRONTAGE = 1.5 x 42.5 LF = 63.75 SF | 147.6 SF | VARIANCE |

G

ALERT TO CONTRACTOR:

PRIOR TO THE CONSTRUCTION OF OR CONNECTION TO ANY STORM DRAIN, SANITARY SEWER, WATER MAIN OR ANY OF THE DRY UTILITIES, THE CONTRACTOR SHALL EXCAVATE, VERIFY AND CALCULATE ALL POINTS OF CONNECTION AND ALL UTILITY CROSSINGS AND INFORM ENGINEER AND THE OWNER OF ANY CONFLICT OR REQUIRED DEVIATIONS FROM THE PLAN. NOTIFICATION SHALL BE MADE A MINIMUM OF 48 HOURS PRIOR TO CONSTRUCTION. ENGINEER AND OWNER SHALL BE HELD HARMLESS IN THE EVENT THAT THE CONTRACTOR FAILS TO MAKE SUCH NOTIFICATION.

User: MLEWIS Plot Date/Time: Jul. 01, 20 - 10:20:20 Drawing: P:\J.P. Morgan Chase\Portsmouth, NH (1574 Woodbury Avenue) OVP# 38100P322370 - JPM.27086\CIVIL\Drawings\Presentation\JPM.27086-P-SITE-NEW.dwg ;C2-SITE



- PROPOSED CHASE PROTOTYPICAL PYLON SIGN (56.2 S.F.).
 PROPOSED WHITE W/BLUE OCTAGON ILLUMINATED CHANNEL "CHASE" LOGO SIGN
- WALL SIGN (36.9 S.F.).
- PROPOSED DOUBLE FACED NON-ILLUMINATED DIRECTIONAL SIGN (2.3 S.F.).
 PROPOSED CLEARANCE SIGN AND HEADACHE BAR.
- PROPOSED CLEARANCE SIGN AND HEADACHE BAR FOR FUTURE DRIVE-UP ATM.
 PROPOSED SIGNATURE DRIVE-UP "CHASE" LOGO LETTERS (5.6 S.F.).
- 7. PROPOSED SIGNATURE DRIVE-UP OCTAGON (3.9 S.F.).
- 8. FUTURE SIGNATURE DRIVE-UP "CHASE" LOGO LETTERS (5.6 S.F.).
 9. FUTURE SIGNATURE DRIVE-UP OCTAGON (3.9 S.F.).

RAILROAD SPIKE -(SET)

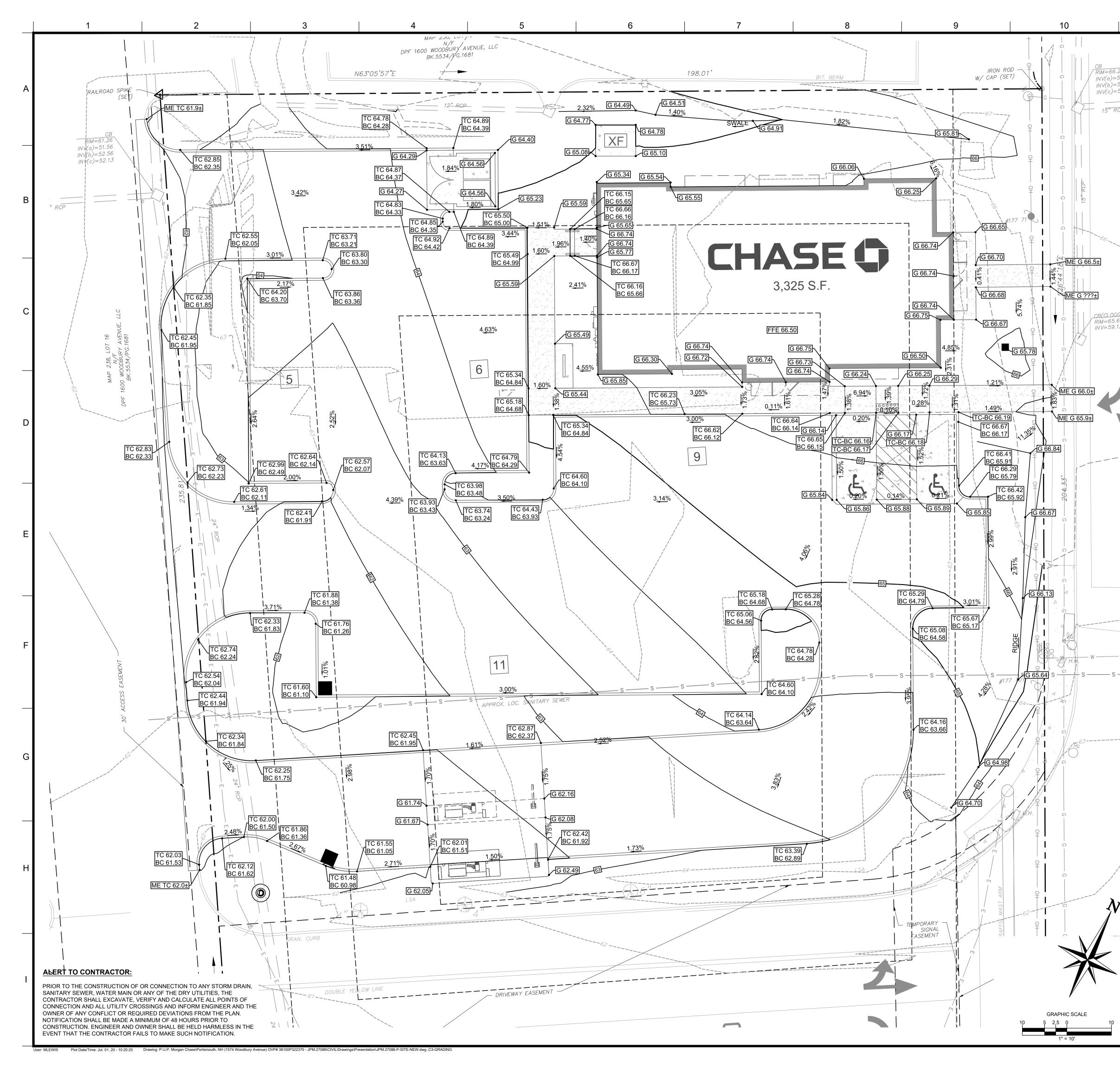
> T 16 AVEI 1681

> > 38, ×38,

10. TEMPORARY "COMING SOON" SIGN MOUNTED ON CONSTRUCTION FENCE (60S.F.).

D. PROPOSED DEPRESSED CONCRETE CURB. REFER TO CONSTRUCTION . PROPOSED TRANSITION CURB SECTION. REFER TO CONSTRUCTION D . PROPOSED BIKE RACK ON CONCRETE PAD. REFER TO CONSTRUCTION G. PROPOSED CONCRETE PAD. REFER TO CONSTRUCTION DETAILS SHE H. PROPOSED 9' X 19' ACCESSIBLE PARKING SPACE AND AISLE WITH SYM CONSTRUCTION DETAILS SHEET. PROPOSED VAN ACCESSIBLE PARKING SIGN. REFER TO CONSTRUCTION . PROPOSED ACCESSIBLE PARKING SIGN. REFER TO CONSTRUCTION D K. PROPOSED NO PARKING ANYTIME SIGN. REFER TO CONSTRUCTION D .. PROPOSED 9' X 19' STANDARD PARKING SPACE. REFER TO CONSTRUC M. PROPOSED SITE MARKINGS. REFER TO CONSTRUCTION DETAILS SHEE N. PROPOSED ASPHALT PAVEMENT. REFER TO CONSTRUCTION DETAILS O. PROPOSED STOP SIGN. REFER TO CONSTRUCTION DETAILS SHEET. P. PROPOSED STOP & DO NOT ENTER SIGN. REFER TO CONSTRUCTION I Q. PROPOSED DETECTABLE WARNING SURFACE. REFER TO CONSTRUCT R. PROPOSED DRIVE-UP SIGNATURE ATM CANOPY. REFER TO CONSTRU S. PROPOSED "FUTURE" DRIVE-UP SIGNATURE ATM CANOPY. REFER TO T. PROPOSED LANDSCAPE AREA. REFER TO LANDSCAPE PLAN FOR DETA U. PROPOSED DO NOT ENTER SIGN. REFER TO CONSTRUCTION DETAILS V. PROPOSED TRASH ENCLOSURE. W. PROPOSED TRASH BIN. REFER TO ARCHITECT PLANS FOR DETAIL. X. RELOCATED ELECTRIC TRANSFORMER AND CONCRETE PAD. REFER T Y. PROPOSED HANDRAIL. REFER TO CONSTRUCTION DETAILS SHEET. Z. PROPOSED WHEEL STOP. REFER TO CONSTRUCTION DETAILS SHEET. AA. PROPOSED STORM DRAIN INLET. REFER TO CONSTRUCTION DETAILS

| | 11 | ĺ | 12 | 13 | |
|----------------------------------|--|---|---|---|---|
| GE | NERAL NOTES: | | | | |
| | DRAWING REFERENCES: | 1574 WOODBURY / PORTSMOUTH, NH TOPOGRAPHIC PL PREPARED BY: AL 400 HARVEY ROAE MANCHESTER, NH CONTACT: JAMES TEL: (603) 627-5500 DATED: 06/23/2020 | I AN OF LAND LEN & MAJOR ASSOCI) 03103 P. SMITH NH LLS) | ATES, INC. | INC. |
| PROF | PERTY OWNER: | RICHARD P. FUSE 201 KEARSARGE V PORTSMOUTH, NH CONTACT: SCOTT (603) 475-377 | VAY 03801 | | |
| APPL | ICANT: | J.P. MORGAN CHA 1450 BRICKELL AV MIAMI, FL 33131 CONTACT: CHRIS I TEL: (786) 473-1769 | ENUE 3RD FLOOR | | TO HILLCREST ROAD TO HILLCREST ROAD Ite 192 Al 377-5060 Ww.core-eng.com |
| , | SITE ADDRESS: | 1574 WOODBURY / PORTSMOUTH, NH COUNTY OF ROCK | AVENUE 03801 INGHAM | | DOCUMENTS PREPARED BY CORESTATES, INC., INCLUDING THIS DOCUMENT, ARE TO BE USED ONLY FOR THE SPECIFIC PROJECT AND SPECIFIC |
| 2) | ZONING DATA: | EXISTING USE: RE PROPOSED USE: E | BANK (PERMITTED BY | ESDAYS (PERMITTED) | USE FOR WHICH THEY WERE INTENDED. ANY EXTENSION OF USE TO ANY OTHER PROJECTS, BY OWNER OR BY ANY OTHER PARTY, WITHOUT THE EXPRESSED WRITTEN CONSENT OF CORESTATES, INC. IS DONE UNLAWFULLY AND AT THE USERS OWN RISK. IF USED IN A WAY OTHER THAN THAT SPECIFICALLY INTENDED, USER WILL HOLD CORESTATES, INC. HARMLESS |
| | §10.5B34.60 (SMALL COMMERCIAL | BUILDING) <u>REQUIRED</u> | EXISTING | PROPOSED | FROM ALL CLAIMS AND LOSSES. |
| | MIN. LOT AREA, SF: MIN. LOT FRONTAGE, FT: FRONT YARD SETBACK, FT: MIN. SIDE YARD SETBACK, FT: MIN. REAR YARD SETBACK, FT: MAX. HEIGHT, FT: MAX. HEIGHT, STORIES: MIN. STREET FACADE HEIGHT: MIN. OPEN SPACE COVERAGE: MAX. BUILDING COVERAGE: | N/A 50 FT. 0 FT 20 FT. 10 FT. 15 FT. 40 FT. 3 18 FT. 10% | 43,673 S.F. 204.32 FT. 87.7 FT. 9.7 FT. 16.7 FT. ± 20 FT. 1 ± 20 FT. 17.79% (7,770 S.F.) 10.53% | NO CHANGE NO CHANGE 20 FT. 19.94 FT. 93.6 FT. 21.5 FT. 1 21.5 FT. 18.18% (7,942 S.F.) 7.55% | |
| | MAX. BUILDING FOOTPRINT: MIN. STREET FACING FACADE GLAZING: | 10,000 S.F. 50% | 4,600 S.F. | 3,325 S.F. 52% | |
| | PARKING REQUIREMENTS: §10.1112.30 OFF-STREET PARKING FOR PROFESSIONAL, BUSINESS A 1 SPACE PER 350 SQUARE FEET C GROSS FLOOR GROSS AREA = 3,3 CALCULATION: 1 SPACE X (3,325 S REQUIRED = 10 SPACES EXISTING PARKING SPACES: 61 SH PROPOSED PARKING SPACES: 31 PARKING DIMENSIONS | ND FINANCIAL SER DF GROSS FLOOR G 325 S.F. (MAIN BUILD 3.F. / 350 S.F.) PACES (INCLUDING | VICES: ROSS AREA VING) 3 ACCESSIBLE SPACE | S) | |
| | EXISTING: VARIES REQUIRED: 8.5' X 19' PROPOSED: 9' X 19' | | | | Know what's below. |
| 5) 6) | ALL EXISTING FEATURES ARE TO ALL PAVEMENT MARKINGS SHALL PRIOR TO STARTING CONSTRUCT REQUIRED PERMITS AND APPROV BEGIN UNTIL THE CONTRACTOR H | . BE LONG LIFE EPO TON, THE CONTRAC /ALS HAVE BEEN OF | XY. TOR SHALL BE RESPO TAINED. NO CONSTR | | THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON DESIGN DRAWINGS, RECORDS OF THE VARIOUS UTILITY COMPANIES, AND WHERE POSSIBLE. MEASUREMENTS TAKEN IN THE FIELD. CORE STATES, INC. DOES NOT GUARANTEE THAT LOCATIONS SHOWN ARE EXACT. THE CONTRACTOR MUST CONTACT THE APPROPRIATE UTILITY COMPANIES AT LEAST 72 HOURS BEFORE ANY EXCAVATION TO REQUEST EXACT FIELD LOCATIONS OF UTILITIES. |
| 7) | DOCUMENTS BY ALL OF THE PERI ALL WORK SHALL BE PERFORMED REQUIREMENTS AND STANDARDS ALL DIMENSIONS SHOWN ON THE | MITTING AUTHORITI D IN ACCORDANCE \ S OF THE LOCAL GO | ES. WITH THESE PLANS AI VERNING AUTHORITY | ND SPECIFICATIONS AND THE | REVISIONS REV DATE COMMENT BY |
| 9) | WITH CONSTRUCTION FOR NECES CONTRACTOR FOR WORK HAVING THESE PLANS IF SUCH NOTIFICAT | SSARY PLAN CHANG G TO BE REDONE DU ION HAS NOT BEEN | GES. NO EXTRA COMP JE TO DIMENSIONS OF GIVEN. | PANCIES EXIST PRIOR TO PROCEEDING PENSATION SHALL BE PAID TO THE R GRADES SHOWN INCORRECTLY ON TH ALL LOCAL, STATE AND FEDERAL | |
| 11) | ALL EXCAVATED UNSUITABLE MA CONTRACTOR IS RESPONSIBLE F IN ACCORDANCE WITH CURRENT STABILITY OF CONTIGUOUS STRU | OR ALL SHORING R OSHA STANDARDS, | EQUIRED DURING EXC AS WELL AS ADDITIO | CAVATION AND SHALL BE PERFORMED NAL PROVISIONS TO ASSURE | |
| | | | | | |
| | | | | | DOCUMENT SITE PLAN APPROVAL FOR CHASE BANK |
| 20 | | | | | SITE LOCATION 1574 WOODBURY AVENUE, PORTSMOUTH, NH 03801 |
| AILS SHEE ⁻ SHEET. | | | SITE LE | GEND | ENGINEER SEAL |
| HEET. YMBOLS C | | | | EXISTING PROPERTY BOUNDARY LINE EXISTING ADJOINING PROPERTY LINE EXISTING ROAD CENTERLINE PROPOSED ROAD CENTERLINE | THOMAS C. PICKERING No. 10218 PHOTOS POCKERING No. 10218 CENSED OTHOUSUND |
| I DETAILS S I DETAILS S | SHEET. SHEET. ETAILS SHEET/ | | · · · | PROPOSED DITCH CENTERLINE PROPOSED LIMITS OF BMP / DETENTION PROPOSED SAWCUT LINE EXISTING CURB PROPOSED CURB | SHEET TITLE SITE PLAN |
| RUCTION D | SHEET. TAILS SHEET. DETAILS SHEET. RUCTION DETAILS SHEET. | | · · · · · · · · · · · · · · · · · · · | PROPOSED DEPRESSED CURB PROPOSED BUILDING PROPOSED ASPHALT | JOB #: JPM.27086 |
| LS SHEET. | TING PLANS FOR DETAIL. | | | PROPOSED CONCRETE EXISTING SANITARY STRUCTURES | DATE: 07/01/2020 SCALE: AS NOTED DRAWN BY: MAL |
| ET. LS SHEET. | | 0 | → OH × | EXISTING WATER STRUCTURES EXISTING OVERHEAD WIRES PROPOSED PARKING COUNT | CHECKED BY: KGF SHEET NO. C-2 |
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| 6.25 =56.65 =58.95 =58.95 | EXIS CHA 1574 POF ALL | STING COND ASE BANK SI 4 WOODBUR RTSMOUTH, I | TE Y AVENUE NH ASSOCIATE | ΈY | EY PREPARED E | 3Y: | | | | ATES | | |
| RCP | G | | G NOTE | ADING CONS | STRUCTION M | | | | ALL | | | Q _ |
| | 2. | INCLUDIN | G REMOVAI | OF ANY EX | FOR DEMOLIT | TIES SER | RVING THE | | | OR | | 12700 HILLCREST ROAD Suite 192 DALLAS, TX 75230 (214) 377-5060 www.core-eng.com |
| | 3. | ELEVATIO RECORDS MEASURE ON AS BE APPROPR EXCAVATI THE RESF | N OF EXIST OF THE VA MENTS TAU ING EXACT LATE UTILIT ON TO REC PONSIBILITY WHICH COI | ING UTILITIE RIOUS UTIL EN IN THE F OR COMPLE Y COMPANI UEST EXAC | LLY CAUTION ES AS SHOWN ITY COMPANI FIELD. THE IN ETE. THE CON ES AT LEAST T FIELD LOCA INTRACTOR T I THE PROPO | I ON THE ES, AND FORMAT TRACTO 72 HOUF ATION OF O RELO(| ESE PLANS WHERE P TON IS NO OR MUST O RS BEFOR UTILITIES CATE ALL | S IS BASED POSSIBLE, PT TO BE RE CALL THE E ANY S. IT SHALL EXISTING | ON LIED BE | INCLUDING TH ONLY FOR THE USE FOR WHI EXTENSION OF BY OWNER OR THE EXPRE CORESTATES AT THE USER OTHER THAN USER WILL HO | REPARED BY COF IS DOCUMENT, AF SPECIFIC PROJEC CH THEY WERE IN USE TO ANY OTHER P. SSED WRITTEN C INC. IS DONE UNIN S OWN RISK. IF L THAT SPECIFICAL D CORESTATES, ALL CLAIMS AND L | RESTATES, INC., RE TO BE USED T AND SPECIFIC ITENDED. ANY HER PROJECTS, ARTY, WITHOUT ONSENT OF LAWFULLY AND ISED IN A WAY LY INTENDED, INC. HARMLESS |
| | 4. | ALL CUT (NOTED. | OR FILL SLC | PES SHALL | BE 4:1 OR FL | ATTER U | INLESS OT | THERWISE | | | CLIENT | |
|) <u>GGED)</u> 5.62 9.12 | 5. | PRECAST | STRUCTUR | ES ARE REC | QUIRED. | | | | | | | |
| 7.12 | 6. | | PES TO BE INSPECTIO | | UT TO REMO | VE ALL S | SILT AND D | ebris. Pri | OR | | Ш | |
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| | | | | | S SHOWN AT O REMAIN AR | | | NG | | | | |
| | | AND/OR R EXISTING | EPLACE TH | E EXISTING IS OR BETTE | | AS NEC | ESSARY T | O RETURN I | IT TO | | T | |
| | 10. | | | | UCTURES SH, WATERTIGHT | | GROUTED | TO ASSURE | | | Ū | |
| | 11. | PAVEMEN | T, AND SHA AREAS SH | LL HAVE TR | N PAVED ARE AFFIC BEARII BOVE FINISH (| NG RING | & COVER | S. MANHOLE | | | | |
| | 12. | IN THE AP | PLICABLE S | TATE GENE | RE TO ALL TE RAL PERMIT ON ACTIVITIE | FOR STC | | | | | at's below. Call befo FICALLY CAUTIONED THAT THI ILITIES AS SHOWN ON THESE I HE VARIOUS UTILITY COMPANI THE FIELD. CORE STATES. INC RE EXACT. THE CONTRACTOR PANIES AT LEAST 72 HOURS B | |
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| | 14. | | | ASSURE PO | DSITIVE DRAII 3. | NAGE AV | VAY FROM | 1 BUILDINGS | 6 FOR | | | |
| | 15. | LAND SUR TOPOGRA CONTRAC | RVEYORS. I APHY AS SH STOR SHALL | F THE CONT OWN ON TH . SUPPLY, A | TAKEN FROM RACTOR DOB E PLANS, WIT T THEIR EXPE TO THE OWN | ES NOT A HOUT EX NSE, A 1 | ACCEPT EX XCEPTION TOPOGRA | XISTING I, THEN THE | | | | |
| — w — s —— | 16. | 4 INCHES ALL SLOP AREAS IN | OF TOPSO ES 3H:1V O ACCORDAN | L. CONTRA R STEEPER. | RBED BY GRA CTOR SHALL CONTRACTO OVERNING SF INED. | APPLY S DR SHAL | TABILIZAT L STABILIZ | TION FABRIC ZE DISTURB | C TO ED | | | |
| | 17. | | | LL COMPLY ED TO SAME | WITH ALL AP | PLICABL | E GOVERI | | 6 | | OCUMEN | |
| | 18. | - | | - | . HAVE A SMC RT OUT, UNL | | | | TAR | APP | ROVAL | FOR |
| | 19. | AND CRO | SSWALKS. | CONTRACT | MAXIMUM CI OR TO MODIF IUM CROSS-S | Y PAVEN | /IENT GRA | DES AS | LKS | | TE LOCATI | |
| | 20 | WITH 2% I | MINIMUM SI | | TIVE DRAINA VIOUS AREA ISE NOTED. | | | | | | AVENUE SMOUT 03801 | , |
| V | 21 | AND ADDI TO FILL PI PROOF-CO (15-TONST SURFACE THE TIME ROLLER IN THROUGH UNSTABLE EXCAVATI REPRESE UTILITIES AND PAVE COMPLY N PLACEME | TIONAL CU LACEMENT OMPACTED TATIC WEIG SOILS. HOW OF CONSTIN STATIC M THESE TY SOILS IDE ED TO AN A NTATIVE. IF SHOULD BE SHOULD BE MENT ARE WITH SECTINT OF A GE | TTING TO RE AND NEW C WITH MULT HT) VIBRATO VEVER, DEF RUCTION, IT ODE IN ORD PICALLY GR NTIFIED DU CCEPTABLE PRESENT, A REMOVED AS. ANY REO ON 3.3 STRU | RING/GRUBBIN EMOVE UNSU ONSTRUCTIO IPLE PERPEN ORY ROLLER PENDING ON T MAY BE NEC ER TO MINIMI ANULAR SOIL RING THE PRO BEARING ST ABANDONED ENTIRELY BE QUIRED BACK JCTURAL FILL ND/OR COAR DERCUT | ITABLE C N, THE E DICULAF TO COM THE GRC ESSARY ZE DRAV ZE DRAV ZE DRAV ZE DRAV SOF-COI RATUM A FOUNDA ELOW TH FOUNDA | CONDITION ENTIRE SIT R PASSES PACT LOC DUNDWATE TO OPER VING WAT LOOSE, SO MPACTION AS DETER TIONS, SL E PROPOS NEW FILL IAL AND P | NS AND PRIC TE SHOULD OF A LARGE OSE, NEAR ER LEVEL A ⁻ ATE THE ER UP OFT, WET O N SHOULD B MINED BY A ABS AND SED BUILDIN SHOULD LACEMENT. | OR BE T T R E S S S I S I S I S I S I S I S I S I S | GR | GINEER SE | |
| • | 22 | EXTERIOF | R GRADE IS | PROPOSED | ROOFING AT A ABOVE THE I WITH ARCHIT | FINISHED | D FLOOR E | ELEVATION. | 0 | JOB #: DATE: SCALE: DRAWN BY: CHECKED BY | : SHEET NO. | JPM.27086 07/01/2020 AS NOTED MAL KGF |
|) | | | | | | | | | | (| C-3 | } |
| | | | | | | | | | | | | |
| | | | | | | | | | | | | |

| SANITARY STRUCTURE TABL |
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| STRUCTURE NAME | STRUCTURE TYPE | RIM | UPSTREAM PIPE NAME | UPSTREAM PIPE INV | DOWNSTREAM PIPE NAME | DOWNSTREAM PIPE INV | DOWNSTREAM PIPE LENGTH | DOWNSTREAM PIPE SIZE AND TYPE | DOWNSTREAM PIPE SLOPE |
|----------------|-------------------------|-------|-----------------------|----------------------|-------------------------|------------------------|---------------------------|----------------------------------|--------------------------|
| A1 | BUILDING STUB | 65.45 | | | SAN - 1 | 63.75' | 3.00' | 4" PVC | 2.00% |
| A2 | CLEANOUT | 65.27 | SAN - 1 | 63.69' | SAN - 2 | 59 <u>.</u> 62' | 20.98' | 4" PVC | 2.00% |
| A3 | 45° BEND | 64.87 | SAN - 2 | 59.20' | SAN - 3 | 59.20' | 52 <u>.</u> 17' | 4" PVC | 2.00% |
| A4 | 45° BEND | 63.37 | SAN - 3 | 58.16' | SAN - 4 | 58.16' | 83.03' | 4" PVC | 2.00% |
| A5 | EXISTING LATERAL TIE-IN | 62.08 | SAN - 4 | 56.50' | | | | | |

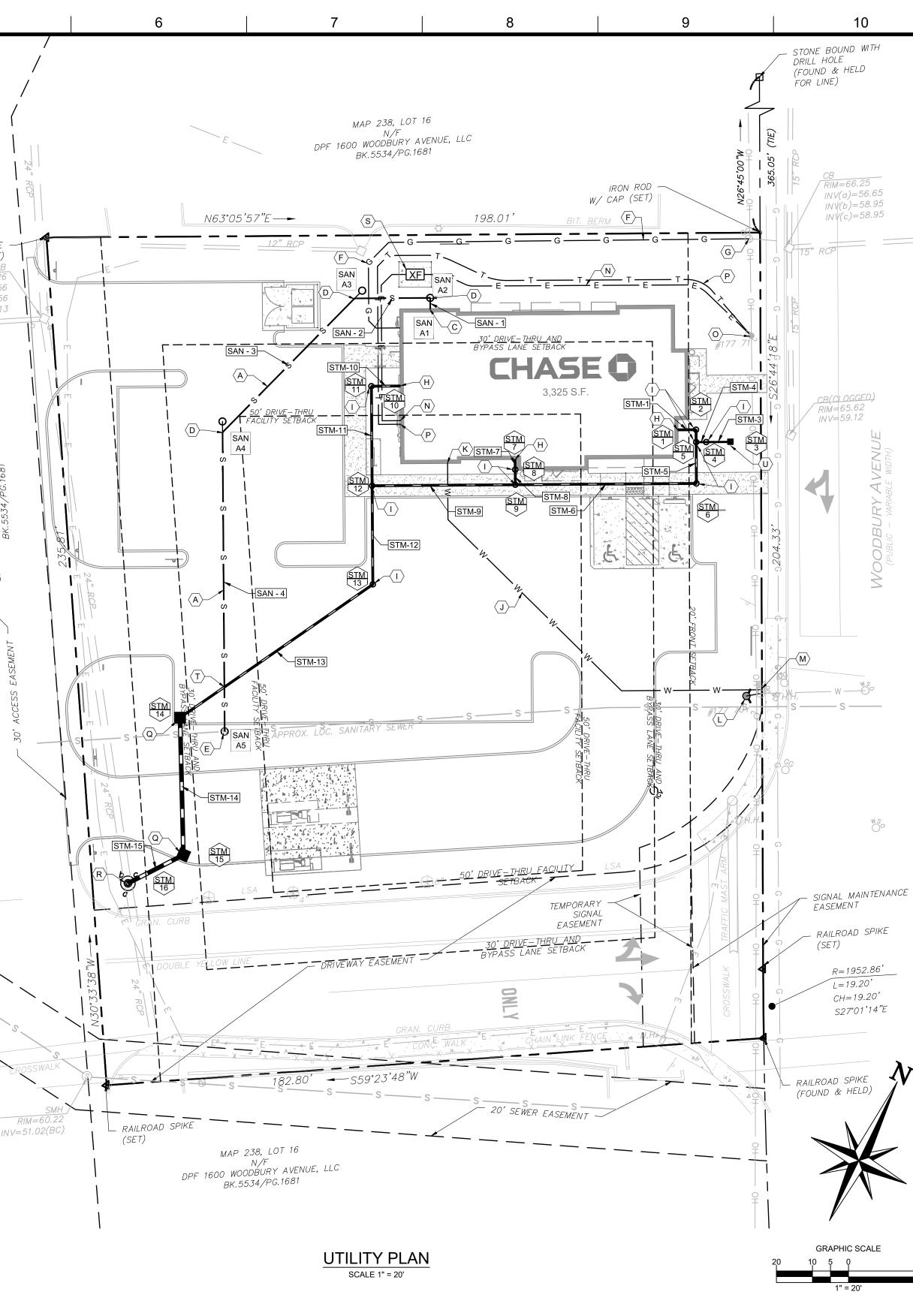
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| | | | | | STORM S | TRUCTURE TABLE | E | | | |
|---|----------------|---------------------|-------|-----------------------|----------------------|-------------------------|------------------------|---------------------------|----------------------------------|--------------------------|
| ; | STRUCTURE NAME | STRUCTURE TYPE | RIM | UPSTREAM PIPE NAME | UPSTREAM PIPE INV | DOWNSTREAM PIPE NAME | DOWNSTREAM PIPE INV | DOWNSTREAM PIPE LENGTH | DOWNSTREAM PIPE SIZE AND TYPE | DOWNSTREAM PIPE SLOPE |
| | 1 | BUILDING STUB | 66.68 | | | STM-1 | 63.54' | 5.00' | 6" PVC | 3.50% |
| | 2 | СО | 66.54 | STM-1 | 63.37' | STM-2 | 63.37' | 3.44' | 6" PVC | 3.13% |
| | 3 | PROPOSED AREA DRAIN | 65.78 | | | STM-3 | 64.50' | 6.65' | 4" PVC | 1.00% |
| | 4 | CLEANOUT | 66.22 | STM-3 | 64.43' | STM-4 | 64.43' | 2.79' | 4" PVC | 42.19% |
| | 5 | WYE BEND | 66.38 | STM-2 STM-4 | 63.26' 63.26' | STM-5 | 63.26' | 11.46' | 6" PVC | 2.96% |
| | 6 | СО | 66.21 | STM-5 | 62.92' | STM-6 | 62.92' | 50.25' | 6" PVC | 2.50% |
| | 7 | BUILDING STUB | 66.74 | | | STM-7 | 63.22' | 2.82' | 6" PVC | 2.50% |
| | 8 | СО | 66.72 | STM-7 | 63.15' | STM-8 | 63.15' | 4.21' | 6" PVC | 35.45% |
| 1 | 9 | со | 66.65 | STM-6 STM-8 | 61.66' 61.66' | STM-9 | 61.66' | 39.74' | 6" PVC | 2.50% |
| | 10 | BUILDING STUB | 65.78 | | | STM-10 | 62.49' | 7.99' | 6" PVC | 2.50% |
| | 11 | со | 65.60 | STM-10 | 62.29' | STM-11 | 62.29' | 27.61' | 6" PVC | 5.86% |
| | 12 | со | 65.45 | STM-9 STM-11 | 60.67' 60.67' | STM-12 | 60.67' | 27.38' | 6" PVC | 2.50% |
| | 13 | со | 63.92 | STM-12 | 59.98' | STM-13 | 59.98' | 65.06' | 6" PVC | 2.00% |
| | 14 | PROP CURB INLET | 61.18 | STM-13 | 58.68' | STM-14 | 57.68' | 38.07' | 12" HDPE | 1.50% |
| | 15 | PROP CURB INLET | 61.07 | STM-14 | 57.11' | STM-15 | 57.11' | 17.30' | 12" HDPE | 1.50% |
| | 16 | PROP MANHOLE | 61.13 | STM-15 | 56.85' | | | | | |

Iser: MLEWIS Plot Date/Time: Jul. 01, 20 - 10:20:30 Drawing: P:\J.P. Morgan Chase\Portsmouth, NH (1574 Woodbury Avenue) OVP# 38100P322370 - JPM.27086\CIVIL\Drawings\Presentation\JPM.27086-P-SITE-NEW.dwg; C4-UTIL

ALERT TO CONTRACTOR:

PRIOR TO THE CONSTRUCTION OF OR CONNECTION TO ANY STORM DRAIN, SANITARY SEWER, WATER MAIN OR ANY OF THE DRY UTILITIES, THE CONTRACTOR SHALL EXCAVATE, VERIFY AND CALCULATE ALL POINTS OF CONNECTION AND ALL UTILITY CROSSINGS AND INFORM ENGINEER AND THE OWNER OF ANY CONFLICT OR REQUIRED DEVIATIONS FROM THE PLAN. NOTIFICATION SHALL BE MADE A MINIMUM OF 48 HOURS PRIOR TO CONSTRUCTION. ENGINEER AND OWNER SHALL BE HELD HARMLESS IN THE EVENT THAT THE CONTRACTOR FAILS TO MAKE SUCH NOTIFICATION.



AILROAD SPIKE

RIM=61.2 INV(a)=51.56

INV(b) = 52.56

INV(c) = 52.13

% ≥<u>n</u>

 \searrow

15" RCP

(SET)

UTILITY NOTES:

1. THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND / OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF THE VARIOUS UTILITY COMPANIES, AND WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED UPON AS BEING COMPLETE OR EXACT. THE CONTRACTOR MUST CALL THE APPROPRIATE UTILITY COMPANIES AT LEAST 72 HOURS BEFORE ANY EXCAVATION TO REQUEST EXACT FIELD LOCATION OF UTILITIES. IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO RELOCATE ALL EXISTING UTILITIES WHICH CONFLICT WITH THE PROPOSED IMPROVEMENTS SHOWN ON THE PLANS.

| GENERAL | NOTES: |
|---------|--------|

11

1. THIS PROJECT REFERENCES A SURVEY PREPARED BY:

EXISTING CONDITIONS SURVEY

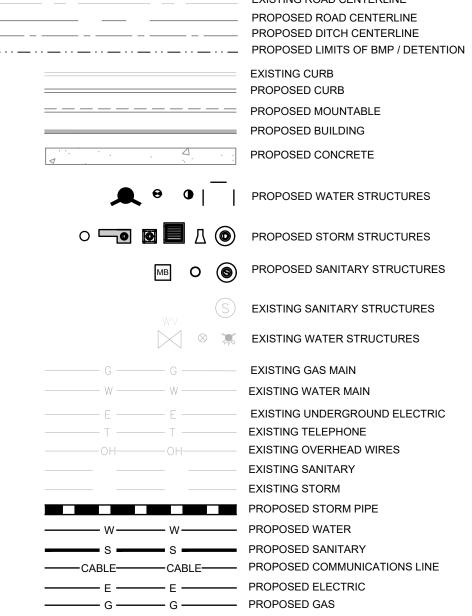
CHASE BANK SITE 1574 WOODBURY AVENUE

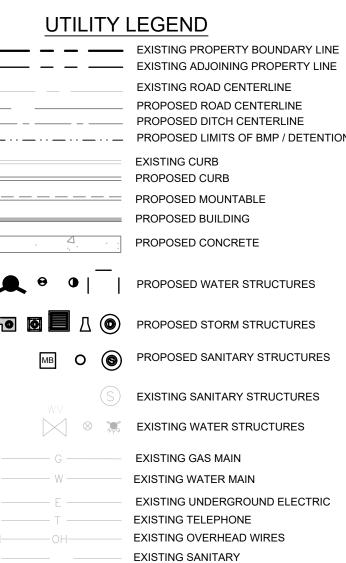
PORTSMOUTH, NH ALLEN & MAJOR ASSOCIATES, INC,

DATED 06/23/2020

○ UTILITY KEY NOTES:

- A. PROPOSED 4" SDR PVC SANITARY SEWER LINE AT A MINIMUM OF 2.0% SLOPE B. PROPOSED SANITARY CONNECTION TO EXISTING LATERAL. CONTRACTOR TO PROVIDE LOCATION OF ENTIRE LATERAL SIZE AND INVERTS PRIOR TO ANY PROPOSED WORK TO ENGINEER OF RECORD. IF EXISTING LATERAL EXTENDS UNDER THE PROPOSED BUILDING LOCATION AND FOOTING, CONTRACTOR TO WORK WITH ENGINEER OF RECORD TO RE-ROUTE EXISTING LATERAL.
- C. PROPOSED SANITARY LATERAL CONNECTION TO PROPOSED BUILDING. REFER TO ARCHITECTURAL PLANS FOR DETAILS.
- D. PROPOSED SANITARY SEWER CLEANOUT.
- E. PROPOSED SANITARY SEWER CONNECTION AT EXISTING SEWER LINE. CONTRACTOR TO VERIFY LOCATION PRIOR TO COMMENCING OF PROPOSED WORK.
- F. PROPOSED GAS SERVICE LINE. CONTRACTOR TO COORDINATE WITH UTILITY COMPANY PRIOR TO INSTALLATION.
- G. PROPOSED GAS CONNECTION TO EXISTING GAS VALVE. CONTRACTOR TO COORDINATE WITH UTILITY COMPANY TO VERIFY IF EXISTING CONNECTION IS ADEQUATE.
- H. PROPOSED 6" PVC UNDERGROUND ROOF DRAIN.
- I. PROPOSED STORM DRAIN CLEAN OUT. J. PROPOSED 1-1/2" WATER SERVICE LINE.
- K. PROPOSED WATER SERVICE CONNECTION TO BUILDING.
- PROPOSED FIRE HYDRANT TO BE INSTALLED PER TOWNSHIP REQUEST.
- M. PROPOSED WATER SERVICE CONNECTION TO EXISTING WATER VALVE. CONTRACTOR TO DETERMINE SIZE OF EXISTING SERVICE AND COORDINATE WITH UTILITY COMPANY PRIOR TO COMMENCING OF WORK.
- N. PROPOSED ELECTRICAL CONDUITS. O. PROPOSED ELECTRICAL POINT OF CONNECTION AT EXISTING POWER POLE. CONTRACTOR TO VERIFY IF SERVICE PROVIDED AT POWER POLE IS ADEQUATE FOR THE PROPOSED BUILDING. CONTRACTOR TO COORDINATE WITH UTILITY COMPANY PRIOR TO COMMENCING ANY WORK.
- P. PROPOSED DATA CONDUITS.
- Q. PROPOSED STORM DRAIN INLET. R. PROPOSED WATER QUALITY CDS UNIT BY CONTECH. REFER TO CONSTRUCTION DETAILS SHEET.
- S. PROPOSED ELECTRICAL TRANSFORMER AND CONCRETE PAD. REFER TO LIGHTING PLANS FOR DETAIL.
- T. PROPOSED SANITARY AND STORM CROSSING. TOP OF SANITARY PIPE = 57.09'
- BOTTOM OF STORM PIPE = 58.97'
- U. PROPOSED 12-INCH YARD DRAIN. REFER TO CONSTRUCTION DETAILS SHEET.





