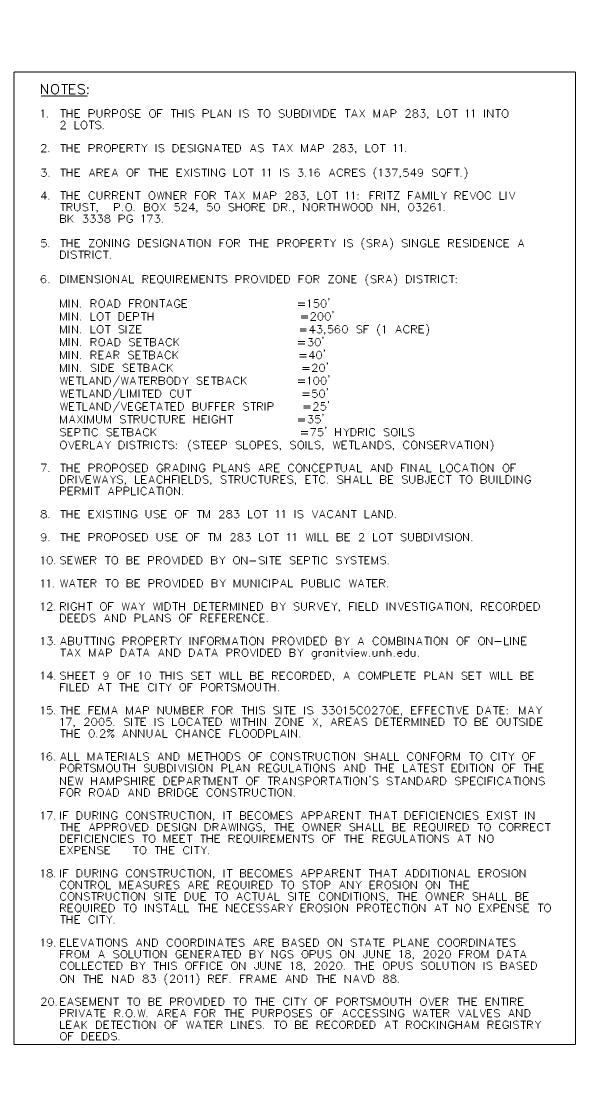
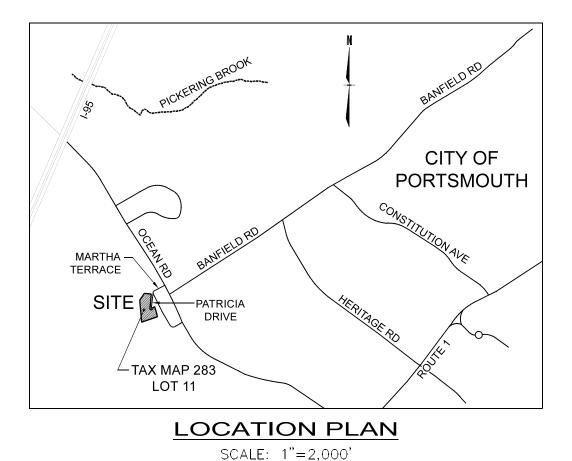
2 LOT SUBDIVISION PLAN FOR

DUBE PLUS CONSTRUCTION

TAX MAP 283, LOT 11

HEMLOCK WAY, PORTSMOUTH, NH 03801 ROCKINGHAM CO.





PROFESSIONAL CONSULTANTS LIST

NEW HAMPSHIRE LAND CONSULTANTS, PLLC. NORTHWOOD, NH 03261 PH:(603) 942-9220

WETLAND/SOIL SCIENTIST

GOVE ENVIRONMENTAL SERVICES, INC. 8 CONTINENTAL DR., BLDG. 2, UNIT H, EXETER, NH 03833 PH: (603) 778-0644

CIVIL ENGINEER

RJB ENGINEERING, LLC 2 GLENDALE ROAD CONCORD, NH 03301

SHEET INDEX

<u>DWG</u>	SHT NO.	<u>DESCRIPTION</u>
CVR	1 OF 10	COVFR SHFFT
ECP	2 OF 10	EXISTING CONDITIONS PLAN
DMP	3 OF 10	DEMOLITION PLAN
PGP	4 OF 10	PROPOSED GRADING PLAN
PDPP	5 OF 10	PROPOSED DRIVEWAY PLAN & PROFILE
PBIP	6 OF 10	PROPOSED BUFFER IMPACT PLAN
PUP	7 OF 10	PROPOSED UTILITY PLAN
PCP	8 OF 10	PROPOSED CONDITIONS PLAN
PSP	9 OF 10	PROPOSED SUBDIVISION
DET	10 OF 10	DETAIL SHEET

Consultants

N.H. LAND

INITIAL PLAN SET SUBMISSION DATE

SEPTEMBER 23, 2020

Latest revision date: JULY 8, 2021

OWNER:

FRITZ FAMILY REVOC LIV TRUST, EDGAR H FRITZ, TRUSTEE P.O. BOX 524, 50 SHORE DR. NORTHWOOD, NH 03261 BK 3338 PG 0173

APPLICANT:

DUBE PLUS CONSTRUCTION, 10 BRICKETTS MILL ROAD, HAMPSTEAD, NH 03841

AGENCY APPROVALS

NHDES SUBDIVISION : ____



CONTACT DIG SAFE 72 HOURS PRIOR TO CONSTRUCTION

THE LOCATION OF ANY UTILITY INFORMATION SHOWN ON THIS PLAN IS APPROXIMATE. NEW HAMPSHIRE LAND CONSULTANTS, PLLC. MAKES NO CLAIM TO THE ACCURACY OR COMPLETENESS OF UTILITIES SHOWN. THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING ANY UTILITIES WHETHER THEY BE ABOVE OR BELOW GROUND. PRIOR TO ANY EXCAVATION ON SITE THE CONTRACTOR SHALL CONTACT DIG SAFE AT 1-888-DIG-SAFE (1-888-344-7233).

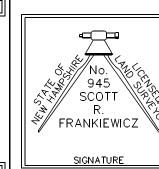
NOTE:

ALL MATERIALS AND METHODS OF CONSTRUCTION SHALL CONFORM TO THE CITY OF PORTSMOUTH REGULATIONS AND THE NEW HAMPSHIRE DEPARTMENT OF TRANSPORTATION "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION", LATEST EDITION.

	REVISIONS						
NO.	DATE	DESCRIPTION	BY				
13	06/29/2021	REVISED PER CITY OF PORTSMOUTH COMMENTS	TDB				
14	07/08/2021	REVISED PER CITY OF PORTSMOUTH COMMENTS	TDB				



, 🕅 Designer Subsurface Disposal Systems *** Scott R. Frankiewicz No. 1348



COVER SHEET TAX MAP 283 LOT 11 **DUBE PLUS CONSTRUCTION**

HEMLOCK WAY, PORTSMOUTH NH 03801 OWNED BY

FRITZ FAMILY REVOC LIV TRUST, EDGAR H FRITZ, TRUSTEE P.O. BOX 524, 50 SHORE DR., NORTHWOOD NH, 03261

BOOK 3338 PAGE 0173

CVR

JOB NO: 258.00

ROCKINGHAM CO.

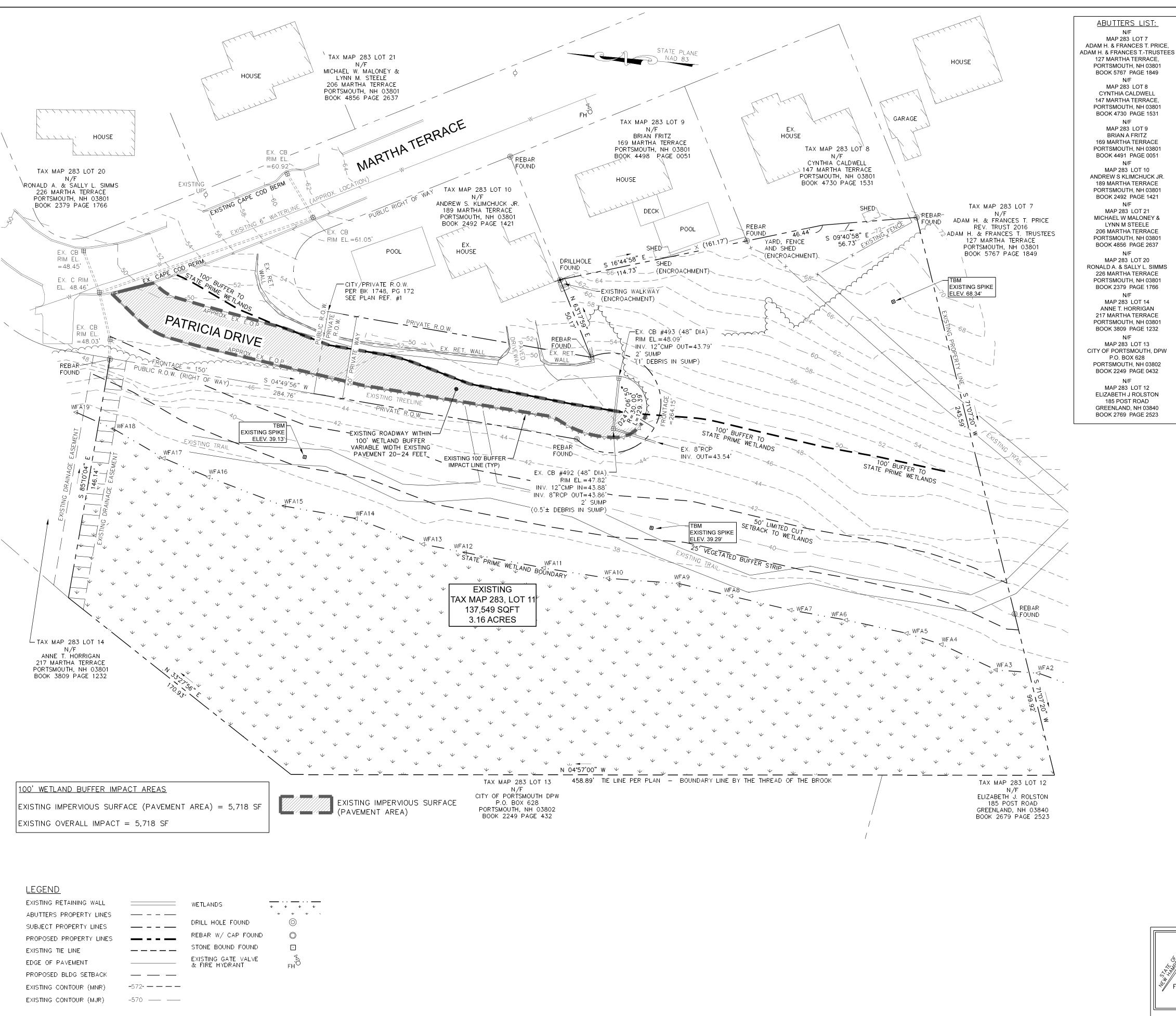
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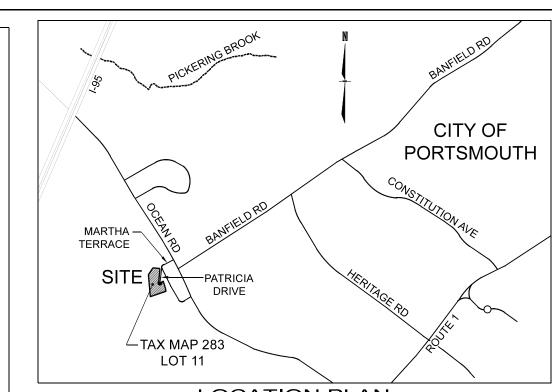
SHT. 1 of 10