EXISTING STORM

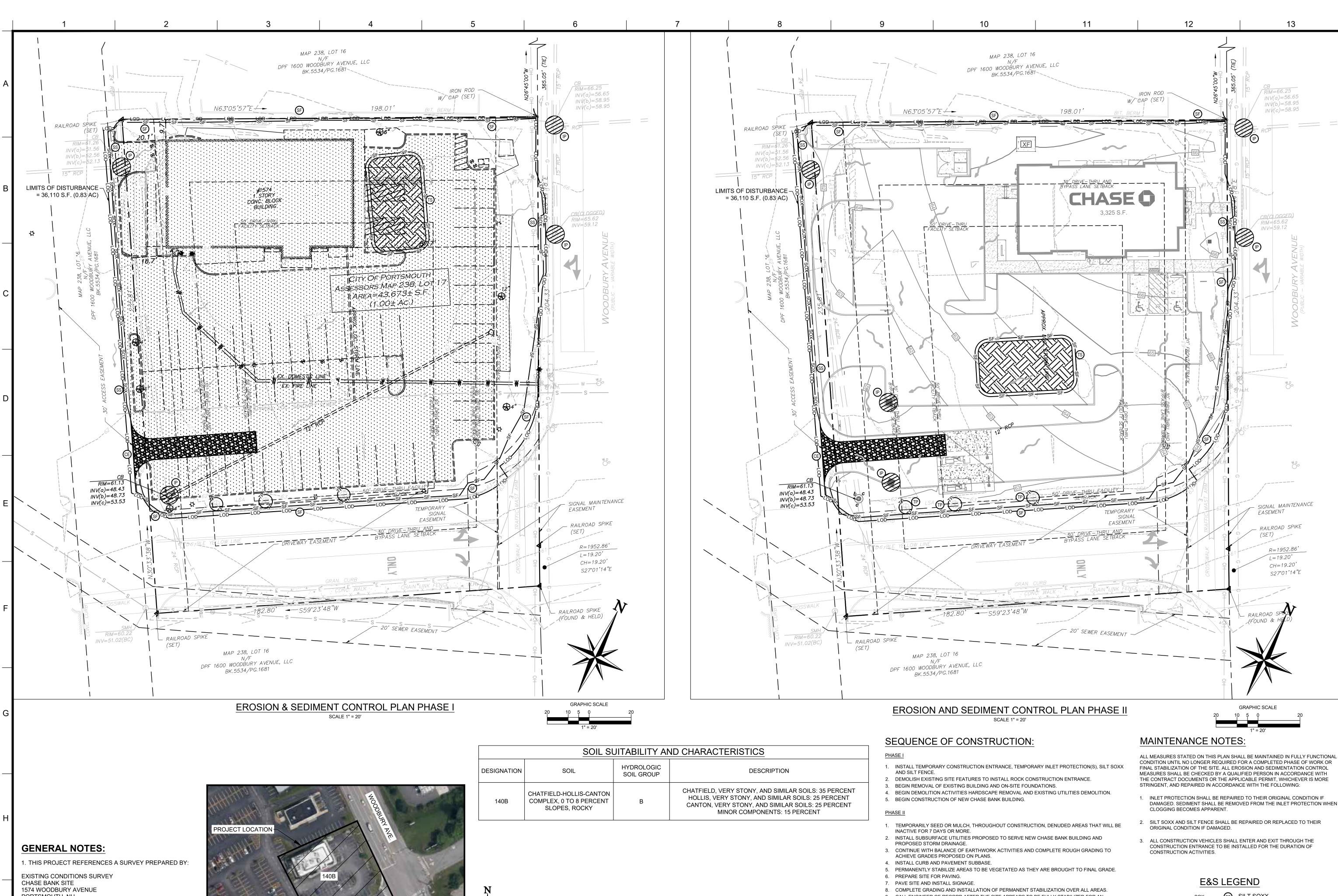


SHEET TITLE

UTILITY PLAN

| OB #: | JPM.27086 |
|-------------|------------|
| DATE: | 07/01/2020 |
| SCALE: | AS NOTED |
| DRAWN BY: | MAL |
| CHECKED BY: | KGF |
| SHEET NO. | |
| C-4 | |

12



PORTSMOUTH, NH ALLEN & MAJOR ASSOCIATES, INC, DATED 06/23/2020

ALERT TO CONTRACTOR:

ser: MLEWIS

PRIOR TO THE CONSTRUCTION OF OR CONNECTION TO ANY STORM DRAIN, SANITARY SEWER, WATER MAIN OR ANY OF THE DRY UTILITIES, THE CONTRACTOR SHALL EXCAVATE, VERIFY AND CALCULATE ALL POINTS OF CONNECTION AND ALL UTILITY CROSSINGS AND INFORM ENGINEER AND THE OWNER OF ANY CONFLICT OR REQUIRED DEVIATIONS FROM THE PLAN. NOTIFICATION SHALL BE MADE A MINIMUM OF 48 HOURS PRIOR TO CONSTRUCTION, ENGINEER AND OWNER SHALL BE HELD HARMLESS IN THE EVENT THAT THE CONTRACTOR FAILS TO MAKE SUCH NOTIFICATION.





Plot Date/Time: Jul. 01, 20 - 10:20:37 Drawing: P:\J.P. Morgan Chase\Portsmouth, NH (1574 Woodbury Avenue) OVP# 38100P322370 - JPM.27086\CIVIL\Drawings\Presentation\JPM.27086-P-SITE-NEW.dwg ;C5-EROS

| | SOIL SUITABILITY AND CHARACTERISTICS | | | | | | | | | |
|------------|---|--------------------------|--|--|--|--|--|--|--|--|
| ESIGNATION | SOIL | HYDROLOGIC SOIL GROUP | DESCRIPTION | | | | | | | |
| 140B | CHATFIELD-HOLLIS-CANTON COMPLEX, 0 TO 8 PERCENT SLOPES, ROCKY | В | CHATFIELD, VERY STONY, AND SIMILAR SOILS: 35 PERCENT HOLLIS, VERY STONY, AND SIMILAR SOILS: 25 PERCENT CANTON, VERY STONY, AND SIMILAR SOILS: 25 PERCENT MINOR COMPONENTS: 15 PERCENT | | | | | | | |

- 9. CALL ENGINEER OF RECORD AFTER THE SITE APPEARS TO BE FULLY STABILIZED FOR AN
- INSPECTION. 10. REMOVE ALL TEMPORARY EROSION AND SEDIMENT CONTROL DEVICES AFTER APPROVAL OF THE
- ENGINEER OF RECORD AND STABILIZE ANY AREAS DISTURBED BY THE REMOVAL OF THE BMP. 11. CONTINUE DAILY INSPECTION REPORTS UNTIL THE FINAL DAILY INSPECTION IS SIGNED OFF BY

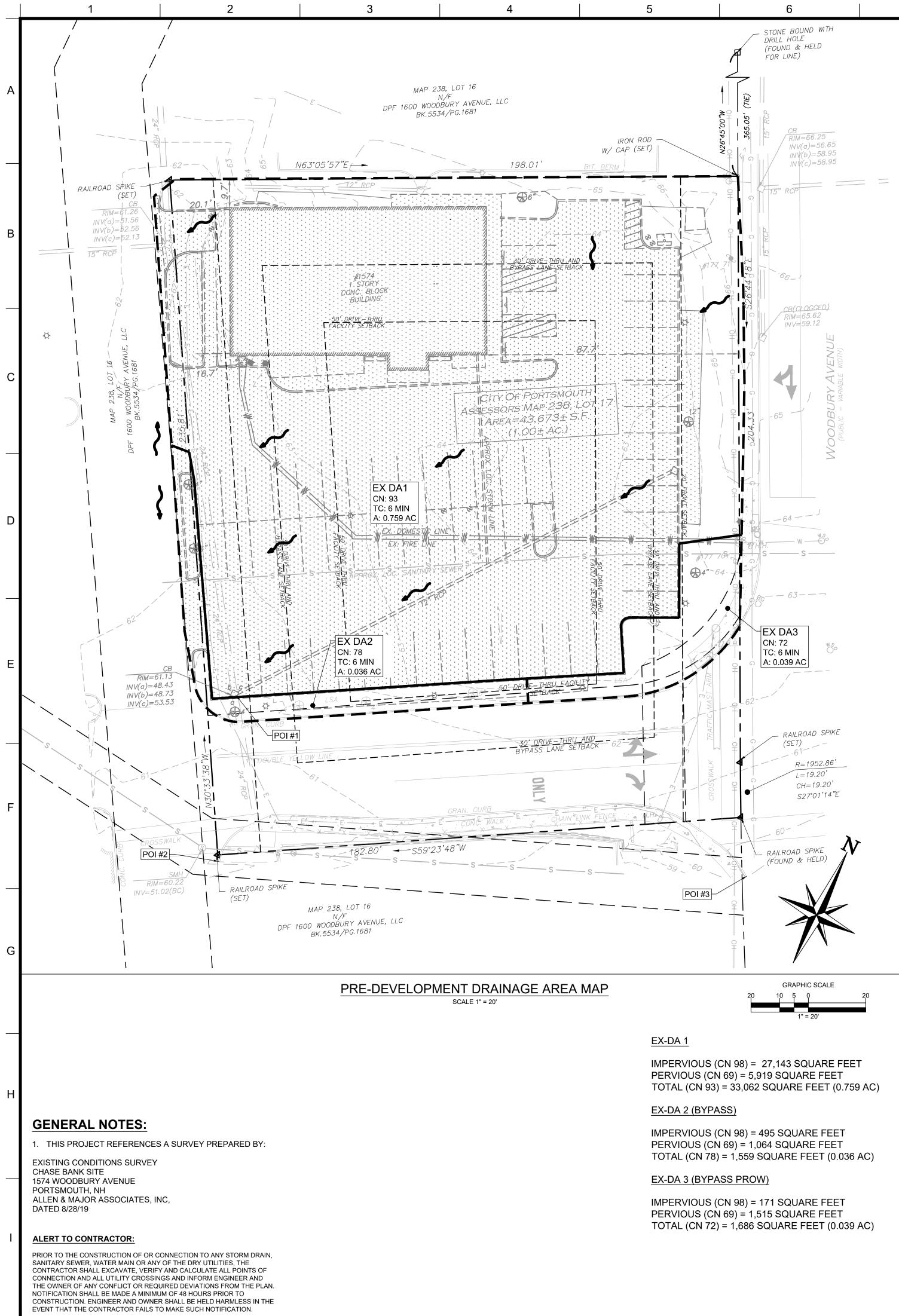
NOTE: THE GENERAL CONTRACTOR MAY COMPLETE CONSTRUCTION-RELATED ACTIVITIES CONCURRENTLY ONLY IF ALL PRECEDING BMPS HAVE BEEN COMPLETELY INSTALLED.

THE OWNER THAT THE SITE IS FULLY STABILIZED AND THE PERMIT MAY BE TERMINATED.

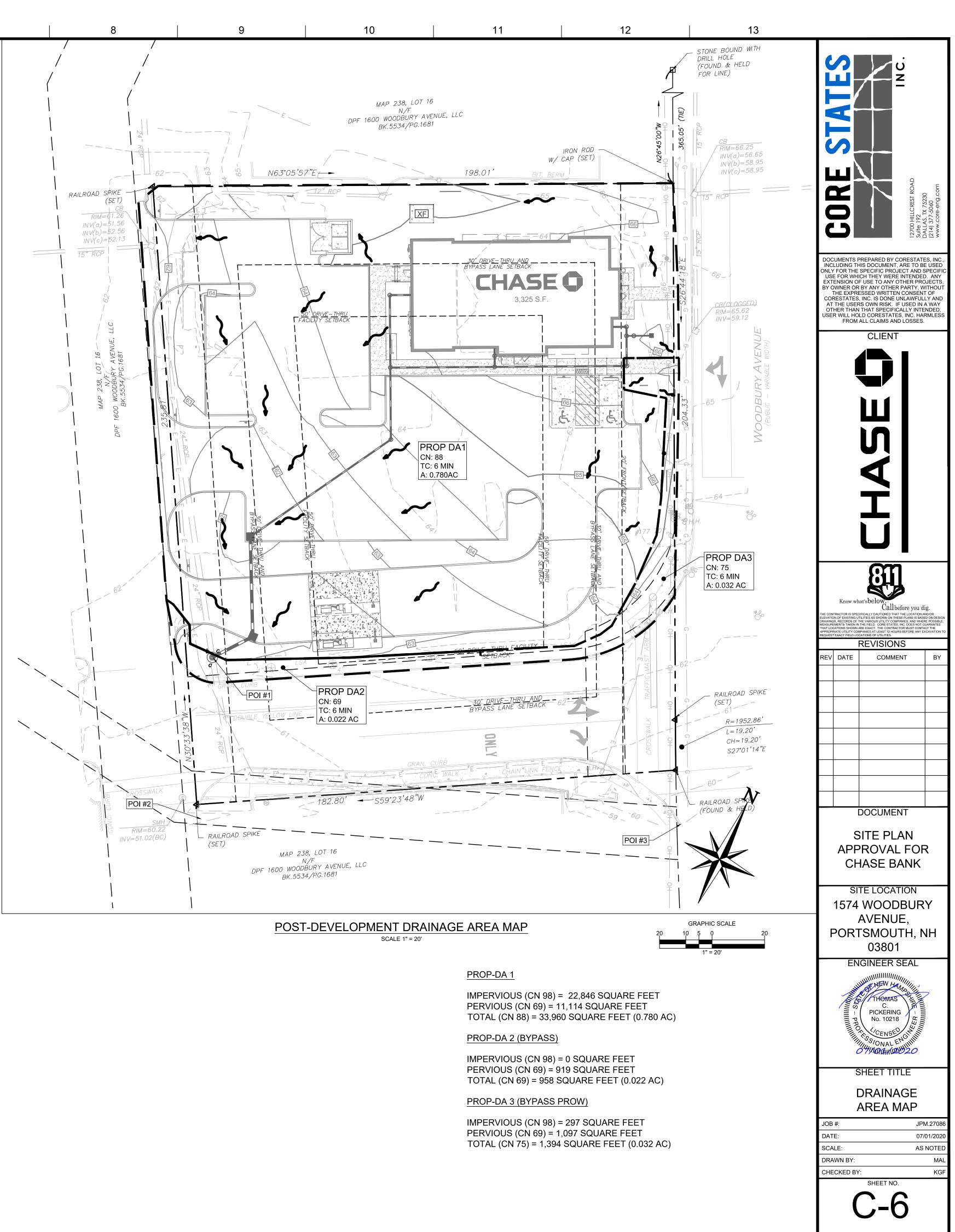
DAMAGED. SEDIMENT SHALL BE REMOVED FROM THE INLET PROTECTION WHEN

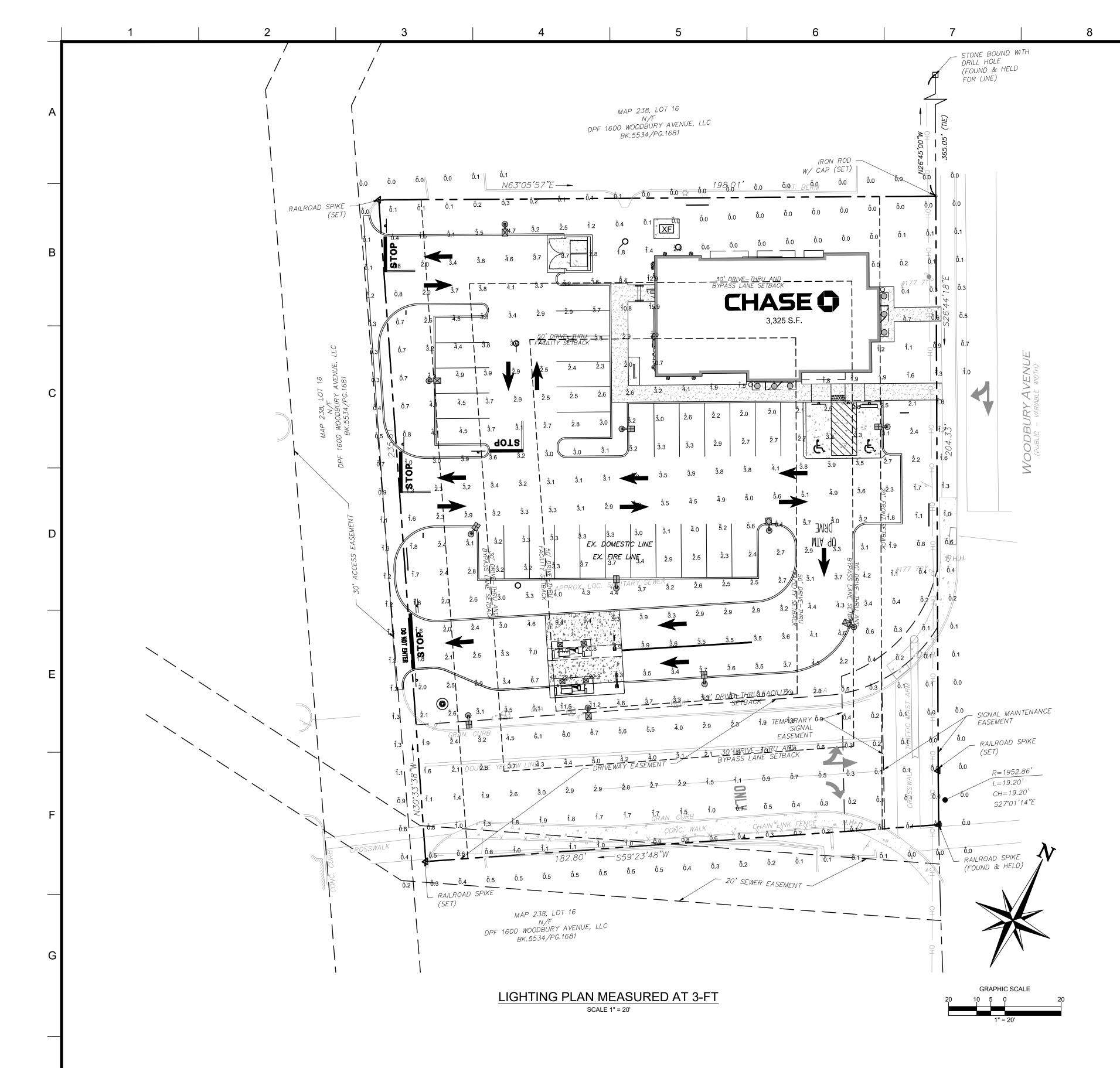
TS TOPSOIL STOCKPILE CONSTRUCTION ENTRANCE LIMITS OF DISTURBANCE

| | GURE VIAIES | | 12700 HILLCREST ROAD Suite 192 Suite 192 | UALLAS, 1X 73230 (214) 377-5060 www.core-eng.com |
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ALERT TO CONTRACTOR:

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| | LUMINAIRE SCHEDULE | | | | | | | |
|-----------|--------------------|-----------------------|--|-----|--------------|--------------|--|--|
| SYMBOL | MANUFACTURER | MODEL | CATALOG | QTY | DISTRIBUTION | MOUNT HT. | | |
| œŧ | COOPER LIGHTING | GLEON GALLEON LED | GLEON-AF-02-LED-E1-5WQ-7030 | 6 | TYPE 5 | 20'-0" | | |
| œ | COOPER LIGHTING | GLEON GALLEON LED | GLEON AF-02-E1-SL4-7030 | 4 | TYPE 4 | 20'-0" | | |
| Ē | COOPER LIGHTING | GLEON GALLEON LED | GLEON-AF-02-E1-SL4-7030-HSS | 1 | TYPE 4 | 20'-0" | | |
| \square | CREE LIGHTING | LED SQUARE CANOPY | C-CP-A-SQ-49L-50K-DB | 4 | TYPE 5 | 9'-10" | | |
| 0 | AMERLUX | HORNET HP | HDL-HP-R-NC-A17-T-16-120-0-10V/HDL-HP-RL | 6 | TYPE 5 | 9'-10" | | |
| \oplus | EATON | LANTERRA 9004 | 9004-W2-RW-LED-3090-W-BK-L1-UNV | 6 | TYPE 5 | 9'-10" | | |
| | LUMARK | XTOR CROSSTOUR LED | XTOR6B-W-BZ-MS/DIM-L20-CBP | 1 | TYPE 4 | 12'-0" | | |

| CALCULATION SUMMARY | | | | | |
|--------------------------------------|---------|---------|--------|--|--|
| AREA | AVERAGE | MAX | MIN | | |
| PROPERTY ANALYSIS MEASURED @ 3 FT | 2.79 fc | 22.4 fc | 0.0 fc | | |

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LIGHTING NOTES

- LIGHT ANALYSIS CONDUCTED AT 3-FEET ABOVE FINISHED GRADE. 2. MOUNTING HEIGHT OF THE SITE LIGHT FIXTURES ARE MEASURED FROM FINISH ASPHALT GRADE.
- ALL PROPOSED FIXTURES ON TIMER TO OPERATE DURING NIGHT TIME HOURS, 30 MINUTES AFTER SUNSET AND 30 MINUTES BEFORE SUNRISE. CONTRACTOR TO INSTALL FIXTURE ON COPPER LIGHTING POLE
- RSS-6-A-XX-S-Y-N-XX. MOUNT HEIGHTS ARE BASED ON HEIGHT ABOVE FINISHED ASPHALT GRADE.

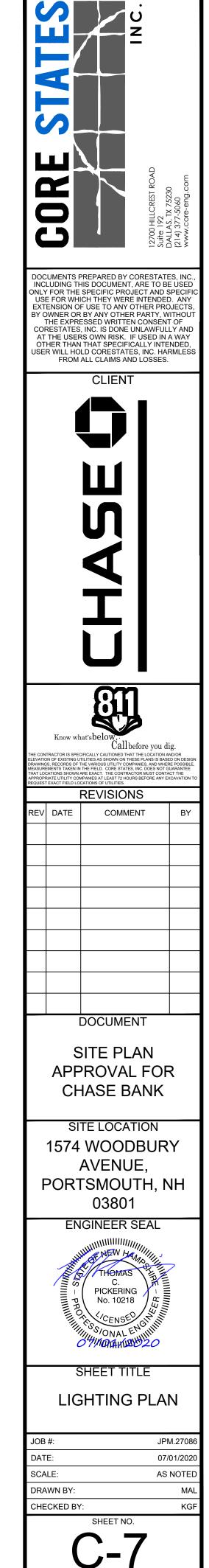
LIGHTING LEGEND

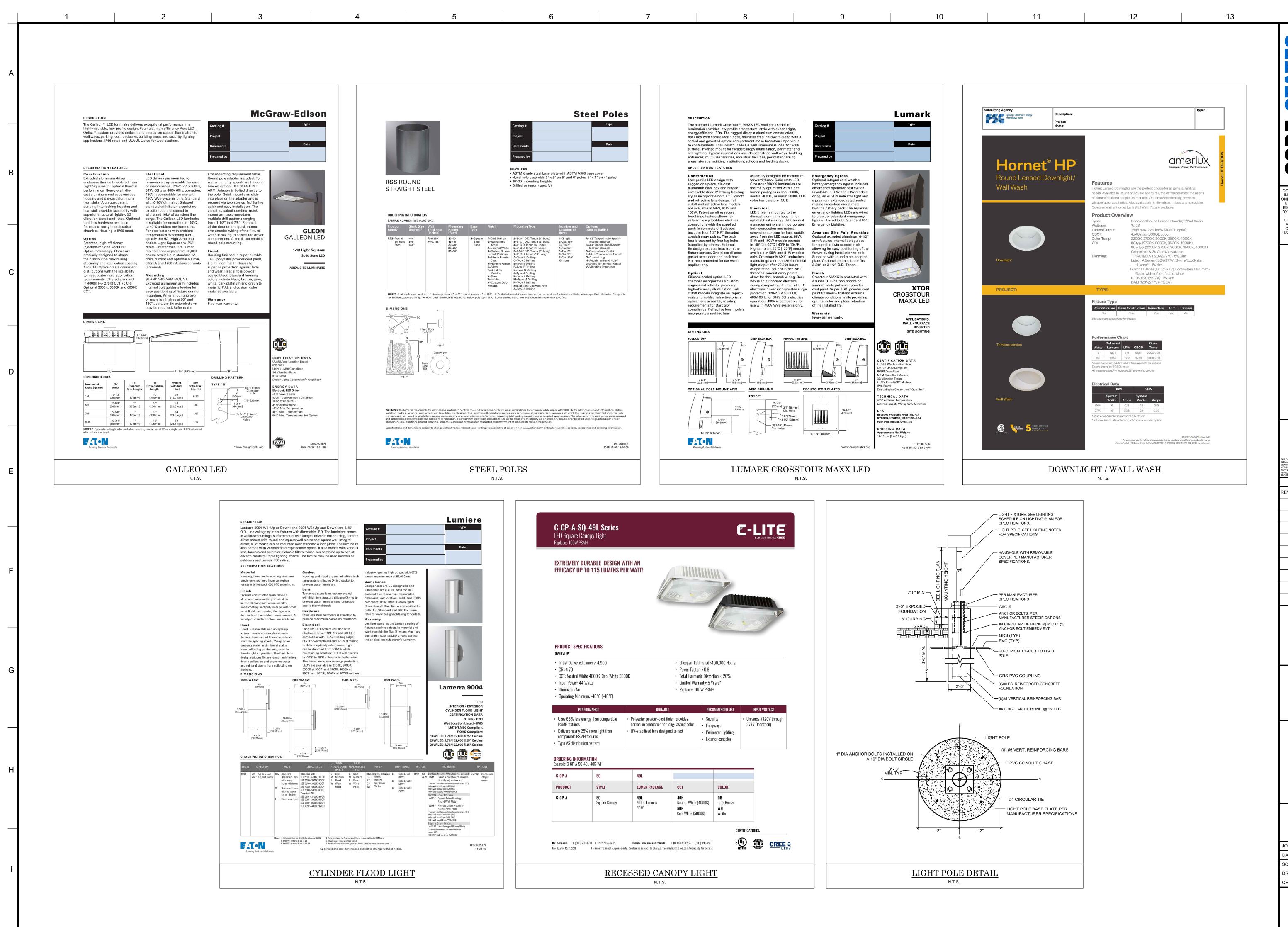
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PROPERTY BOUNDARY LINE ADJOINING PROPERTY LINE ROAD CENTERLINE G CURB SED CURB ANDLE CALCULATION POINT

SED SITE LIGHT FIXTURE

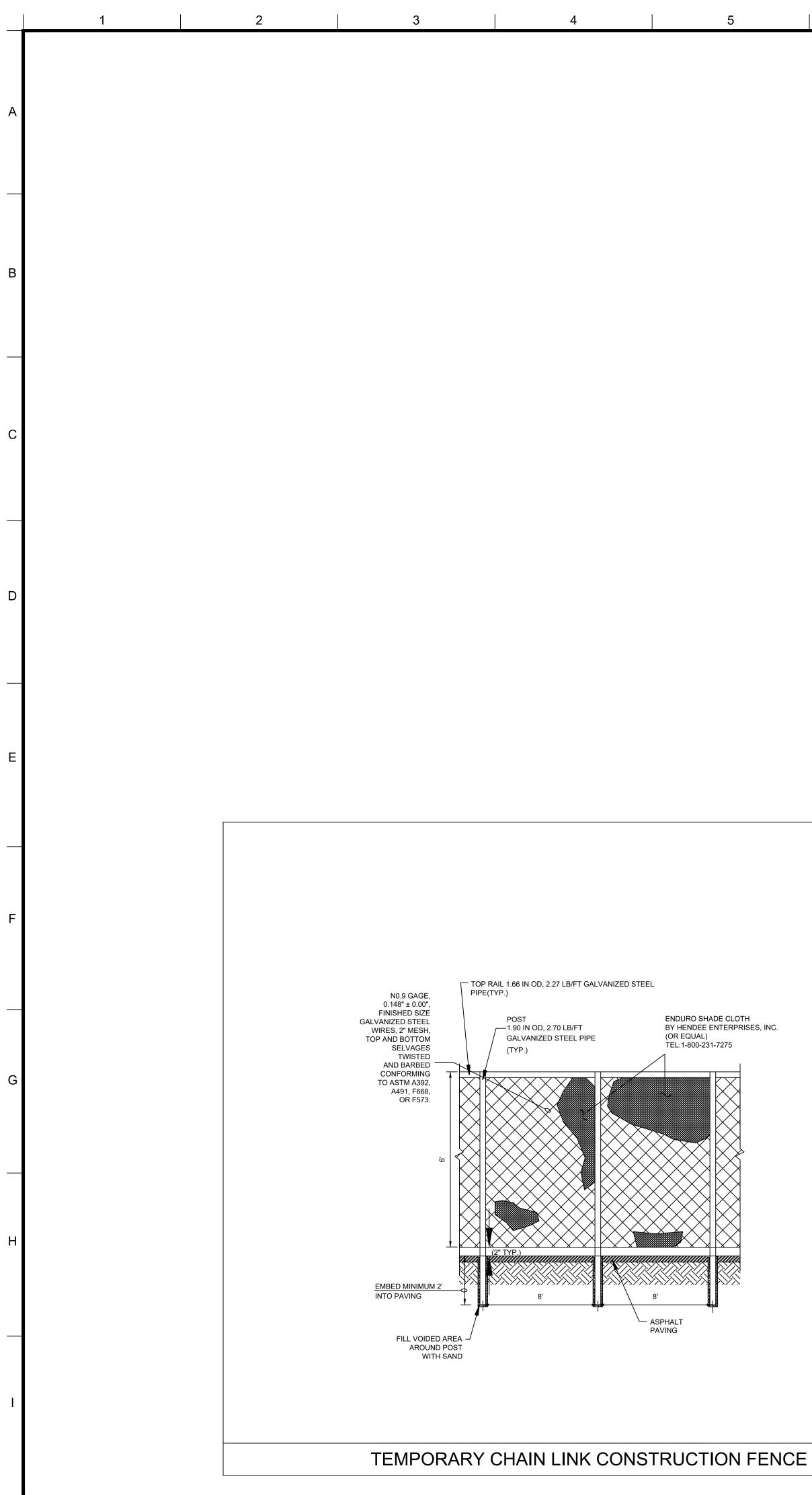
G LIGHT FIXTURES



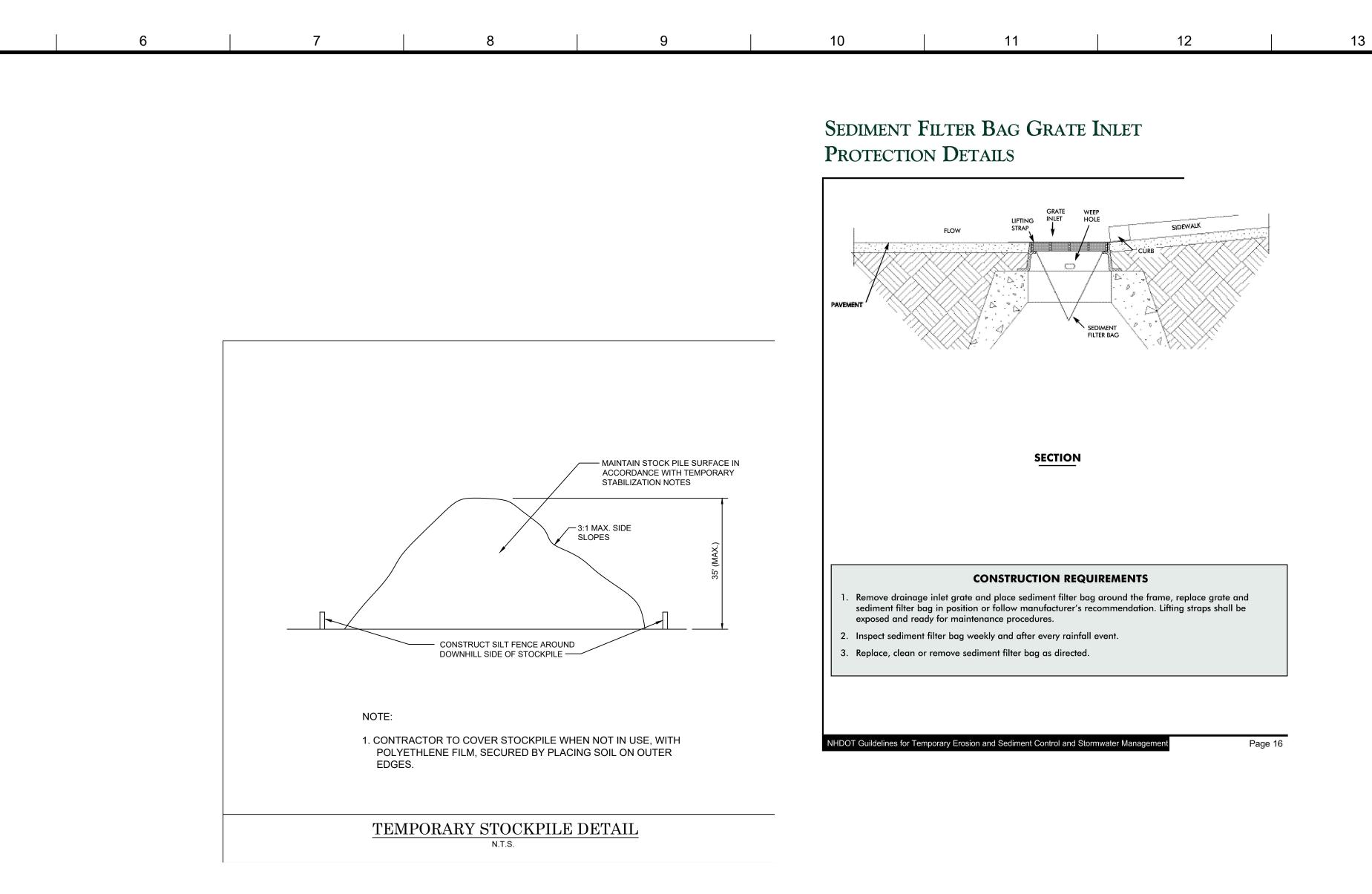


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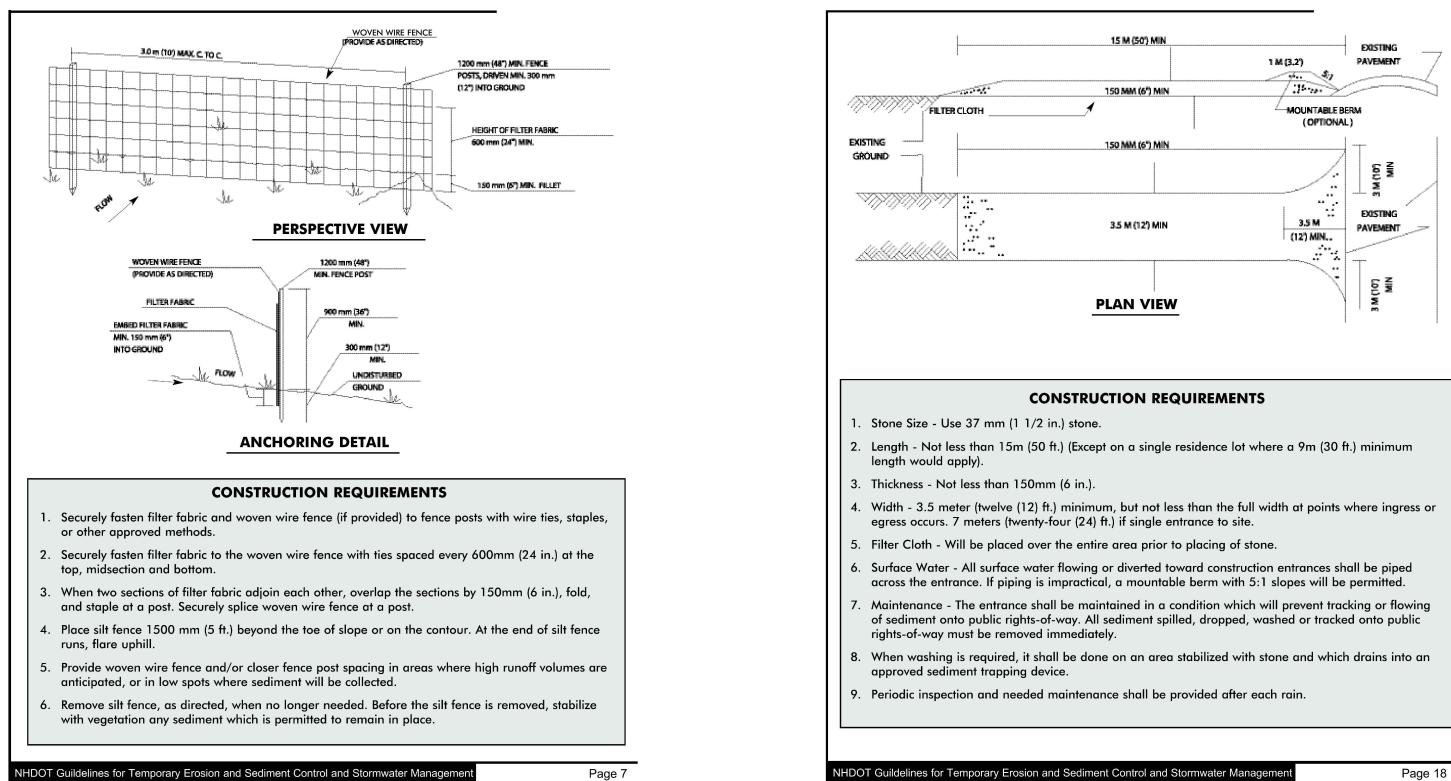
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| CORE ST | | 12700 HILLCREST ROAD Suite 192 DALLAS, TX 75230 (214) 377-5060 www.core-eng.com |
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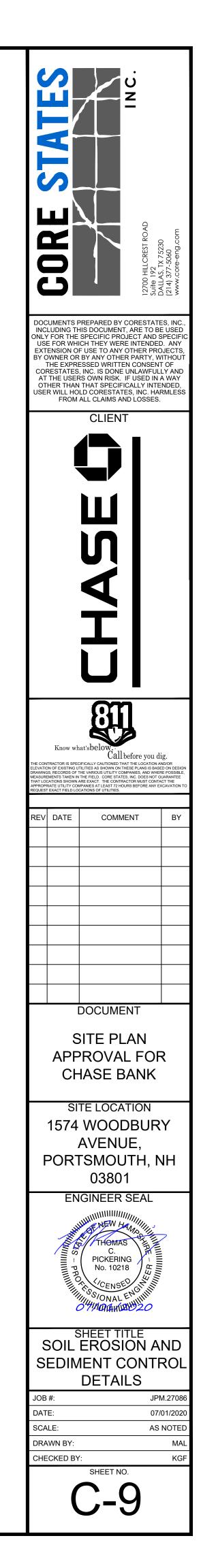
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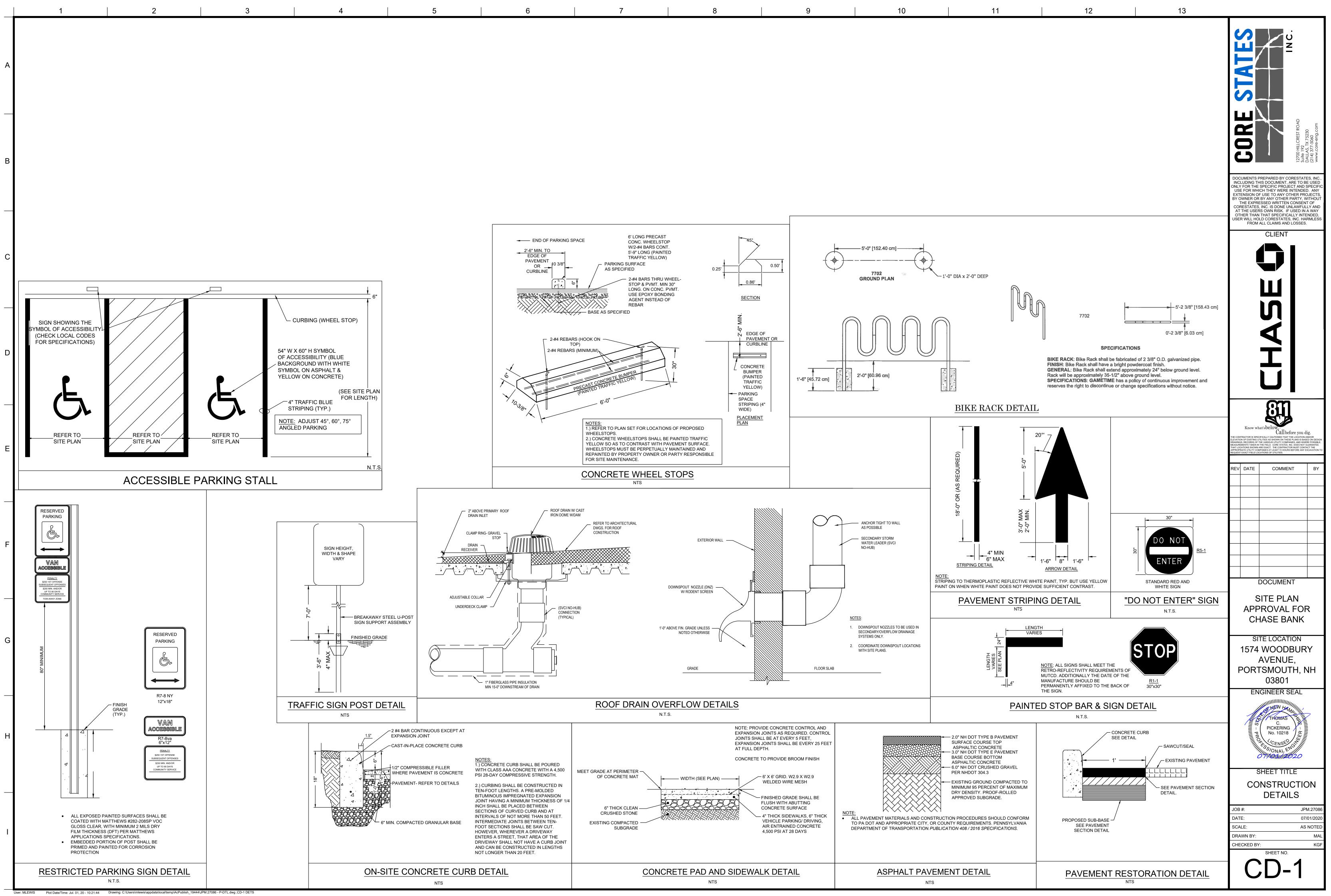


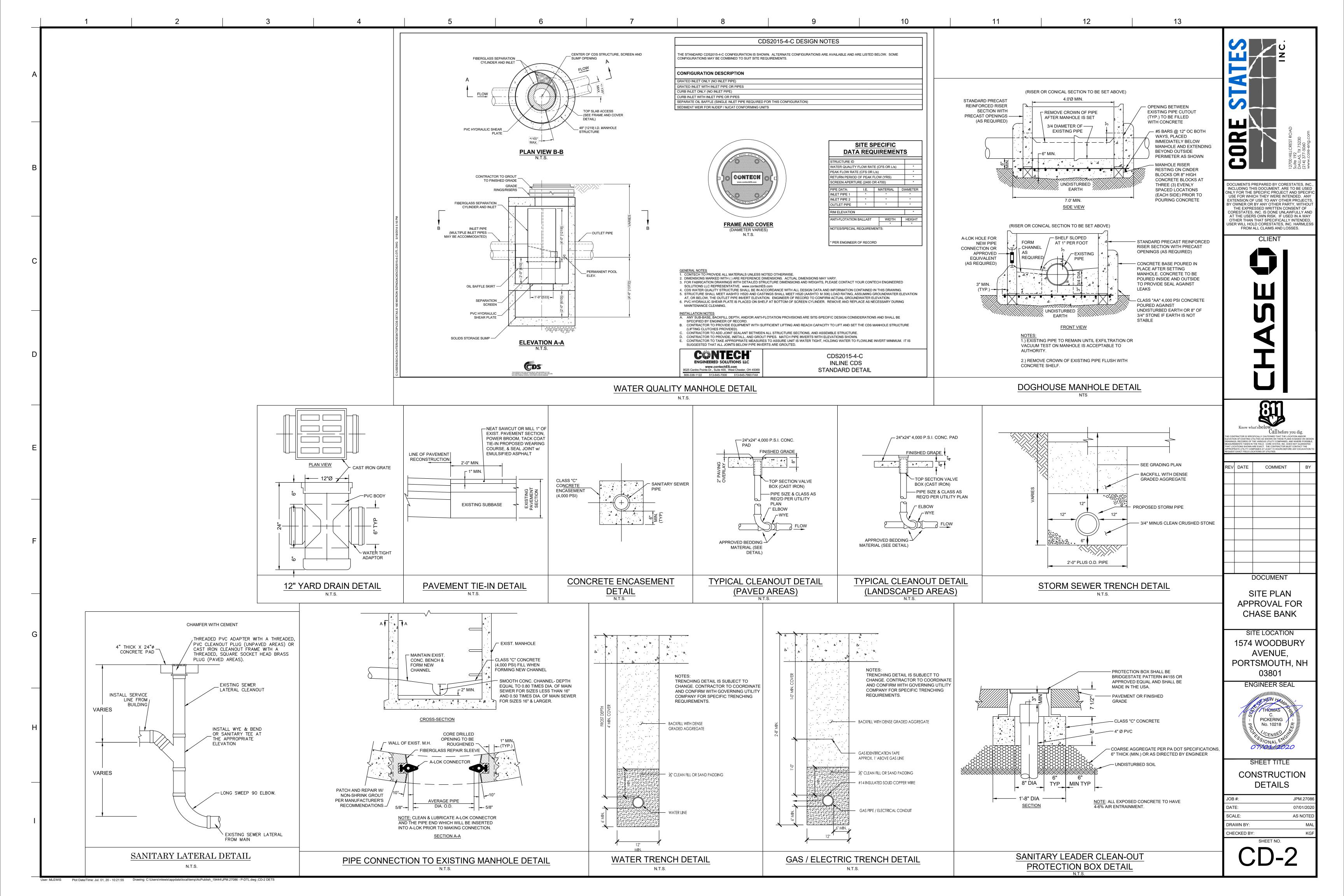
SILT FENCE PERIMETER BARRIER DETAILS

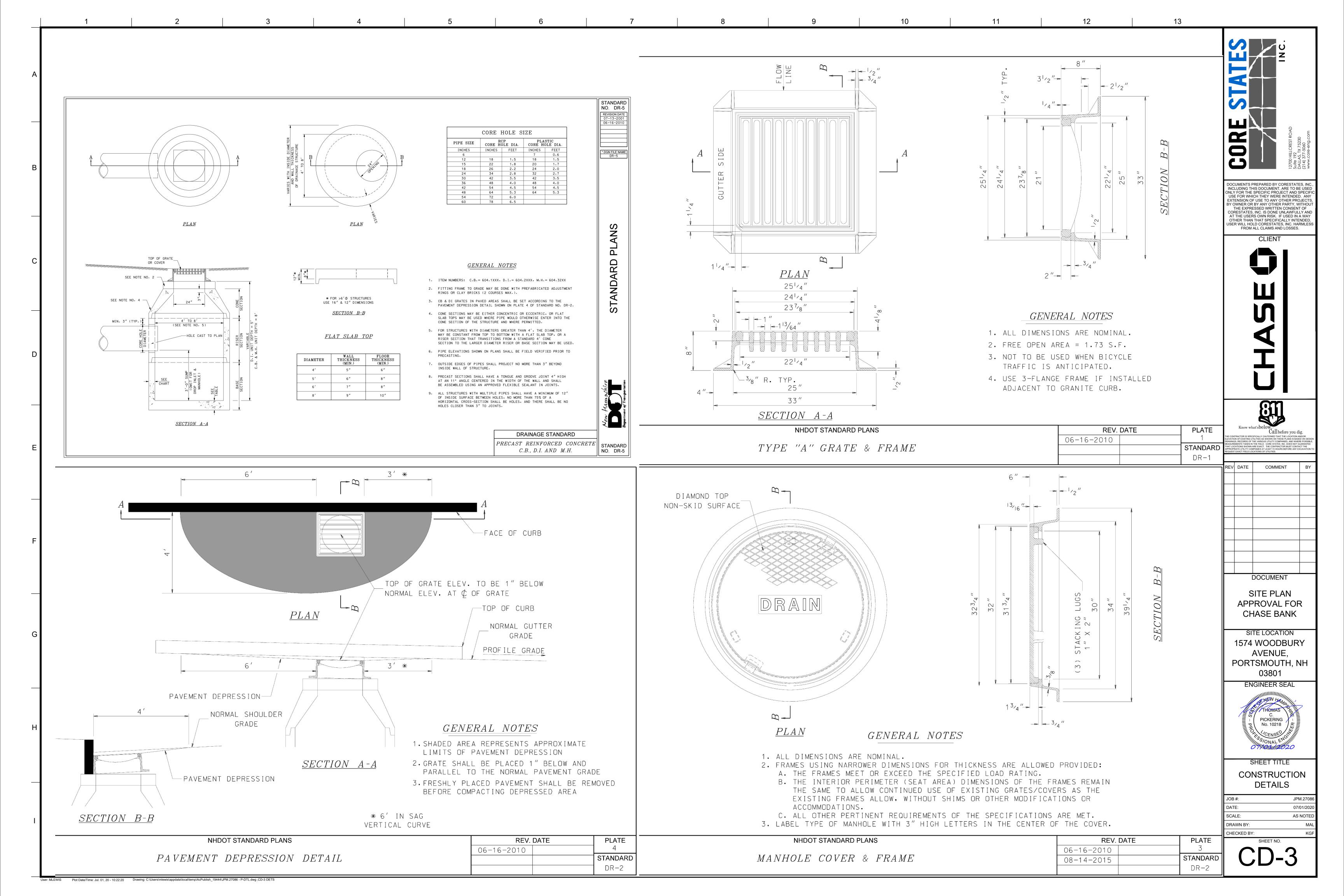


STABILIZED CONSTRUCTION ENTRANCE DETAILS

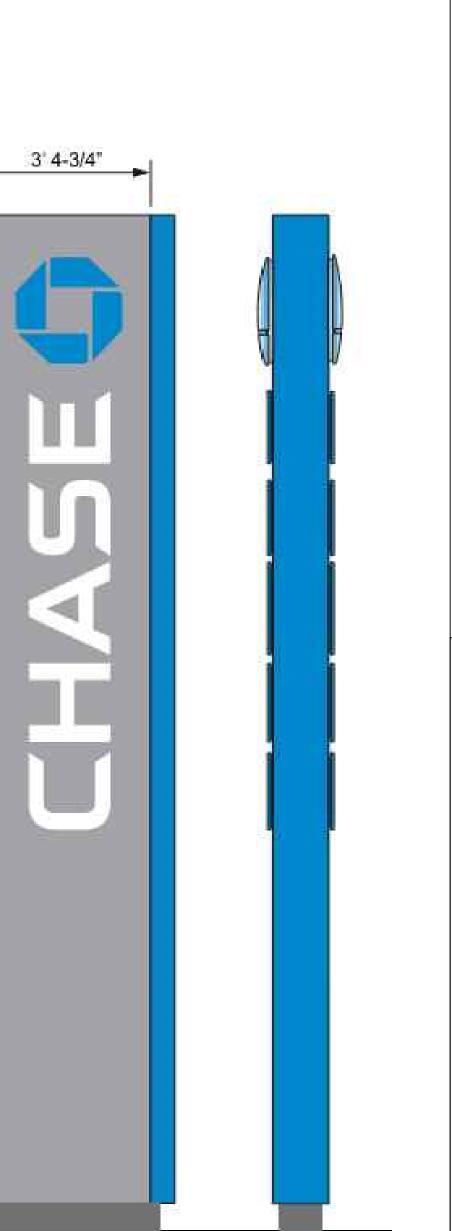


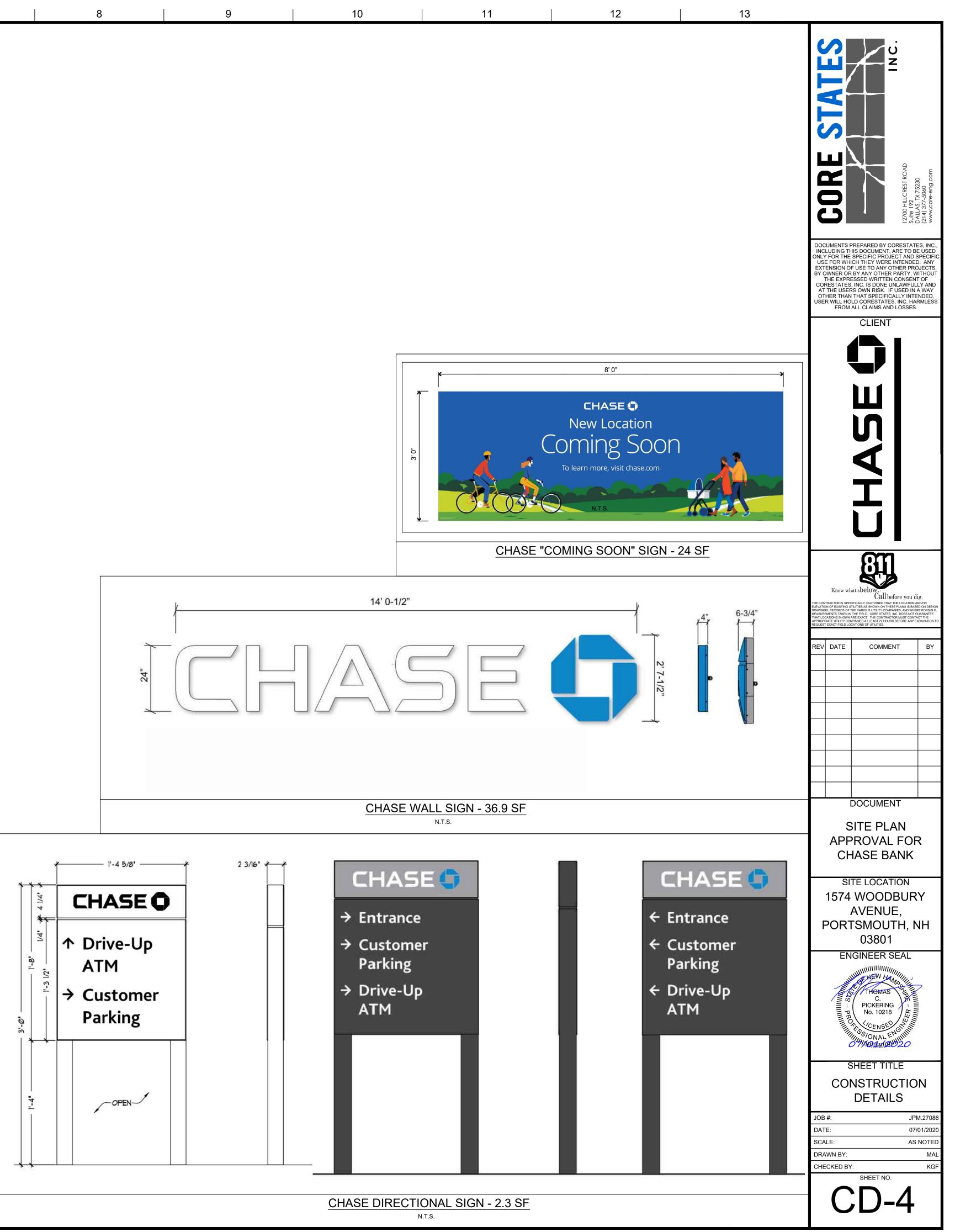


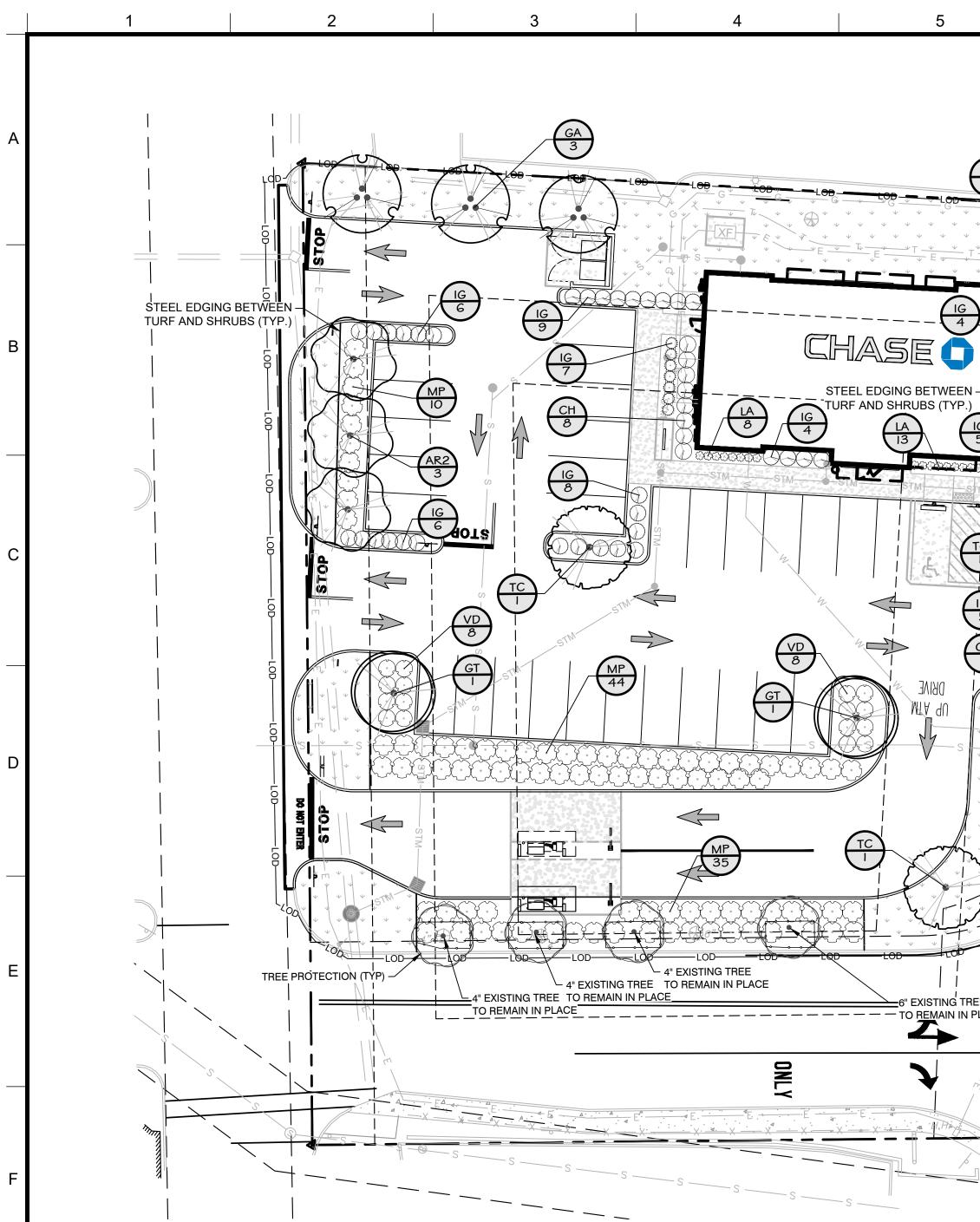




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ROOT BARRIERS

THE CONTRACTOR SHALL INSTALL ROOT BARRIERS NEAR ALL NEWLY-PLANTED TREES THAT ARE LOCATED WITHIN FIVE (5) FEET OF PAVING OR CURBS. ROOT BARRIERS SHALL BE "CENTURY" OR "DEEP-ROOT" 24" DEEP PANELS (OR EQUAL). BARRIERS SHALL BE LOCATED IMMEDIATELY ADJACENT TO HARDSCAPE. INSTALL PANELS PER MANUFACTURER'S RECOMMENDATIONS. UNDER NO CIRCUMSTANCES SHALL THE CONTRACTOR USE ROOT BARRIERS OF A TYPE THAT COMPLETELY ENCIRCLE THE ROOTBALL.

MULCHES

AFTER ALL PLANTING IS COMPLETE, CONTRACTOR SHALL INSTALL 3" THICK LAYER OF 1-1/2" SHREDDED WOOD MULCH, NATURAL (UNDYED), OVER LANDSCAPE FABRIC IN ALL PLANTING AREAS (EXCEPT FOR TURF AND SEEDED AREAS). CONTRACTOR SHALL SUBMIT SAMPLES OF ALL MULCHES TO LANDSCAPE ARCHITECT AND OWNER FOR APPROVAL PRIOR TO CONSTRUCTION. ABSOLUTELY NO EXPOSED GROUND SHALL BE LEFT SHOWING ANYWHERE ON THE PROJECT AFTER MULCH HAS BEEN INSTALLED (SUBJECT TO THE CONDITIONS AND REQUIREMENTS OF THE "GENERAL GRADING AND PLANTING NOTES" AND SPECIFICATIONS).

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| PLANT SCH | IEDU | LE | | | | |
|-------------------------|------|--|-----------|-------|----------------|------------|
| TREES | | BOTANICAL / COMMON NAME | CAL | CONT. | SIZE | <u>QTY</u> |
| \bigcirc | AR2 | Acer rubrum `Red Sunset` Red Sunset Maple | 2" Cal. | B&B | 8`-10` | 3 |
| | СС | Cercis canadensis Eastern Redbud Multi-trunk - 3-5 canes | 1.5" Cal. | B&B | 6`-8` | 3 |
| e e e | GA | Ginkgo biloba `Autumn Gold` TM Autumn Gold Maidenhair Tree | 2" Cal. | B&B | 8`-10` | 3 |
| ∂_{-} | GT | Gleditsia triacanthos inermis `Skycole` TM Skyline Thornless Honey Locust | 2" Cal. | B&B | 8`-10` | 2 |
| | TC | Tilia cordata Littleleaf Linden | 2" Cal. | B&B | 8`-10` | 5 |
| SHRUBS | | BOTANICAL / COMMON NAME | CONTAINER | SIZE | <u>SPACING</u> | <u>QTY</u> |
| \bigcirc | СН | Clethra alnifolia `Hummingbird` Summersweet | 5 gal. | | | 16 |
| Sinner Ce | HP | Hypericum prolificum Broombrush | 3 gal. | | | 18 |
| \bigcirc | IG | llex glabra `Shamrock` Inkberry | 5 gal. | | | 66 |
| $\langle + \rangle$ | LA | Lavandula angustifolia English Lavender | 3 gal. | | | 21 |
| $\langle \cdot \rangle$ | MP | Myrica pensylvanica `Bobzam` Bobee Northern Bayberry | 5 gal. | | | 89 |
| | VD | Viburnum dentatum `Arrowwood` Arrowwood Viburnum | 5 gal. | | | 16 |
| GROUND COVERS | | BOTANICAL / COMMON NAME | CONT | SIZE | SPACING | <u>QTY</u> |
| | TURF | Poa pratensis Kentucky Bluegrass | sod | | | 7,852 sf |

GENERAL GRADING AND PLANTING NO

| 1. | BY SUBMITTING A PROPOSAL FOR THE LANDSCAPE PLANTING SCO |
|----|---|
| 2. | HAS READ, AND WILL COMPLY WITH, THE ASSOCIATED NOTES, SPE THE GENERAL CONTRACTOR IS RESPONSIBLE FOR REMOVING ALL |
| | REMAIN). |
| 3. | IN THE CONTEXT OF THESE PLANS, NOTES, AND SPECIFICATIONS, |
| | THE SOIL SURFACE (NOT TOP OF MULCH) AS INDICATED ON THE G |
| | a. BEFORE STARTING WORK, THE LANDSCAPE CONTRACTOR S |
| | LANDSCAPE AREAS ARE WITHIN +/-0.1' OF FINISH GRADE. SE |
| | ON TURF AREA AND PLANTING BED PREPARATION. |
| | b. CONSTRUCT AND MAINTAIN FINISH GRADES AS SHOWN ON G |
| | SLOPES AS RECOMMENDED BY THE GEOTECHNICAL REPORT |
| | DRAINAGE AWAY FROM STRUCTURES AT THE MINIMUM SLOP |
| | PLANS, AND AREAS OF POTENTIAL PONDING SHALL BE REGR |
| | AND ELIMINATE PONDING POTENTIAL. c. THE LANDSCAPE CONTRACTOR SHALL DETERMINE WHETHEI |
| | TAKING INTO ACCOUNT THE ROUGH GRADE PROVIDED, THE |
| | ON A SOIL TEST, PER SPECIFICATIONS), AND THE FINISH GRA |
| | d. ENSURE THAT THE FINISH GRADE IN SHRUB AREAS IMMEDIA |
| | SURFACES, AFTER INSTALLING SOIL AMENDMENTS, IS 3" BEL |
| | ALLOW FOR PROPER MULCH DEPTH. TAPER THE SOIL SURF. |
| | GRADING PLANS, AT APPROXIMATELY 18" AWAY FROM THE V |
| | e. ENSURE THAT THE FINISH GRADE IN TURF AREAS IMMEDIATE |
| | SURFACES, AFTER INSTALLING SOIL AMENDMENTS, IS 1" BEI |
| | SOIL SURFACE TO MEET FINISH GRADE, AS SPECIFIED ON TH |
| | FROM THE WALKS. |
| | f. SHOULD ANY CONFLICTS AND/OR DISCREPANCIES ARISE BE |
| | THESE NOTES AND PLANS, AND ACTUAL CONDITIONS, THE C |
| | TO THE ATTENTION OF THE LANDSCAPE ARCHITECT, GENER |
| 4. | ALL PLANT LOCATIONS ARE DIAGRAMMATIC. ACTUAL LOCATIONS |
| | OR DESIGNER PRIOR TO PLANTING. THE LANDSCAPE CONTRACTO |
| | PERMITTING AUTHORITY ARE MET (I.E., MINIMUM PLANT QUANTITIE |
| | ETC.). a. THE LANDSCAPE CONTRACTOR IS RESPONSIBLE FOR DETER |
| | ON LEGENDS AND CALLOUTS ARE FOR GENERAL INFORMATI |
| | THE PLAN AND THE PLANT LEGEND, THE PLANT QUANTITY AS |
| | CALLOUT (FOR GROUNDCOVER PATTERNS) SHALL TAKE PR |
| | b. NO SUBSTITUTIONS OF PLANT MATERIALS SHALL BE ALLOW |
| | LANDSCAPE ARCHITECT. IF SOME OF THE PLANTS ARE NOT |
| | NOTIFY THE LANDSCAPE ARCHITECT IN WRITING (VIA PROPE |
| | c. THE CONTRACTOR SHALL, AT A MINIMUM, PROVIDE REPRESE |
| | PROJECT. THE CONTRACTOR SHALL ALLOW THE LANDSCAF |
| | REPRESENTATIVE TO INSPECT, AND APPROVE OR REJECT, A |
| | SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS FOR SUB |
| 5. | THE CONTRACTOR SHALL MAINTAIN THE LANDSCAPE IN A HEALTH |
| | OWNER. REFER TO SPECIFICATIONS FOR CONDITIONS OF ACCEPT |
| | AND FOR FINAL ACCEPTANCE AT THE END OF THE MAINTENANCE |

6. SEE SPECIFICATIONS AND DETAILS FOR FURTHER REQUIREMENTS.

| 01 | ΓE | S |
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OPE OF WORK, THE CONTRACTOR CONFIRMS THAT HE ECIFICATIONS, AND DETAILS WITH THIS PROJECT. L EXISTING VEGETATION (EXCEPT WHERE NOTED TO 12

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6, "FINISH GRADE" REFERS TO THE FINAL ELEVATION OF GRADING PLANS. SHALL VERIFY THAT THE ROUGH GRADES OF ALL EE SPECIFICATIONS FOR MORE DETAILED INSTRUCTION

GRADING PLANS, AND CONSTRUCT AND MAINTAIN RT. ALL LANDSCAPE AREAS SHALL HAVE POSITIVE OPE SPECIFIED IN THE REPORT AND ON THE GRADING RADED TO BLEND IN WITH THE SURROUNDING GRADES

R OR NOT THE EXPORT OF ANY SOIL WILL BE NEEDED, AMOUNT OF SOIL AMENDMENTS TO BE ADDED (**BASED** ADES TO BE ESTABLISHED. ATELY ADJACENT TO WALKS AND OTHER WALKING LOW THE ADJACENT FINISH SURFACE, IN ORDER TO FACE TO MEET FINISH GRADE, AS SPECIFIED ON THE

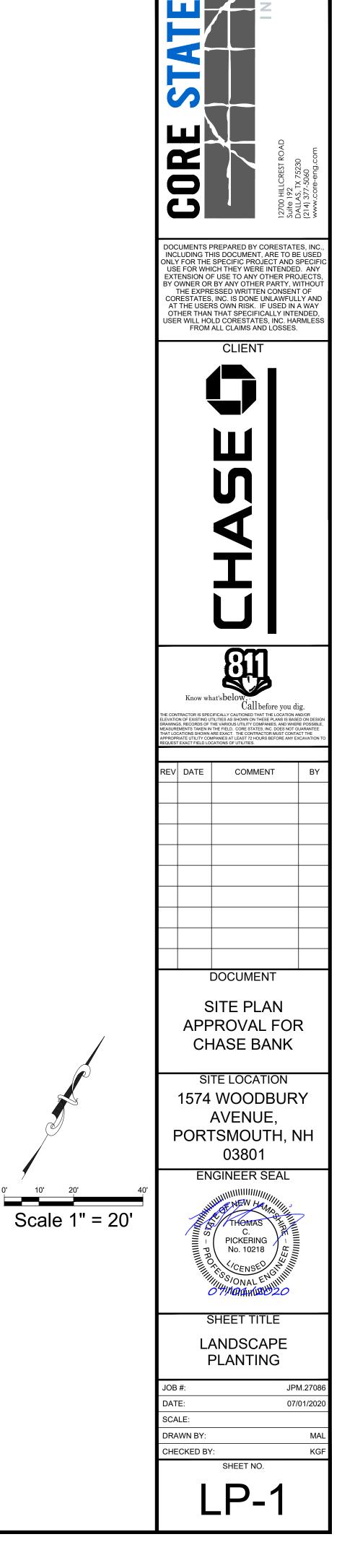
WALKS. FELY ADJACENT TO WALKS AND OTHER WALKING ELOW THE FINISH SURFACE OF THE WALKS. TAPER THE THE GRADING PLANS, AT APPROXIMATELY 18" AWAY

TWEEN THE GRADING PLANS, GEOTECHNICAL REPORT, CONTRACTOR SHALL IMMEDIATELY BRING SUCH ITEMS RAL CONTRACTOR, AND OWNER. SHALL BE VERIFIED WITH THE LANDSCAPE ARCHITECT OR SHALL ENSURE THAT ALL REQUIREMENTS OF THE

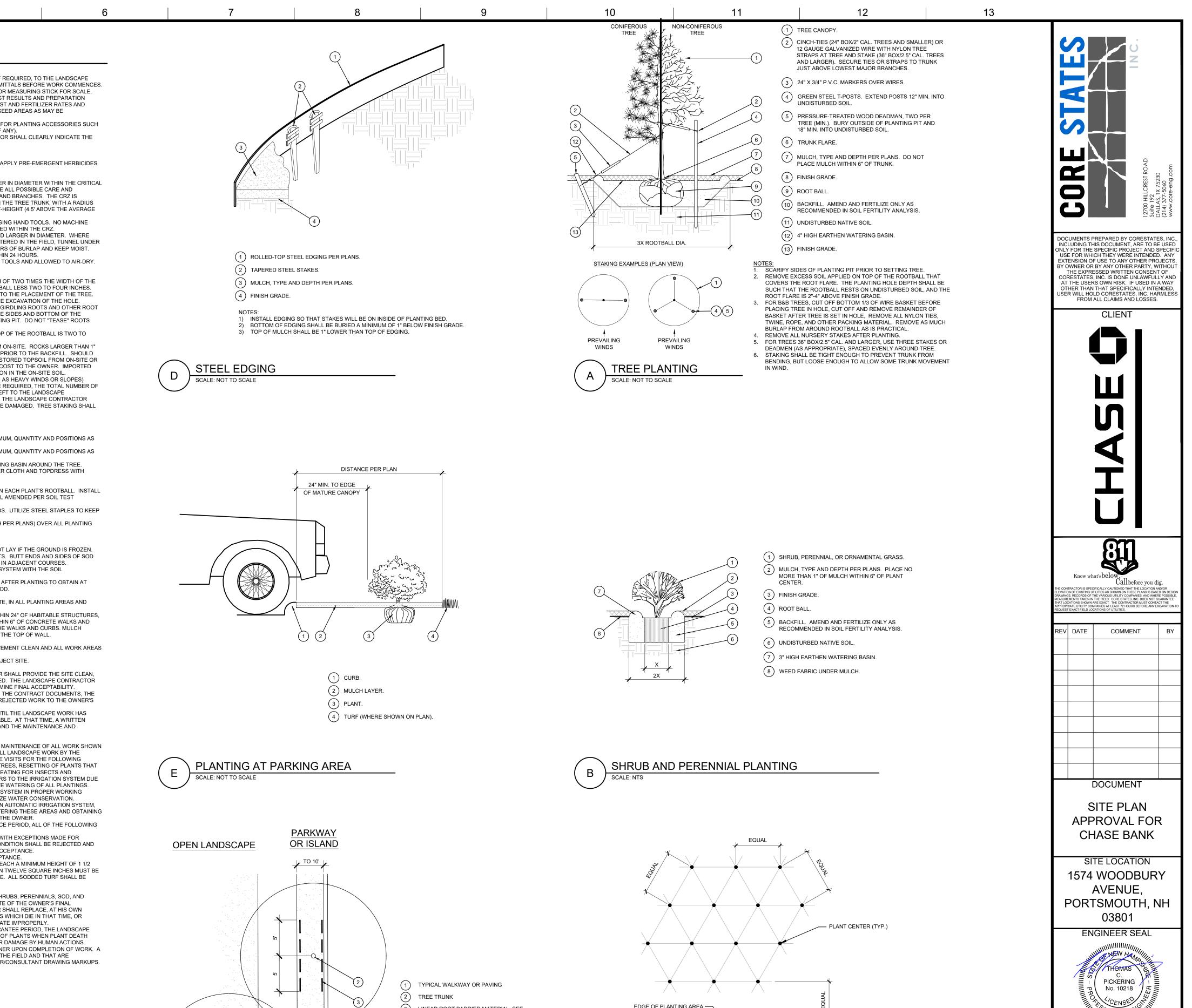
IES, PLANTING METHODS, TREE PROTECTION METHODS, RMINING PLANT QUANTITIES; PLANT QUANTITIES SHOWN TION ONLY. IN THE EVENT OF A DISCREPANCY BETWEEN AS SHOWN ON THE PLAN (FOR INDIVIDUAL SYMBOLS) OR RECEDENCE. WED WITHOUT THE WRITTEN PERMISSION

ALCEDENCE. **DWED WITHOUT THE WRITTEN PERMISSION OF THE** DT AVAILABLE, THE LANDSCAPE CONTRACTOR SHALL PER CHANNELS). SENTATIVE PHOTOS OF ALL PLANTS PROPOSED FOR THE APE ARCHITECT AND THE OWNER/OWNER'S , ALL PLANTS DELIVERED TO THE JOBSITE. REFER TO DURITAL O

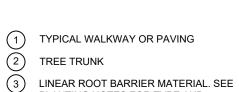
BMITTALS. HY CONDITION FOR 90 DAYS AFTER ACCEPTANCE BY THE TANCE FOR THE START OF THE MAINTENANCE PERIOD, PERIOD. S.