683C FIRST NH TURNPIKE, NORTHWOOD, NH 03261 PH. 603-942-9220 WEBSITE: NHLANDCONSULTANTS.COM

SCOTT R. FRANKIEWICZ





LOCATION PLAN SCALE: 1"=2,000'

- THE PURPOSE OF THIS PLAN IS TO SUBDIVIDE TAX MAP 283, LOT 11 INTO
- THE PROPERTY IS DESIGNATED AS TAX MAP 283, LOT 11.
- THE AREA OF THE EXISTING LOT 11 IS 3.16 ACRES (137,549 SQFT.)
- THE CURRENT OWNER FOR TAX MAP 283, LOT 11: FRITZ FAMILY REVOC LIV TRUST, P.O. BOX 524, 50 SHORE DR., NORTHWOOD NH, 03261. BK 3338 PG 173.
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- MIN. ROAD FRONTAGE MIN. LOT DEPTH =200'=43,560 SF (1 ACRE)MIN. LOT SIZE MIN. ROAD SETBACK MIN. REAR SETBACK MIN. SIDE SETBACK
- WETLAND/WATERBODY SETBACK WETLAND / LIMITED CUT WETLAND/VEGETATED BUFFER STRIP MAXIMUM STRUCTURE HEIGHT =75' HYDRIC SOILS SEPTIC SETBACK
- OVERLAY DISTRICTS: (STEEP SLOPES, SOILS, WETLANDS, CONSERVATION)
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- 4. SHEET 9 OF 10 THIS SET WILL BE RECORDED, A COMPLETE PLAN SET WILL BE FILED AT THE CITY OF PORTSMOUTH.
- 5.THE FEMA MAP NUMBER FOR THIS SITE IS 33015C0270E, EFFECTIVE DATE: MAY 17, 2005. SITE IS LOCATED WITHIN ZONE X, AREAS DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOODPLAIN.
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- B. R.C.R.D. PLAN #C8102, RECORDED SEPTEMBER 18, 1978, TITLED: "LOT LINE REVISION, LAND OF LEVESQUE AND GERACI, PORTSMOUTH NH", PREPARED BY: JOHN W. DURGIN ASSOCIATES INC., ENGINEERS, SURVEYORS & DESIGNERS OF PORTSMOUTH AND ROCHESTER, DATED SEPTEMBER 1978, SCALE: 1"=50', APPROVED BY PORTSMOUTH PLANNING BOARD ON SEPTEMBER 18, 1978.
- 4. R.C.R.D. PLAN #D33328, RECORDED DECEMBER 6, 2005, TITLED: "SUBDIVISION AND LOT LINE RELOCATION PLAN, MAP 283 - LOTS 7 & 11", PREPARED FOR: ADAM H. & FRANCES PRICE AND ADAM H. PRICE & FRITZ FAMILY REV. LIVING TRUST, 127 MARTHA TERRACE & PATRICIA DRIVE, PORTSMOUTH NH, PREPARED BY: AMBIT ENGINEERING, INC., CIVIL ENGINEERS & LAND SURVEYORS, PORTSMOUTH NH., SCALE: 1"=50', DATED MARCH 2005, APPROVED BY PORTSMOUTH PLANNING BOARD ON OCTOBER 24, 2005.

SCOTT R. FRANKIEWICZ, LLS



I CERTIFY THAT THIS PLAT IS BASED UPON THE PLAN REFERENCES AND A FIELD SURVEY CONDUCTED ON THE GROUND IN SPRING OF 2020, MEETING THE MINIMUM REQUIREMENTS FOR ACCURACY, 1:10,000 AND COMPLETENESS PER THE STATE OF NEW HAMPSHIRE AND THE CITY OF PORTSMOUTH, NH.

ECP

DATE: SHT. 2 of 10

GRAPHIC

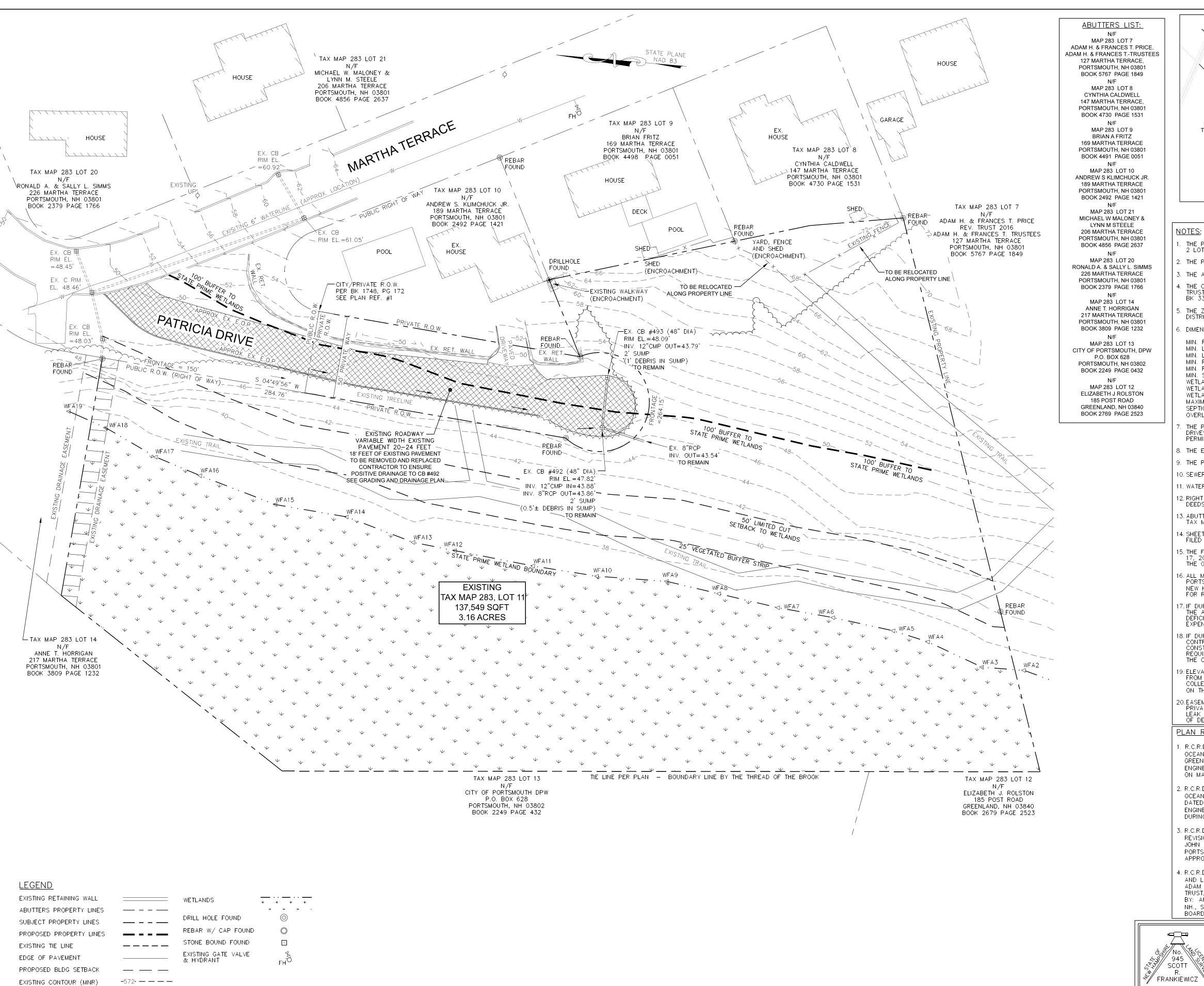
SCALE

SCALE: 1"=30'

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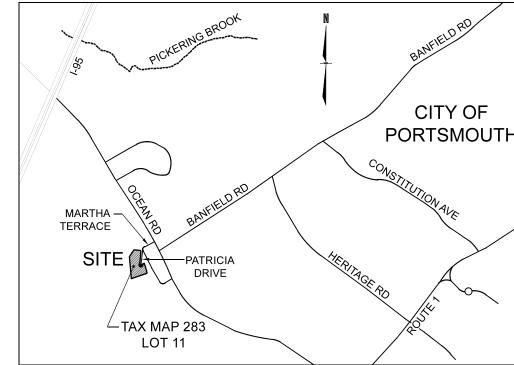
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ROCKINGHAM CO. JOB NO: 258.00 DATE: SEPTEMBER 23, 2020



EXISTING CONTOUR (MJR)

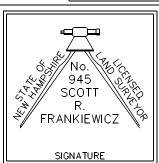
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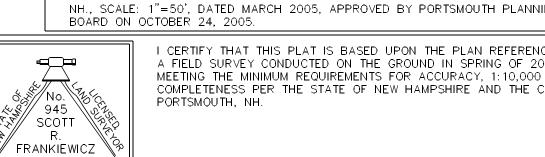
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DATE: SEPTEMBER 23, 2020

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DMP SHT. 3 of 10

R H



SCOTT R. FRANKIEWICZ, LLS

PORTSMOUTH LOCATION PLAN SCALE: 1"=2,000' THE PURPOSE OF THIS PLAN IS TO SUBDIVIDE TAX MAP 283, LOT 11 INTO THE PROPERTY IS DESIGNATED AS TAX MAP 283, LOT 11. THE CURRENT OWNER FOR TAX MAP 283, LOT 11: FRITZ FAMILY REVOC LIV TRUST, P.O. BOX 524, 50 SHORE DR., NORTHWOOD NH, 03261. BK 3338 PG 173.