| | TING SPECIFICATIONS | | | | | |
|-----------------|--|-----------|--------------------------------|--|--|-----------------------|
| | IFICATIONS OF LANDSCAPE CONTRACTOR | В. | ARCHITE | TRACTOR SHALL PROVI CT, AND RECEIVE APPR | OVAL IN WRITIN | NG FOR S |
| | ALL LANDSCAPE WORK SHOWN ON THESE PLANS SHALL BE PERFORMED BY A SINGLE FIRM SPECIALIZING IN LANDSCAPE PLANTING. A LIST OF SUCCESSFULLY COMPLETED PROJECTS OF THIS TYPE, SIZE AND NATURE MAY BE | | PHOTOS | ALS SHALL INCLUDE PH OR SAMPLES OF ANY RI ENDATIONS FROM THE | EQUIRED MULC | CHES, AND |
| | REQUESTED BY THE OWNER FOR FURTHER QUALIFICATION MEASURES. THE LANDSCAPE CONTRACTOR SHALL HOLD A VALID NURSERY AND FLORAL CERTIFICATE IS | | TYPES, AI APPROPF | ND OTHER AMENDMENT RIATE). | TS FOR TREE/S | SHRUB, TL |
| | THE TEXAS DEPARTMENT OF AGRICULTURE, AS WELL AS OPERATE UNDER A COMMERCIAL P APPLICATOR LICENSE ISSUED BY EITHER THE TEXAS DEPARTMENT OF AGRICULTURE OR THI STRUCTURAL PEST CONTROL BOARD. | | AS TREE | ALS SHALL ALSO INCLUI STAKES AND TIES, EDG IULTIPLE ITEMS ARE SH | SING, AND LAND | DSCAPE F |
| | E OF WORK WORK COVERED BY THESE SECTIONS INCLUDES THE FURNISHING AND PAYMENT OF ALL MA | | GENERAL PLAN | | | |
| 1. | LABOR, SERVICES, EQUIPMENT, LICENSES, TAXES AND ANY OTHER ITEMS THAT ARE NECESS THE EXECUTION, INSTALLATION AND COMPLETION OF ALL WORK, SPECIFIED HEREIN AND / O | ARY FOR | 2. EXCEPT I | ALL NURSERY TAGS AN N AREAS TO BE PLANTE ANUFACTURER'S RECC | ED WITH ORNAM | MENTAL (|
| 2. | ON THE LANDSCAPE PLANS, NOTES, AND DETAILS. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE LAWS, CODES AND REGULATIONS REQUIRED BY AUTHORITIES HAVING JURISDICTION OVER SUCH WORK. INCLU | | a. CON | NG NEAR EXISTING TRE NTRACTOR SHALL NOT I DT ZONE (CRZ) OF EXIS | DISTURB ROOT | |
| | INSPECTIONS AND PERMITS REQUIRED BY FEDERAL, STATE AND LOCAL AUTHORITIES IN SUP TRANSPORTATION AND INSTALLATION OF MATERIALS. | | PRE | ECAUTIONS TO AVOID IN INED AS A CIRCULAR A | JURY TO TREE | E ROOTS, |
| 3. | THE LANDSCAPE CONTRACTOR SHALL VERIFY THE LOCATION OF ALL UNDERGROUND UTILIT' (WATER, SEWER, ELECTRICAL, TELEPHONE, GAS, CABLE, TELEVISION, ETC.) PRIOR TO THE S' ANY WORK. | | GRA | JAL TO 1' FOR EVERY 1" ADE AT THE TRUNK). EXCAVATION WITHIN T | | |
| | | | EXC c. ALT | AVATION OR TRENCHIN | NG OF ANY KINI E TO AVOID TRE | ID SHALL EE ROOT |
| A. ALL I | IANUFACTURED PRODUCTS SHALL BE NEW. | | SUC | E ROOTS 1-1/2" AND LA CH ROOTS. WRAP EXPO DSE ALL TRENCHES WIT | OSED ROOTS W | VITH SEVI |
| | AINER AND BALLED-AND-BURLAPPED PLANTS: FURNISH NURSERY-GROWN PLANTS COMPLYING WITH ANSI Z60.1-2014. PROVIDE WELL-SHAF BRANCHED. HEALTHY. VIGOROUS STOCK FREE OF DISEASE. INSECTS. EGGS. LARVAE. AND D | | | SEVERED ROOTS SHAI | | |
| | SUCH AS KNOTS, SUN SCALD, INJURIES, ABRASIONS, AND DISFIGUREMENT. ALL PLANTS WIT SPECIES SHALL HAVE SIMILAR SIZE, AND SHALL BE OF A FORM TYPICAL FOR THE SPECIES. A | HIN A | 1. TREE PLA | , NTING HOLES SHALL B L, AND TO A DEPTH EQ | | |
| C | SHALL BE OBTAINED FROM SOURCES WITHIN 200 MILES OF THE PROJECT SITE, AND WITH SIN CLIMACTIC CONDITIONS. ROOT SYSTEMS SHALL BE HEALTHY, DENSELY BRANCHED ROOT SYSTEMS, NON-POT-BOUND | | REMOVE | THE SIDES AND BOTTO ANY GLAZING THAT MAY | Y HAVE BEEN C | CAUSED I |
| | FROM ENCIRCLING AND/OR GIRDLING ROOTS, AND FREE FROM ANY OTHER ROOT DEFECTS (J-SHAPED ROOTS). | SUCH AS | DEFECTS | TAINER AND BOX TREES , THE CONTRACTOR SH L OF ALL TREES JUST E | IALL SHAVE A 1 | 1" LAYER |
| | TREES MAY BE PLANTED FROM CONTAINERS OR BALLED-AND-BURLAPPED (B&B), UNLESS SP ON THE PLANTING LEGEND. BARE-ROOT TREES ARE NOT ACCEPTABLE. | ECIFIED | 4. INSTALL 1 | M THE ROOTBALL. THE TREE ON UNDISTUR | | |
| 4. | ANY PLANT DEEMED UNACCEPTABLE BY THE LANDSCAPE ARCHITECT OR OWNER SHALL BE IMMEDIATELY REMOVED FROM THE SITE AND SHALL BE REPLACED WITH AN ACCEPTBLE PLA TYPE AND SIZE AT THE CONTRACTOR'S OWN EXPENSE. ANY PLANTS APPEARING TO BE UNH | EALTHY, | 5. BACKFILL DIA. AND | HES ABOVE THE SURRO THE TREE HOLE UTILIZ ALL OTHER DEBRIS SHA | ZING THE EXIST ALL BE REMOVE | TING TOP ED FROM |
| 5 | EVEN IF DETERMINED TO STILL BE ALIVE, SHALL NOT BE ACCEPTED. THE LANDSCAPE ARCHI OWNER SHALL BE THE SOLE JUDGES AS TO THE ACCEPTABILITY OF PLANT MATERIAL. ALL TREES SHALL BE STANDARD IN FORM, UNLESS OTHERWISE SPECIFIED. TREES WITH CEN | | IMPORT A | IAL SOIL BE REQUIRED DDITIONAL TOPSOIL FR SHALL BE OF SIMILAR T | ROM OFF-SITE A | AT NO AE |
| | LEADERS WILL NOT BE ACCEPTED IF LEADER IS DAMAGED OR REMOVED. PRUNE ALL DAMAG AFTER PLANTING. | ED TWIGS | 6. TREES SH REQUIRE | HALL NOT BE STAKED U STAKES TO KEEP TREE | INLESS LOCAL (ES UPRIGHT. SI | CONDITIO |
| 6. | CALIPER MEASUREMENTS FOR STANDARD (SINGLE TRUNK) TREES SHALL BE AS FOLLOWS: \$ ABOVE THE ROOT FLARE FOR TREES UP TO AND INCLUDING FOUR INCHES IN CALIPER, AND 1 INCHES ABOVE THE ROOT FLARE FOR TREES EXCEEDING FOUR INCHES IN CALIPER. | | CONTRAC | KES (BEYOND THE MIN CTOR'S DISCRETION. SH RAIGHTEN THE TREE. C | HOULD ANY TR | REES FAL |
| 7. | MULTI-TRUNK TREES SHALL BE MEASURED BY THEIR OVERALL HEIGHT, MEASURED FROM TH THE ROOT BALL. WHERE CALIPER MEASUREMENTS ARE USED, THE CALIPER SHALL BE CALC | | ADHERE ⁻ a. 1"-2 | TO THE FOLLOWING GU " TREES | JIDELINES: TWO STAKES | S PER TF |
| 8. | AS ONE-HALF OF THE SUM OF THE CALIPER OF THE THREE LARGEST TRUNKS. ANY TREE OR SHRUB SHOWN TO HAVE EXCESS SOIL PLACED ON TOP OF THE ROOT BALL, SO THE ROOT FLARE HAS BEEN COMPLETELY COVERED, SHALL BE REJECTED. | THAT | c. TRE | 2"-4" TREES ES OVER 4" CALIPER _TI-TRUNK TREES | THREE STAK GUY AS NEE THREE STAK | DED |
| HEAL | PROVIDE WELL-ROOTED SOD OF THE VARIETY NOTED ON THE PLANS. SOD SHALL BE CUT FF THY, MATURE TURF WITH SOIL THICKNESS OF 3/4" TO 1". EACH PALLET OF SOD SHALL BE | ROM | e. MUL | DED TO STABILIZE THE TI-TRUNK TREES | TREE THREE STAK | |
| D. TOPS | MPANIED BY A CERTIFICATE FROM SUPPLIER STATING THE COMPOSITION OF THE SOD. OIL: SANDY TO CLAY LOAM TOPSOIL, FREE OF STONES LARGER THAN ½ INCH, FOREIGN MAT TS, ROOTS, AND SEEDS. | ſER, | 7. UPON CO | EDED TO STABILIZE THE MPLETION OF PLANTING HE INTERIOR OF THE TF | G, CONSTRUCT | |
| E. COM MOIS | POST: WELL-COMPOSTED, STABLE, AND WEED-FREE ORGANIC MATTER, pH RANGE OF 5.5 TO FURE CONTENT 35 TO 55 PERCENT BY WEIGHT; 100 PERCENT PASSING THROUGH 3/4-INCH SIE | VE; D. | MULCH (T SHRUB, PEREN | YPE AND DEPTH PER P NIAL, AND GROUNDCO\ | VER PLANTING | |
| | BLE SALT CONTENT OF 5 TO 10 DECISIEMENS/M; NOT EXCEEDING 0.5 PERCENT INERT CONTA REE OF SUBSTANCES TOXIC TO PLANTINGS. NO MANURE OR ANIMAL-BASED PRODUCTS SH/ | | THE PLAN | PLANTING HOLES TWICE IT IN THE HOLE. BACKF ENDATIONS. | | |
| F. FERT NUTF | LIZER: GRANULAR FERTILIZER CONSISTING OF NITROGEN, PHOSPHORUS, POTASSIUM, AND (IENTS IN PROPORTIONS, AMOUNTS, AND RELEASE RATES RECOMMENDED IN A SOIL REPORT | | 2. INSTALL T THE WEE | THE WEED BARRIER CLO D BARRIER CLOTH IN PI | LACE. | |
| G. MULO | IFIED SOIL-TESTING AGENCY (SEE BELOW). H: SIZE AND TYPE AS INDICATED ON PLANS, FREE FROM DELETERIOUS MATERIALS AND SUIT)RESSING OF TREES AND SHRUBS. | | | ANTING IS COMPLETE, I VERING THE ENTIRE PL | | • |
| 1. | STAKING AND GUYING STAKES: 6' LONG GREEN METAL T-POSTS. GUY AND TIE WIRE: ASTM A 641, CLASS 1, GALVANIZED-STEEL WIRE, 2-STRAND, TWISTED, 0.1 | | 2. LAY SOD | ETY TO BE AS SPECIFIE WITHIN 24 HOURS FROM | M THE TIME OF | STRIPPI |
| | DIAMETER. STRAP CHAFING GUARD: REINFORCED NYLON OR CANVAS AT LEAST 1-1/2 INCH WIDE, WITH | | STRIPS - I | SOD TO FORM A SOLID I DO NOT OVERLAP. STA E SOD TO ENSURE GOO | GGER STRIPS | TO OFFS |
| L. STEE | GROMMETS TO PROTECT TREE TRUNKS FROM DAMAGE. LEDGING: PROFESSIONAL STEEL EDGING, 14 GAUGE THICK X 4 INCHES WIDE, FACTORY PAIN N. ACCEPTABLE MANUFACTURERS INCLUDE COL-MET OR APPROVED EQUAL. | TED DARK | UNDERNE 5. WATER T | EATH. HE SOD THOROUGHLY V | WITH A FINE SF | PRAY IMI |
| M. PRE- FOR | MERGENT HERBICIDES: ANY GRANULAR, NON-STAINING PRE-EMERGENT HERBICIDE THAT IS THE SPECIFIC ORNAMENTALS OR TURF ON WHICH IT WILL BE UTILIZED. PRE-EMERGENT HERE | | MULCH 1. INSTALL M | K INCHES OF PENETRAT | | |
| SHAL | L BE APPLIED PER THE MANUFACTURER'S LABELED RATES. | | TREE RIN 2. DO NOT II | | 6" OF TREE RO | OOT FLAF |
| METHODS | | | CURBS SI COVER W | HALL NOT PROTRUDE A THIN 12" OF WALLS SH | BOVE THE FINI | ISH SURF |
| | PREPARATION BEFORE STARTING WORK, THE LANDSCAPE CONTRACTOR SHALL VERIFY THAT THE GRADE C LANDSCAPE AREAS ARE WITHIN +/-0.1' OF FINISH GRADE. THE CONTRACTOR SHALL NOTIFY 1 | FALL | IN A NEAT | ANDSCAPE PREPARATI | l. | - , |
| 2. | OWNER IMMEDIATELY SHOULD ANY DISCREPANCIES EXIST. SOIL TESTING: a. AFTER FINISH GRADES HAVE BEEN ESTABLISHED, CONTRACTOR SHALL HAVE SOIL SAM | H. | 2. DISPOSEI INSPECTION AN | D LEGALLY OF ALL EXC ID ACCEPTANCE | AVATED MATER | |
| | FROM THE PROJECT'S LANDSCAPE AREAS TESTED BY AN ESTABLISHED SOIL TESTING LABORATORY. EACH SAMPLE SUBMITTED TO THE LAB SHALL CONTAIN NO LESS THAN (| DNE | FREE OF SHALL TH | MPLETION OF THE WOF DEBRIS AND TRASH, AN IEN REQUEST AN INSPE | ND SUITABLE FO | OR USE A |
| | QUART OF SOIL, TAKEN FROM BETWEEN THE SOIL SURFACE AND 6" DEPTH. IF NO SAMI LOCATIONS ARE INDICATED ON THE PLANS, THE CONTRACTOR SHALL TAKE A MINIMUM SAMPLES FROM VARIOUS REPRESENTATIVE LOCATIONS FOR TESTING. | | LANDSCA | E INSPECTED PLANTING PE CONTRACTOR SHAL | L REPLACE AN | |
| | b. THE CONTRACTOR SHALL HAVE THE SOIL TESTING LABORATORY PROVIDE RESULTS FOR FOLLOWING: SOIL TEXTURAL CLASS, GENERAL SOIL FERTILITY, pH, ORGANIC MATTER (| | 3. THE LANE | CTION WITHIN 24 HOURS DSCAPE MAINTENANCE INSPECTED BY THE OW | PERIOD WILL N | |
| | SALT (CEC), LIME, SODIUM ADSORPTION RATIO (SAR) AND BORON CONTENT. c. THE CONTRACTOR SHALL ALSO SUBMIT THE PROJECT'S PLANT LIST TO THE LABORATC WITH THE SOIL SAMPLES. | | GUARANT | F FINAL ACCEPTANCE | | D BY THE |
| | d. THE SOIL REPORT PRODUCED BY THE LABORATORY SHALL CONTAIN RECOMMENDATION THE FOLLOWING (AS APPROPRIATE): SEPARATE SOIL PREPARATION AND BACKFILL MIX | NS FOR | ON THESE | SCAPE CONTRACTOR S E PLANS FOR 90 DAYS E | BEYOND FINAL | ACCEPT |
| | RECOMMENDATIONS FOR GENERAL ORNAMENTAL PLANTS, XERIC PLANTS, TURF, AND I SEED, AS WELL AS PRE-PLANT FERTILIZER APPLICATIONS AND RECOMMENDATIONS FO OTHER SOIL RELATED ISSUES. THE REPORT SHALL ALSO PROVIDE A FERTILIZER PROG | R ANY | OWNER. ACTIONS | LANDSCAPE MAINTENA (AS APPROPRIATE): PF TLED, MOWING AND AE | NCE SHALL INC | CLUDE W IG, REST/ |
| 3. | THE ESTABLISHMENT PERIOD AND FOR LONG-TERM MAINTENANCE. THE CONTRACTOR SHALL INSTALL SOIL AMENDMENTS AND FERTILIZERS PER THE SOILS REP | ORT | DISEASES TO FAULT | S, REPLACEMENT OF MU | JLCH, REMOVAL KMANSHIP, ANI | L OF LITT D THE AF |
| 4. | RECOMMENDATIONS. ANY CHANGE IN COST DUE TO THE SOIL REPORT RECOMMENDATIONS INCREASE OR DECREASE, SHALL BE SUBMITTED TO THE OWNER WITH THE REPORT. FOR BIDDING PURPOSES ONLY, THE SOIL PREPARATION SHALL CONSIST OF THE FOLLOWING | | THE LANE ORDER, V | DSCAPE CONTRACTOR S VITH SCHEDULING ADJU SEEDED AND/OR SODDE | SHALL MAINTAI | IN THE IF SEASON |
| 4. | a. TURF: INCORPORATE THE FOLLOWING AMENDMENTS INTO THE TOP 8" OF SOIL BY MEA ROTOTILLING AFTER CROSS-RIPPING: | | THE LANE A FULL, H | SCAPE CONTRACTOR SEALTHY STAND OF PLA | SHALL BE RESP | PONSIBL DITIONAL |
| | NITROGEN STABILIZED ORGANIC AMENDMENT - 4 CU. YDS. PER 1,000 S.F. PREPLANT TURF FERTILIZER (10-20-10 OR SIMILAR, SLOW RELEASE, ORGANIC) - 15 LBS S.F. | PER 1,000 | CONDITIC | VE FINAL ACCEPTANCE DNS MUST OCCUR: E LANDSCAPE SHALL SH | | |
| | "CLAY BUSTER" OR EQUAL - USE MANUFACTURER'S RECOMMENDED RATE TREES, SHRUBS, AND PERENNIALS: INCORPORATE THE FOLLOWING AMENDMENTS INT | O THE TOP | SEA REF | SONAL DORMANCY). A PLACED BY HEALTHY PL | ALL PLANTS NO | T MEETII |
| | 8" OF SOIL BY MEANS OF ROTOTILLING AFTER CROSS-RIPPING: NITROGEN STABILIZED ORGANIC AMENDMENT - 4 CU. YDS. PER 1,000 S.F. 12-12-12 FERTILIZER (OR SIMILAR, ORGANIC, SLOW RELEASE) - 10 LBS. PER CU. YD. | | c. SOE | HARDSCAPE SHALL BE DDED AREAS MUST BE A HES BEFORE FIRST MO | ACTIVELY GRO | WING AN |
| | "CLAY BUSTER" OR EQUAL - USE MANUFACTURER'S RECOMMENDED RATE IRON SULPHATE - 2 LBS. PER CU. YD. | | RES NEA | ODDED (AS APPROPRIA | ATE) PRIOR TO | FINAL A |
| 5. | IN THE CONTEXT OF THESE PLANS, NOTES, AND SPECIFICATIONS, "FINISH GRADE" REFERS TO FINAL ELEVATION OF THE SOIL SURFACE (NOT TOP OF MULCH) AS INDICATED ON THE GRADI A. BEFORE STARTING WORK, THE LANDSCAPE CONTRACTOR SHALL VERIFY THAT THE RO | IG PLANS. | 1. THE LANE | RIOD, PLANT GUARANTI DSCAPE CONTRACTOR S DN SYSTEMS FOR A PEF | SHALL GUARAN | NTEE ALL |
| | GRADES OF ALL LANDSCAPE AREAS ARE WITHIN +/-0.1' OF FINISH GRADE. SEE SPECIFI FOR MORE DETAILED INSTRUCTION ON TURF AREA AND PLANTING BED PREPARATION. | CATIONS | ACCEPTA EXPENSE | NCE (90 DAYS FOR ANN AND TO THE SATISFAC | NUAL PLANTS). TION OF THE C | THE CO DWNER, A |
| | b. CONSTRUCT AND MAINTAIN FINISH GRADES AS SHOWN ON GRADING PLANS, AND CONS AND MAINTAIN SLOPES AS RECOMMENDED BY THE GEOTECHNICAL REPORT. ALL LAND AREAS SHALL HAVE POSITIVE DRAINAGE AWAY FROM STRUCTURES AT THE MINIMUM S | SCAPE | 2. AFTER TH | NY PORTIONS OF THE I IE INITIAL MAINTENANC CTOR SHALL ONLY BE R | E PERIOD AND | DURING |
| | SPECIFIED IN THE REPORT AND ON THE GRADING PLANS, AND AREAS OF POTENTIAL PC SHALL BE REGRADED TO BLEND IN WITH THE SURROUNDING GRADES AND ELIMINATE F | ONDING | CANNOT I PROVIDE A MIN | BE ATTRIBUTED DIRECT IMUM OF (2) COPIES OF | TLY TO OVERWA | ATERING |
| | POTENTIAL. c. THE LANDSCAPE CONTRACTOR SHALL DETERMINE WHETHER OR NOT THE EXPORT OF WILL BE NEEDED, TAKING INTO ACCOUNT THE ROUGH GRADE PROVIDED, THE AMOUNT | | | 'ING IS A RECORD OF AI THROUGH CHANGE ORI | | |
| | AMENDMENTS TO BE ADDED (BASED ON A SOIL TEST , PER SPECIFICATIONS), AND THE GRADES TO BE ESTABLISHED. | FINISH | | | | |
| | d. ENSURE THAT THE FINISH GRADE IN SHRUB AREAS IMMEDIATELY ADJACENT TO WALKS OTHER WALKING SURFACES, AFTER INSTALLING SOIL AMENDMENTS, IS 3" BELOW THE FINISH SURFACE, IN ORDER TO ALLOW FOR PROPER MULCH DEPTH. TAPER THE SOIL S | ADJACENT | | | | |
| | TO MEET FINISH GRADE, AS SPECIFIED ON THE GRADING PLANS, AT APPROXIMATELY 1 FROM THE WALKS. | 3" AWAY | | | | |
| | e. ENSURE THAT THE FINISH GRADE IN TURF AREAS IMMEDIATELY ADJACENT TO WALKS A OTHER WALKING SURFACES, AFTER INSTALLING SOIL AMENDMENTS, IS 1" BELOW THE SURFACE OF THE WALKS. TAPER THE SOIL SURFACE TO MEET FINISH GRADE, AS SPEC | FINISH | | | | |
| | SURFACE OF THE WALKS. TAPER THE SOIL SURFACE TO MEET FINISH GRADE, AS SPEC THE GRADING PLANS, AT APPROXIMATELY 18" AWAY FROM THE WALKS. SHOULD ANY CONFLICTS AND/OR DISCREPANCIES ARISE BETWEEN THE GRADING PLAN | | | | | |
| | GEOTECHNICAL REPORT, THESE NOTES AND PLANS, AND ACTUAL CONDITIONS, THE CONTRACTOR SHALL IMMEDIATELY BRING SUCH ITEMS TO THE ATTENTION OF THE LAN | | | | | |
| | ARCHITECT, GENERAL CONTRACTOR, AND OWNER. | | | | | |



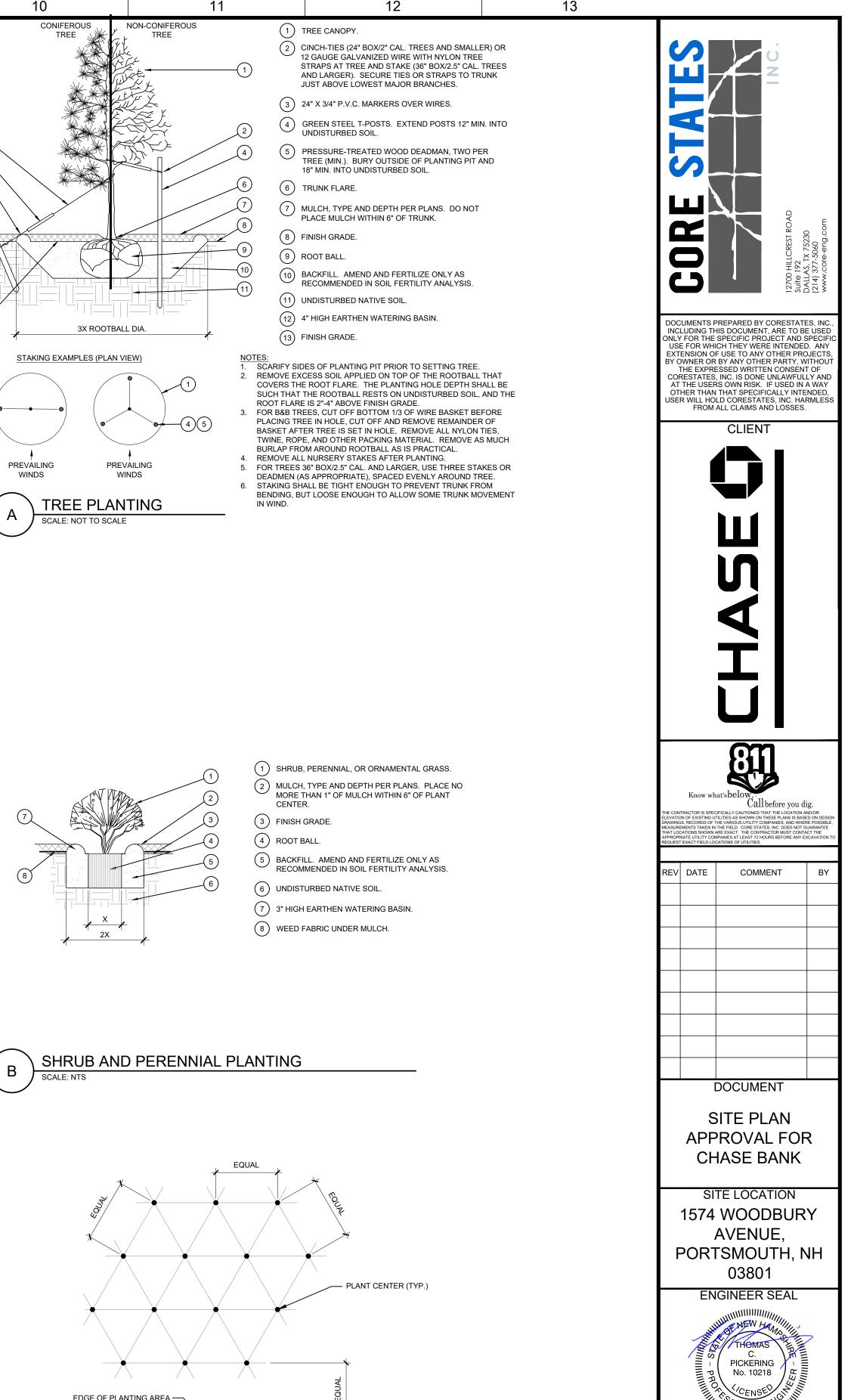
-ROOT BARRIER - PLAN VIEW SCALE: NOT TO SCALE



PLANTING NOTES FOR TYPE AND MANUFACTURER. INSTALL PER MANUFACTURER'S SPECIFICATIONS. (4) TREE CANOPY

5 TYPICAL PLANTING AREA (6) TYPICAL CURB AND GUTTER

NOTES: 1) INSTALL ROOT BARRIERS NEAR ALL NEWLY-PLANTED TREES THAT ARE LOCATED WITHIN FIVE (5) FEET OF PAVING OR CURBS. 2) BARRIERS SHALL BE LOCATED IMMEDIATELY ADJACENT TO HARDSCAPE. UNDER NO CIRCUMSTANCES SHALL THE CONTRACTOR USE ROOT BARRIERS OF A TYPE THAT COMPLETELY ENCIRCLE THE ROOTBALL.



SHEET TITLE

PLANTING

DETAILS &

SPECIFICATIONS

SHEET NO.

P-2

JPM.2708

07/01/2020

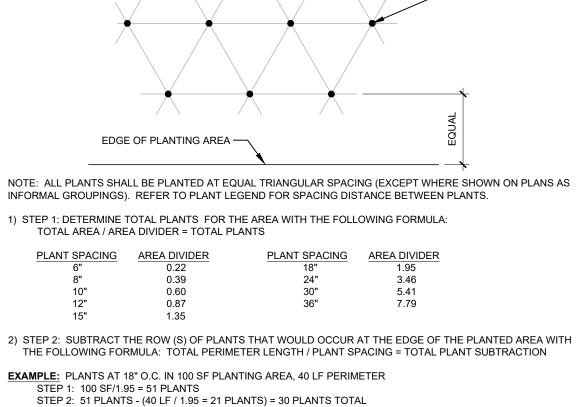
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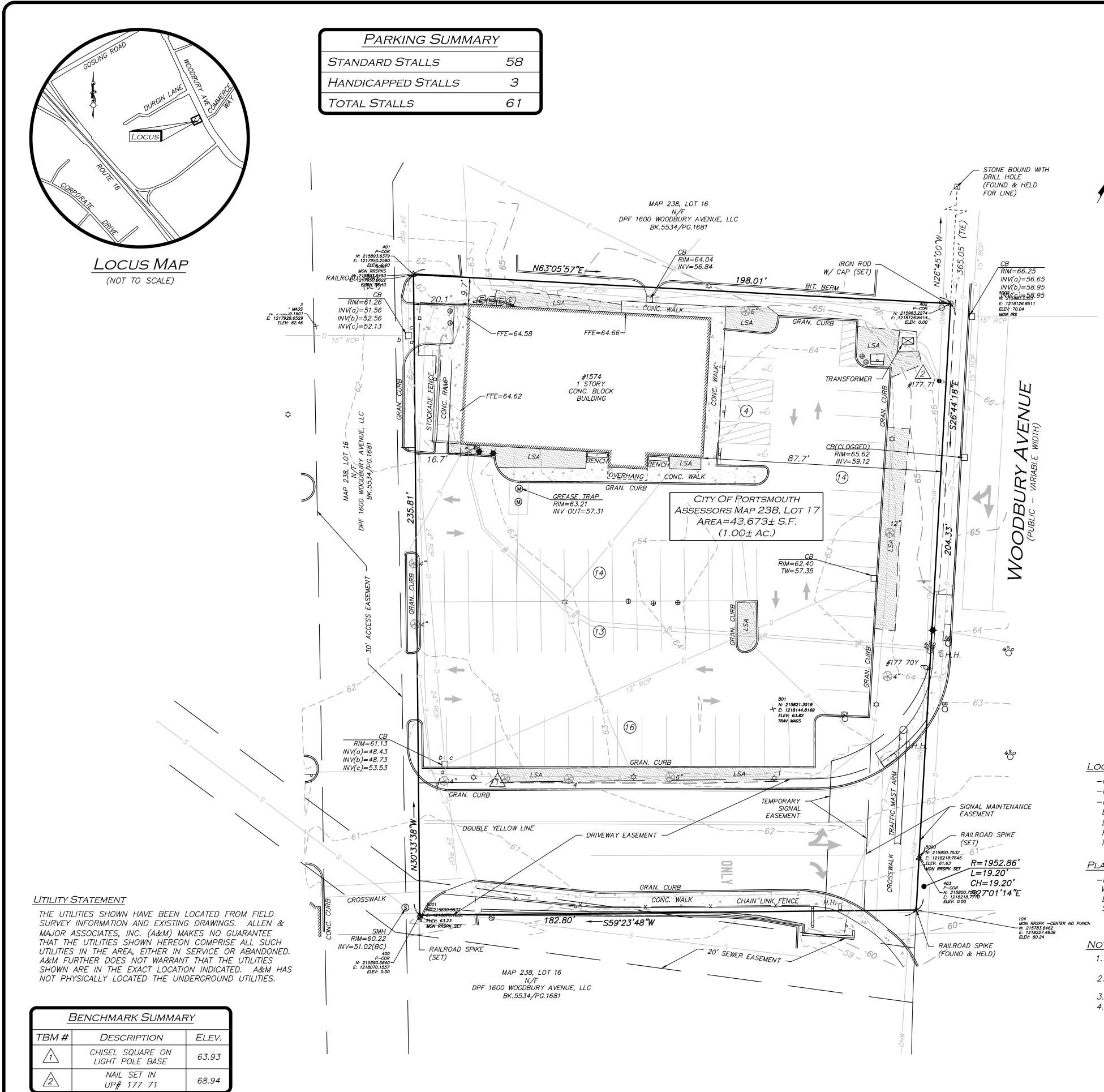
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Locus References

-CITY OF PORTSMOUTH ASSESSORS MAP 23 -R.C.R.D. BOOK 4452, PAGE 881

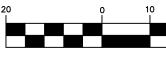
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-PLAN ENTITLED, "SIGNAL MAINTENANCE EAS WOODBURY AVENUE PORTSMOUTH, ROCKING BY RICHARD P. FUSEGNI", 1"=20', DATED SURVEYING, INC., AND ON FILE AT THE R.C.

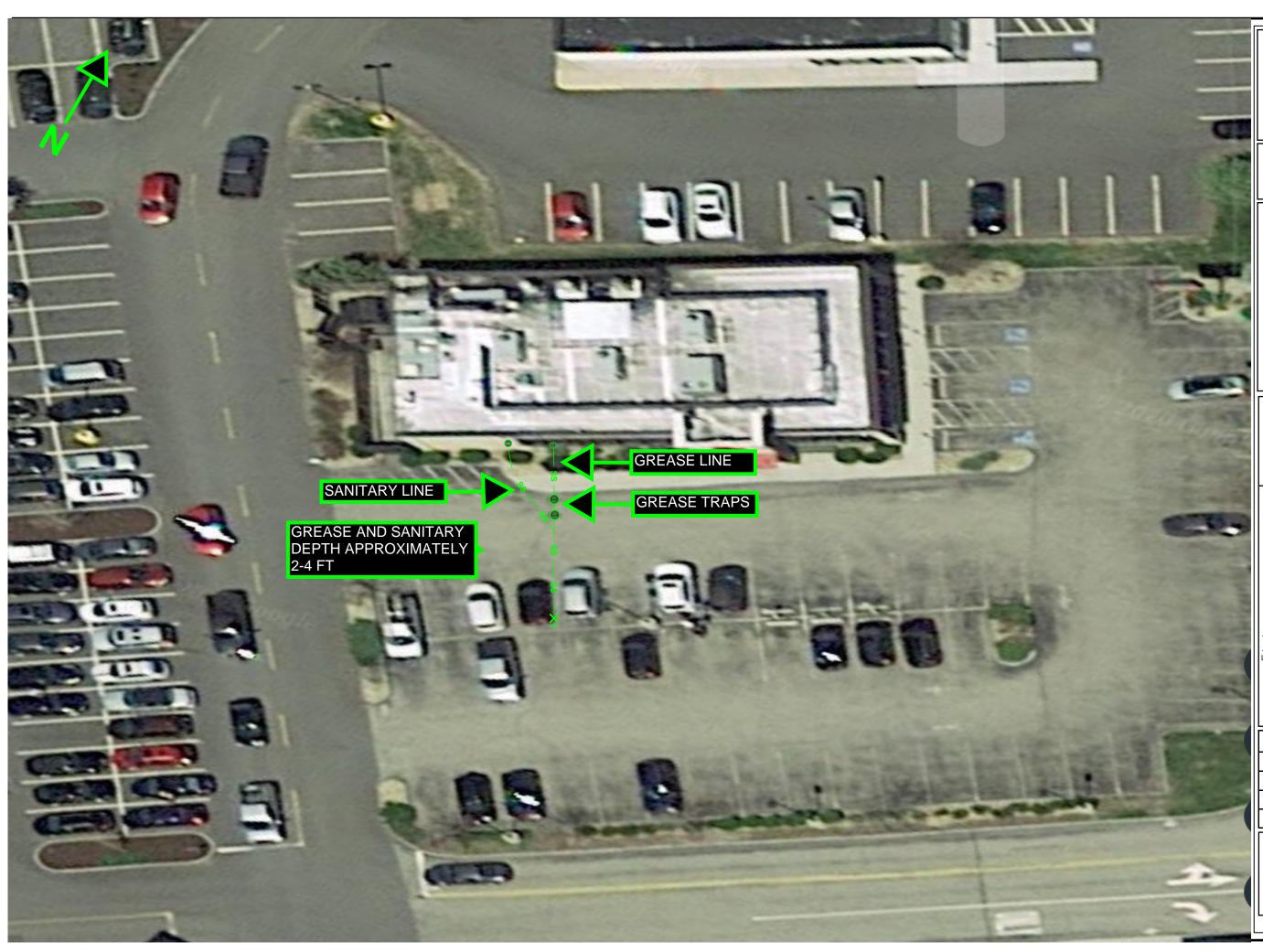
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- 3. VERTICAL DATUM IS NAVD 88.
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master locators

800.495.4248 info@masterlocators.com www.masterlocators.com

Site Name and Location: 1574 Woodbury Ave Portsmouth NH 03801

Assumptions & Clarifications

Assumptions & Clarifications 1. UNLESS OTHERWISE NOTED UNDERGROUND UTILITY DATA IS CONSIDERED QUALITY LEVEL 8 (QLB) AS DEFINED IN ASCE 38-02: STANDARD GUIDELINE FOR THE COLLECTION AND DEPICTION OF EXISTING SUBSURFACE UTILITY DATA AND IS INTENDED TO SHOW THE APPROXIMATE HORIZONTAL LOCATIONS OF EXISTING UNDERGROUND UTILITIES AS MARKED BY MASTER LOCATORS DURING A GEOPHYSICAL INVESTIGATION PERFORMED WITHIN THE OUTLINED SCOPE OF WORK.

OF WORK. 2. ALL UTILITY LOCATIONS SHOWN ON THIS PLAN ARE FOR REFERENCE ONLY. THIS PLAN SHOULD NOT BE USED FOR CONSTRUCTION OR DESIGN PURPOSES AND MASTER LOCATORS IS NOT RESPONSIBLE FOR DAMAGE TO UTILITIES RESULTING FROM ANY CONSTRUCTION WORK BASED ON THESE PLAN.

3. NO BOUNDARY OR PROPERTY SURVEY WORK WAS CONDUCTED IN THE DEVELOPMENT OF THIS PLAN. THE PLAN IS NOT DRAWN TO SCALE.

4 ANY DEPTH INFORMATION PROVIDED IS CONSIDERED APPROXIMATE AND IS NOT GUARANTEED UNLESS LABELED AS QUALITY LEVEL A (QLA) DATA.

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STORMWATER MANAGEMENT & SOIL EROSION CONTROL REPORT

FOR

JP MORGAN CHASE BANK – PORTSMOUTH

LOT 17, ASSESSOR MAP 238

1574 WOODBURY AVENUE

CITY OF PORTSMOUTH

ROCKINGHAM COUNTY, NEW HAMPSHIRE

PREPARED BY:

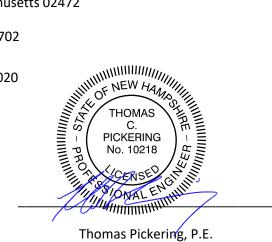
CORE STATES GROUP

9 Galen Street, Suite 117

Watertown, Massachusetts 02472

857-500-4702

July 1st, 2020



Thomas Pickering, P.E.

NH License No. 10218

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- C. Hydraulic Calculations Report
- D. Drainage Area Maps
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I. INTRODUCTION

The proposed Chase Bank is a commercial re-development project within the G1, Gateway Corridor District, located on Woodbury Avenue between Durgin Lane and Arthur Brady Drive part of the overall development known as "Durgin Square" situated in the City of Portsmouth, New Hampshire. The overall site **(See Appendix "A")** consists of a total tract area of 1.002 acres or 43,673 square-feet, as reference from Assessors Map 238 Lot 17. The proposed redevelopment of this property will include the demolition of approximately 4,600-square foot existing restaurant building, foundations, landscaping, pavement, concrete and associated utilities. The re-development proposes the construction of a new single-story Chase Bank with one (1) remote drive-thru ATM and bypass lane. The proposed redevelopment will disturb approximately 0.830 acres (36,110 sf) of the subject property. The design intent for stormwater management is to meet City of Portsmouth stormwater requirements under Ordinance Section-7.4 and in conjunction with New Hampshire Stormwater Manual.

The site is located solely within a commercialized area in the City of Portsmouth, New Hampshire. No streams or waterways are located on the subject site or adjoining the property. The subject property is occupying a parcel lot that is part of an overall shopping center. The intent of this report is to show the proposed stormwater analysis and conditions for the Chase Bank development. The majority of the site's stormwater discharges to three (3) points of analyses that discharge directly to study points located as shown on the attached drainage area maps (See Appendix "D").

II. METHODOLOGY

Run-off has been generated under both pre- and post-development conditions in accordance with "Technical Release No. 55-Urban Hydrology for Small Watersheds" and City of Portsmouth stormwater requirements under Ordinance Section-7.4.

Runoff curve numbers (CN's) have been established by use of the hydrologic soil groups associated with the soils found in the Rockingham County Soil Survey. A composite soil survey map **(See Appendix "B")** is provided for review of the general soil characteristics. See the chart below of CN number breakdown for existing and proposed.

| | <u>CN</u> |
|------------------------------------|-----------|
| Grass/Lawn Coverage (Soil Group B) | 69 |
| Impervious Coverage | 98 |

The existing soil stratum found within the project area is comprised of one soil type, Chatfield-Jollis-Canton (140B) as identified from mapping available from the Natural Resource Conservation Service Web Soil Survey and found in **Appendix "B"**.

The proposed time of concentration is based on overland and sheet flow. The time of concentration for the drainage areas has been set to a minimum value of 6 minutes (0.10 hours), per the TR-55 Manual, for the purpose of the calculations.

Hydrographs were generated using "Hydraflow Hydrographs Extension for AutoCAD" by Autodesk, Inc. This program is based upon the Soil Conservation Service methodology for tabular hydrographs using the Type III storm event as detailed in" Technical Release No. 55-Urban Hydrology for Small Watersheds." The 24-hour rainfall for the four (4) respective storms studied have been gathered from the National Oceanic and Atmospheric Administration rainfall data:

| <u>Storm Event</u> | 24 Hour Rainfall |
|--------------------|------------------|
| 2-yr* | 3.32″ |
| 10-yr* | 5.33" |
| 25-yr* | 6.59" |
| 50-yr* | 7.51″ |

*Based on Rainfall Event over a 24-hour period.

III. HYDROLOGIC ANALYSIS

A. Existing Drainage Area Conditions

As shown on the Existing Drainage Area Map (See Appendix "D"), the existing project is comprised of three (3) drainage areas, 0.834 acres, which all discharge into three (3) study points. Study Point 1 is identified as the majority of the onsite flow that flows to the existing stormwater conveyance system. Existing Drainage Area 1, which is primarily located on the north and west part of the site, drains to Study Point-1. Study Point 2 which collects Existing Drainage Area 2 is identified by the flow that bypasses the existing stormwater conveyance system and flows to overall shopping center. The last study point, Study Point 3, is identified as the overland flow that discharges onto the public Right-of-Way and does not flow into the shopping center system. The hydraulic calculations for the existing drainage areas for each study point can be found in Appendix "C".

B. Proposed Drainage Area Conditions

The proposed drainage conditions are design to replicate the existing drainage conditions while meeting the City of Portsmouth's stormwater regulations. As shown on the Proposed Drainage Area Map (See Appendix "D"). As it is existing, the proposed drainage area map comprises of three drainages areas which discharge to three separate study points. The hydraulic calculations for the proposed drainage areas for each study point can be found in Appendix "C". The description of each drainage area is located below.

IV. STORMWATER MANAGEMENT REQUIREMENTS

A. Best Management Practices (Section 7.4.2)

Per the City of Portsmouth Stormwater Management Regulations as described in Section 7.4 of Site Plan Review regulations all developments under site plan review regardless of limit of disturbance shall meet, as applicable, the 23 requirements for Best Management Practices. Below you will find how the development meets the applicable management practices.

• Section 7.4.2.1-3

The Best Management Practices for sections 1-3 are not applicable to the project and therefore do not need to be met.

• Section 7.4.2.4

"Snow storage areas shall be located such that no direct discharges to receiving waters are possible from the storage site. Runoff from snow storage areas shall enter treatment areas to remove suspended solids and other contaminants before being discharged to receiving waters or preferably be allowed to infiltrate into the groundwater."

The proposed project is a redevelopment and near no streams or waterways, therefore would not have any direct discharge into receiving waters. Additionally, as part of the redevelopment the project is proposing water quality system at the most downstream part of the existing conveyance system to treat site runoff. Further details on the proposed water quality system of the project can be found below.

• Section 7.4.2.5

"Every effort shall be made to retain stormwater on the site using the natural or existing flow patterns of the site."

The proposed project is a redevelopment where a majority of the existing drainage patterns are contained on the on-site storm conveyance system and do not discharge to adjacent properties. As part of the proposed redevelopment the existing drainage patterns will be maintained.

• Section 7.4.2.6-7

The Best Management Practices for sections 6-7 are not applicable to the project and therefore do not need to be met. Since the overall site is being reduced in impervious coverage and on-site water quality system is being proposed, the need for infiltration practices are not required.

• Section 7.4.2.8

"Measure shall be taken to control the post-development peak rate of runoff so that it does not exceed pre-development runoff for the 2-, 10-, 25-, 50-year, 24 hour storm event.

In order to meet this standard, a pre- and post- development comparison of the 2-, 10-, 25-, 50- year storm events for each study point can be found in **Appendix "D"**. The calculations show that at any point the post-development peak discharge rate does not exceed the pre-development peak discharge rate for any storm event. A summary of the runoff quantities can be found on the table below:

| Summary of Existing and Proposed Runoff Quantity | | | |
|--|-------------|------------|------------|
| Area | Storm Event | Existing | Proposed |
| | | Flow (cfs) | Flow (cfs) |
| | 2-year | 2.131 | 1.858 |
| Study Point - 1 | 10-year | 3.640 | 3.428 |
| | 25-year | 4.575 | 4.408 |
| | 50-year | 5.253 | 5.120 |
| | 2-year | 0.055 | 0.019 |
| Study Point - 2 | 10-year | 0.123 | 0.054 |
| | 25-year | 0.167 | 0.079 |
| | 50-year | 0.200 | 0.099 |
| Study Point -3 | 2-year | 0.042 | 0.041 |
| | 10-year | 0.108 | 0.099 |
| | 25-year | 0.155 | 0.138 |
| | 50-year | 0.189 | 0.167 |
| | 2-year | 2.227 | 1.918 |
| TOTAL LOT | 10-year | 3.871 | 3.581 |
| RUNOFF | 25-year | 4.897 | 4.626 |
| | 50-year | 5.643 | 5.385 |

Therefore, described above and through the hydraulic analysis of the pre-development and post-development conditions of the site, the proposed development meets the criteria for stormwater quantity as defined in Section 7.4.2.8 in the City of Portsmouth Ordinance.

• Section 7.4.2.9

"The applicant shall demonstrate that on- and off-site downstream channel or system capacity is sufficient to carry the stormwater run-off volume flow without adverse effects, such as flooding and erosion of stream banks and shoreland areas.

As previously stated, the overall combined site runoff of the post-development drainage is being reduced from the pre-development conditions. Therefore, it is determined that no adverse effects of the downstream bank for the proposed project.

• Section 7.4.2.10

The Best Management Practices for section 10 are not applicable to the project and therefore do not need to be met.

• Section 7.4.2.11

"For a storm event of ½ inch or less, the applicant shall demonstrate that stormwater management practices will remove contaminants from the stormwater runoff that leaves the site. The use of oil and grit traps in manholes, on-site vegetated waterways, and vegetated buffer strips along waterways and drainage swales, and the reduction in use of deicing salts and fertilizers may be required by the Planning Board."

Per the New Hampshire Stormwater Manual and the City of Portsmouth, a water quality device was added at the most downstream pipe conveyance system to meet the requirements. The proposed water quality device meets the Standards depicted in the New Hampshire Stormwater Manual which requires 80% TSS Removal rate of the "first wash" water quality storm event.

The New Hampshire Stormwater Manual defines the "first wash" water quality storm as the first 1" rainfall of any given storm event. As mentioned previously a CDS 2015-4 unit by Contech has been provided to meet these standards, a CDS Removal Rate calculated for this project has been provided in **Appendix "E"**. The CDS unit uses indirect screening technique to remove suspended solids, fine sands and larger particles. Additionally, the unit has an internal weir/bypass system to only provide solids removal for the water quality storm event and not inhibit the flow of the other storm events. A design summary of the CDS Unit can be found in **Appendix "E"**.

• Section 7.4.2.12

The Best Management Practices for section 12 are not applicable to the project and therefore do not need to be met.

• Section 7.4.2.13

"The design of the on-site stormwater drainage systems shall not increase or impede existing flows."

As previously stated, the overall combined site runoff of the post-development drainage is being reduced from the pre-development conditions. Therefore, it is determined that no adverse effects to the existing flows.

• Section 7.4.2.14-19

The Best Management Practices for sections 14-19 are met through the proposed project. An extensive landscape and soil erosion plans are proposed to maintain integrity of downstream drainage systems in and the proposed development shall be stabilized as per these requirements.

• Section 7.4.2.20-23

The Best Management Practices for section 20-23 are not applicable to the project and therefore do not need to be met.

B. Groundwater Recharge

Per the New Hampshire Stormwater Manual the proposed development groundwater recharge must meet the pre-development groundwater recharge. Since the overall impervious coverage of the site is being reduced the annual post development groundwater recharge will naturally exceed what is existing. It is determined, that no additionally BMP techniques are required to meet the groundwater recharge volume requirement.

V. SOIL EROSION AND SEDIMENT CONTROL

A. Overview

The Soil Erosion and Sediment Control Measures for this project include adequately installed perimeter silt fencing, temporary and permanent seeding and mulching, inlet protection, and the installation of temporary stone tracking pads at the project site entrance. A Phase I and Phase II plan has been provided in the drawings set for your reference. All provisions are to be in accordance with the "New Hampshire Stormwater Manual Volume 3".

The soil erosion and sediment control plan will minimize the downstream erosion hazard by controlling runoff at its source, minimizing runoff from disturbed areas and de- concentrating storm water runoff. The objectives of the erosion control plan will be achieved through the management of storm water runoff during construction.

B. Temporary Erosion and Sediment Control Measures

The temporary soil erosion and sediment control measures will include, but not limited to, silt fences, diversion ditches, stabilization of the construction entrance, sediment traps and basins, storm drain inlet protection, hydro-seeding and dust control. Detailed descriptions of each of the measures that will be employed on the project have been included in the following paragraphs:

- Silt Fences Silt fences consist of standard strength filter fabric with wire mesh reinforcement (or extra strength synthetic filter fabric) secured to supporting posts and entrenched at the base. Filter fabric requirements and installation design criteria will be in accordance with the requirements in the "New Hampshire Stormwater Manual Volume 1-3". Silt fences will be installed on the down slope side of work areas, as close to the disturbed areas as possible. Sediment will be removed from behind silt fences when sediment has accumulated to one-third of the original height of the fence.
- **Dust Control** Dust Control shall be accomplished through the use of vegetative cover, mulch, spray adhesive, sprinkling or barriers. Water will be applied by sprinkler or water truck as necessary during grading operations to minimize sediment transport and maintain acceptable air quality conditions. Repetitive treatments will be done as needed until grades are paved or stabilized with vegetation.
- Stabilized Construction Entrance A ramp of crushed stone extending a minimum distance of 50 feet will be installed at each point of ingress and egress from the site. The purpose of the device is to minimize the potential of tracking mud from the site onto public rights-of-way or adjoining properties. The entrance shall be maintained in a condition, which will prevent tracking, or flowing of sediment onto public rights-of-way, all sediment spilled, dropped, washed or tracked onto public rights-of-way must be removed immediately. When washing is required, it shall be done on an area stabilized with stone and which drains into an approved sediment trapping device.

• Soil Stockpiling - Topsoil and earth material shall be stockpiled for reuse at the location shown on the Erosion Control Plans. All stockpiles shall be protected using a perimeter dike of silt fence or straw bale sediment barriers to prevent sediment runoff. This applies to all stockpiles remaining in place for more than two weeks. Stockpile side slopes shall not exceed 2 horizontal to 1 vertical (2:1). Temporary seeding or covering of stockpiles shall be completed within two weeks of formation.

VI. CONCLUSION

The implementation of the City of Portsmouth standards for stormwater management design have been presented and achieved through the proposed stormwater management analysis and design. Using stormwater conveyance systems, the runoff throughout the developed site has been designed to meet the necessary requirements. Based upon this analysis, the proposed storm water management system will benefit the existing downstream conveyance system by providing a reduction in impervious area and reducing peak flow stormwater rates to them.

<u>APPENDIX A</u>

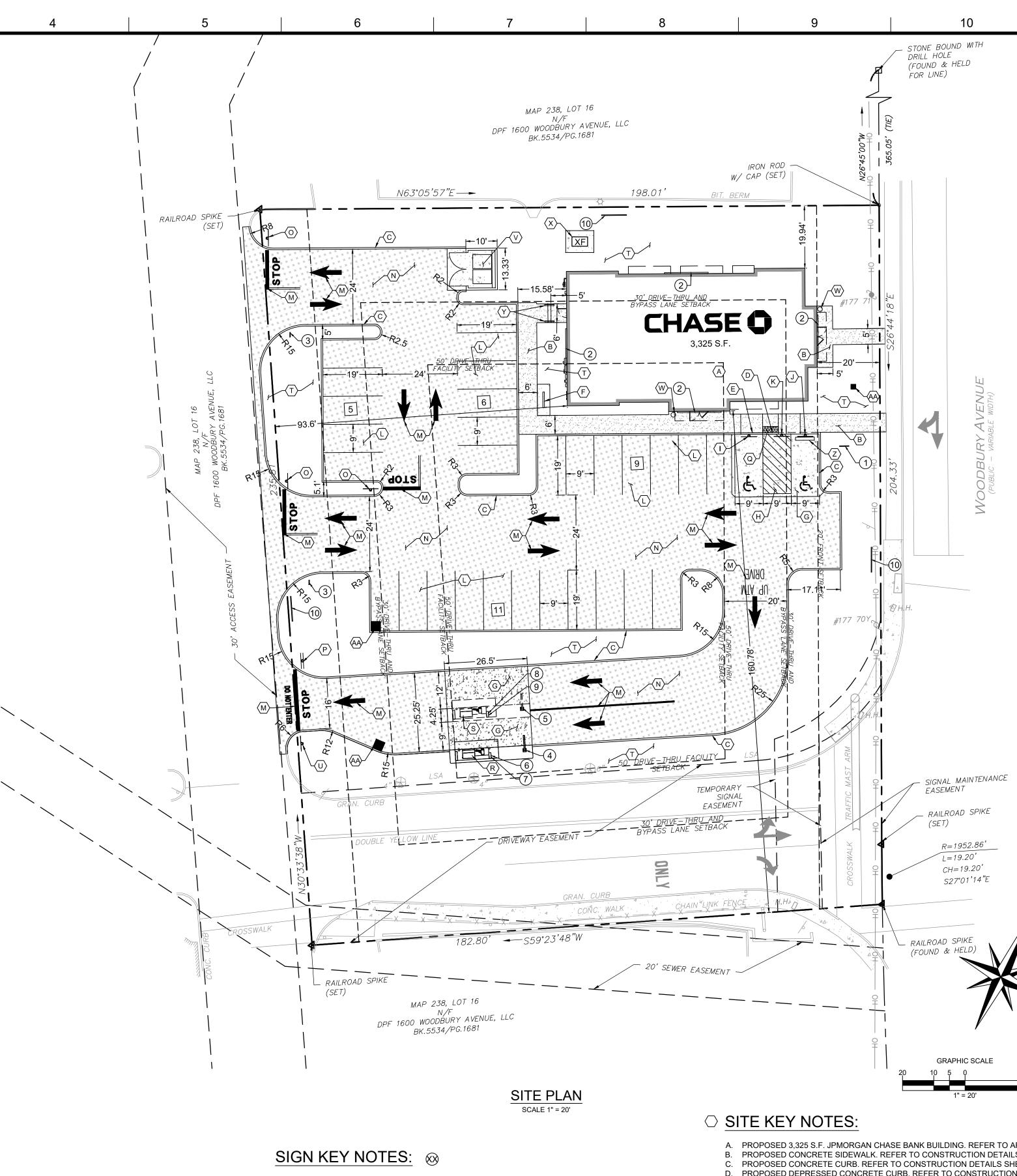
OVERALL SITE PLAN

| SIGNA | GE TABLE (SIGI | N DISTRICT 5) | | |
|-----------------------------------|---|--|---|--|
| | TEMPORARY S | IGNS | | |
| PROVISION | REQUIRED | PROPOSED | COMMENT | |
| MAXIMUM SIGN AREA | 64 SF | 60 SF | COMPLIANT | |
| MAXIMUM SIGN HEIGHT | 12 FEET | <12 FT (MOUNTED ON CONSTRUCTION FENCE) | COMPLIANT | |
| | FREESTANDING | SIGN | | |
| MAXIMUM ALLOWED PER LOT | 1 | 1 MONUMENT SIGN 1 ATM SIGN 1 FUTURE ATM SIGN | COMPLIANT VARIANCE VARIANCE | |
| MAXIMUM SIGN AREA | 100 SF | 56.2 SF 9.5 SF 9.5 SF | COMPLIANT | |
| MAXIMUM SETBACK FRONT LOT LINE | 10 FT | 10 FT | COMPLIANT | |
| | WALL SIGI | N | | |
| MAXIMUM ALLOWED | ONE PER STREET FRONTAGE AND/OR AT MAIN ENTRANCE | 1 EAST ELEVATION (WOODBURY AVE) 1 SOUTH ELEVATION 1 NORTH ELEVATION 1 WEST ELEVATION | COMPLIANT - COMPLIANT VARIANCE VARIANCE | |
| MAXIMUM SIGN AREA | 100 SF | 36.9 SF | COMPLIANT | |
| TOTAL AGGREGATE SIGNS | | | | |
| MAXIMUM AGGREGATE SIGN AREA | 1.5 SF PER LINEAR FEET OF BUILDING FRONTAGE = 1.5 x 42.5 LF = 63.75 SF | 147.6 SF | VARIANCE | |

ALERT TO CONTRACTOR:

PRIOR TO THE CONSTRUCTION OF OR CONNECTION TO ANY STORM DRAIN, SANITARY SEWER, WATER MAIN OR ANY OF THE DRY UTILITIES, THE CONTRACTOR SHALL EXCAVATE, VERIFY AND CALCULATE ALL POINTS OF CONNECTION AND ALL UTILITY CROSSINGS AND INFORM ENGINEER AND THE OWNER OF ANY CONFLICT OR REQUIRED DEVIATIONS FROM THE PLAN. NOTIFICATION SHALL BE MADE A MINIMUM OF 48 HOURS PRIOR TO CONSTRUCTION. ENGINEER AND OWNER SHALL BE HELD HARMLESS IN THE EVENT THAT THE CONTRACTOR FAILS TO MAKE SUCH NOTIFICATION.

User: MLEWIS Plot Date/Time: Jul. 01, 20 - 09:58:32 Drawing: P:\J.P. Morgan Chase\Portsmouth, NH (1574 Woodbury Avenue) OVP# 38100P322370 - JPM.27086\CIVIL\Drawings\Presentation\JPM.27086-P-SITE-NEW.dwg ;C2-SITE

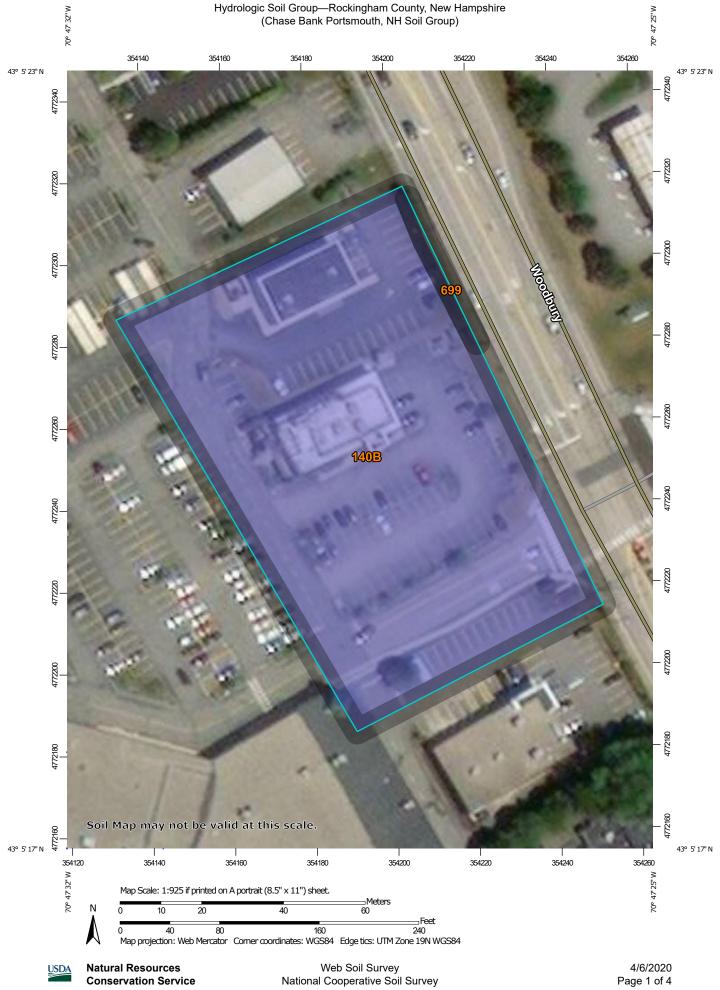


- PROPOSED CHASE PROTOTYPICAL PYLON SIGN (56.2 S.F.).
 PROPOSED WHITE W/BLUE OCTAGON ILLUMINATED CHANNEL "CHASE" LOGO SIGN
- WALL SIGN (36.9 S.F.).
- PROPOSED DOUBLE FACED NON-ILLUMINATED DIRECTIONAL SIGN (2.3 S.F.).
 PROPOSED CLEARANCE SIGN AND HEADACHE BAR.
- PROPOSED CLEARANCE SIGN AND HEADACHE BAR FOR FUTURE DRIVE-UP ATM.
 PROPOSED SIGNATURE DRIVE-UP "CHASE" LOGO LETTERS (5.6 S.F.).
- 7. PROPOSED SIGNATURE DRIVE-UP OCTAGON (3.9 S.F.).
- FUTURE SIGNATURE DRIVE-UP "CHASE" LOGO LETTERS (5.6 S.F.).
 FUTURE SIGNATURE DRIVE-UP OCTAGON (3.9 S.F.).
- 10. TEMPORARY "COMING SOON" SIGN MOUNTED ON CONSTRUCTION FENCE (60S.F.).
- D. PROPOSED DEPRESSED CONCRETE CURB. REFER TO CONSTRUCTION E. PROPOSED TRANSITION CURB SECTION. REFER TO CONSTRUCTION D PROPOSED BIKE RACK ON CONCRETE PAD. REFER TO CONSTRUCTION G. PROPOSED CONCRETE PAD. REFER TO CONSTRUCTION DETAILS SHE H. PROPOSED 9' X 19' ACCESSIBLE PARKING SPACE AND AISLE WITH SYM CONSTRUCTION DETAILS SHEET. PROPOSED VAN ACCESSIBLE PARKING SIGN. REFER TO CONSTRUCTION J. PROPOSED ACCESSIBLE PARKING SIGN. REFER TO CONSTRUCTION D K. PROPOSED NO PARKING ANYTIME SIGN. REFER TO CONSTRUCTION D _. PROPOSED 9' X 19' STANDARD PARKING SPACE. REFER TO CONSTRUCT M. PROPOSED SITE MARKINGS. REFER TO CONSTRUCTION DETAILS SHEE N. PROPOSED ASPHALT PAVEMENT. REFER TO CONSTRUCTION DETAILS O. PROPOSED STOP SIGN. REFER TO CONSTRUCTION DETAILS SHEET. P. PROPOSED STOP & DO NOT ENTER SIGN. REFER TO CONSTRUCTION I Q. PROPOSED DETECTABLE WARNING SURFACE. REFER TO CONSTRUCT R. PROPOSED DRIVE-UP SIGNATURE ATM CANOPY. REFER TO CONSTRUC S. PROPOSED "FUTURE" DRIVE-UP SIGNATURE ATM CANOPY. REFER TO T. PROPOSED LANDSCAPE AREA. REFER TO LANDSCAPE PLAN FOR DETA U. PROPOSED DO NOT ENTER SIGN. REFER TO CONSTRUCTION DETAILS V. PROPOSED TRASH ENCLOSURE. W. PROPOSED TRASH BIN. REFER TO ARCHITECT PLANS FOR DETAIL. X. RELOCATED ELECTRIC TRANSFORMER AND CONCRETE PAD. REFER T
- Y. PROPOSED HANDRAIL. REFER TO CONSTRUCTION DETAILS SHEET.Z. PROPOSED WHEEL STOP. REFER TO CONSTRUCTION DETAILS SHEET.
- 2. PROPOSED WHEEL STOP. REFER TO CONSTRUCTION DETAILS SHEET. AA. PROPOSED STORM DRAIN INLET. REFER TO CONSTRUCTION DETAILS

| | 11 | | 12 | 13 | | |
|-----------------------------|---|---|---|--|--|---|
| GE | ENERAL NOTES: | | | | | |
| THIS | THIS DRAWING REFERENCES: 1574 WOODBURY AVENUE PORTSMOUTH, NH TOPOGRAPHIC PLAN OF LAND PREPARED BY: ALLEN & MAJOR ASSOCIATES, INC. 400 HARVEY ROAD MANCHESTER, NH 03103 CONTACT: JAMES P. SMITH NH LLS TEL: (603) 627-5500 | | | | | |
| PRO | PERTY OWNER: | DATED: 06/23/2020 RICHARD P. FUSEGNI 201 KEARSARGE WAY PORTSMOUTH, NH 03801 CONTACT: SCOTT MITCHELL | | | | |
| APPI | LICANT: | (603) 475-377 J.P. MORGAN CHA 1450 BRICKELL AV MIAMI, FL 33131 CONTACT: CHRIS TEL: (786) 473-176 | 'ENUE 3RD FLOOR FOIT | | | le 172 LLAS, TX 75230 4) 377-5060 M.Core-eng.com |
| 1) | SITE ADDRESS: | 1574 WOODBURY | TEL: (786) 473-1769 1574 WOODBURY AVENUE PORTSMOUTH, NH 03801 COUNTY OF ROCKINGHAM | | | 5 ATES, INC., |
| 2) | ZONING DATA: | ZONED: G-1 GATE | WAY CORRIDOR DISTR | RICT | INCLUDING THIS DOCUMENT, ARE TO ONLY FOR THE SPECIFIC PROJECT AND USE FOR WHICH THEY WERE INTEND EXTENSION OF USE TO ANY OTHER P | D SPECIFIC DED. ANY |
| | | PROPOSED USE: I | STAURANT - RUBY TUB BANK (PERMITTED BY F DRIVE THROUGH (CON | BY OWNER OR BY ANY OTHER PARTY, THE EXPRESSED WRITTEN CONSE CORESTATES, INC. IS DONE UNLAWFI AT THE USERS OWN RISK. IF USED I OTHER THAN THAT SPECIFICALLY IN USER WILL HOLD CORESTATES, INC. F | , WITHOUT ENT OF ULLY AND IN A WAY ITENDED, | |
| | §10.5B34.60 (SMALL COMMERCIAL | BUILDING) <u>REQUIRED</u> | EXISTING | PROPOSED | | ES. |
| | MIN. LOT AREA, SF: MIN. LOT FRONTAGE, FT: | N/A 50 FT. | 43,673 S.F. 204.32 FT. | NO CHANGE NO CHANGE | | |
| | FRONT YARD SETBACK, FT: MIN. SIDE YARD SETBACK, FT: | 0 FT 20 FT. 10 FT. | 87.7 FT. 9.7 FT. | 20 FT. 19.94 FT. | | |
| | MIN. REAR YARD SETBACK, FT: MAX. HEIGHT, FT: | 15 FT. 40 FT. | 16.7 FT. ± 20 FT. | 93.6 FT. 21.5 FT. | | |
| | MAX. HEIGHT, STORIES: MIN. STREET FACADE HEIGHT: | 3 18 FT. 10% | 1 ± 20 FT. 17.79% | 1 21.5 FT. 18.18% | Ш | |
| | MIN. OPEN SPACE COVERAGE: | 70% | (7,770 S.F.) 10.53% | (7,942 S.F.) 7.55% | | |
| | MAX. BUILDING FOOTPRINT: MIN. STREET FACING | 10,000 S.F. | 4,600 S.F. | 3,325 S.F. | | |
| | FACADE GLAZING: | 50% | "" | 52% | | |
| 3) | PARKING REQUIREMENTS: §10.1112.30 OFF-STREET PARKING | G REQUIREMENTS F | PER THE CITY OF PORT | SMOUTH ZONING ORDINANCE: | | |
| | FOR PROFESSIONAL, BUSINESS A 1 SPACE PER 350 SQUARE FEET (| | | | | |
| | GROSS FLOOR GROSS AREA = 3,3 CALCULATION: 1 SPACE X (3,325 S REQUIRED = 10 SPACES | | DING) | | | |
| | EXISTING PARKING SPACES: 61 SPACES (INCLUDING 3 ACCESSIBLE SPACES) PROPOSED PARKING SPACES: 31 SPACES (INCLUDING 2 ACCESSIBLE SPACES) | | | | | |
| | PARKING DIMENSIONS EXISTING: VARIES REQUIRED: 8.5' X 19' PROPOSED: 9' X 19' | | | | | |
| 4) | ALL EXISTING FEATURES ARE TO | REMAIN UNLESS C | THERWISE NOTED. | | Know what's below. Call before you the contractor is specifically cautioned that the locatic | |
| 5) 6) | ALL PAVEMENT MARKINGS SHALI PRIOR TO STARTING CONSTRUCT | TION, THE CONTRAC | TOR SHALL BE RESPO | | ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS E DRAWINGS, RECORDS OF THE VARIOUS UTILITY COMPARIES, AND W MEASUREMENTS TAKEN IN THE FIELD. CORE STATES, INC. DOES IN THAT LOCATIONS SHOWN ARE EXACT. THE CONTRACTOR MUST CO APPROPRIATE UTILITY COMPANIES AT LEAST 72 HOURS BEFORE AN | BASED ON DESIGN |
| | REQUIRED PERMITS AND APPROV BEGIN UNTIL THE CONTRACTOR I DOCUMENTS BY ALL OF THE PER | HAS RECEIVED AND | THOROUGHLY REVIEW | | REQUEST EXACT FIELD LOCATIONS OF UTILITIES. | |
| 7) | ALL WORK SHALL BE PERFORME REQUIREMENTS AND STANDARD | D IN ACCORDANCE S OF THE LOCAL GO | WITH THESE PLANS AN VERNING AUTHORITY. | | REV DATE COMMENT | BY |
| 8) | | SHALL NOTIFY ENG | NEER IF ANY DISCREP | ANCIES EXIST PRIOR TO PROCEEDING | | |
| | | G TO BE REDONE D | JE TO DIMENSIONS OR | ENSATION SHALL BE PAID TO THE GRADES SHOWN INCORRECTLY ON | | |
| 9) | | | - | H ALL LOCAL, STATE AND FEDERAL | | |
| 10) 11) | | OR ALL SHORING R | EQUIRED DURING EXC | AVATION AND SHALL BE PERFORMED | | |
| 2 | IN ACCORDANCE WITH CURRENT STABILITY OF CONTIGUOUS STRU | | - | | | |
| Ą | T | | | | | |
| | | | | | | |
| | | | | | DOCUMENT | |
| | | | | | SITE PLAN | |
| | | | | | APPROVAL FO | R |
| | | | | | CHASE BANK | K |
| | | | | | SITE LOCATION | |
| 20 | | | | | 1574 WOODBUI | RY |
| | | | | | AVENUE, | |
| | | | | | PORTSMOUTH, 03801 | ΝH |
| | | | | | ENGINEER SEAL | |
| ARCHITE .S SHEE IEET. | ECTURAL PLANS FOR DETAILS. T. | | | | | |
| N DETAI | LS SHEET. SHEET. | | SITE LE | | | |
| | ILS SHEET. | | | EXISTING PROPERTY BOUNDARY LINE EXISTING ADJOINING PROPERTY LINE | | |
| | OF ACCESSIBILITY. REFER TO AILS SHEET. | | | EXISTING ROAD CENTERLINE PROPOSED ROAD CENTERLINE | | |
| ETAILS | SHEET. SHEET. | | | PROPOSED DITCH CENTERLINE PROPOSED LIMITS OF BMP / DETENTION | | |
| ET. | DETAILS SHEET/ | | | PROPOSED SAWCUT LINE EXISTING CURB | SHEET TITLE | |
| SHEET | S SHEET. | | | PROPOSED CURB PROPOSED DEPRESSED CURB | SITE PLAN | |
| TION DE | SHEET. TAILS SHEET. DETAILS SHEET. | | | PROPOSED BUILDING | | |
| CONST AILS. | RUCTION DETAILS SHEET. | | | | | JPM.27086 07/01/2020 |
| SHEET | | | | PROPOSED CONCRETE | | AS NOTED |
| TO LIGH | TING PLANS FOR DETAIL. | | | EXISTING SANITARY STRUCTURES | DRAWN BY: CHECKED BY: | MAL KGF |
| SHEET. | | 0 | HOH | EXISTING OVERHEAD WIRES | SHEET NO. | |
| | | | X | PROPOSED PARKING COUNT | C? | |
| | | | | | | |

<u>APPENDIX B</u>

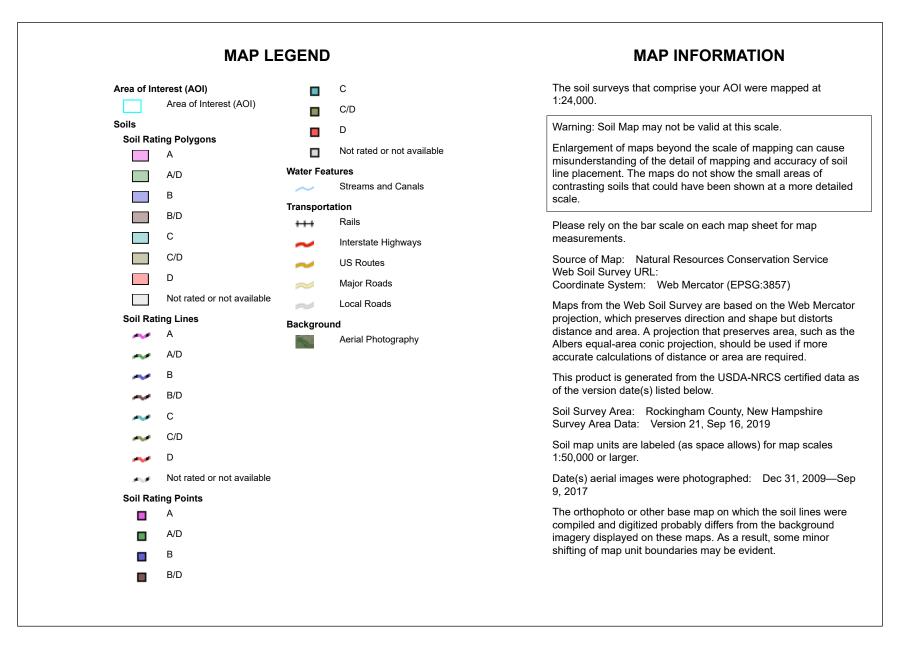
WEB SOIL SURVEY MAP



USDA

Natural Resources **Conservation Service**

Web Soil Survey National Cooperative Soil Survey



Hydrologic Soil Group

| Map unit symbol | Map unit name | Rating | Acres in AOI | Percent of AOI | | |
|---------------------------|---|--------|--------------|----------------|--|--|
| 140B | Chatfield-Hollis-Canton complex, 0 to 8 percent slopes, rocky | В | 2.1 | 99.8% | | |
| 699 | Urban land | | 0.0 | 0.2% | | |
| Totals for Area of Intere | est | | 2.1 | 100.0% | | |

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

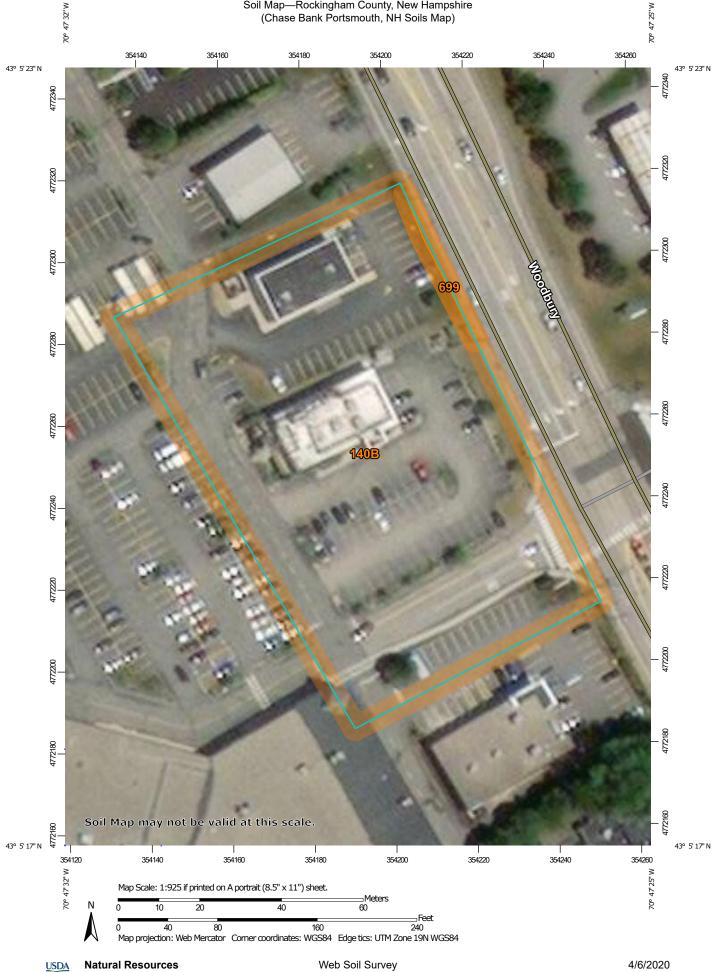
Aggregation Method: Dominant Condition

USDA

Component Percent Cutoff: None Specified Tie-break Rule: Higher



Soil Map—Rockingham County, New Hampshire (Chase Bank Portsmouth, NH Soils Map)



Conservation Service

Web Soil Survey National Cooperative Soil Survey

| MAF | LEGEND | MAP INFORMATION | | |
|---|---|--|--|--|
| Area of Interest (AOI) Area of Interest (AOI) Soils Soil Map Unit Polygo Soil Map Unit Polygo Soil Map Unit Polygo Soil Map Unit Polygo Soil Map Unit Points Special Point Features Image: Solid Map Unit Points Special Point Features Image: Solid Map Unit Points Special Point Features Image: Solid Map Unit Points Soil Map Unit Points Special Point Features Image: Solid Map Unit Points Special Point Features Image: Solid Map Unit Points Soil Map Unit Points Soil Map Unit Points Soil Map Unit Points Image: Solid Map Unit Points Soil Map Unit Points Image: Solid Map | Spoil Area Stony Spot Very Stony Spot | The soil surveys that comprise your AOI were mapped at 1:24,000. Warning: Soil Map may not be valid at this scale. Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale. Please rely on the bar scale on each map sheet for map measurements. Source of Map: Natural Resources Conservation Service Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857) Maps from the Web Soil Survey are based on the Web Mercato projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data a of the version date(s) listed below. Soil Survey Area: Rockingham County, New Hampshire Survey Area Data: Version 21, Sep 16, 2019 | | |
| Perennial Water Rock Outcrop Saline Spot Sandy Spot Severely Eroded Spot Sinkhole Slide or Slip Sodic Spot | | Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. Date(s) aerial images were photographed: Dec 31, 2009—Sej 9, 2017 The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. | | |