> **GRAPHIC** SCALE

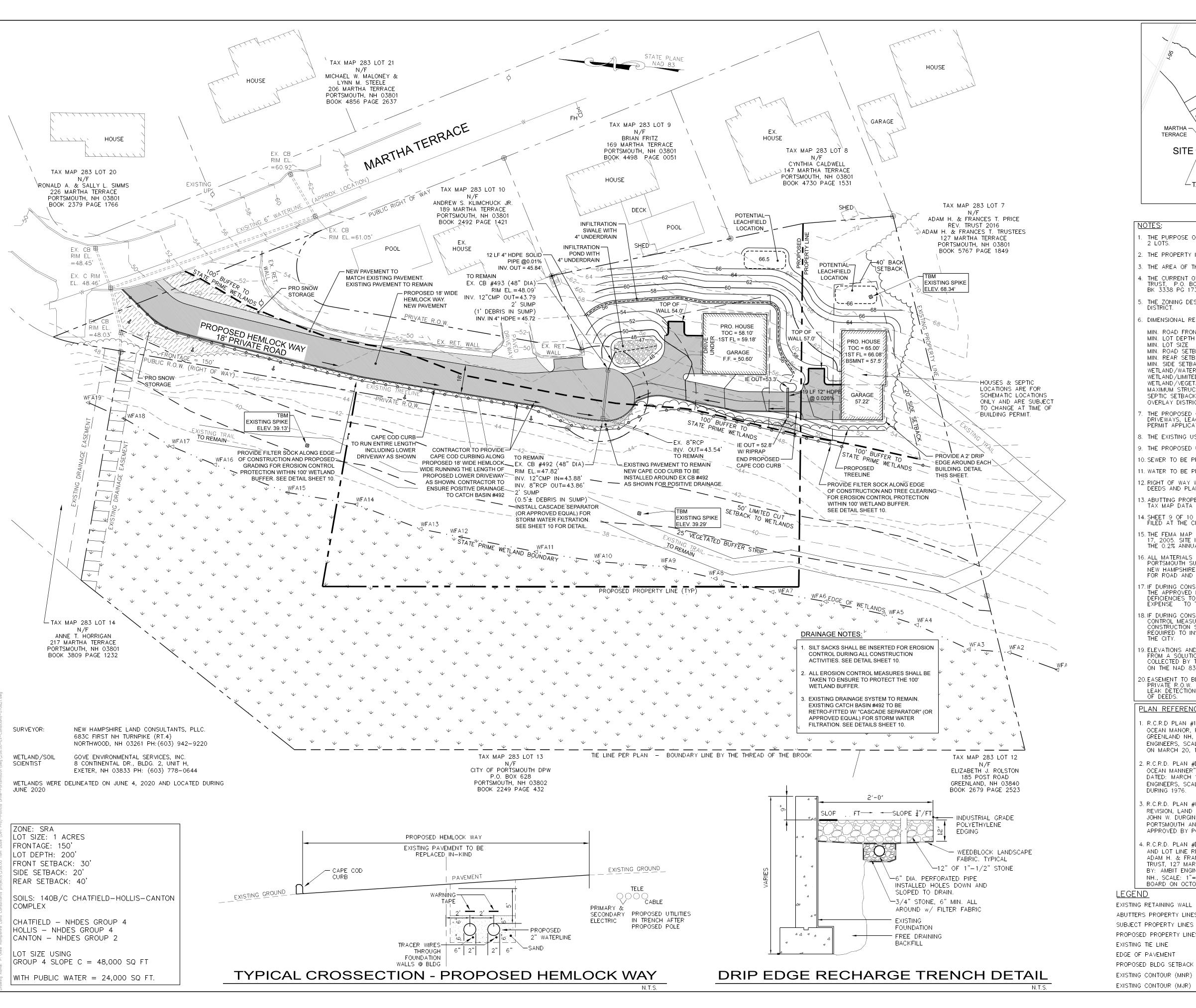
SCALE: 1"=30'

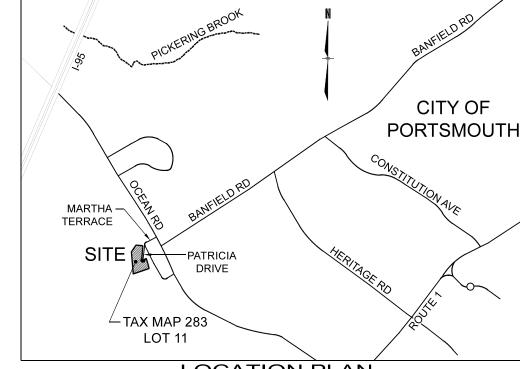




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ROCKINGHAM CO. JOB NO: 258.00





LOCATION PLAN SCALE: 1"=2,000

		=												
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SEPTIC SETBACK

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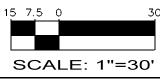
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BUTTERS PROPERTY LINES			ν ν
JBJECT PROPERTY LINES		DRILL HOLE FOUND	\odot
ROPOSED PROPERTY LINES		REBAR W/ CAP FOUND	
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OGE OF PAVEMENT		EXISTING GATE VALVE & HYDRANT	, FH
ROPOSED BLDG SETBACK		ω 111 <u>Β</u> 17/1111	FH
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	ВҮ	TDB	TDB					
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SCALE



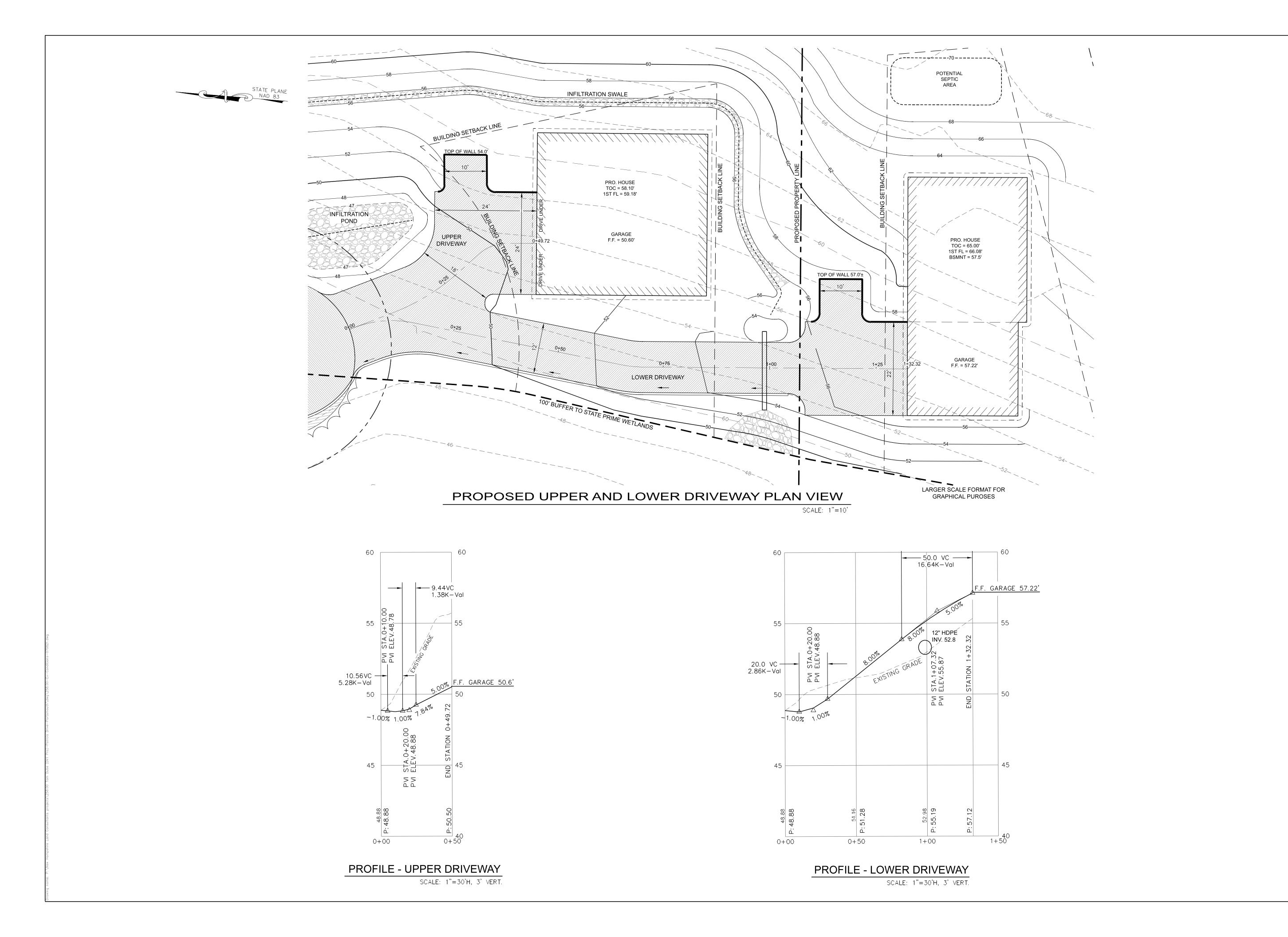




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ROCKINGHAM CO. JOB NO: 258.00 DATE: SEPTEMBER 23, 2020

> **PGP** SHT. 4 of 10



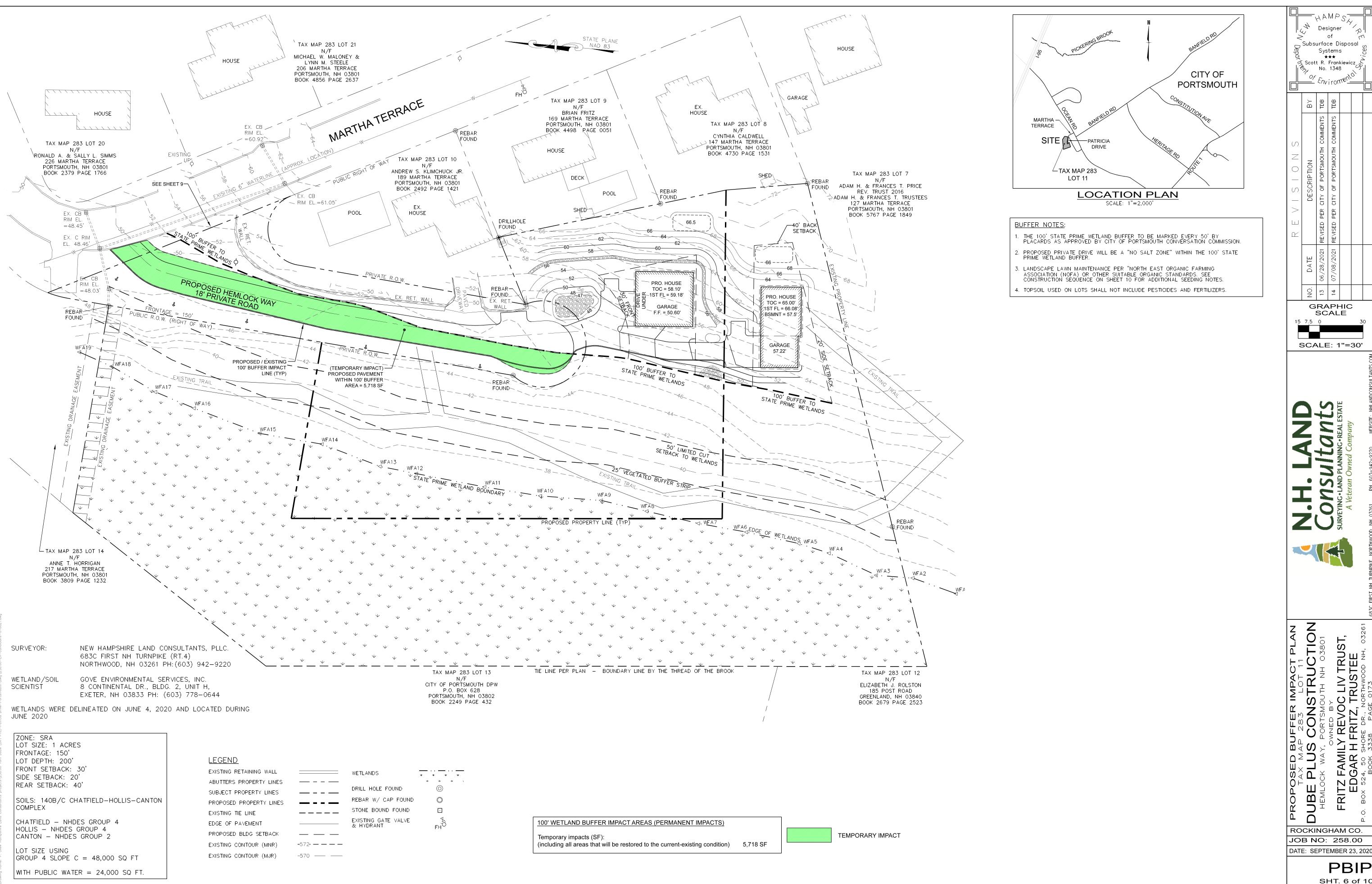
GRAPHIC SCALE AS SHOWN





ROCKINGHAM CO. JOB NO: 258.00 DATE: SEPTEMBER 23, 2020

> PDPP SHT. 5 of 10



, M Designer _ Subsurface Disposal Systems *** 、 ろ Scott R. Frankiewicz No. 1348 <u>—</u>' Envirom

GRAPHIC SCALE

SCALE: 1"=30'





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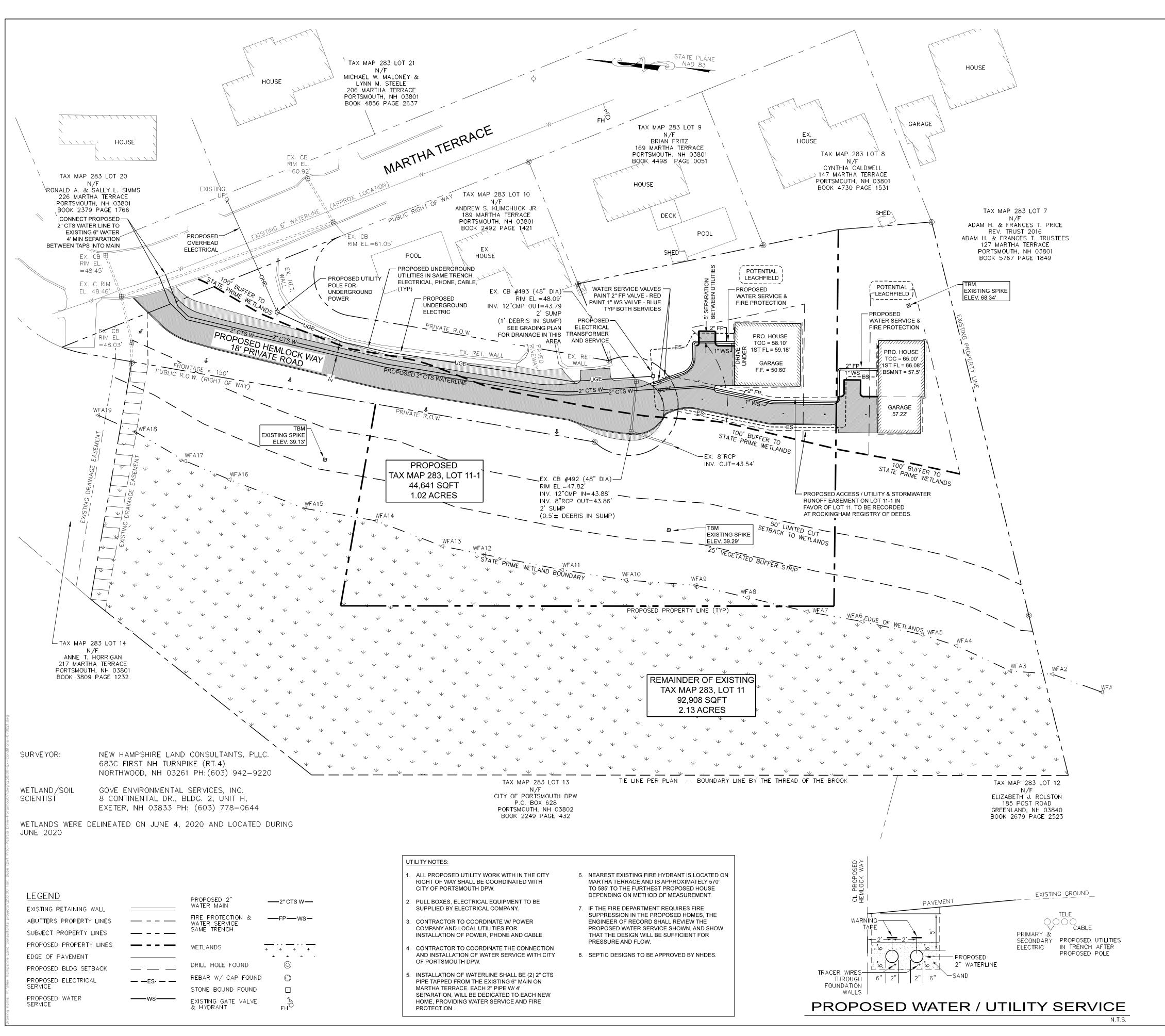
283 LOT 11

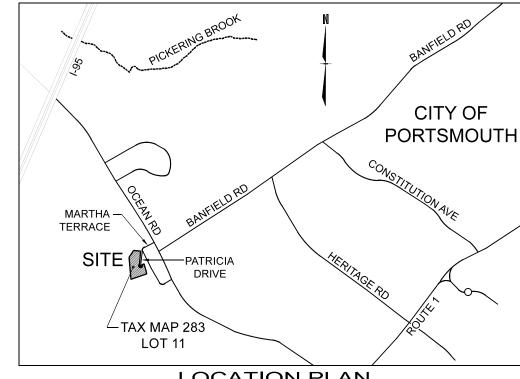
CONSTRUCT

PORTSMOUTH NH 03 **B**

ROCKINGHAM CO. JOB NO: 258.00

> **PBIP** SHT. 6 of 10





LOCATION PLAN SCALE: 1"=2,000'

NOTES:

- 1. THE PURPOSE OF THIS PLAN IS TO SUBDIVIDE TAX MAP 283, LOT 11 INTO
- 2. THE PROPERTY IS DESIGNATED AS TAX MAP 283, LOT 11.
- 3. THE AREA OF THE EXISTING LOT 11 IS 3.16 ACRES (137,549 SQFT.)
- 4. THE CURRENT OWNER FOR TAX MAP 283, LOT 11: FRITZ FAMILY REVOC LIV TRUST, P.O. BOX 524, 50 SHORE DR., NORTHWOOD NH, 03261. BK 3338 PG 173.
- 5. THE ZONING DESIGNATION FOR THE PROPERTY IS (SRA) SINGLE RESIDENCE A DISTRICT.
- 6. DIMENSIONAL REQUIREMENTS PROVIDED FOR ZONE (SRA) DISTRICT:
- MIN. ROAD FRONTAGE =150'

 MIN. LOT DEPTH =200'

 MIN. LOT SIZE =43,560 SF (1 ACRE)

 MIN. ROAD SETBACK =30'

 MIN. REAR SETBACK =40'

 MIN. SIDE SETBACK =20'

 WETLAND/WATERBODY SETBACK =100'

 WETLAND/LIMITED CUT =50'

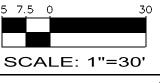
 WETLAND/VEGETATED BUFFER STRIP =25'
- MAXIMUM STRUCTURE HEIGHT = 35'
 SEPTIC SETBACK = 75' HYDRIC SOILS
 OVERLAY DISTRICTS: (STEEP SLOPES, SOILS, WETLANDS, CONSERVATION)
- . THE PROPOSED GRADING PLANS ARE CONCEPTUAL AND FINAL LOCATION OF DRIVEWAYS, LEACHFIELDS, STRUCTURES, ETC. SHALL BE SUBJECT TO BUILDING PERMIT APPLICATION.
- B. THE EXISTING USE OF TM 283 LOT 11 IS VACANT LAND.
- 9. THE PROPOSED USE OF TM 283 LOT 11 WILL BE 2 LOT SUBDIVISION.
- 10. SEWER TO BE PROVIDED BY ON-SITE SEPTIC SYSTEMS.
- 11. WATER TO BE PROVIDED BY MUNICIPAL PUBLIC WATER.
- 12. RIGHT OF WAY WIDTH DETERMINED BY SURVEY, FIELD INVESTIGATION, RECORDED DEEDS AND PLANS OF REFERENCE.
- 13. ABUTTING PROPERTY INFORMATION PROVIDED BY A COMBINATION OF ON-LINE TAX MAP DATA AND DATA PROVIDED BY granitview.unh.edu.
- 14.SHEET 9 OF 10 THIS SET WILL BE RECORDED, A COMPLETE PLAN SET WILL BE FILED AT THE CITY OF PORTSMOUTH.
- 15. THE FEMA MAP NUMBER FOR THIS SITE IS 33015C0270E, EFFECTIVE DATE: MAY 17, 2005. SITE IS LOCATED WITHIN ZONE X, AREAS DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOODPLAIN.
- I6. ALL MATERIALS AND METHODS OF CONSTRUCTION SHALL CONFORM TO CITY OF PORTSMOUTH SUBDIVISION PLAN REGULATIONS AND THE LATEST EDITION OF THE NEW HAMPSHIRE DEPARTMENT OF TRANSPORTATION'S STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION.
- 7. IF DURING CONSTRUCTION, IT BECOMES APPARENT THAT DEFICIENCIES EXIST IN THE APPROVED DESIGN DRAWINGS, THE OWNER SHALL BE REQUIRED TO CORRECT DEFICIENCIES TO MEET THE REQUIREMENTS OF THE REGULATIONS AT NO EXPENSE TO THE CITY.
- 18. IF DURING CONSTRUCTION, IT BECOMES APPARENT THAT ADDITIONAL EROSION CONTROL MEASURES ARE REQUIRED TO STOP ANY EROSION ON THE CONSTRUCTION SITE DUE TO ACTUAL SITE CONDITIONS, THE OWNER SHALL BE REQUIRED TO INSTALL THE NECESSARY EROSION PROTECTION AT NO EXPENSE TO THE CITY.
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- O.EASEMENT TO BE PROVIDED TO THE CITY OF PORTSMOUTH OVER THE ENTIRE PRIVATE R.O.W. AREA FOR THE PURPOSES OF ACCESSING WATER VALVES AND LEAK DETECTION OF WATER LINES. TO BE RECORDED AT ROCKINGHAM REGISTRY OF DEEDS.