Map Unit Legend

| Map Unit Symbol | Map Unit Name | Acres in AOI | Percent of AOI | |
|-----------------------------|---|--------------|----------------|--|
| 140B | Chatfield-Hollis-Canton complex, 0 to 8 percent slopes, rocky | 2.1 | 99.8% | |
| 699 | Urban land | 0.0 | 0.2% | |
| Totals for Area of Interest | | 2.1 | 100.0% | |



<u>APPENDIX C</u>

HYDRAULIC CALCULATIONS REPORT

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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

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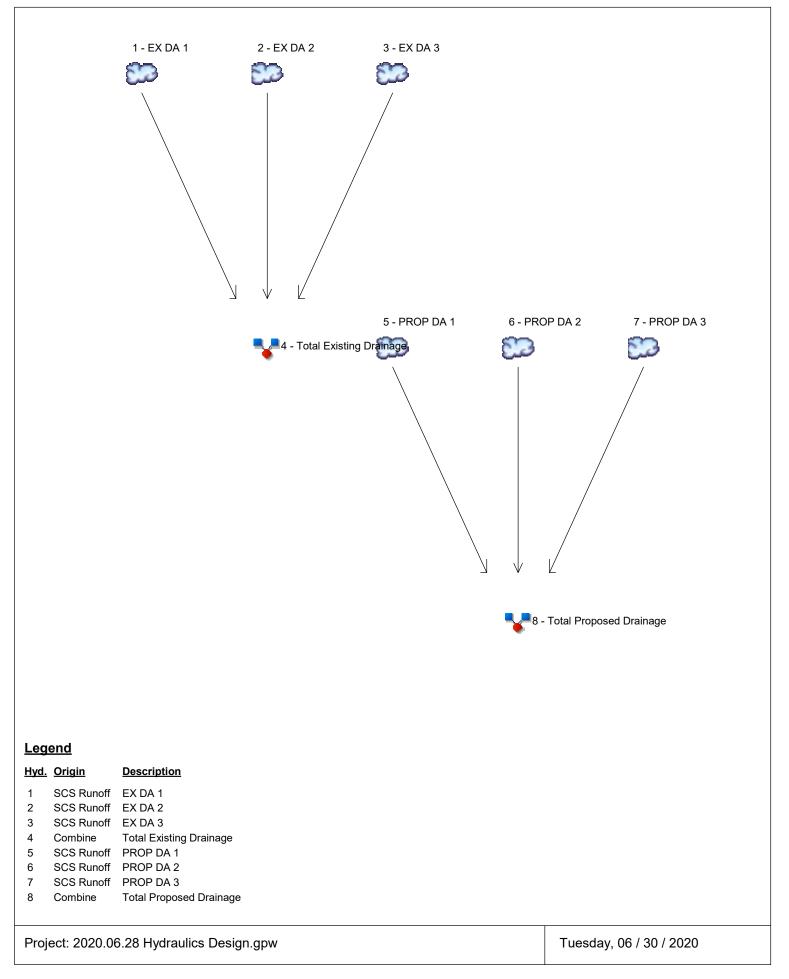
50 - Year

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|--|----|
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| Hydrograph No. 2, SCS Runoff, EX DA 2 | |
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Watershed Model Schematic

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020



Hydrograph Return Period Recap Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

| | | Inflow | Peak Outflow (cfs) | | | | | | | | Hydrograph |
|-----|------------------|------------|--------------------|----------|------|------|-------|-------|-------|---------|-------------------------|
| о. | type (origin) | hyd(s) | 1-yr | 2-yr | 3-yr | 5-yr | 10-yr | 25-yr | 50-yr | 100-yr | Description |
| 1 | SCS Runoff | | | 2.131 | | | 3.640 | 4.575 | 5.253 | | EX DA 1 |
| 2 | SCS Runoff | | | 0.055 | | | 0.123 | 0.167 | 0.200 | | EX DA 2 |
| 3 | SCS Runoff | | | 0.042 | | | 0.108 | 0.155 | 0.189 | | EX DA 3 |
| 4 | Combine | 1, 2, 3 | | 2.227 | | | 3.871 | 4.897 | 5.643 | | Total Existing Drainage |
| 5 | SCS Runoff | | | 1.858 | | | 3.428 | 4.408 | 5.120 | | PROP DA 1 |
| 6 | SCS Runoff | | | 0.019 | | | 0.054 | 0.079 | 0.099 | | PROP DA 2 |
| 7 | SCS Runoff | | | 0.041 | | | 0.099 | 0.138 | 0.167 | | PROP DA 3 |
| 8 | Combine | 5, 6, 7 | | 1.918 | | | 3.581 | 4.626 | 5.385 | | Total Proposed Drainage |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| Pro | j. file: 2020.0 |)6 28 Hvdi | raulics D | esian an | w | 1 | 1 | 1 | | esday 0 | 6 / 30 / 2020 |

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

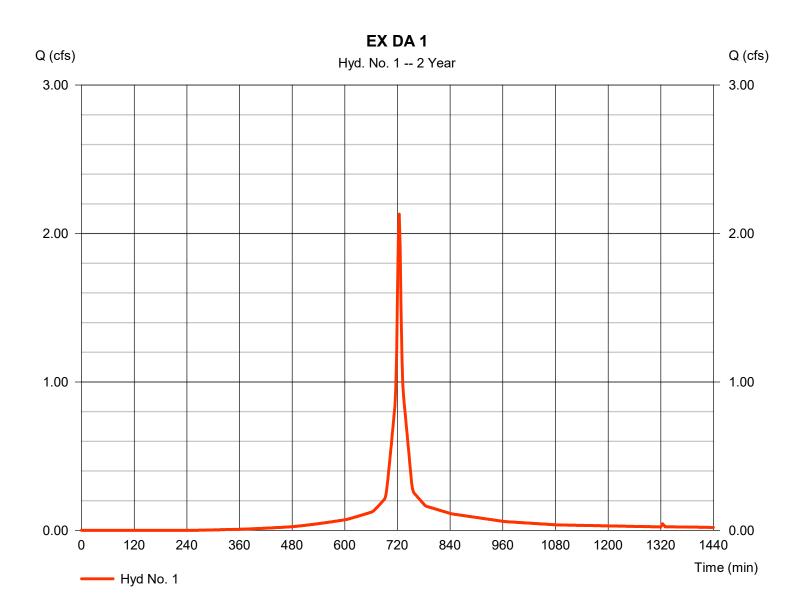
| Hyd. No. | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to Peak (min) | Hyd. volume (cuft) | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph Description |
|-------------|--------------------------------|-----------------------|---------------------------|--------------------------|--------------------------|------------------|------------------------------|-------------------------------|---------------------------|
| 1 | SCS Runoff | 2.131 | 2 | 724 | 6,624 | | | | EX DA 1 |
| 2 | SCS Runoff | 0.055 | 2 | 724 | 167 | | | | EX DA 2 |
| 3 | SCS Runoff | 0.042 | 2 | 724 | 133 | | | | EX DA 3 |
| 4 | Combine | 2.227 | 2 | 724 | 6,925 | 1, 2, 3 | | | Total Existing Drainage |
| 5 | SCS Runoff | 1.858 | 2 | 724 | 5,588 | | | | PROP DA 1 |
| 6 | SCS Runoff | 0.019 | 2 | 724 | 63 | | | | PROP DA 2 |
| 7 | SCS Runoff | 0.041 | 2 | 724 | 128 | | | | PROP DA 3 |
| 8 | Combine | 1.918 | 2 | 724 | 5,780 | 5, 6, 7 | | | Total Proposed Drainage |
| | | | | | | | | | |
| | | | | | | | | | |
| | | | | | | | | | |
| 202 | 20.06.28 Hyd | raulics De | sign.gpw | | Return | Period: 2 Ye | ear | Tuesday, (| 06 / 30 / 2020 |

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 1

EX DA 1

| Hydrograph type | = SCS Runoff | Peak discharge | = 2.131 cfs |
|-----------------|--------------|--------------------|--------------|
| Storm frequency | = 2 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 6,624 cuft |
| Drainage area | = 0.760 ac | Curve number | = 93 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 3.32 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | - | |

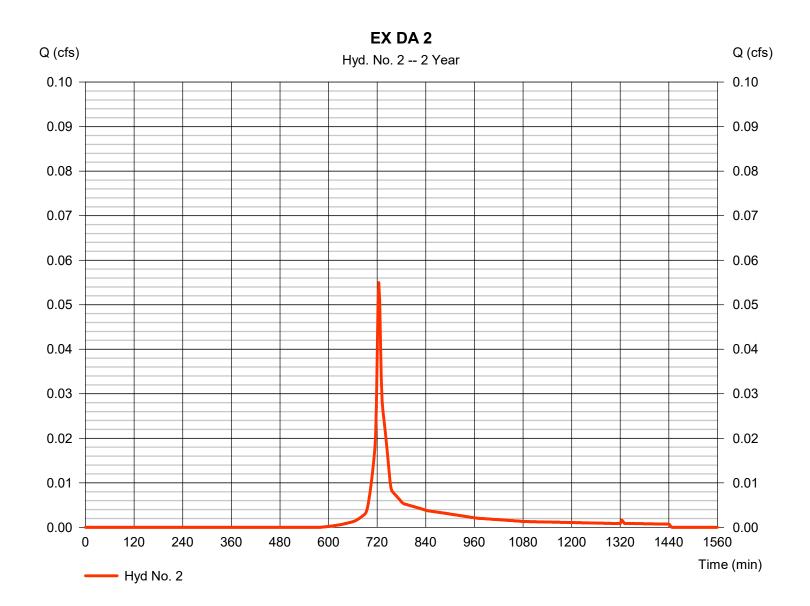


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 2

EX DA 2

| Hydrograph type | = SCS Runoff | Peak discharge | = 0.055 cfs |
|-----------------|--------------|--------------------|-------------|
| Storm frequency | = 2 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 167 cuft |
| Drainage area | = 0.036 ac | Curve number | = 78 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 3.32 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | | |



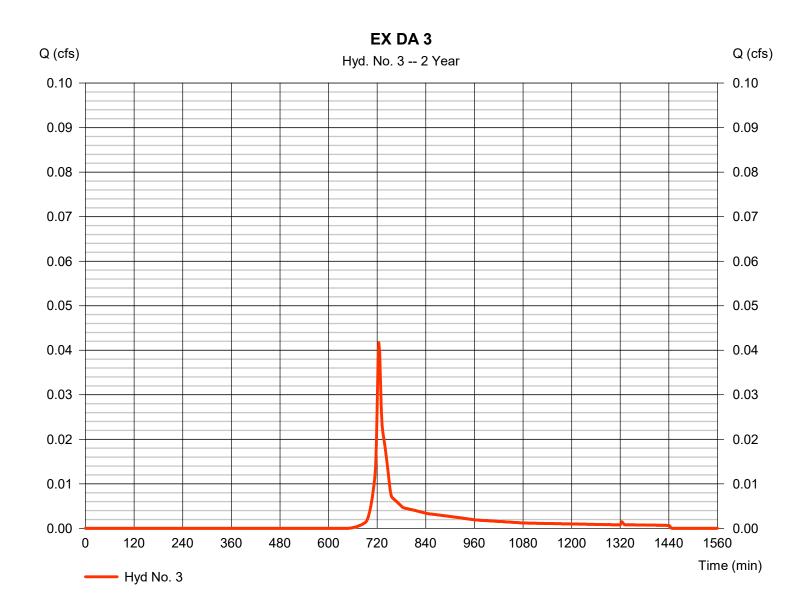
5

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 3

EX DA 3

| Hydrograph type | = SCS Runoff | Peak discharge | = 0.042 cfs |
|-----------------|--------------|--------------------|-------------|
| Storm frequency | = 2 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 133 cuft |
| Drainage area | = 0.039 ac | Curve number | = 72 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 3.32 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | | |

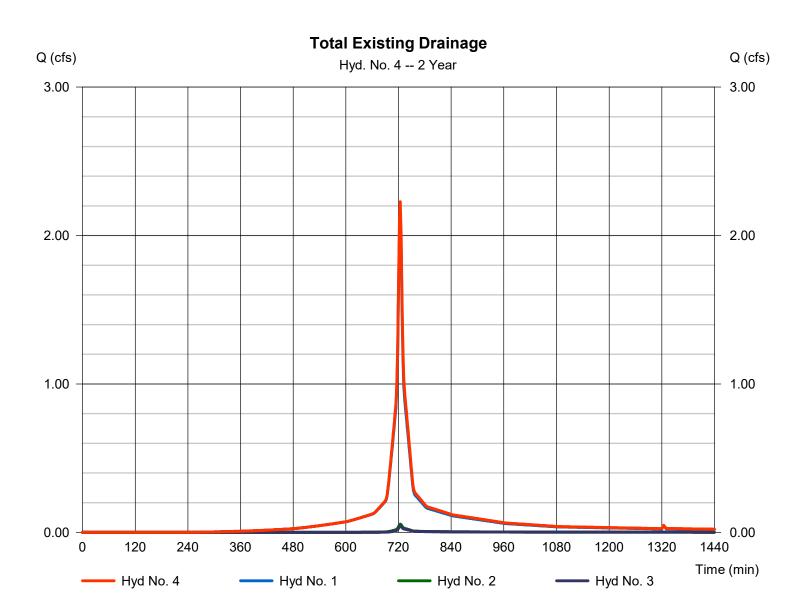


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 4

Total Existing Drainage

| Hydrograph type = Combi | ne Peak discharge | = 2.227 cfs |
|---|-------------------|----------------------------|
| Storm frequency = 2 yrs | Time to peak | = 724 min |
| Time interval= 2 minInflow hyds.= 1, 2, 3 | Hyd. volume | = 6,925 cuft = 0.835 ac |



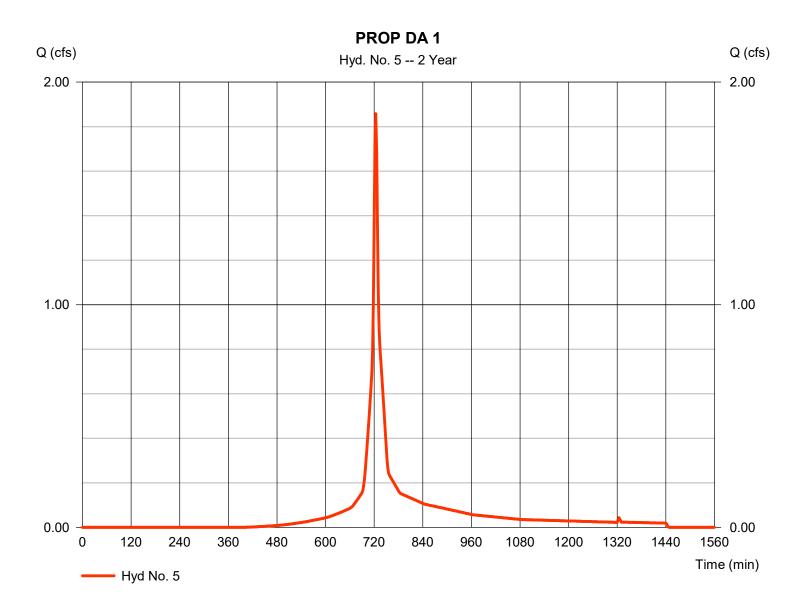
7

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 5

PROP DA 1

| Hydrograph type | = SCS Runoff | Peak discharge | = 1.858 cfs |
|-----------------|--------------|--------------------|--------------|
| Storm frequency | = 2 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 5,588 cuft |
| Drainage area | = 0.780 ac | Curve number | = 88 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 3.32 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | | |

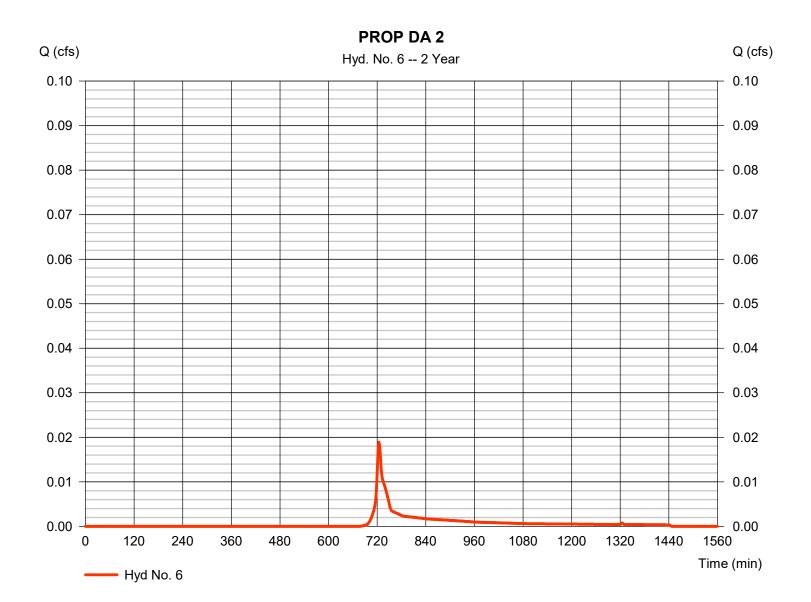


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 6

PROP DA 2

| Hydrograph type | = SCS Runoff | Peak discharge | = 0.019 cfs |
|-----------------|--------------|--------------------|-------------|
| Storm frequency | = 2 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 63 cuft |
| Drainage area | = 0.022 ac | Curve number | = 69 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 3.32 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |

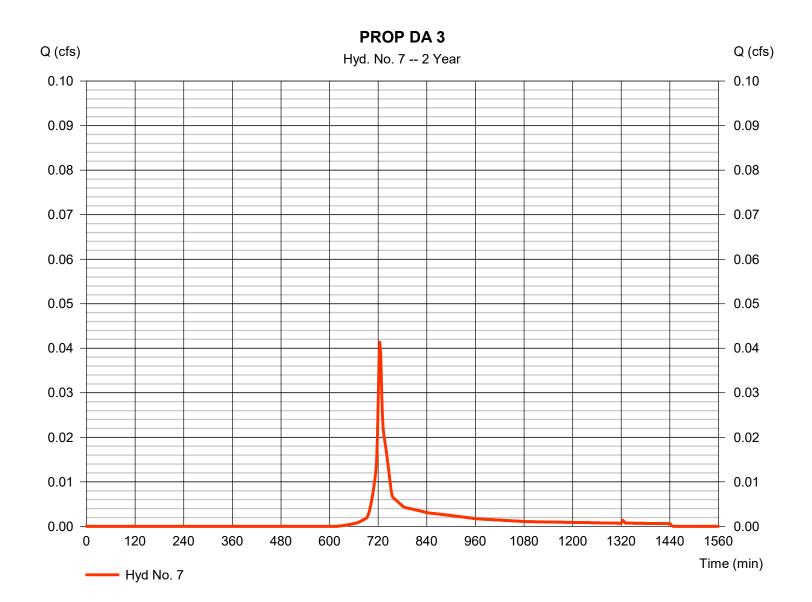


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 7

PROP DA 3

| Hydrograph type | = SCS Runoff | Peak discharge | = 0.041 cfs |
|-----------------|--------------|--------------------|-------------|
| Storm frequency | = 2 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 128 cuft |
| Drainage area | = 0.032 ac | Curve number | = 75 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 3.32 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |

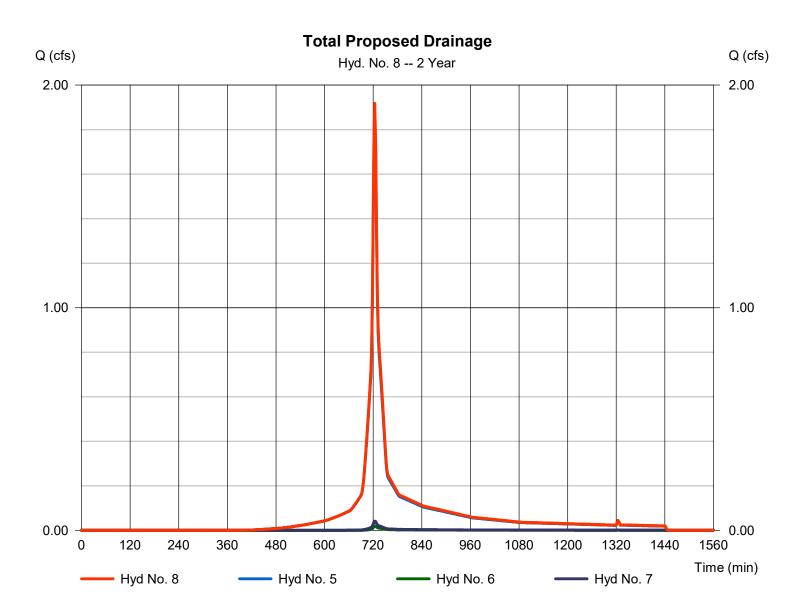


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 8

Total Proposed Drainage

| Storm frequency = 2 yrs | Peak discharge Time to peak | = 1.918 cfs = 724 min |
|--|-------------------------------------|----------------------------|
| Time interval $= 2 \text{ min}$ Inflow hyds. $= 5, 6, 7$ | Hyd. volume Contrib. drain. area | = 5,780 cuft = 0.834 ac |



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Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

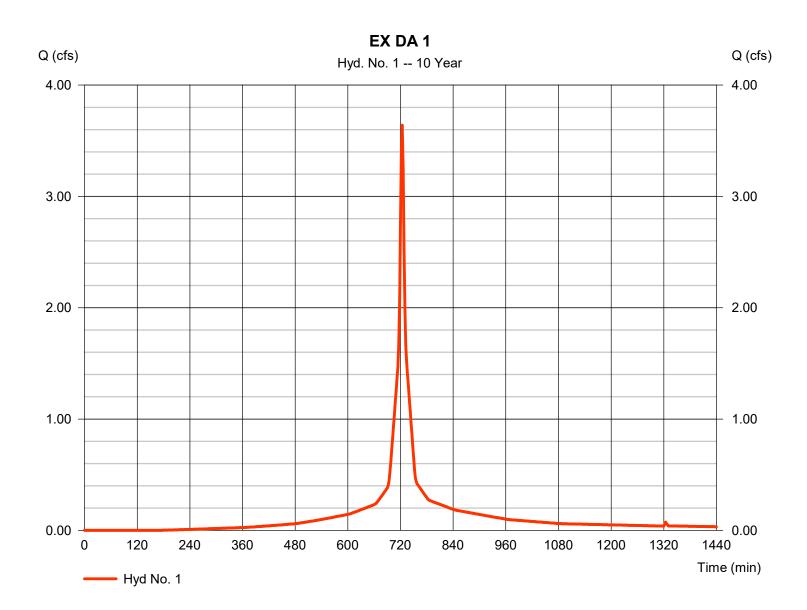
| Hyd. No. | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to Peak (min) | Hyd. volume (cuft) | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph Description |
|-------------|--------------------------------|-----------------------|---------------------------|--------------------------|--------------------------|------------------|------------------------------|-------------------------------|---------------------------|
| 1 | SCS Runoff | 3.640 | 2 | 724 | 11,696 | | | | EX DA 1 |
| 2 | SCS Runoff | 0.123 | 2 | 724 | 367 | | | | EX DA 2 |
| 3 | SCS Runoff | 0.108 | 2 | 724 | 326 | | | | EX DA 3 |
| 4 | Combine | 3.871 | 2 | 724 | 12,389 | 1, 2, 3 | | | Total Existing Drainage |
| 5 | SCS Runoff | 3.428 | 2 | 724 | 10,573 | | | | PROP DA 1 |
| 6 | SCS Runoff | 0.054 | 2 | 724 | 165 | | | | PROP DA 2 |
| 7 | SCS Runoff | 0.099 | 2 | 724 | 296 | | | | PROP DA 3 |
| 8 | Combine | 3.581 | 2 | 724 | 11,034 | 5, 6, 7 | | | Total Proposed Drainage |
| | | | | | | | | | |
| | | | | | | | | | |
| | | | | | | | | | |
| | | | | | | | | | |
| | | | | | | | | | |
| 202 | 20.06.28 Hyd | raulics De | sign.gov | / | Return F | Period: 10 \ | /ear | Tuesday 0 | 6 / 30 / 2020 |

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 1

EX DA 1

| Hydrograph type | = SCS Runoff | Peak discharge | = 3.640 cfs |
|-----------------|--------------|--------------------|---------------|
| Storm frequency | = 10 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 11,696 cuft |
| Drainage area | = 0.760 ac | Curve number | = 93 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 5.33 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | | |

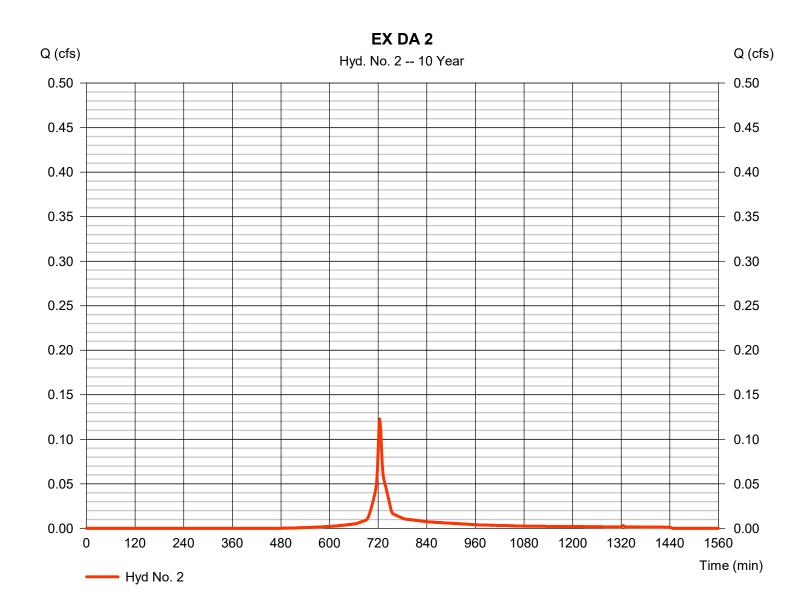


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 2

EX DA 2

| Hydrograph type | = SCS Runoff | Peak discharge | = 0.123 cfs |
|-----------------|--------------|--------------------|-------------|
| Storm frequency | = 10 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 367 cuft |
| Drainage area | = 0.036 ac | Curve number | = 78 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 5.33 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |



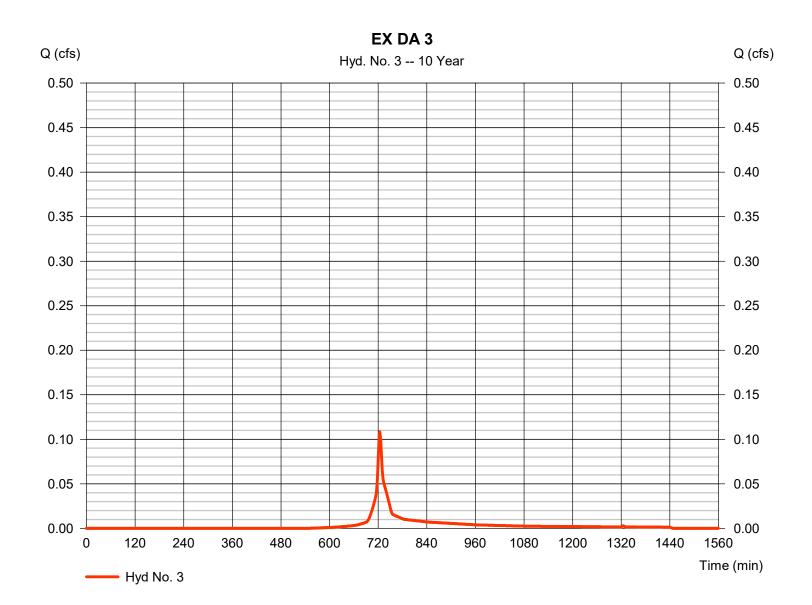
14

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 3

EX DA 3

| ograph type | = SCS Runoff | Peak discharge | = 0.108 cfs |
|--|---|---|---|
| m frequency | = 10 yrs | Time to peak | = 724 min |
| e interval | = 2 min | Hyd. volume | = 326 cuft |
| nage area | = 0.039 ac | Curve number | = 72 |
| n Slope | = 0.0 % | Hydraulic length | = 0 ft |
| nethod | = User | Time of conc. (Tc) | = 6.00 min |
| l precip. | = 5.33 in | Distribution | = Type III |
| m duration | = 24 hrs | Shape factor | = 484 |
| m frequency e interval nage area n Slope nethod I precip. | = 10 yrs = 2 min = 0.039 ac = 0.0 % = User = 5.33 in | Time to peak Hyd. volume Curve number Hydraulic length Time of conc. (Tc) Distribution | 724 min 326 cuft 72 0 ft 6.00 min Type III |



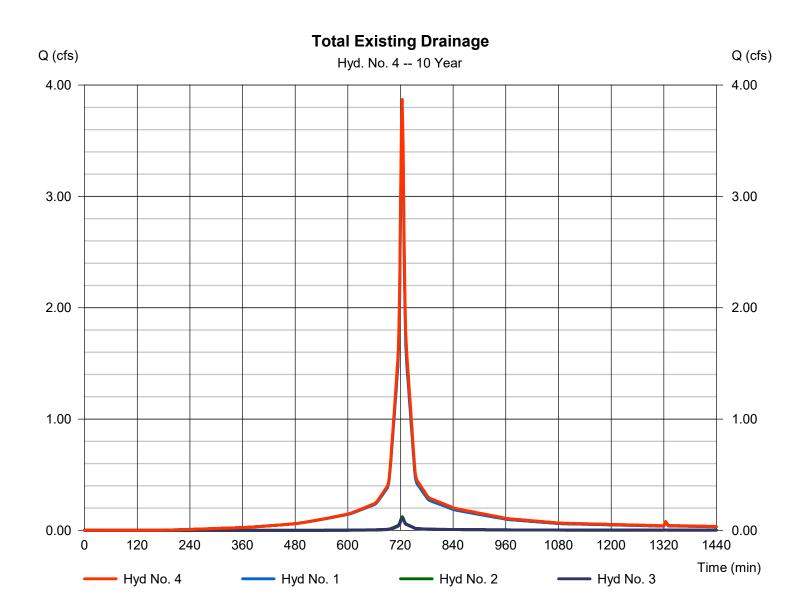
15

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 4

Total Existing Drainage

| Hydrograph type Storm frequency | = Combine = 10 yrs | Peak discharge Time to peak | = 3.871 cfs = 724 min |
|------------------------------------|-----------------------|--------------------------------|--------------------------|
| Time interval | = 2 min | Hyd. volume | = 12,389 cuft |
| Inflow hyds. | = 1, 2, 3 | Contrib. drain. area | = 0.835 ac |



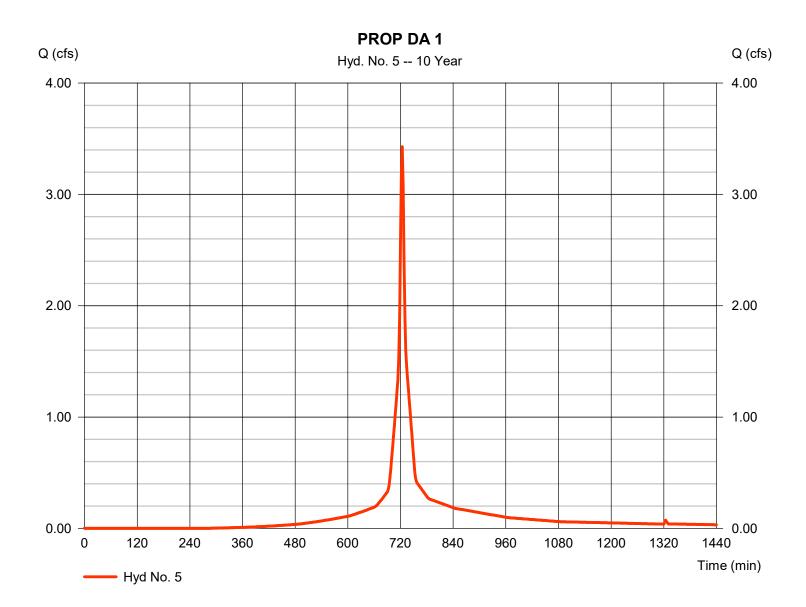
16

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 5

PROP DA 1

| Hydrograph type | = SCS Runoff | Peak discharge | = 3.428 cfs |
|-----------------|--------------|--------------------|---------------|
| Storm frequency | = 10 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 10,573 cuft |
| Drainage area | = 0.780 ac | Curve number | = 88 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 5.33 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | | |



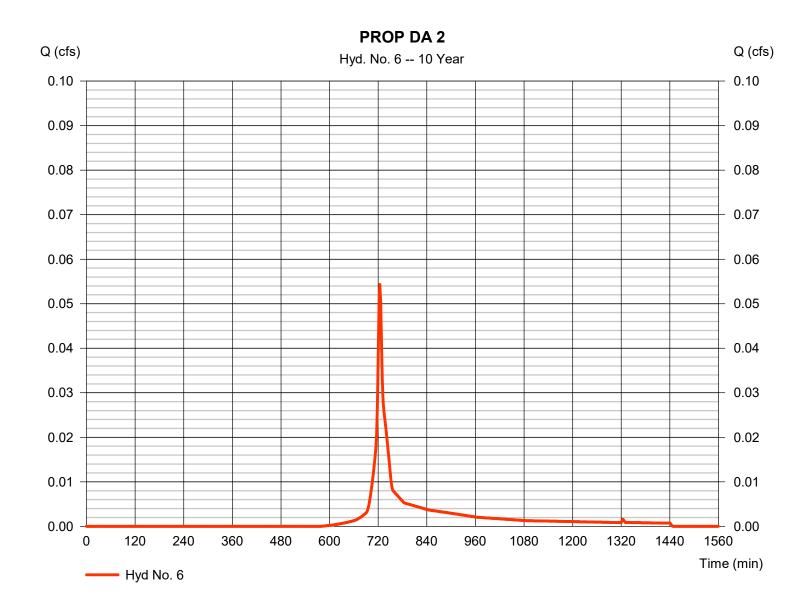
17

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 6

PROP DA 2

| Hydrograph type | = SCS Runoff | Peak discharge | = 0.054 cfs |
|-----------------|--------------|--------------------|-------------|
| Storm frequency | = 10 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 165 cuft |
| Drainage area | = 0.022 ac | Curve number | = 69 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 5.33 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |

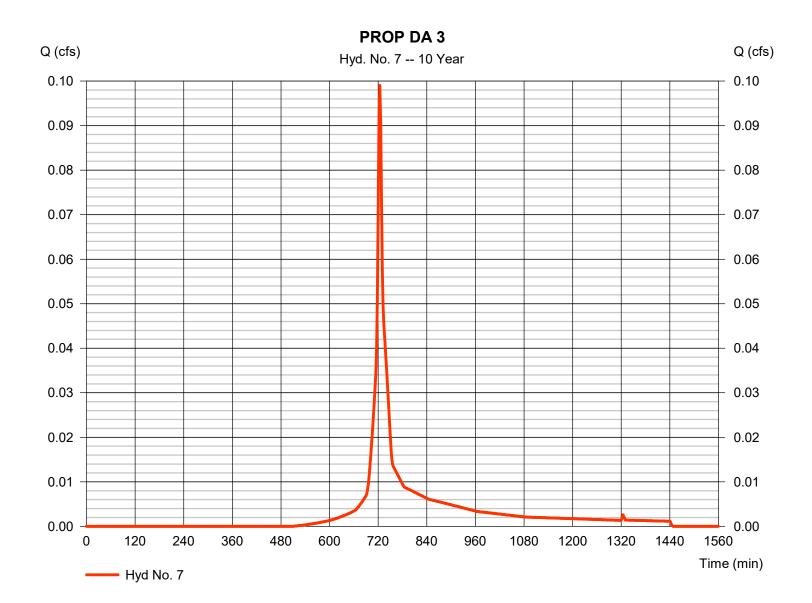


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 7

PROP DA 3

| Hydrograph type | = SCS Runoff | Peak discharge | = 0.099 cfs |
|-----------------|--------------|--------------------|-------------|
| Storm frequency | = 10 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 296 cuft |
| Drainage area | = 0.032 ac | Curve number | = 75 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 5.33 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | | |

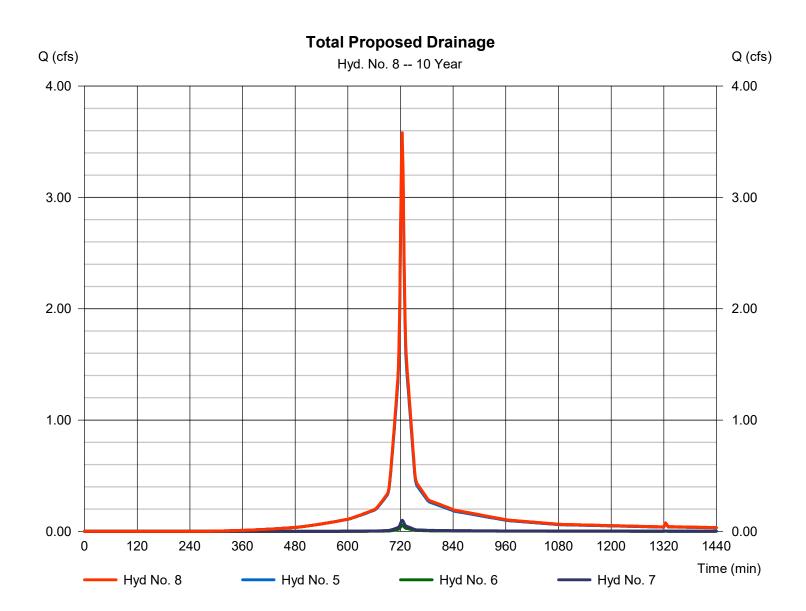


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 8

Total Proposed Drainage

| Hydrograph type Storm frequency | = Combine = 10 yrs | Peak discharge Time to peak | = 3.581 cfs = 724 min |
|------------------------------------|-----------------------|--------------------------------|--------------------------|
| Time interval | = 2 min | Hyd. volume | = 11,034 cuft |
| Inflow hyds. | = 5, 6, 7 | Contrib. drain. area | = 0.834 ac |



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Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