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- 3. R.C.R.D. PLAN #C8102, RECORDED SEPTEMBER 18, 1978, TITLED: "LOT LINE REVISION, LAND OF LEVESQUE AND GERACI, PORTSMOUTH NH", PREPARED BY: JOHN W. DURGIN ASSOCIATES INC., ENGINEERS, SURVEYORS & DESIGNERS OF PORTSMOUTH AND ROCHESTER, DATED SEPTEMBER 1978, SCALE: 1"=50', APPROVED BY PORTSMOUTH PLANNING BOARD ON SEPTEMBER 18, 1978.
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	ВХ	TDB	8QL		
N Z O - N - > H Z	DESCRIPTION	13 06/28/2021 REVISED PER CITY OF PORTSMOUTH COMMENTS TDB	14 07/08/2021 REVISED PER CITY OF PORTSMOUTH COMMENTS		
	DATE	06/28/2021	07/08/2021		
	NO.	13	14		

GRAPHIC SCALE







PROPOSED UTILITY PLAN
TAX MAP 283 LOT 11

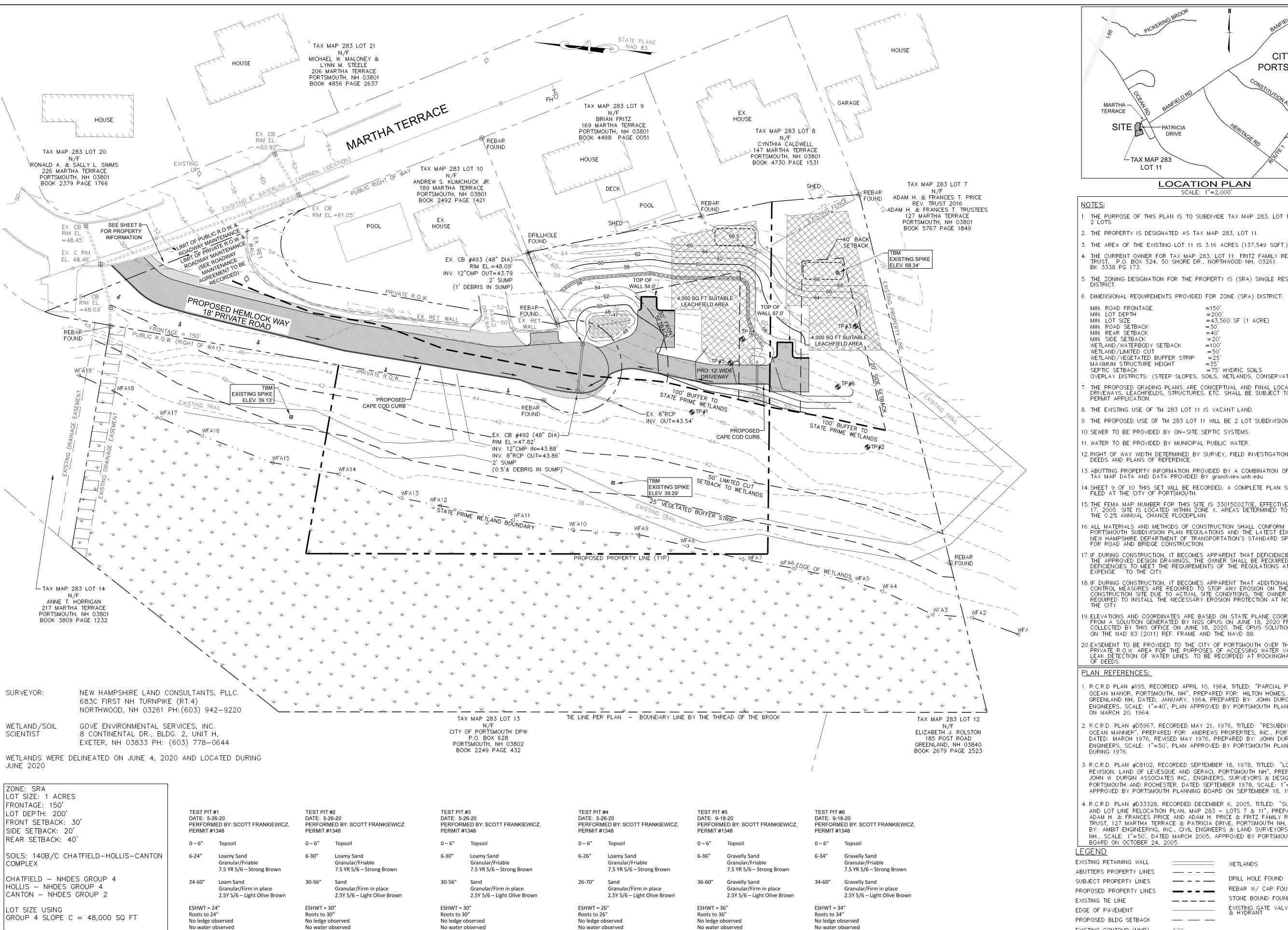
BE PLUS CONSTRUCTION
EMLOCK WAY, PORTSMOUTH NH 03801
OWNED BY
RITZ FAMILY REVOC LIV TRUST,
EDGAR H FRITZ, TRUSTEE

ROCKINGHAM CO.

JOB NO: 258.00

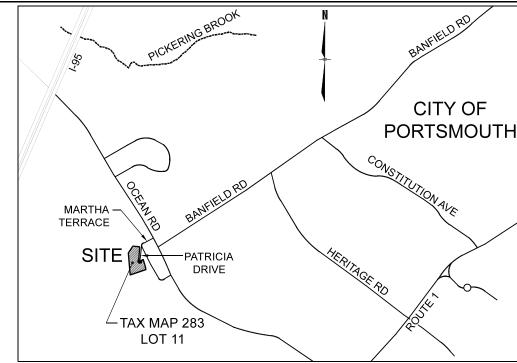
DATE: SEPTEMBER 23, 2020

PUP



|WITH PUBLIC WATER = 24,000 SQ FT.

Many stones throughout hole



LOCATION PLAN SCALE: 1"=2,000"

THE PURPOSE OF THIS PLAN IS TO SUBDIVIDE TAX MAP 283, LOT 11 INTO

- 2. THE PROPERTY IS DESIGNATED AS TAX MAP 283, LOT 11.
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 BK 3338 PG 173.
- . THE ZONING DESIGNATION FOR THE PROPERTY IS (SRA) SINGLE RESIDENCE A DISTRICT.

=200'

=43,560 SF (1 ACRE)

- . DIMENSIONAL REQUIREMENTS PROVIDED FOR ZONE (SRA) DISTRICT
- MIN. ROAD FRONTAGE MIN. LOT DEPTH
- MIN. LOT SIZE MIN. ROAD SETBACK
- MIN. REAR SETBACK
- MIN. SIDE SETBACK WETLAND/WATERBODY SETBACK
- WETLAND/LIMITED CUT
- WETLAND/VEGETATED BUFFER STRIP MAXIMUM' STRUCTURE HEIGHT SEPTIC SETBACK
- =75' HYDRIC SOILS OVERLAY DISTRICTS: (STEEP SLOPES, SOILS, WETLANDS, CONSERVATION)
- THE PROPOSED GRADING PLANS ARE CONCEPTUAL AND FINAL LOCATION OF DRIVEWAYS, LEACHFIELDS, STRUCTURES, ETC. SHALL BE SUBJECT TO BUILDING PERMIT APPLICATION.
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<u>_E</u>	GE	N	D

<u>LEGEND</u>			
EXISTING RETAINING WALL		WETLANDS	<u>Ψ</u> ···
ABUTTERS PROPERTY LINES			Ψ.
SUBJECT PROPERTY LINES		DRILL HOLE FOUND	(
PROPOSED PROPERTY LINES		REBAR W/ CAP FOUND	(
EXISTING TIE LINE		STONE BOUND FOUND	
EDGE OF PAVEMENT		EXISTING GATE VALVE & HYDRANT	_
PROPOSED BLDG SETBACK		& IIIBIAAA	F
EXISTING CONTOUR (MNR)	-572 - — — — —		

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	BY	TDB	TDB		
REVISIONS	DESCRIPTION	13 06/28/2021 REVISED PER CITY OF PORTSMOUTH COMMENTS TDB	14 07/08/2021 REVISED PER CITY OF PORTSMOUTH COMMENTS TDB		
	DATE	06/28/2021	07/08/2021		
	Ö N	13	14		

, 🖒 Designer

, Subsurface Disposal

Systems

GRAPHIC SCALE

SCALE: 1"=30'

1. WATER TO BE PROVIDED BY MUNICIPAL PUBLIC WATER.

- DEEDS AND PLANS OF REFERENCE.

- S. ALL MATERIALS AND METHODS OF CONSTRUCTION SHALL CONFORM TO CITY OF PORTSMOUTH SUBDIVISION PLAN REGULATIONS AND THE LATEST EDITION OF THE
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\setminus	<u>D</u>	
G	RETAINING	WALL

EXISTING CONTOUR (MJR)

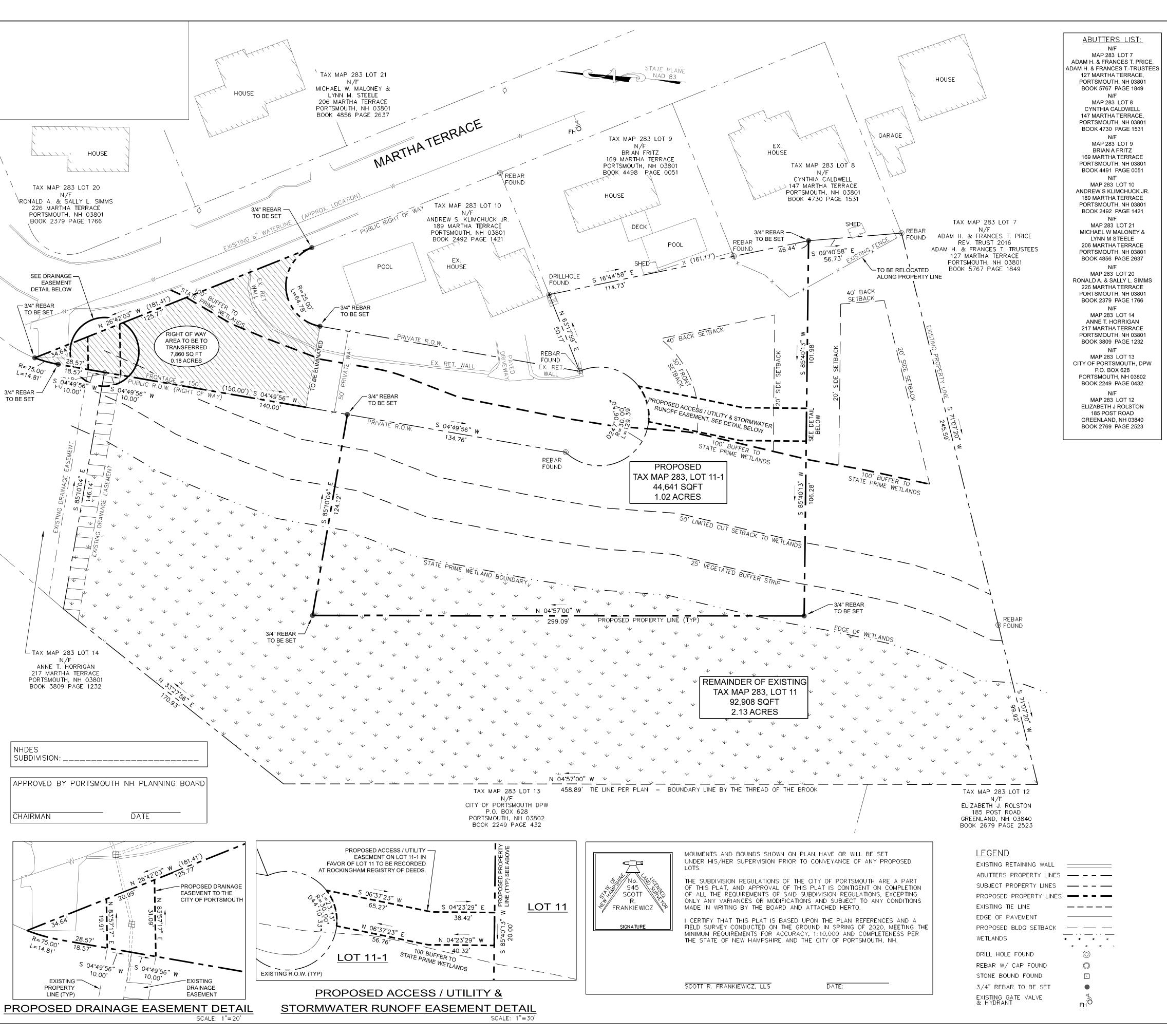
WALL		WETLANDS	<u> </u>
TY LINES			• •
LINES		DRILL HOLE FOUND	0
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		STONE BOUND FOUND	
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ETBACK			111
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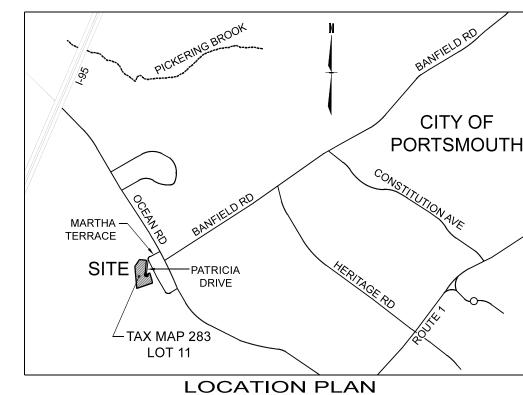
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ROCKINGHAM CO. JOB NO: 258.00 DATE: SEPTEMBER 23, 2020

> PCP SHT. 8 of 10





NOTES

THE PURPOSE OF THIS PLAN IS TO SUBDIVIDE TAX MAP 283, LOT 11 INTO 2 LOTS.

SCALE: 1"=2,000'

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 THE CURRENT OWNER FOR TAX MAP 283, LOT 11: FRITZ FAMILY REVOC LIV
- TRUST, P.O. BOX 524, 50 SHORE DR., NORTHWOOD NH, 03261.
 BK 3338 PG 173.

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REQUIRED NOTES:

I. THE DUMPING OF ANY KIND IS PROHIBITED IN THE WETLAND BUFFER.

2. SALTING OF ROAD IS PROHIBITED DUE TO CLOSE PROXIMITY TO A PRIME

	ВҮ	TDB	TDB		
REVISIONS	DESCRIPTION	13 06/28/2021 REVISED PER CITY OF PORTSMOUTH COMMENTS TDB	14 07/08/2021 REVISED PER CITY OF PORTSMOUTH COMMENTS TDB		
	DATE	06/28/2021	07/08/2021		
	NO.	13	14		

GRAPHIC SCALE

SCALE: 1"=30'

LAND SALESTATE

H. LAND DE LAND PLANNING. REAL



TRUCTION
H NH 03801
IN TRUST,
USTEE

ROPOSED SUBDIVISION PLATAX MAP 283 LOT 11

BE PLUS CONSTRUCTI
EMLOCK WAY, PORTSMOUTH NH 038
OWNED BY
RITZ FAMILY REVOC LIV TRUS
EDGAR H FRITZ, TRUSTEE

ROCKINGHAM CO. JOB NO: 258.00

DATE: SEPTEMBER 23, 2020

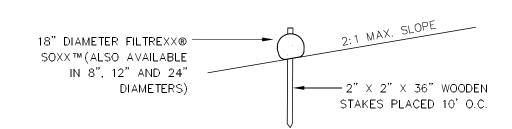
PSP SHT. 9 of 10

CONSTRUCTION SEQUENCE:

- 1. CUT AND CLEAR TREES, REMOVE EXISTING PAVEMENT WITHIN LIMIT OF WORK (PROPOSED TREELINE), UNLESS OTHERWISE NOTED.
 ALL STUMPS, BRANCHES, TOPS AND BRUSH TO BE PROPERLY DISPOSED OF, PREFERABLY OFF SITE.
- 2. CONSTRUCT TEMPORARY AND PERMANENT EROSION CONTROL FACILITIES (DETENTION BASIN, DIVERSION BERM, GRASS SWALE) PRIOR TO ANY EARTH MOVING OPERATION.
- 3. ALL AREAS SHALL BE PROTECTED FROM EROSION. SIDE SLOPES AND DETENTION POND SHALL BE STABILIZED PRIOR TO DIRECTING RUNOFF TO THEM.
- 4. POND SHALL BE INSTALLED EARLY ON IN THE CONSTRUCTION SEQUENCE (BEFORE ROUGH GRADING THE SITE).
- 5. ALL STORM DRAINAGE SYSTEMS SUCH AS DETENTION/RETENTION BASINS, LEVEL SPREADERS SHALL BE PROTECTED FROM EROSION. ALL STORM DRAINAGE SYSTEMS SHALL BE STABILIZED PRIOR TO DIRECTING FLOW INTO THEM.
- 6. CONSTRUCT TEMPORARY CULVERTS, DIVERSION DITCHES/SWALES OR BERMS AS REQUIRED TO MINIMIZE THE EROSIVE AFFECTS OF STORMWATER RUNOFF DURING ALL CONSTRUCTION ACTIVITIES. TEMPORARY WATER DIVERSION (SWALES, BASINS, ETC.) MUST BE USED AS NECESSARY UNTIL AREAS STABILIZED.
- 7. ALL MATERIAL SUITABLE FOR USE AS TOPSOIL SHALL BE STOCKPILED IN UPLANDS AREAS. ALL STOCKPILES SHALL BE SEEDED WITH WINTER RYE AND IF NECESSARY, SURROUNDED WITH SILT FENCE, AND/OR STRAW BALES, IN ORDER TO PREVENT OR CONTAIN SOIL EROSION.
- 8. ALL MATERIAL SUITABLE FOR FILL OR SELECT MATERIAL SHALL BE STOCKPILED IN UPLANDS AREAS. ALL STOCKPILES SHALL BE SURROUNDED WITH SILT FENCE, AND/OR STRAW BALES, IN ORDER TO CONTAIN SOIL EROSION.
- 9. REMOVE ALL IMPROPER ROADWAY MATERIAL WITHIN 18" OF SUBGRADE. REPLACE WITH COMPACTED GRANULAR FILL ACCEPTABLE
 TO THE STATE/TOWN SPECIFICATIONS. ALL SUITABLE FILL MATERIAL SHALL BE COMPACTED TO AT LEAST 95% OF THE DRY
 WEIGHT AS DETERMINED BY MODIFIED PROCTOR TESTING (ASTM D-1556) REQUIREMENTS.
- 10. CONSTRUCT ALL UNDERGROUND UTILITIES INCLUDING, BUT NOT LIMITED TO DRAIN, DATA, CABLE AND POWER.
- 11. ROUGH GRADE SITE WITHIN LIMIT OF WORK AND COMMENCE CONSTRUCTION OF ROADWAY.
- 12. SITE SHALL BE STABILIZED WITHIN 72 HOURS OF FINISHED GRADE.
- 13. COMPLETE ROADWAY SLOPE GRADING/EMBANKMENT CONSTRUCTION. ALL SLOPES SHALL BE STABILIZED AND SEEDED IMMEDIATELY AFTER GRADING. THE CONTRACTOR SHALL STABILIZE SLOPES WITH APPROPRIATE SEEDING PROGRAM OR JUTE MAT, WHEREVER SPECIFIED. ALL CUT AND FILL SLOPES SHALL BE SEEDED/LOAMED WITHIN 72 HOURS OF ACHIEVING FINISH GRADE.
- 14. APPLY TOPSOIL TO SITE SLOPES AND OTHER AREAS DISTURBED BY CONSTRUCTION. TOPSOIL USED SHALL BE NATIVE ORGANIC MATERIAL SCREENED AS TO BE FREE FROM ROOTS, BRANCHES, STONES, AND OTHER DELETERIOUS MATERIALS. TOPSOIL SHALL BE APPLIED SO AS TO PROVIDE A MINIMUM OF A 4-INCH COMPACTED THICKNESS. UPON COMPLETION OF TOPSOILING, FINISHED SECTIONS ARE TO BE LIMED, SEEDED, AND MULCHED. CONSERVATION SEED MIX SHALL BE USED ALONG "PROPOSED PRIVATE DRIVE" AND WILDFLOWER MIX TO BE USED IN DETENTION BASIN AND OTHER OPEN AREAS. THE CONTRACTOR SHALL INSPECT COMPLETED SECTIONS OF WORK ON A REGULAR BASIS AND REMEDY ANY PROBLEM AREAS UNTIL A HEALTHY STAND OF GRASS IS ESTABLISHED.
- 15. MAINTAIN, REPAIR, AND REPLACE TEMPORARY EROSION CONTROL MEASURES AS NECESSARY FOR A MINIMUM PERIOD OF 12 MONTHS FOLLOWING SUBSTANTIAL COMPLETION.
- 16. AFTER STABILIZATION (12 MONTHLY FOLLOWING SUBSTANTIAL COMPLETION), REMOVE AND PROPERLY DISPOSE OF TEMPORARY EROSION CONTROL MEASURES, PREFERABLY OFF SITE.
- 17. THE SMALLEST PRACTICAL AREA SHALL BE DISTURBED DURING CONSTRUCTION, BUT IN NO CASE SHALL EXCEED 5 ACRES AT ANY ONE TIME BEFORE DISTURBED AREAS ARE STABILIZED.

DEFINITION OF THE WORD STABLE: AN AREA SHALL BE CONSIDERED STABLE IF ONE OF THE FOLLOWING HAS OCCURRED:

- A: BASE COURSE GRAVELS HAVE BEEN INSTALLED IN AREAS TO BE PAVED.
- B: A MINIMUM OF 85 PERCENT VEGETATED GROWTH HAS BEEN ESTABLISHED.
- C: A MINIMUM OF 3 INCHES OF NON-EROSIVE MATERIAL SUCH AS STONE OR RIPRAP HAS BEEN INSTALLED.
- D: OR, EROSION CONTROL BLANKETS HAVE BEEN PROPERTY INSTALLED.
- 18. ALL AREAS SHALL BE STABILIZED WITHIN 45 DAYS OF INITIAL DISTURBANCE.



FILTER SOCK DETAIL

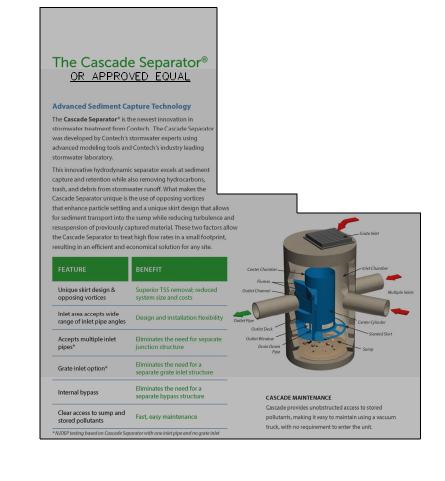
FILTREXX ® OR APPROVED EQUAL

NOT TO SCALE

PLAN VIEW B-B NOT TO SCALE CONTRACTOR TO GROUT TO PRISHED GRADE BROWNESS MUSICAL BROWNESS



STORM WATER FILTER DETAILS



GRATE FO 1. STRUCTURES

RECOMMENDED MAINTENANCE SCHEDULE

-EACH SITLSACK SHOULD BE INSPECTED AFTER EVERY MAJOR RAIN EVENT

-IF THERE HAVE BEEN NO MAJOR EVENTS, SILTSACK

SHOULD BE INSPECTED EVERY 2-3 WEEKS

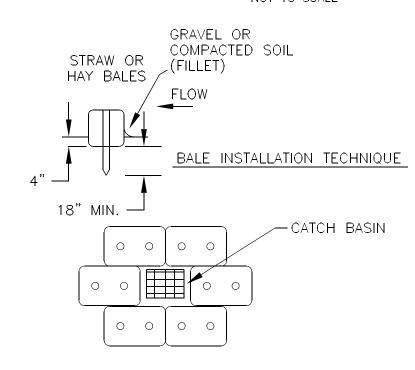
-THE RESTRAINT CORD SHOULD BE VISIBLE AT ALL
TIMES. IF CORD IS COVERED WITH SEDIMENT, THE

SILTSACK DETAIL

LIFTING STRAPS

NOT TO SCALE

SILTSACK SHOULD BE EMPTIED.