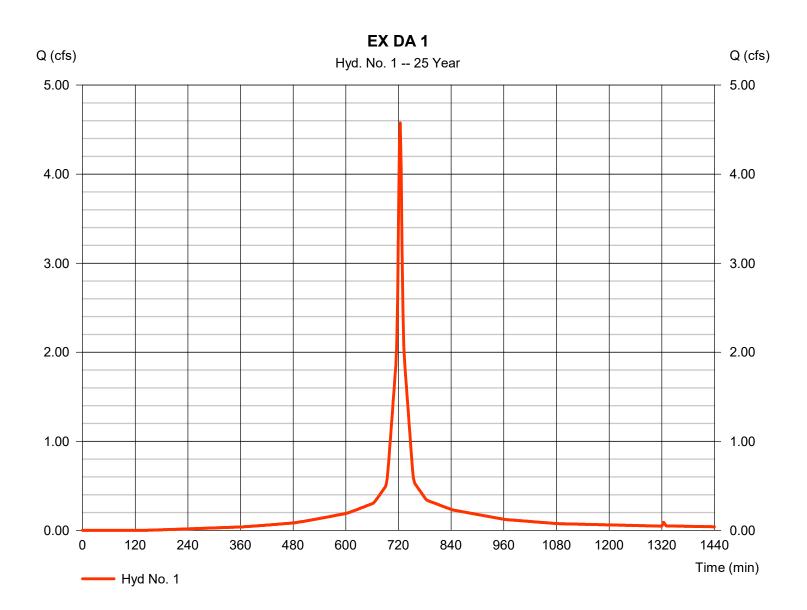
| Hyd. No. | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to Peak (min) | Hyd. volume (cuft) | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph Description |
|-------------|--------------------------------|-----------------------|---------------------------|--------------------------|--------------------------|------------------|------------------------------|-------------------------------|---------------------------|
| 1 | SCS Runoff | 4.575 | 2 | 724 | 14,912 | | | | EX DA 1 |
| 2 | SCS Runoff | 0.167 | 2 | 724 | 503 | | | | EX DA 2 |
| 3 | SCS Runoff | 0.155 | 2 | 724 | 462 | | | | EX DA 3 |
| 4 | Combine | 4.897 | 2 | 724 | 15,877 | 1, 2, 3 | | | Total Existing Drainage |
| 5 | SCS Runoff | 4.408 | 2 | 724 | 13,792 | | | | PROP DA 1 |
| 6 | SCS Runoff | 0.079 | 2 | 724 | 238 | | | | PROP DA 2 |
| 7 | SCS Runoff | 0.138 | 2 | 724 | 413 | | | | PROP DA 3 |
| 8 | Combine | 4.626 | 2 | 724 | 14,443 | 5, 6, 7 | | | Total Proposed Drainage |
| | | | | | | | | | |
| | | | | | | | | | |
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| | | | | | | | | | |
| | | | | | | | | | |
| 202 | 20.06.28 Hyd | raulics De | sign.gpv | | Return F | Period: 25 \ | /ear | Tuesday, 0 | 6 / 30 / 2020 |

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 1

EX DA 1

| Hydrograph type | = SCS Runoff | Peak discharge | = 4.575 cfs |
|-----------------|--------------|--------------------|---------------|
| Storm frequency | = 25 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 14,912 cuft |
| Drainage area | = 0.760 ac | Curve number | = 93 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 6.59 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | | |

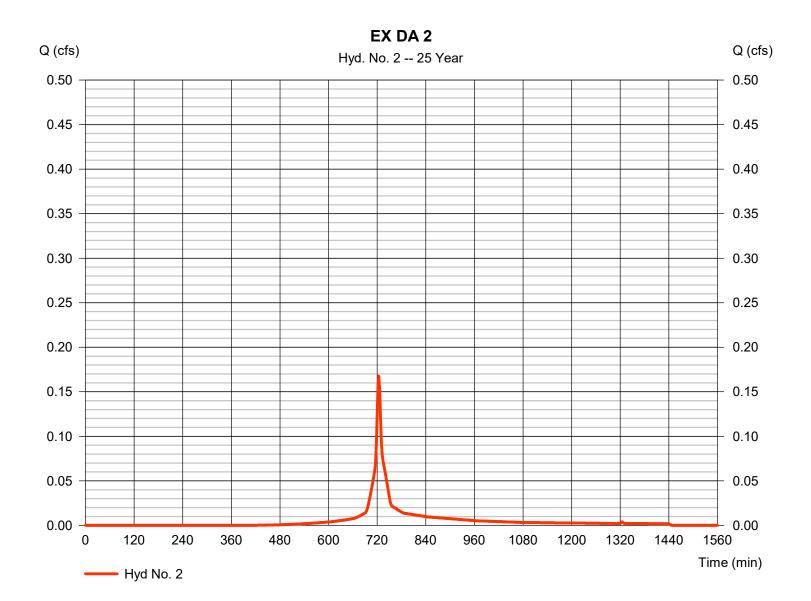


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 2

EX DA 2

| Hydrograph type | = SCS Runoff | Peak discharge | = 0.167 cfs |
|-----------------|--------------|--------------------|-------------|
| Storm frequency | = 25 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 503 cuft |
| Drainage area | = 0.036 ac | Curve number | = 78 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 6.59 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |

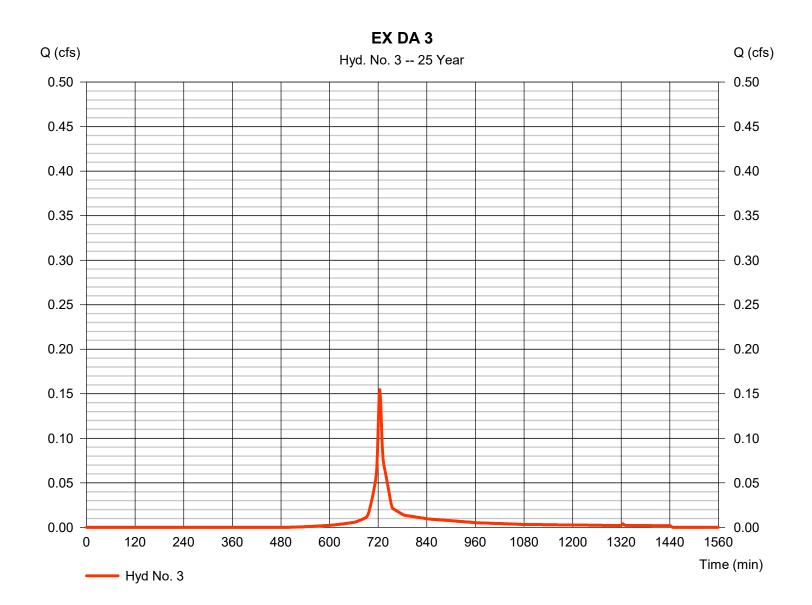


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 3

EX DA 3

| Hydrograph type | = SCS Runoff | Peak discharge | = 0.155 cfs |
|-----------------|--------------|--------------------|-------------|
| Storm frequency | = 25 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 462 cuft |
| Drainage area | = 0.039 ac | Curve number | = 72 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 6.59 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | | |



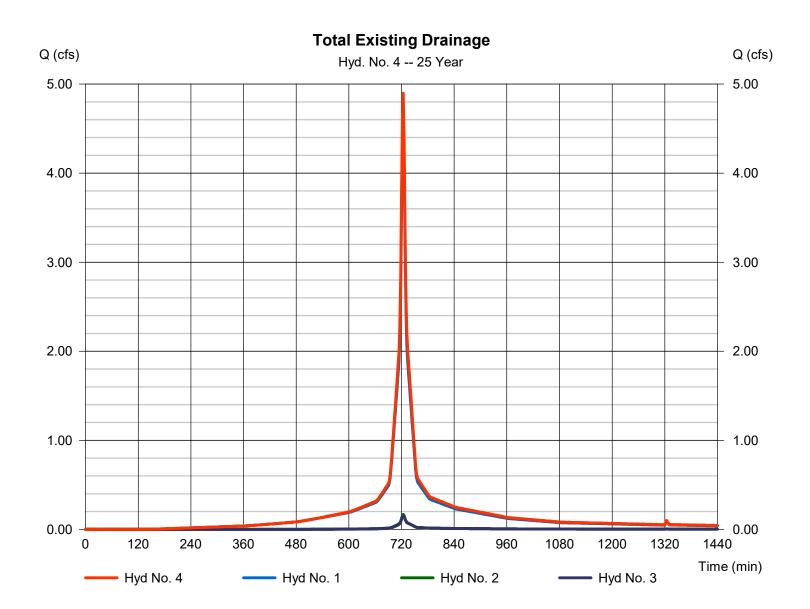
24

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 4

Total Existing Drainage

| Hydrograph type Storm frequency | = Combine = 25 yrs | Peak discharge Time to peak | = 4.897 cfs = 724 min |
|------------------------------------|-----------------------|--------------------------------|--------------------------|
| Time interval | = 2 min | Hyd. volume | = 15,877 cuft |
| Inflow hyds. | = 1, 2, 3 | Contrib. drain. area | = 0.835 ac |



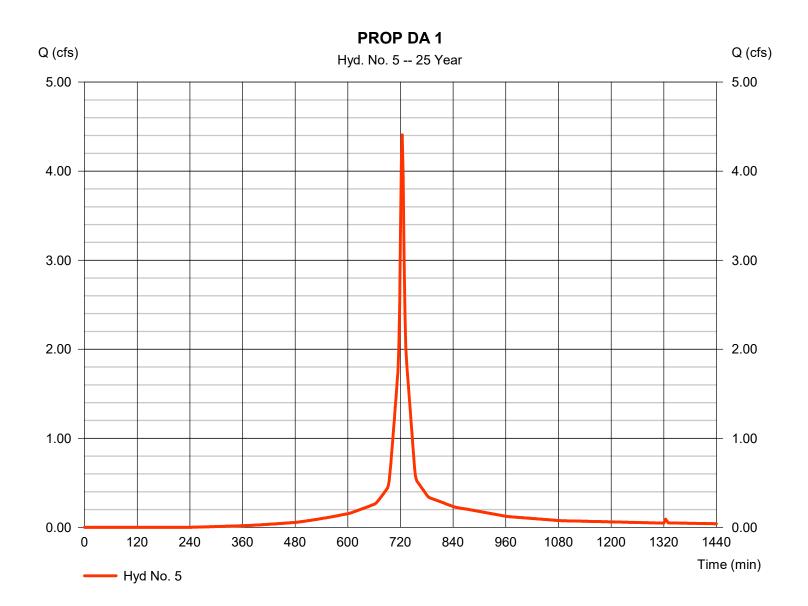
25

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 5

PROP DA 1

| Hydrograph type | = SCS Runoff | Peak discharge | = 4.408 cfs |
|-----------------|--------------|--------------------|---------------|
| Storm frequency | = 25 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 13,792 cuft |
| Drainage area | = 0.780 ac | Curve number | = 88 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 6.59 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | - | |

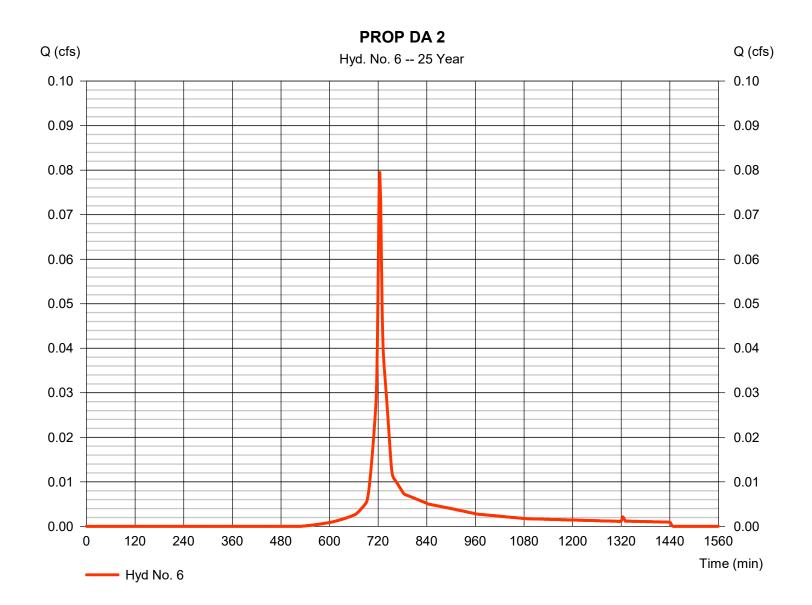


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 6

PROP DA 2

| Hydrograph type | = SCS Runoff | Peak discharge | = 0.079 cfs |
|-----------------|--------------|--------------------|-------------|
| Storm frequency | = 25 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 238 cuft |
| Drainage area | = 0.022 ac | Curve number | = 69 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 6.59 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | | |

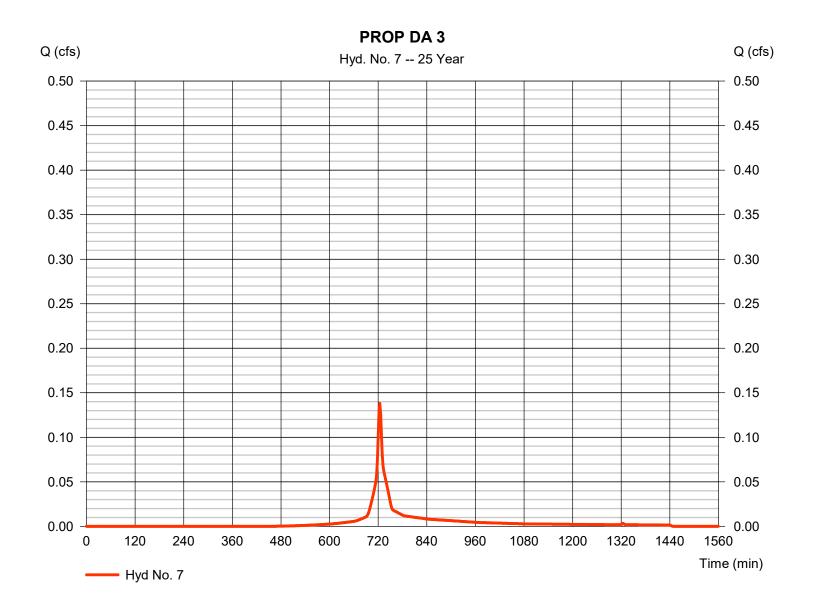


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 7

PROP DA 3

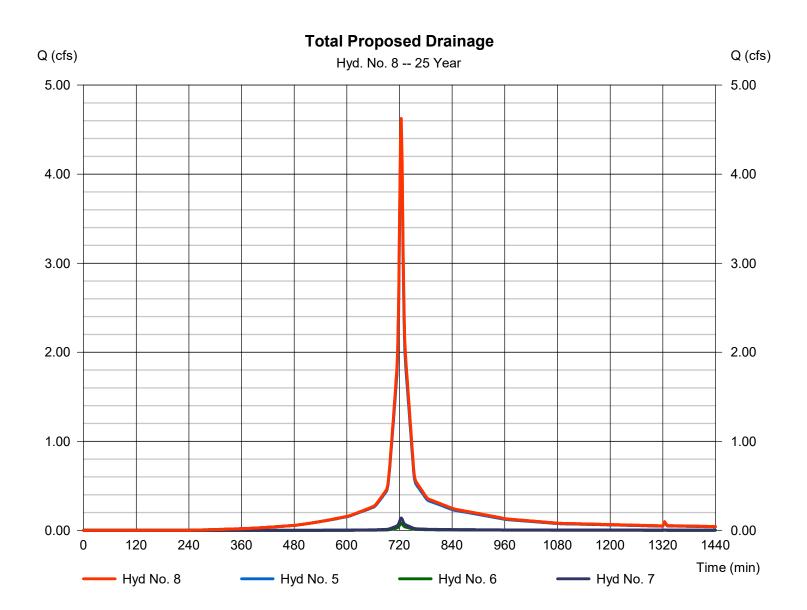
| Hydrograph type | = SCS Runoff | Peak discharge | = 0.138 cfs |
|-----------------|--------------|--------------------|-------------|
| Storm frequency | = 25 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 413 cuft |
| Drainage area | = 0.032 ac | Curve number | = 75 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 6.59 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | | |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 8

Total Proposed Drainage



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

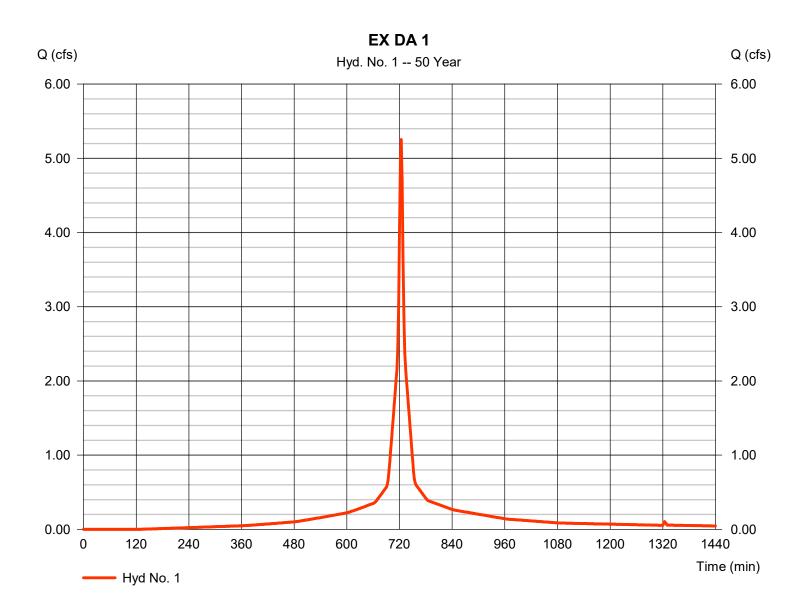
| Hyd. No. | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to Peak (min) | Hyd. volume (cuft) | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph Description |
|-------------|--------------------------------|-----------------------|---------------------------|--------------------------|--------------------------|------------------|------------------------------|-------------------------------|---------------------------|
| 1 | SCS Runoff | 5.253 | 2 | 724 | 17,268 | | | | EX DA 1 |
| 2 | SCS Runoff | 0.200 | 2 | 724 | 605 | | | | EX DA 2 |
| 3 | SCS Runoff | 0.189 | 2 | 724 | 566 | | | | EX DA 3 |
| 4 | Combine | 5.643 | 2 | 724 | 18,440 | 1, 2, 3 | | | Total Existing Drainage |
| 5 | SCS Runoff | 5.120 | 2 | 724 | 16,165 | | | | PROP DA 1 |
| 6 | SCS Runoff | 0.099 | 2 | 724 | 295 | | | | PROP DA 2 |
| 7 | SCS Runoff | 0.167 | 2 | 724 | 501 | | | | PROP DA 3 |
| 8 | Combine | 5.385 | 2 | 724 | 16,961 | 5, 6, 7 | | | Total Proposed Drainage |
| | | | | | | | | | |
| | | | | | | | | | |
| | | | | | | | | | |
| 202 | 20.06.28 Hyd | raulics De | sign.gpv | / | Return F | Period: 50 \ | /ear | Tuesday, 0 | 6 / 30 / 2020 |

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 1

EX DA 1

| Hydrograph type Storm frequency Time interval Drainage area Basin Slope Tc method | = SCS Runoff = 50 yrs = 2 min = 0.760 ac = 0.0 % = User | Peak discharge Time to peak Hyd. volume Curve number Hydraulic length Time of conc. (Tc) | = 5.253 cfs = 724 min = 17,268 cuft = 93 = 0 ft = 6.00 min |
|--|--|---|---|
| | | | • |
| 0 | | | |
| Basin Slope | = 0.0 % | , , | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 7.51 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | | |

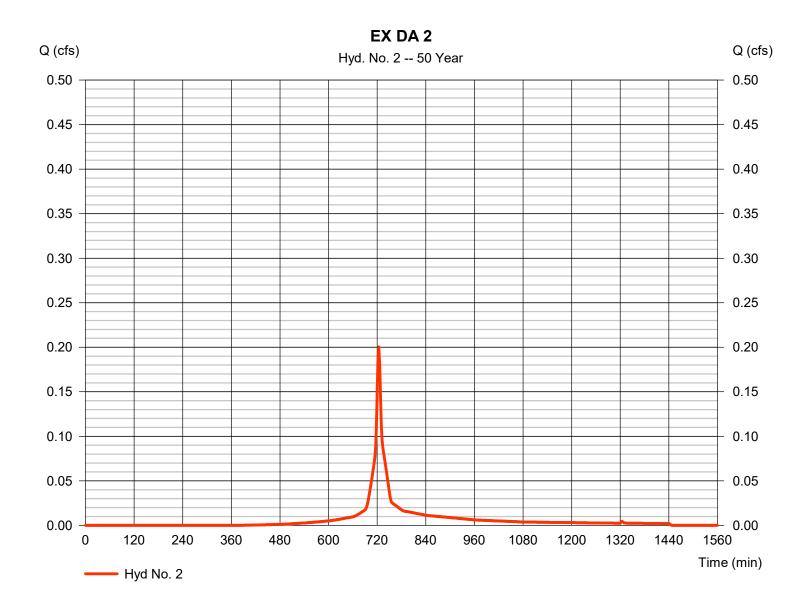


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 2

EX DA 2

| Hydrograph type | = SCS Runoff | Peak discharge | = 0.200 cfs |
|-----------------|--------------|--------------------|-------------|
| Storm frequency | = 50 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 605 cuft |
| Drainage area | = 0.036 ac | Curve number | = 78 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 7.51 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | | |

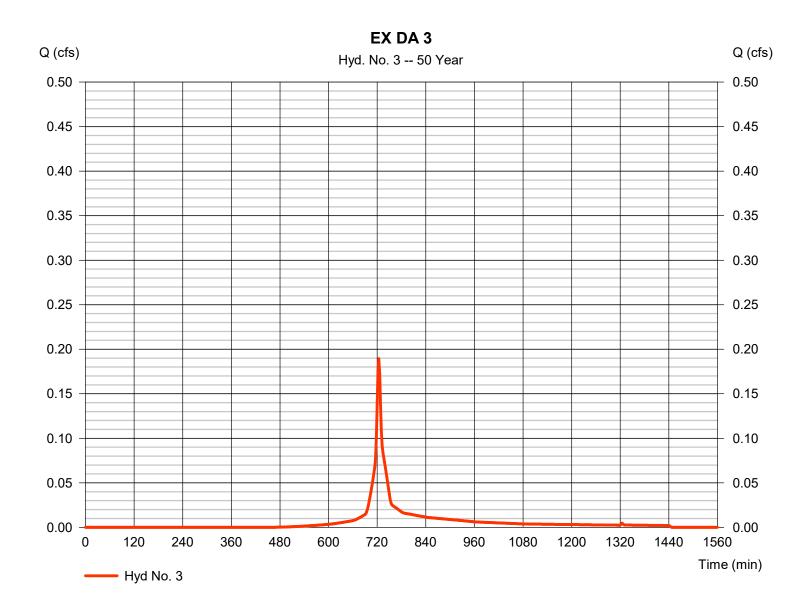


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 3

EX DA 3

| Hydrograph type | = SCS Runoff | Peak discharge | = 0.189 cfs |
|-----------------|--------------|--------------------|-------------|
| Storm frequency | = 50 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 566 cuft |
| Drainage area | = 0.039 ac | Curve number | = 72 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 7.51 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | | |

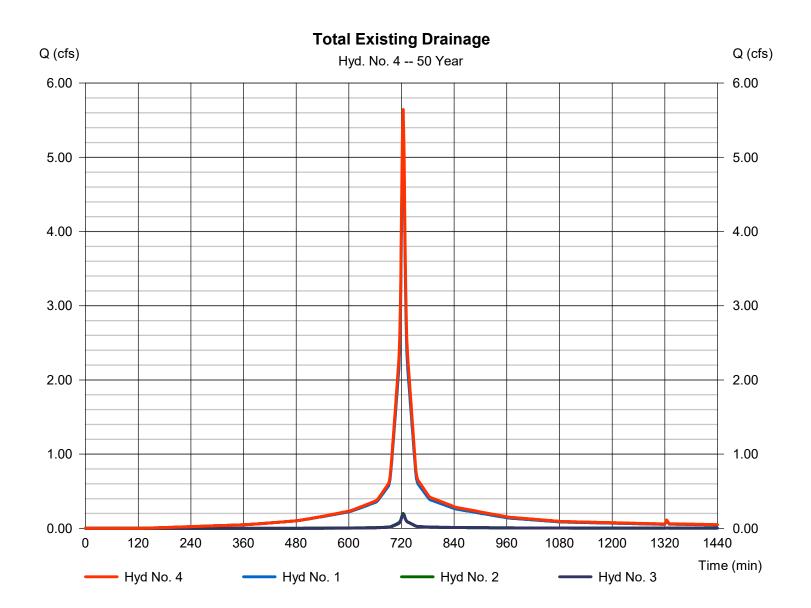


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 4

Total Existing Drainage

| Hydrograph type | = Combine | Peak discharge | = 5.643 cfs |
|-----------------|-----------|----------------------|---------------|
| Storm frequency | = 50 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 18,440 cuft |
| Inflow hyds. | = 1, 2, 3 | Contrib. drain. area | = 0.835 ac |
| innow nyus. | - 1, 2, 3 | Contrib. Grain. area | - 0.055 ac |



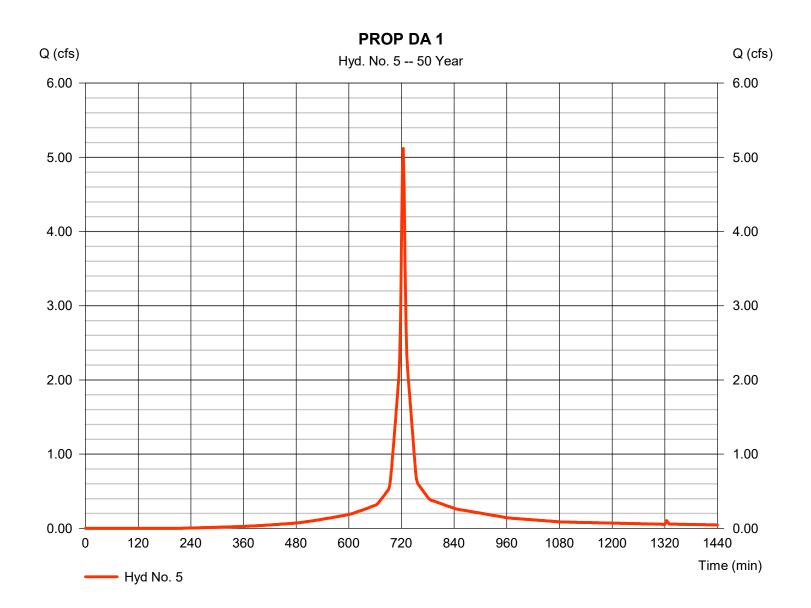
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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 5

PROP DA 1

| Hydrograph type | = SCS Runoff | Peak discharge | = 5.120 cfs |
|-----------------|--------------|--------------------|---------------|
| Storm frequency | = 50 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 16,165 cuft |
| Drainage area | = 0.780 ac | Curve number | = 88 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 7.51 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | | |



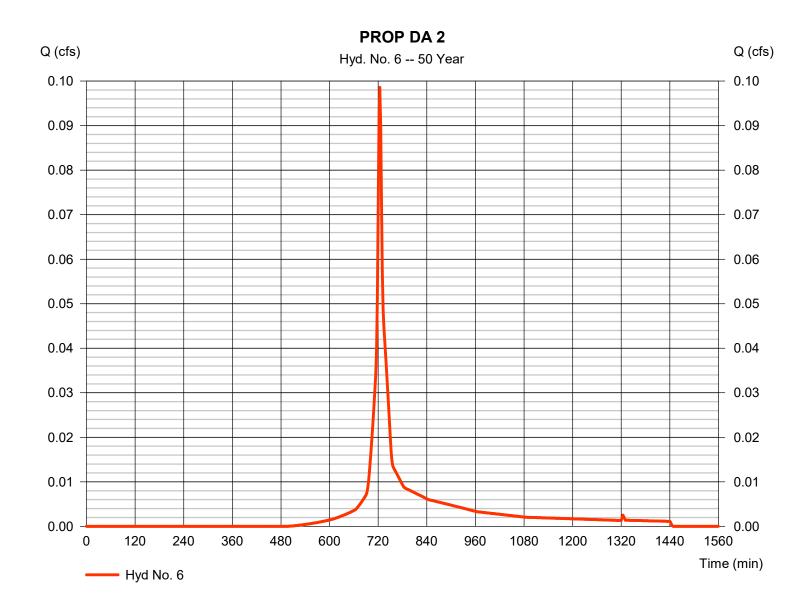
35

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 6

PROP DA 2

| Hydrograph type | = SCS Runoff | Peak discharge | = 0.099 cfs |
|-----------------|--------------|--------------------|-------------|
| Storm frequency | = 50 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 295 cuft |
| Drainage area | = 0.022 ac | Curve number | = 69 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 7.51 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | - | |

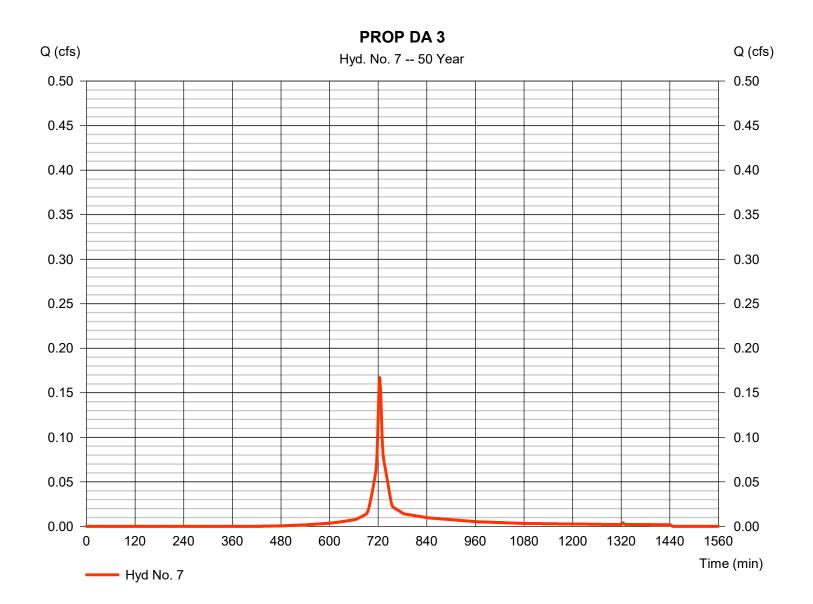


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 7

PROP DA 3

| Hydrograph type | = SCS Runoff | Peak discharge | = 0.167 cfs |
|-----------------|--------------|--------------------|-------------|
| Storm frequency | = 50 yrs | Time to peak | = 724 min |
| Time interval | = 2 min | Hyd. volume | = 501 cuft |
| Drainage area | = 0.032 ac | Curve number | = 75 |
| Basin Slope | = 0.0 % | Hydraulic length | = 0 ft |
| Tc method | = User | Time of conc. (Tc) | = 6.00 min |
| Total precip. | = 7.51 in | Distribution | = Type III |
| Storm duration | = 24 hrs | Shape factor | = 484 |
| | | | |

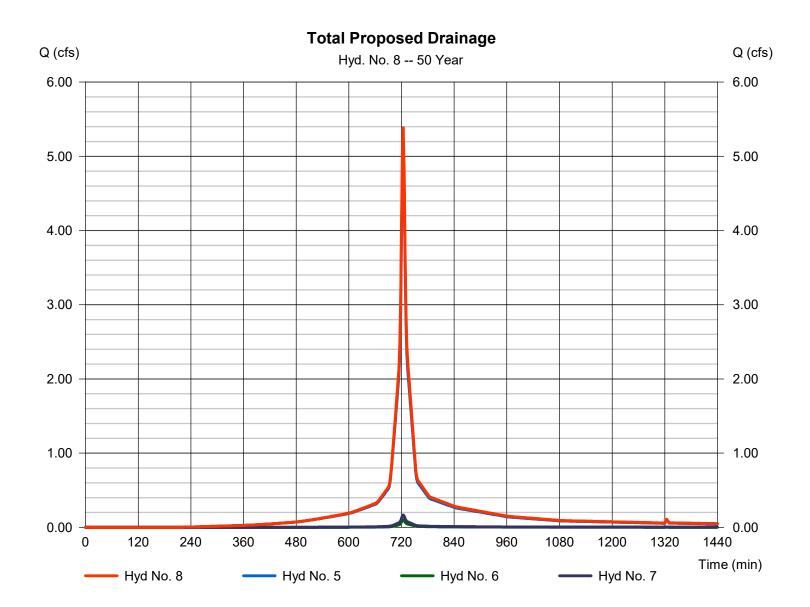


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 8

Total Proposed Drainage

| Hydrograph type | Combine 50 yrs 2 min 5, 6, 7 | Peak discharge | = 5.385 cfs |
|-----------------|---|----------------------|---------------|
| Storm frequency | | Time to peak | = 724 min |
| Time interval | | Hyd. volume | = 16,961 cuft |
| Inflow hyds. | | Contrib. drain. area | = 0.834 ac |
| innett nyde. | 0, 0, 1 | | 0.00140 |



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Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

| Intensity-Duration-Frequency Equation Coefficients (FHA) | | | | | | | | |
|--|---|---|--|--|--|--|--|--|
| В | D | E | (N/A) | | | | | |
| 18.0672 | 3.8000 | 0.7285 | | | | | | |
| 22.0221 | 3.9000 | 0.7320 | | | | | | |
| 0.0000 | 0.0000 | 0.0000 | | | | | | |
| 27.7805 | 3.8000 | 0.7295 | | | | | | |
| 32.0891 | 3.7000 | 0.7233 | | | | | | |
| 39.6397 | 3.8000 | 0.7285 | | | | | | |
| 43.8573 | 3.7000 | 0.7237 | | | | | | |
| 50.1557 | 3.8000 | 0.7284 | | | | | | |
| | B 18.0672 22.0221 0.0000 27.7805 32.0891 39.6397 43.8573 | B D 18.0672 3.8000 22.0221 3.9000 0.0000 0.0000 27.7805 3.8000 32.0891 3.7000 39.6397 3.8000 43.8573 3.7000 | B D E 18.0672 3.8000 0.7285 22.0221 3.9000 0.7320 0.0000 0.0000 0.0000 27.7805 3.8000 0.7295 32.0891 3.7000 0.7285 43.8573 3.7000 0.7237 | | | | | |

File name: Portsmouth NH.IDF

Intensity = B / (Tc + D)^E

| Return | | | | | | | | | | | | | |
|-----------------|-------|------|------|------|------|------|------|------|------|------|------|------|--|
| Period (Yrs) | 5 min | 10 | 15 | 20 | 25 | 30 | 35 | 40 | 45 | 50 | 55 | 60 | |
| 1 | 3.71 | 2.67 | 2.13 | 1.79 | 1.56 | 1.39 | 1.26 | 1.15 | 1.06 | 0.99 | 0.93 | 0.88 | |
| 2 | 4.45 | 3.21 | 2.56 | 2.16 | 1.88 | 1.67 | 1.51 | 1.38 | 1.28 | 1.19 | 1.11 | 1.05 | |
| 3 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5 | 5.69 | 4.09 | 3.27 | 2.75 | 2.39 | 2.13 | 1.93 | 1.76 | 1.63 | 1.52 | 1.42 | 1.34 | |
| 10 | 6.71 | 4.83 | 3.86 | 3.25 | 2.83 | 2.52 | 2.28 | 2.09 | 1.93 | 1.80 | 1.69 | 1.59 | |
| 25 | 8.13 | 5.86 | 4.68 | 3.94 | 3.43 | 3.05 | 2.76 | 2.53 | 2.33 | 2.17 | 2.04 | 1.92 | |
| 50 | 9.17 | 6.60 | 5.27 | 4.44 | 3.86 | 3.44 | 3.11 | 2.85 | 2.64 | 2.46 | 2.30 | 2.17 | |
| 100 | 10.29 | 7.41 | 5.92 | 4.98 | 4.34 | 3.86 | 3.49 | 3.20 | 2.95 | 2.75 | 2.58 | 2.43 | |
| | | | | | | | | | | | | | |

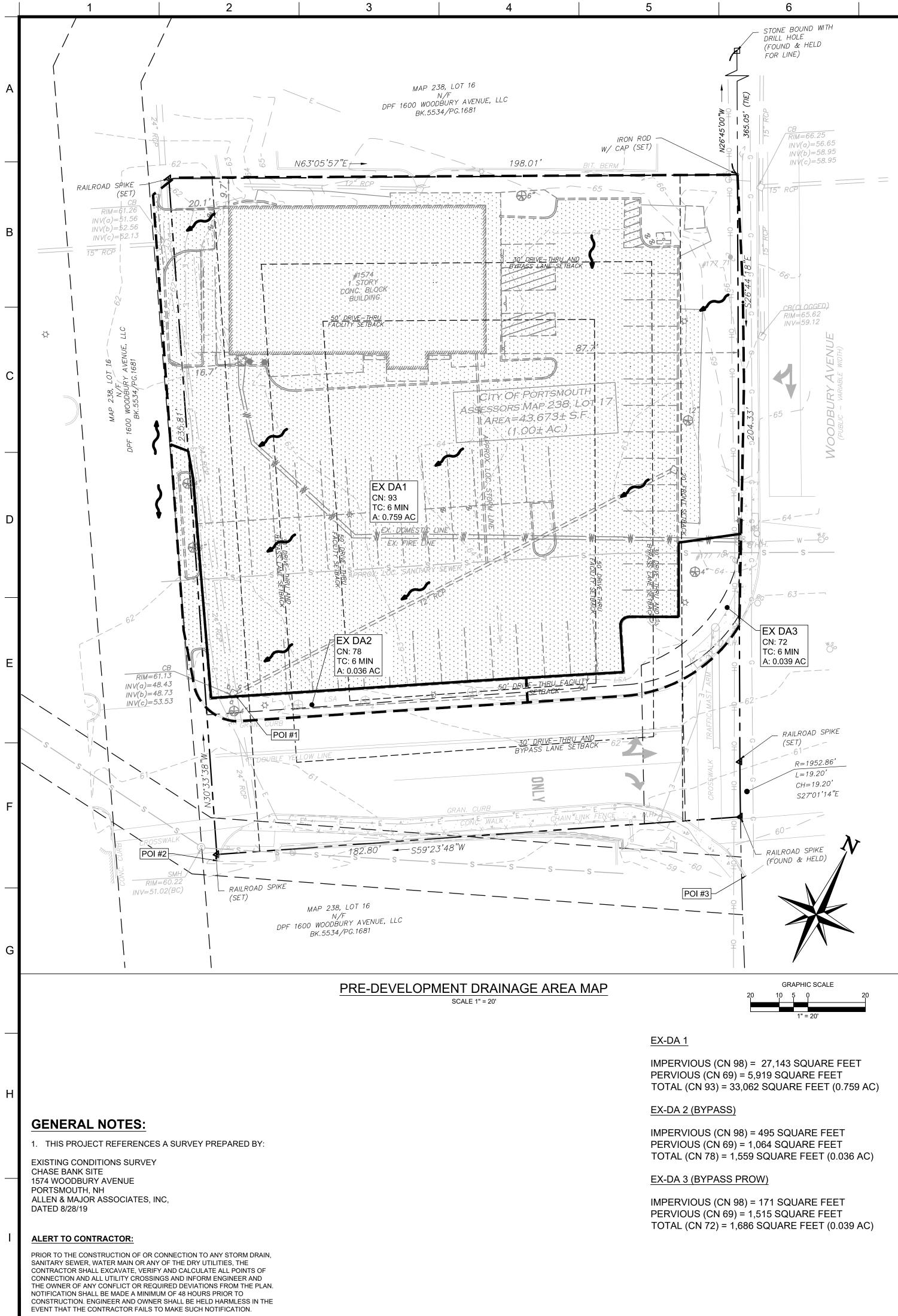
Tc = time in minutes. Values may exceed 60.