EROSION PROTECTION

TYPE "E"

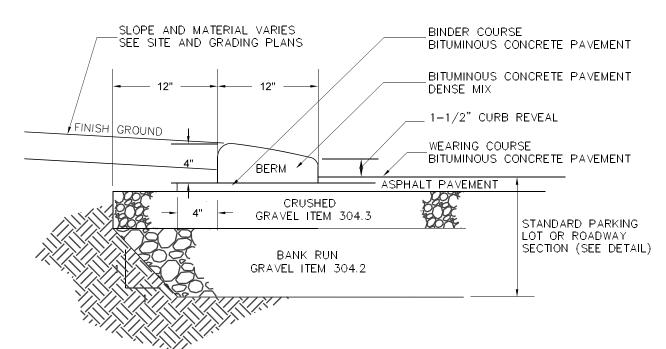
NORMAL USE AROUND CATCH BASINS
NOT TO SCALE

CONSTRUCTION SPECIFICATIONS FOR STRAW OR HAY BALE BARRIERS

LEAST 18 INCHES INTO THE SOIL.

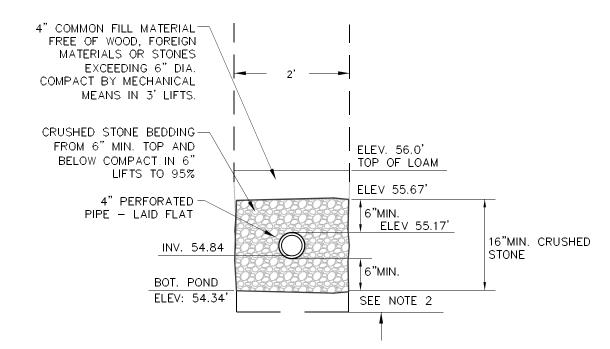
- 1. STRUCTURES SHALL BE INSTALLED ACCORDING TO THE DIMENSIONS SHOWN ON THE PLANS AT THE APPROPRIATE SPACING.
- 2. CONSTRUCTION OPERATIONS SHALL BE CARRIED OUT IN SUCH A MANNER SO THAT FROSION AND AIR AND WATER POLLUTION WILL BE MINIMIZED.
- EROSION AND AIR AND WATER POLLUTION WILL BE MINIMIZED.

 3. WHEN HAY BALES ARE USED, THE BALES SHALL BE EMBEDDED AT LEAST 4 INCHES INTO THE SOIL. WHEN TIMBER STRUCTURES ARE USED, THE TIMBER SHALL EXTEND AT
- 4. HAY OR STRAW BALES SHALL BE ANCHORED INTO THE SOIL USING 2" X 2" STAKES DRIVEN THROUGH THE BALES AND AT LEAST 18 INCHES INTO THE SOIL.
- 5. SEEDING, FERTILIZING, AND MULCHING SHALL CONFORM TO THE RECOMMENDATIONS IN THE APPROPRIATE VEGETATIVE BMP.
- 6. STRUCTURES SHALL BE REMOVED FROM THE CHANNEL WHEN THEIR USEFUL LIFE HAS BEEN COMPLETED.



CAPE COD CURB (ASPHALT) DETAIL

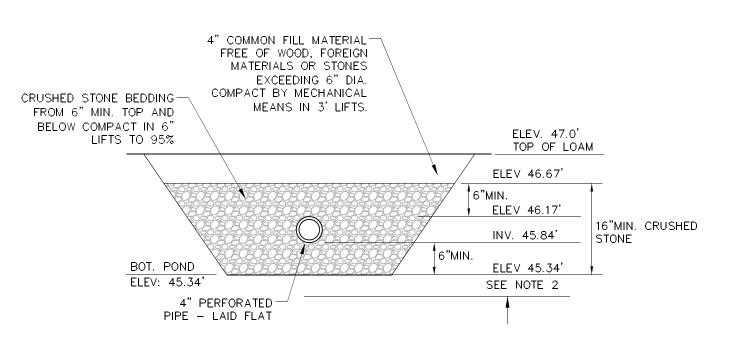
NOT TO SCAL



- 1. FOR CROSS COUNTRY CONSTRUCTION, BACKFILL OR FILL SHALL BE MOUNDED TO A HEIGHT OF 6 INCHES ABOVE THE ORIGINAL GROUND SURFACE.
- 2. ORDERED EXCAVATION OF UNSUITED MATERIAL BELOW GRADE, RE-FILL WITH BEDDING MATERIAL.

INFILTRATION SWALE WITH 4" UNDERDRAIN DETAIL

NOT TO SCALE



- FOR CROSS COUNTRY CONSTRUCTION, BACKFILL OR FILL SHALL BE MOUNDED TO A HEIGHT OF 6 INCHES ABOVE THE ORIGINAL GROUND SURFACE.
- 2. ORDERED EXCAVATION OF UNSUITED MATERIAL BELOW GRADE, RE—FILL WITH BEDDING MATERIAL.

INFILTRATION POND WITH
4" UNDERDRAIN PIPE DETAIL

NOT TO SCALE

MAINTENANCE

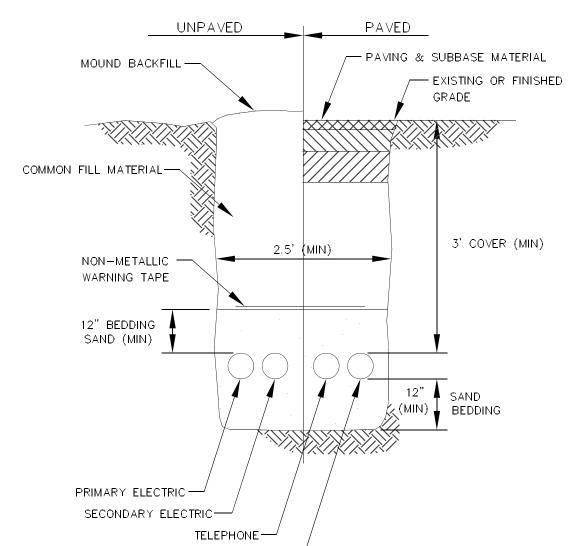
THE LEVEL SPREADER SHOULD BE CHECKED PERIODICALLY AND AFTER EVERY MAJOR STORM TO DETERMINE IF THE LIP HAS BEEN DAMAGED AND TO DETERMINE THAT THE DESIGN CONDITIONS HAVE NOT CHANGED. ANY DETRIMENTAL SEDIMENT ACCUMULATION SHOULD BE REMOVED. IF RILLING HAS TAKEN PLACE ON THE LIP, THEN THE DAMAGE SHOULD BE REPAIRED AND RE?VEGETATED. THE VEGETATION SHOULD BE MOWED OCCASIONALLY TO CONTROL WEEDS AND THE ENCROACHMENT OF WOODY VEGETATION. CLIPPINGS SHOULD BE REMOVED AND DISPOSED OF OUTSIDE THE SPREADER AND AWAY FROM THE OUTLET AREA. FERTILIZATION SHOULD BE DONE AS NECESSARY TO KEEP THE VEGETATION HEALTHY AND DENSE.

CONSTRUCTION SPECIFICATIONS

- 1) CONSTRUCT THE LEVEL SPREADER LIP ON A ZERO GRADE TO INSURE UNIFORM SPREADING RUNOFF.
- LEVEL SPREADER SHALL BE CONSTRUCTED ON UNDISTURBED SOIL AND NOT ON FILL.
- 3) AN EROSION STOP SHALL BE PLACED VERTICALLY A MINIMUM OF SIX INCHES DEEP IN A SILT TRENCH ONE FOOT BACK OF THE LEVEL LIP AND PARALLEL TO THE LIP. THE EROSION STOP SHALL EXTEND THE ENTIRE
- 4) THE ENTIRE LIP AREA SHALL BE PROTECTED BY PLACING TWO STRIPS OF JUTE OR EXCELSIOR MATTING ALONG THE LIP. EACH STRIP SHALL OVERLAP THE EROSION STOP BY AT LEAST SIX INCHES.
- 5) THE ENTRANCE CHANNEL TO THE LEVEL SPREADER SHALL NOT EXCEED A 1 PERCENT GRADE FOR AT LEAST 50 FEET BEFORE ENTERING INTO THE SPREADER.
- 6) THE FLOW FROM THE LEVEL SPREADER SHALL OUTLET ONTO STABILIZED AREAS. WATER SHOULD NOT RE—CONCENTRATE IMMEDIATELY BELOW THE SPREADER.

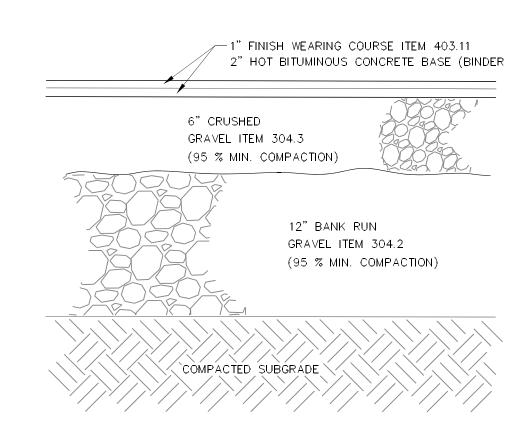
7) PERIODIC INSPECTION AND REQUIRED MAINTENANCE SHALL BE PERFORMED.

LENGTH OF THE LEVEL LIP.



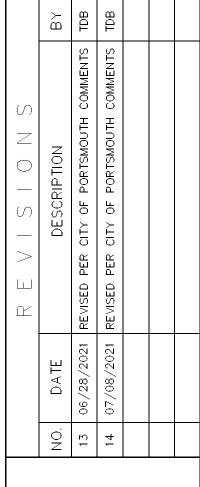
UTILITY TRENCH DETAIL

NOT TO SCALE



PAVEMENT SECTION

NOT TO SCALE



SCALE AS SHOWN

N.H. LAND
CONSUITAND
SURVEYING-LAND PLANNING-REAL ESTATE
A Veteran Owned Company



DETAIL SHEET

TAX MAP 283 LOT 11

BE PLUS CONSTRUCTION

EMLOCK WAY, PORTSMOUTH NH 03801

OWNED BY

RITZ FAMILY REVOC LIV TRUST,

EDGAR H FRITZ, TRUSTEE

ROCKINGHAM CO.

JOB NO: 258.00

DATE: SEPTEMBER 23, 2020

DET SHT. 10 of 10

DRAINAGE ANALYSIS

Prepared for:

DUBE PLUS CONSTRUCTION

TAX MAP 283 LOT 11

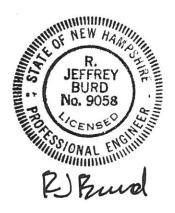
PATRICIA DRIVE

PORTSMOUTH, NH

Prepared by:

NEW HAMPSHIRE LAND CONSULTANTS, PLLC
683C FIRST NH TURNPIKE
NORTHWOOD, NH 03261
&
RJB ENGINEERING
JEFFREY BURD, P.E.