| n, NH (1574 | 4 Woodbury | y Avenue) | OVP# 38100P322370 - JPM.27086\CIVIL\Engineering\Stormwater\Portsmouth NH.pcp | |
|-------------|------------|-----------|--|--|
| | | | | |

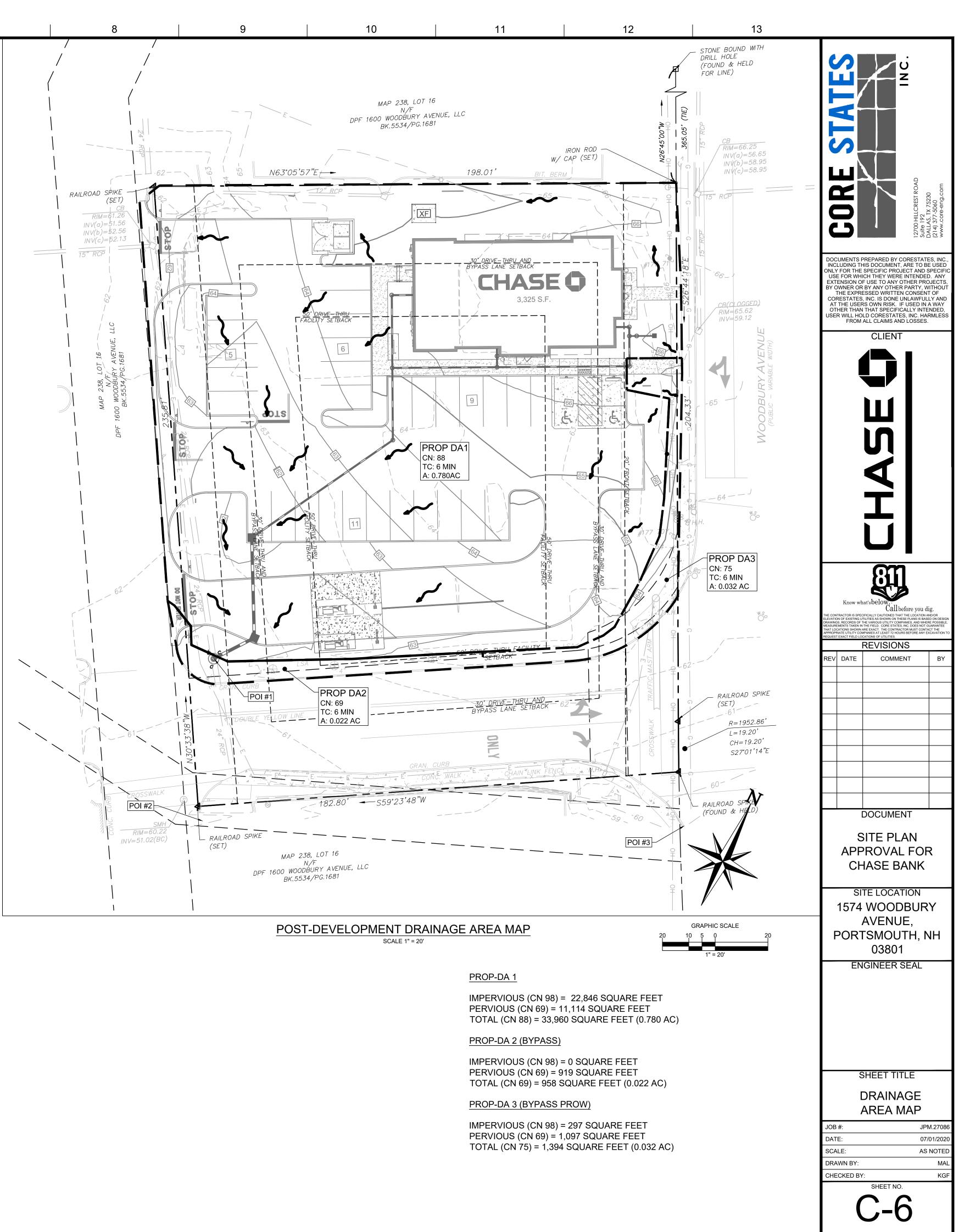
| | Rainfall Precipitation Table (in) | | | | | | | |
|-----------------------|-----------------------------------|------|------|------|-------|-------|-------|--------|
| Storm Distribution | 1-yr | 2-yr | 3-yr | 5-yr | 10-yr | 25-yr | 50-yr | 100-yr |
| SCS 24-hour | 0.00 | 3.32 | 0.00 | 0.00 | 5.33 | 6.59 | 7.51 | 7.28 |
| SCS 6-Hr | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Huff-1st | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Huff-2nd | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Huff-3rd | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Huff-4th | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Huff-Indy | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Custom | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |

APPENDIX D

DRAINAGE AREA MAPS



Plot Date/Time: Jul. 01, 20 - 09:57:04 Drawing: P:\J.P. Morgan Chase\Portsmouth, NH (1574 Woodbury Avenue) OVP# 38100P322370 - JPM.27086\CIVIL\Drawings\Presentation\JPM.27086-P-SITE-NEW.dwg ;C6-DRAIN



<u>APPENDIX E</u>

CDS UNIT SPECIFICATIONS



CDS Design Summary

Chase Bank: WQ Portsmouth, NH

Information Provided:

- Total Contributing Drainage Area = 33,960 sf (0.780 Acres)
- Impervious cover = 22,846 sf (0.524 Acres)
- Design Storm = 1.00" Rainfall
- $T_c = 6$ minutes
- Unit Peak Discharge, qu = 650 cfs/mi²/in

CDS Information:

The CDS technology features a patented non-blocking, indirect screening technique developed to treat stormwater runoff. The unit is highly effective in the capture of suspended solids, fine sands and larger particles. Because of its non-blocking screening capacity, the CDS unit is un-matched in its ability to capture and retain gross pollutants such as trash and debris.

Design Summary:

A CDS 2015-4 was selected for this site. The CDS 2015-4 was sized to treat the 1" first flush of 0.519 cfs, and to remove greater than 80% TSS on an annual basis.

Water Quality Volume (WQV)

| mater Quanty , oran | |
|---------------------|---|
| 0.78 ac | A = Area draining to the practice |
| 0.52 ac | A_{I} = Impervious area draining to the practice |
| 0.67 decimal | I = percent impervious area draining to the practice, in decimal form |
| 0.66 unitless | Rv = Runoff coefficient = 0.05 + (0.9 x I) |
| 0.51 ac-in | WQV = 1" x Rv x A |
| 1,855 cf | WQV conversion (ac-in x 43,560 sf/ac x 1ft/12") |

Water Quality Flow (WQF)

| 1 | inches | P = amount of rainfall. For WQF in NH, $P = 1$ ". | | | | | |
|-------|-------------------------|--|--|--|--|--|--|
| 0.66 | inches | Q = water quality depth. $Q = WQV/A$ | | | | | |
| 96 | unitless | CN = unit peak discharge curve number. CN = 1000/(10+5P+10Q-10*[Q2 + 1.25*Q*P]0.5) | | | | | |
| 0.4 | inches | S = potential maximum retention. $S = (1000/CN) - 10$ | | | | | |
| 0.076 | inches | Ia = initial abstraction. Ia = $0.2S$ | | | | | |
| 6.0 | minutes | $T_c = Time of Concentration$ | | | | | |
| 650.0 | cfs/mi ² /in | qu is the unit peak discharge. Obtain this value from TR-55 exhibits 4-II and 4-III | | | | | |
| 0.519 | cfs | WQF = $q_u x$ WQV. Conversion: to convert "cfs/mi ² /in * ac-in" to "cfs" multiply by $1 \text{mi}^2/640 \text{ac}$ | | | | | |
| | | | | | | | |

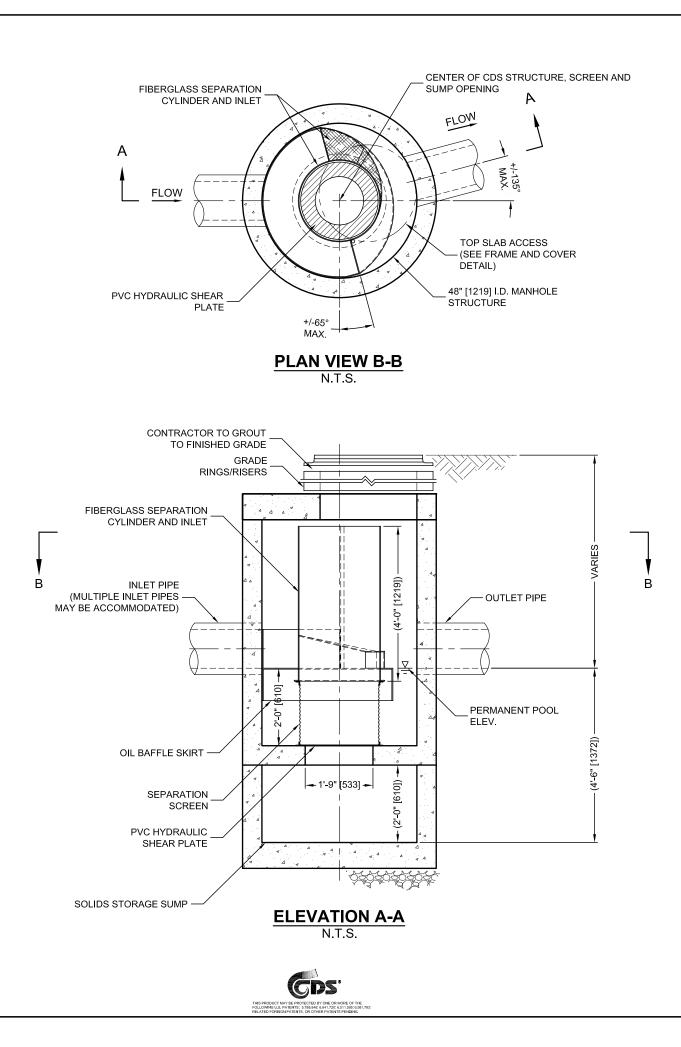
Fig. 1 – BMP Worksheet for WQF



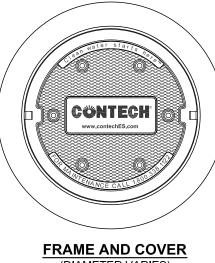


| CDS ESTIMATED NET ANNUAL SOLIDS LOAD REDUCTION BASED ON THE RATIONAL RAINFALL METHOD | | | | | | | |
|---|---------------------------|-------------------|-------------------|--------------------------------|--------------------|--|--|
| Chase Bank Portsmouth, NH | | | | | | | |
| Area0.78 acUnit Site DesignationWQWeighted C0.88Rainfall Station #104t_c6 minCDS Model2015-4CDS Treatment Capacity0.7 cfs | | | | | | | |
| <u>Rainfall</u> Intensity ¹ | Percent Rainfall | <u>Cumulative</u> | Total Flowrate | Treated Flowrate | Incremental | | |
| (in/hr) | <u>Volume¹</u> | Rainfall Volume | <u>(cfs)</u> | <u>(cfs)</u> | <u>Removal (%)</u> | | |
| 0.02 | 13.0% | 13.0% | 0.01 | 0.01 | 12.5 | | |
| 0.04 | 12.2% | 25.2% | 0.03 | 0.03 | 11.7 | | |
| 0.06 | 11.2% | 36.4% | 0.04 | 0.04 | 10.7 | | |
| 0.08 | 10.0% | 46.4% | 0.05 | 0.05 | 9.4 | | |
| 0.10 | 8.2% | 54.6% | 0.07 | 0.07 | 7.7 | | |
| 0.12 | 5.8% | 60.4% | 0.08 | 0.08 | 5.4 | | |
| 0.14 | 6.5% | 66.9% | 0.10 | 0.10 | 6.0 | | |
| 0.16 | 4.6% | 71.5% | 0.11 | 0.11 | 4.3 | | |
| 0.18 | 3.7% | 75.2% | 0.12 | 0.12 | 3.3 | | |
| 0.20 | 3.3% | 78.5% | 0.14 | 0.14 | 3.0 | | |
| 0.25 | 6.7% | 85.2% | 0.17 | 0.17 | 5.9 | | |
| 0.30 | 3.7% | 88.9% | 0.21 | 0.21 | 3.2 | | |
| 0.35 | 2.4% | 91.3% | 0.24 | 0.24 | 2.1 | | |
| 0.40 | 1.8% | 93.1% | 0.27 | 0.27 | 1.5 | | |
| 0.45 | 1.9% | 95.0% | 0.31 | 0.31 | 1.6 | | |
| 0.50 | 1.1% | 96.1% | 0.34 | 0.34 | 0.9 | | |
| 0.75 | 2.6% | 98.7% | 0.51 | 0.51 | 1.9 | | |
| 1.00 | 0.9% | 99.6% | 0.69 | 0.69 | 0.6 | | |
| 1.50 | 0.4% | 100.0% | 1.03 | 0.70 | 0.2 | | |
| 2.00 | 0.0% | 100.0% | 1.37 | 0.70 | 0.0 | | |
| 0.00 | 0.0% | 100.0% | 0.00 | 0.00 | 0.0 | | |
| | | | | | 91.9 | | |
| | | | | ency Adjustment ² = | 6.5% | | |
| | | | Predicted % Annua | al Rainfall Treated = | 93.4% | | |
| Predicted Net Annual Load Removal Efficiency = 85.4% | | | | | | | |

CDS2015-4-C DESIGN NOTES



| THE STANDARD CDS2015-4-C CONFIGURATION IS SHOWN. ALTERNAT CONFIGURATIONS MAY BE COMBINED TO SUIT SITE REQUIREMENTS. |
|--|
| CONFIGURATION DESCRIPTION |
| GRATED INLET ONLY (NO INLET PIPE) |
| GRATED INLET WITH INLET PIPE OR PIPES |
| CURB INLET ONLY (NO INLET PIPE) |
| CURB INLET WITH INLET PIPE OR PIPES |
| SEPARATE OIL BAFFLE (SINGLE INLET PIPE REQUIRED FOR THIS CON |
| SEDIMENT WEIR FOR NJDEP / NJCAT CONFORMING UNITS |



(DIAMETER VARIES) N.T.S.

GENERAL NOTES

- 1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERW
- 2. DIMENSIONS MARKED WITH () ARE REFERENCE DIMENSIONS. AC 3. FOR FABRICATION DRAWINGS WITH DETAILED STRUCTURE DIME SOLUTIONS LLC REPRESENTATIVE. www.contechES.com
- 4. CDS WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND 5. STRUCTURE SHALL MEET AASHTO HS20 AND CASTINGS SHALL MEET HS20 (AASHTO M 306) LOAD RATING, ASSUMING GROUNDWATER ELEVATION
- AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. 6. PVC HYDRAULIC SHEAR PLATE IS PLACED ON SHELF AT BOTTOM OF SCREEN CYLINDER. REMOVE AND REPLACE AS NECESSARY DURING MAINTENANCE CLEANING.

INSTALLATION NOTES

- A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE CDS MANHOLE STRUCTURE В. (LIFTING CLUTCHES PROVIDED).
- CONTRACTOR TO ADD JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS, AND ASSEMBLE STRUCTURE. C.
- D. CONTRACTOR TO PROVIDE, INSTALL, AND GROUT PIPES. MATCH PIPE INVERTS WITH ELEVATIONS SHOWN.
- E. CONTRACTOR TO TAKE APPROPRIATE MEASURES TO ASSURE UNIT IS WATER TIGHT, HOLDING WATER TO FLOWLINE INVERT MINIMUM. IT IS SUGGESTED THAT ALL JOINTS BELOW PIPE INVERTS ARE GROUTED.



NATE CONFIGURATIONS ARE AVAILABLE AND ARE LISTED BELOW. SOME

ONFIGURATION)

| SITE SPECIFIC DATA REQUIREMENTS | | | | | | |
|------------------------------------|-------------|------|-------------|---|---------|--|
| STRUCTURE ID | | | | | | |
| WATER QUALITY | FLOW RAT | E (0 | CFS OR L/s) | | * | |
| PEAK FLOW RAT | E (CFS OR I | _/s) | | | * | |
| RETURN PERIOD | OF PEAK F | LO | W (YRS) | | * | |
| SCREEN APERTL | JRE (2400 C | R 4 | 700) | | * | |
| | | _ | | | 1 | |
| PIPE DATA: | I.E. | 1 | MATERIAL | D | IAMETER | |
| INLET PIPE 1 | * | | * | | * | |
| INLET PIPE 2 | * | | * | | * | |
| OUTLET PIPE | * | | * | | * | |
| | | | | | 1 | |
| RIM ELEVATION | | | | | * | |
| ANTI-FLOTATION | BALLAST | | WIDTH | Т | HEIGHT | |
| | 27 1227 101 | | * | + | * | |
| NOTES/SPECIAL REQUIREMENTS: | | | | | | |
| * PER ENGINEER OF RECORD | | | | | | |

| STRUCTURE ID | | | | | | | |
|--|-----------------------------|-----|----------|---|---------|--|--|
| WATER QUALITY FLOW RATE (CFS OR L/s) * | | | | | | | |
| PEAK FLOW RATE (CFS OR L/s) * | | | | | | | |
| RETURN PERIOD | OF PEAK F | LO | W (YRS) | | * | | |
| SCREEN APERTL | JRE (2400 C | R 4 | 700) | | * | | |
| | | | | | | | |
| PIPE DATA: | I.E. | Ν | MATERIAL | D | IAMETER | | |
| INLET PIPE 1 * * | | | | | * | | |
| INLET PIPE 2 | LET PIPE 2 * * | | | | | | |
| OUTLET PIPE | * | | * | | * | | |
| | | | | | | | |
| RIM ELEVATION | | | | | * | | |
| | | | | | | | |
| ANTI-FLOTATION | BALLAST | | WIDTH | | HEIGHT | | |
| * * | | | | | | | |
| NOTES/SPECIAL | NOTES/SPECIAL REQUIREMENTS: | | | | | | |
| | | | | | | | |
| | | | | | | | |
| | | | | | | | |

CDS2015-4-C

INLINE CDS

STANDARD DETAIL

| ISE. | |
|--|--|
| CTUAL DIMENSIONS MAY VARY. | |
| NSIONS AND WEIGHTS, PLEASE CONTACT YOUR CONTECH ENGINEERED | |
| | |
| TH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING | |



Summary of Underground Utility Locating

Prepared For: Chase Bank - Core States

Prepared By: Peter Kessinger Peter.Kessinger@gprsinc.com Senior Project Manager-New England 603.247.6532 June 11, 2020



June 11, 2020

Chase Bank - Core States Attn: Kevin Furao Site: 1574 Woodbury Ave, Portsmouth, NH

We appreciate the opportunity to provide this report for our work completed on June 11, 2020.

PURPOSE

The purpose of the project was to search for underground utilities within the project boundaries provided by the client. The scope of work consisted of 1 location measuring approximately 1-acre. The client marked the desired locations prior to our scanning and our markings were then placed onto the surface using marking paint.

EQUIPMENT

- Underground Scanning GPR Antenna. The antenna with frequencies ranging from 250 MHz-450 MHz is mounted in a stroller frame which rolls over the surface. The surface needs to be reasonably smooth and unobstructed in order to obtain readable scans. Obstructions such as curbs, landscaping, and vegetation will limit the feasibility of GPR. The data is displayed on a screen and marked in the field in real time. The total depth achieved can be as much as 8' or more with this antenna but can vary widely depending on the types of materials being scanned through. Some soil types such as clay may limit maximum depths to 3' or less. As depth increases, targets must be larger in order to be detected and non-metallic targets can be especially difficult to locate. Depths provided should always be treated as estimates as their accuracy can be affected by multiple factors. For more information, please visit: Link
- Electromagnetic Pipe Locator. The EM locator can passively detect the electromagnetic fields from live AC power or from radio signals travelling along some conductive utilities. It can also be used in conjunction with a transmitter to connect directly to accessible, metallic pipes or tracer wires. A current is sent through the pipe or tracer wire at a specific frequency and the resulting EM field can then be detected by the receiver. A utility's ability to be located depends on a variety of factors including access to the utility, conductivity, grounding, interference from other fields, and many others. Depths provided should always be treated as estimates as their accuracy can be affected by multiple factors. For more information, please visit: Link
- **GPS**. This handheld GPS unit offers accuracy down to 4 inches; however, the accuracy will depend on the satellite environment and obstructions and should not be considered to be survey-grade. Features can be collected as points, lines, or areas and then exported into Google Earth or overlaid on a CAD drawing. For more information, please visit: <u>Link</u>

PROCESS

The process typically begins with using the EM pipe locator to locate pipes or utilities throughout the scan area. First, the transmitter is used to connect to and trace any visible risers, tracer wires, or accessible, conductive utilities provided that there is an exposed, metallic surface. The areas are then swept with the receiver to detect live power or radio frequency signals. Locations and depths are painted or flagged on the surface. Depths cannot always be provided depending on the location method and can be prone to error.

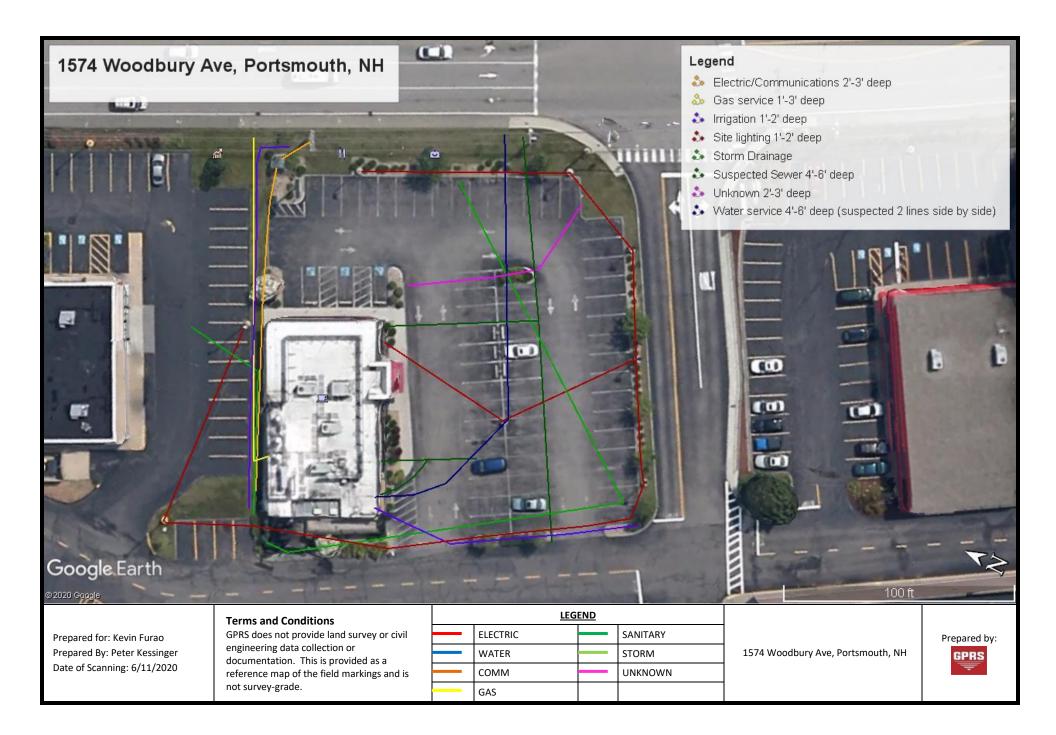
Initial GPR scans were then collected in order to evaluate the data and calibrate the equipment. Based on these findings, a scanning strategy is formed, typically consisting of scanning the entire area in a grid with 2'x2' scan spacing in order to locate any potential utilities that were not found with the pipe locator. The GPR data is viewed in real time and anomalies in the data are located and marked on the surface along with their depths using spray paint, pin flags, etc.

LIMITATIONS

Please keep in mind that there are limitations to any subsurface investigation. The equipment may not achieve maximum effectiveness due to soil conditions, above ground obstructions, reinforced concrete, and a variety of other factors. No subsurface investigation or equipment can provide a complete image of what lies below. Our results should always be used in conjunction with as many methods as possible including consulting existing plans and drawings, exploratory excavation or potholing, visual inspection of above-ground features, and utilization of services such as One Call/811. Depths are dependent on the dielectric of the materials being scanned so depth accuracy can vary throughout a site. Relevant scan examples were saved and will be provided in this report.

FINDINGS

The subsurface conditions at the time of the scanning allowed for maximum GPR depth penetration of 3'-4' in most areas. Multiple utilities were able to be located such as water, gas, electric, communications, etc. using either the GPR or EM pipe locator. The following pages will provide further explanation of the findings.



CLOSING

GPRS, Inc. has been in business since 2001, specializing in underground storage tank location, concrete scanning, utility locating, and shallow void detection for projects throughout the United States. I encourage you to visit our website (<u>www.gprsinc.com</u>) and contact any of the numerous references listed.

GPRS appreciates the opportunity to offer our services, and we look forward to continuing to work with you on future projects. Please feel free to contact us for additional information or with any questions you may have regarding this report.

Signed,

P winger

Peter Kessinger Senior Project Manager—New England



Direct: 603.247.6532 Peter.Kessinger@gprsinc.com

www.gprsinc.com



City of Portsmouth, New Hampshire

Site Plan Application Checklist

This site plan application checklist is a tool designed to assist the applicant in the planning process and for preparing the application for Planning Board review. A pre-application conference with a member of the planning department is strongly encouraged as additional project information may be required depending on the size and scope. The applicant is cautioned that this checklist is only a guide and is not intended to be a complete list of all site plan review requirements. Please refer to the Site Plan review regulations for full details.

Applicant Responsibilities (Section 2.5.2): Applicable fees are due upon application submittal along with required attachments. The application shall be complete as submitted and provide adequate information for evaluation of the proposed site development. <u>Waiver requests must be submitted</u> in writing with appropriate justification.

| Name of Owner/Applicant: Plance Plance | | | 0 | | | |
|--|------|---------|-----------|----------------|------------------------|----|
| Phone Number: _ | | | E-mail: | | | |
| Site Address: | 1514 | WOODBRY | AUQUE | | Map: <u>238</u> Lot: _ | 17 |
| Zoning District: | GI | • | Lot area: | 43,673 sq. ft. | , | |

| | Application Requirements | | |
|---|--|--|---------------------|
| Ø | Required Items for Submittal | Item Location (e.g. Page or Plan Sheet/Note #) | Waiver Requested |
| Ø | Fully executed and signed Application form. (2.5.2.3) | | N/A |
| ত | All application documents, plans, supporting documentation and other materials provided in digital Portable Document Format (PDF). (2.5.2.8) | | N/A |

| | Site Plan Review Application Required Information | | | | | | | |
|---------|--|---|---------------------|--|--|--|--|--|
| Ø | Required Items for Submittal | Item Location (e.g. Page/line or Plan Sheet/Note #) | Waiver Requested | | | | | |
| | Statement that lists and describes "green" building components and systems. (2.5.3.1A) | N/A | | | | | | |
| C | Gross floor area and dimensions of all buildings and statement of uses and floor area for each floor. (2.5.3.1B) | c - 2 | N/A | | | | | |
| Ø | Tax map and lot number, and current zoning of all parcels under Site Plan Review. (2.5.3.1C) | C-Z, SURVEY | N/A | | | | | |
| 2 | Owner's name, address, telephone number, and signature. Name, address, and telephone number of applicant if different from owner. (2.5.3.1D) | cv-1 | N/A | | | | | |

| | Site Plan Review Application Required Info | ormation | |
|---|--|---|---------------------|
| Ø | Required Items for Submittal | Item Location (e.g. Page/line or Plan Sheet/Note #) | Waiver Requested |
| Ø | Names and addresses (including Tax Map and Lot number and zoning districts) of all direct abutting property owners (including properties located across abutting streets) and holders of existing conservation, preservation or agricultural preservation restrictions affecting the subject property. (2.5.3.1E) | SUZUZY PLON | N/A |
| Ø | Names, addresses and telephone numbers of all professionals involved in the site plan design. (2.5.3.1F) | cV-1 | N/A |
| ľ | List of reference plans. (2.5.3.1G) | CV-1 | N/A |
| ত | List of names and contact information of all public or private utilities servicing the site. (2.5.3.1H) | CV-1 | N/A |

| | Site Plan Specifications | | |
|---|---|---|---------------------|
| | Required Items for Submittal | Item Location (e.g. Page/line or Plan Sheet/Note #) | Waiver Requested |
| T | Full size plans shall not be larger than 22 inches by 34 inches with match lines as required, unless approved by the Planning Director. Submittals shall be a minimum of 11 inches by 17 inches as specified by Planning Dept. staff. (2.5.4.1A) | Required on all plan sheets | N/A |
| Ū | Scale: Not less than 1 inch = 60 feet and a graphic bar scale shall be included on all plans. (2.5.4.1B) | Required on all plan sheets | N/A |
| | GIS data should be referenced to the coordinate system New Hampshire State Plane, NAD83 (1996), with units in feet. (2.5.4.1C) | NIA | N/A |
| | Plans shall be drawn to scale. (2.5.4.1D) | Required on all plan sheets | N/A |
| ত | Plans shall be prepared and stamped by a NH licensed civil engineer. (2.5.4.1D) | | N/A |
| | Wetlands shall be delineated by a NH certified wetlands scientist and so stamped. (2.5.4.1E) | NIA | N/A |
| | Title (name of development project), north point, scale, legend. (2.5.4.2A) | SITE PLAN DRWYS | N/A |
| J | Date plans first submitted, date and explanation of revisions. (2.5.4.2B) | ev-1 | N/A |
| V | Individual plan sheet title that clearly describes the information that is displayed. (2.5.4.2C) | Required on all plan sheets | N/A |
| Ø | Source and date of data displayed on the plan. (2.5.4.2D) | | N/A |

Site Plan Application Checklist/April 2019

| Site Plan Specifications | | | |
|---|---|---------------------|--|
| Required Items for Submittal | Item Location (e.g. Page/line or Plan Sheet/Note #) | Waiver Requested | |
| A note shall be provided on the Site Plan stating: "All conditions on this Plan shall remain in effect in perpetuity pursuant to the requirements of the Site Plan Review Regulations." (2.5.4.2E) | 2V-1 | N/A | |
| Plan sheets submitted for recording shall include the following notes: a. "This Site Plan shall be recorded in the Rockingham County Registry of Deeds." b. "All improvements shown on this Site Plan shall be constructed and maintained in accordance with the Plan by the property owner and all future property owners. No changes shall be made to this Site Plan without the express approval of the Portsmouth Planning Director." (2.13.3) | CV-1 | N/A | |
| Plan sheets showing landscaping and screening shall also include the following additional notes: a. "The property owner and all future property owners shall be responsible for the maintenance, repair and replacement of all required screening and landscape materials." b. "All required plant materials shall be tended and maintained in a healthy growing condition, replaced when necessary, and kept free of refuse and debris. All required fences and walls shall be maintained in good repair." c. "The property owner shall be responsible to remove and replace dead or diseased plant materials immediately with the same type, size and quantity of plant materials as originally installed, unless alternative plantings are requested, justified and approved by the Planning Board or Planning Director." | CV-1 | N/A | |

| | Site Plan Specifications – Required Exhibits | | |
|----------|---|---|---------------------|
| M | Required Items for Submittal | Item Location (e.g. Page/line or Plan Sheet/Note #) | Waiver Requested |
| | 1. Existing Conditions: (2.5.4.3A) | | |
| đ | a. Surveyed plan of site showing existing natural and built features; | SURVEY PLAN | |
| B | b. Zoning boundaries; | c-2 | |
| 0Y | c. Dimensional Regulations; | C-2 | |
| B | d. Wetland delineation, wetland function and value assessment; | NID | |
| 3 | e. SFHA, 100-year flood elevation line and BFE data. | NIA | |
| | 2. Buildings and Structures: (2.5.4.3B) | | |
| Ø | Plan view: Use, size, dimensions, footings, overhangs, 1st fl. elevation; | SITE RANS | |
| | Elevations: Height, massing, placement, materials, lighting, façade treatments; | ELOVATION DIZUKIS | - |
| 9 | c. Total Floor Area; | SITE PLOW C-2 | |
| I | d. Number of Usable Floors; | c-2 | |
| 3 | e. Gross floor area by floor and use. | C-2 | |
| | 3. Access and Circulation: (2.5.4.3C) | | |
| 0 | a. Location/width of access ways within site; | c-2 | |
| 3 | Location of curbing, right of ways, edge of pavement and sidewalks; | c-2 | |
| B | Location, type, size and design of traffic signing (pavement markings); | C-2 | |
| ব | d. Names/layout of existing abutting streets; | C-2, SURVERIADO | S |
| 9 | e. Driveway curb cuts for abutting prop. and public roads; | C-2 | |
| 9 | If subdivision; Names of all roads, right of way lines and easements noted; | NIA | |
| B | g. AASHTO truck turning templates, description of minimum vehicle allowed being a WB-50 (unless otherwise approved by TAC). | NLA | |
| | 4. Parking and Loading: (2.5.4.3D) | | |
| đ | a. Location of off street parking/loading areas, landscaped areas/buffers; | c-2 | |
| 3 | b. Parking Calculations (# required and the # provided). | c-2 | |
| | 5. Water Infrastructure: (2.5.4.3E) | | |
| ď | Size, type and location of water mains, shut-offs, hydrants & Engineering data; | C-2 | |
| Ø | b. Location of wells and monitoring wells (include protective radii). | NLA | |
| | 6. Sewer Infrastructure: (2.5.4.3F) | | |
| T | a. Size, type and location of sanitary sewage facilities & Engineering data. | c-2 | |
| | 7. Utilities: (2.5.4.3G) | | |
| 9 | a. The size, type and location of all above & below ground utilities; | C-2 | |
| 9 | b. Size type and location of generator pads, transformers and other fixtures. | NIA | |

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| | Site Plan Specifications – Required Exhibit | ts and Data | |
|----|--|---|---------------------|
| | Required Items for Submittal | Item Location (e.g. Page/line or Plan Sheet/Note #) | Waiver Requested |
| | 8. Solid Waste Facilities: (2.5.4.3H) | | |
| Ø | a. The size, type and location of solid waste facilities. | C-2 | |
| | 9. Storm water Management: (2.5.4.3I) | | |
| 9 | a. The location, elevation and layout of all storm-water drainage. | C-Z, SURVER PLA | J |
| | 10. Outdoor Lighting: (2.5.4.3J) | | |
| I | a. Type and placement of all lighting (exterior of building, parking lot and any other areas of the site) and; b. photometric plan. | C-2, C-7 | |
| ত | Indicate where dark sky friendly lighting measures have been implemented. (10.1) | c-1 | |
| | 12. Landscaping: (2.5.4.3K) | | |
| Ø | Identify all undisturbed area, existing vegetation and that which is to be retained; | C-Z, SITE F | eaus |
| G | b. Location of any irrigation system and water source. | C-2 | |
| | 13. Contours and Elevation: (2.5.4.3L) | | |
| Ø | Existing/Proposed contours (2 foot minimum) and finished grade elevations. | C-3 | |
| | 14. Open Space: (2.5.4.3M) | | |
| Ø | a. Type, extent and location of all existing/proposed open space. | C-7 | |
| e | 15. All easements, deed restrictions and non-public rights of ways. (2.5.4.3N) | C-2 C-2, SURVEY | |
| Q/ | 16. Location of snow storage areas and/or off-site snow removal. (2.5.4.30) | TO BE ADD TOC | 2 |
| Q | 17. Character/Civic District (All following information shall be included): (2.5.4.3Q) | C-2 | |
| | a. Applicable Building Height (10.5A21.20 & 10.5A43.30); | | |
| | b. Applicable Special Requirements (10.5A21.30); | | |
| | c. Proposed building form/type (10.5A43); | | |
| | d. Proposed community space (10.5A46). | | |

| Other Required Information | | | | |
|----------------------------|---|---|---------------------|--|
| | Required Items for Submittal | Item Location (e.g. Page/line or Plan Sheet/Note #) | Waiver Requested | |
| P | Traffic Impact Study or Trip Generation Report, as required. (Four (4) hardcopies of the full study/report and Six (6) summaries to be submitted with the Site Plan Application) (3.2.1-2) | TRIP CIEVERATION | | |
| C | Indicate where Low Impact Development Design practices have been incorporated. (7.1) | c-2 | | |
| 9 | Indicate whether the proposed development is located in a wellhead protection or aquifer protection area. Such determination shall be approved by the Director of the Dept. of Public Works. (7.3.1) | ND | | |
| Ø | Indicate where measures to minimize impervious surfaces have been implemented. (7.4.3) | 2-2 | | |
| Ø | Calculation of the maximum effective impervious surface as a percentage of the site. (7.4.3.2) | C-2 | | |
| V | Stormwater Management and Erosion Control Plan. (Four (4) hardcopies of the full plan/report and Six (6) summaries to be submitted with the Site Plan Application) (7.4.4.1) | c 17 -1 | | |

| | Final Site Plan Approval Required Information | | | |
|---------|--|---|---------------------|--|
| Ø | Required Items for Submittal | Item Location (e.g. Page/line or Plan Sheet/Note #) | Waiver Requested | |
| | All local approvals, permits, easements and licenses required, including but not limited to: a. Waivers; b. Driveway permits; c. Special exceptions; d. Variances granted; e. Easements; f. Licenses. (2.5.3.2A) | | | |
| | Exhibits, data, reports or studies that may have been required as part of the approval process, including but not limited to: a. Calculations relating to stormwater runoff; b. Information on composition and quantity of water demand and wastewater generated; c. Information on air, water or land pollutants to be discharged, including standards, quantity, treatment and/or controls; d. Estimates of traffic generation and counts pre- and post-construction; e. Estimates of noise generation; f. A Stormwater Management and Erosion Control Plan; g. Endangered species and archaeological / historical studies; h. Wetland and water body (coastal and inland) delineations; i. Environmental impact studies. | | | |

| Final Site Plan Approval Required Information | | | |
|---|--|---|---------------------|
| | Required Items for Submittal | Item Location (e.g. Page/line or Plan Sheet/Note #) | Waiver Requested |
| | A document from each of the required private utility service providers indicating approval of the proposed site plan and indicating an ability to provide all required private utilities to the site. (2.5.3.2D) | | |
| | A list of any required state and federal permit applications required for the project and the status of same. (2.5.3.2E) | | |

Applicant's Signature: Mulu & Dom Date: \$1.4 / 2020

Site Plan Application Checklist/April 2019