Project Number: 258.00



PROJECT NARATIVE

1. Table of Contents

- 1. Narrative of the project with summary table of peak discharge rates
- 2. Drainage analysis-Full Pre & Post summary of the 50-YR
- 3. Conclusion

Narrative

Introduction

This drainage analysis details the surface water drainage patterns on a parcel located at Patricia Drive in Portsmouth, NH. Using HydroCAD to model storm events this analysis estimates the amount of storm water surface runoff from this site before and after the proposed parking lot and sidewalk. The design of this project will decrease the runoff.

The proposed improvements are on Patricia Drive and Tax Map 283 Lot 11. The applicant, Dube Plus Construction, wishes to rebuild Patricia Drive and construct 2 single family homes. We are proposing 1 detention basin and one treatment/detention system to control, pre-treat and treat the stormwater runoff from the reconstructed road, driveways and yards. The houses will be constructed with drip edges and all roof runoff will be infiltrated via the drip edge. The roadway stormwater runoff is directed to a detention/infiltration area that is equipped with a sediment forebay, a bio-retention system and detention area. The stormwater the isn't infiltrated will leave this detention/infiltration/filtration system will be directed to a rip rap slope to a level spreader and directed to 75' natural filter strip, which will provide additional overland treatment prior to reaching the prime wetland.

The area that has been analyzed is all upland, Chatfield-Hollis-Canton, Sandy Loam soils (Hydro group B soils) as categorized by the Soil Conservation District.

The following section explains the methods used to determine the runoff quantities generated by the existing conditions site. The objective of this analysis is to obtain surface storm water runoff flow data. This information is compared to evaluate whether there may be an impact to existing drainage system in the area.

Methodology

The drainage analysis performed utilizes nationally recognized techniques developed by the USDA, Soil Conservation Service (SCS). The techniques and models used for this analysis are described in "Urban Hydrology for Small Watersheds, Technical Release Number 55" dated 1986 and in USDOT Federal Highway Administration (FHA) "Hydraulic Design of Highway Culverts" dated September 1985.

Design computations were based on a Type III 24-hour storm event as recommended for New Hampshire. 10 year – 24-hour event of 4.92 inches of precipitation respectively was analyzed. Pre and Post-development conditions were analyzed by the same method. An investigation was conducted to confirm published watershed soil and vegetative characteristics that were used for the input program "HydroCAD Storm water Modeling System, Version 10.00-25". Tabulated summaries of the results are shown in the results section of this report.

Procedure

To begin the stormwater study, the limits and areas of the watershed for this development were identified. The existing watershed area is treated as 1 sub-catchment. The proposed development watershed area is treated as 5 sub-catchments. Weighted runoff curve numbers (CN) were calculated for each sub-catchment watershed area. Runoff curve numbers were chosen based on site investigation, TR-55, USDA Agriculture Handbook 590 (1997), and USDA Soil Conservation

Patricia Drive, Portsmouth, NH Drainage Analysis December 22, 2020, Rev. 7-7-2021 Page 4

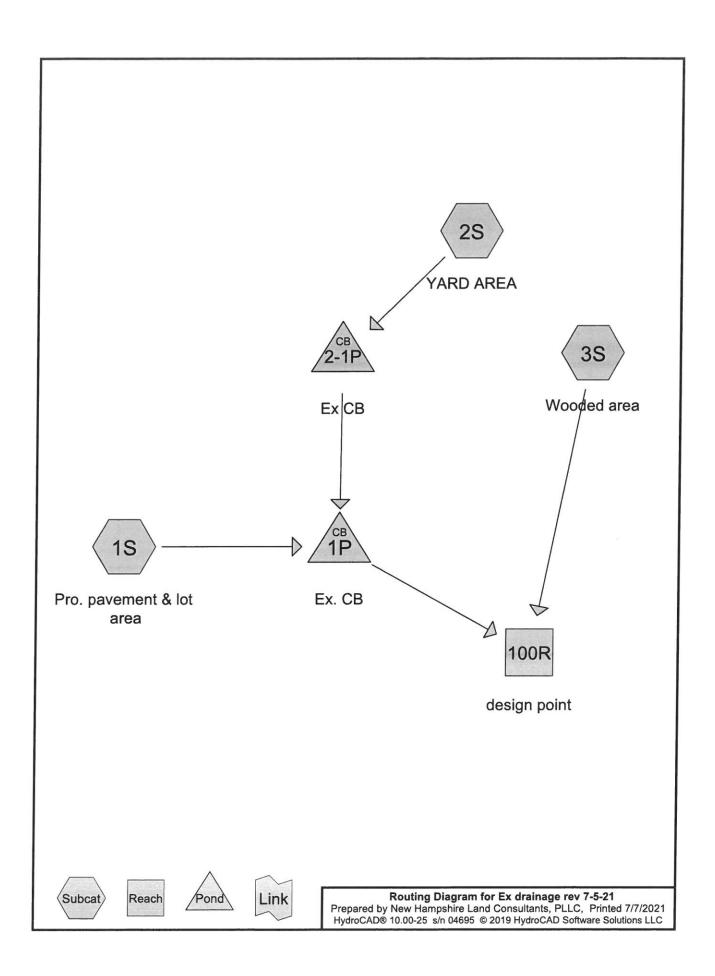
Service Soil Survey, issued October 1994. The value of CN depends on soil type, vegetative cover and hydraulic conditions of the land surface. Surface water run off rate and total volume during and after a storm event is also influenced by: slope of the land, area of the watershed, hydraulic length of watershed, and ponds and swamps. In addition, the amount of surface runoff produced by a given storm event is a function of the duration and intensity of the storm.

Pre-development and post-development conditions for the watershed were analyzed by the method outlined in USDA Soil Conservation Service Soil Survey, issued October 1994. Using this post-development information, computer generated hydrographs were calculated and peak runoff rates determined for each specific storm event.

The entire area to be developed will disturb approximately 34,000 square feet. Re-graded areas along the edge of construction will ultimately become stabilized and generally resume their predevelopment characteristics.

DRAINAGE ANALYSIS PRE & POST

Pre-Conditions Drainage Analysis
Full summary
50 YR – 24 HR rainfall = 7.48"



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Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.261	61	>75% Grass cover, Good, HSG B (1S, 2S, 3S)
0.261	98	Impervious (1S, 2S, 3S)
0.508	55	Woods, Good, HSG B (2S, 3S)
1.030	67	TOTAL AREA

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Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.769	HSG B	1S, 2S, 3S
0.000	HSG C	
0.000	HSG D	
0.261	Other	1S, 2S, 3S
1.030		TOTAL AREA

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Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.261	0.000	0.000	0.000	0.261	>75% Grass cover, Good	1S, 2S,
							3S
0.000	0.000	0.000	0.000	0.261	0.261	Impervious	1S, 2S,
							3S
0.000	0.508	0.000	0.000	0.000	0.508	Woods, Good	2S, 3S
0.000	0.769	0.000	0.000	0.261	1.030	TOTAL AREA	

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Pipe Listing (all nodes)

Line#	Node	In-Invert	Out-Invert	Length	Slope	n	Diam/Width	Height	Inside-Fill
	Number	(feet)	(feet)	(feet)	(ft/ft)		(inches)	(inches)	(inches)
1	1P	43.86	43.54	25.0	0.0128	0.013	8.0	0.0	0.0
2	2-1P	43.79	43.88	28.0	-0.0032	0.013	12.0	0.0	0.0

Type III 24-hr 50 yr 24 hr Rainfall=7.48"

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Time span=5.00-60.00 hrs, dt=0.05 hrs, 1101 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Pro. pavement & lot area Runoff Area=10,927 sf 76.95% Impervious Runoff Depth>6.16"

Tc=5.0 min CN=89 Runoff=1.73 cfs 0.129 af

Subcatchment 2S: YARD AREA Runoff Area=10,031 sf 17.32% Impervious Runoff Depth=3.48"

Tc=5.0 min CN=65 Runoff=0.94 cfs 0.067 af

Subcatchment 3S: Wooded area Runoff Area=23,910 sf 5.06% Impervious Runoff Depth=2.74"

Tc=5.0 min CN=58 Runoff=1.72 cfs 0.125 af

Reach 100R: design point Inflow=4.36 cfs 0.321 af

Outflow=4.36 cfs 0.321 af

Pond 1P: Ex. CB Peak Elev=46.75' Inflow=2.66 cfs 0.195 af

8.0" Round Culvert n=0.013 L=25.0' S=0.0128 '/' Outflow=2.66 cfs 0.195 af

Pond 2-1P: Ex CB Peak Elev=44.50' Inflow=0.94 cfs 0.067 af

12.0" Round Culvert n=0.013 L=28.0' S=-0.0032 '/' Outflow=0.94 cfs 0.067 af

Total Runoff Area = 1.030 ac Runoff Volume = 0.321 af Average Runoff Depth = 3.74" 74.69% Pervious = 0.769 ac 25.31% Impervious = 0.261 ac

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Summary for Subcatchment 1S: Pro. pavement & lot area

[49] Hint: Tc<2dt may require smaller dt

Runoff = 1.73 cfs @

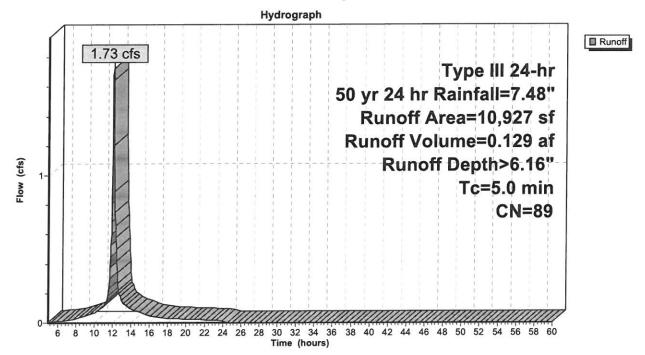
1.73 cfs @ 12.07 hrs, Volume=

0.129 af, Depth> 6.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs Type III 24-hr 50 yr 24 hr Rainfall=7.48"

-	Α	rea (sf)	CN	Description		
*		8,408	98	Impervious		
		2,519	61	>75% Gras	s cover, Go	ood, HSG B
		10,927	89	Weighted A	verage	
		2,519		23.05% Per	vious Area	a
		8,408		76.95% Imp	ervious Ar	rea
	Tc (min)	Length (feet)	Slope (ft/ft)	•	Capacity (cfs)	Description
	5.0					Direct Entry, 1

Subcatchment 1S: Pro. pavement & lot area



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Summary for Subcatchment 2S: YARD AREA

[49] Hint: Tc<2dt may require smaller dt

Runoff =

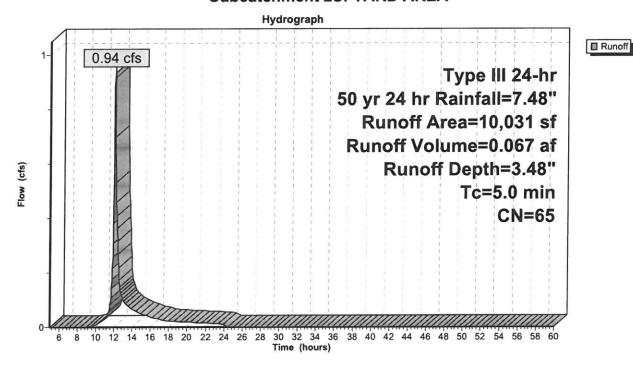
0.94 cfs @ 12.08 hrs, Volume=

0.067 af, Depth= 3.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs Type III 24-hr 50 yr 24 hr Rainfall=7.48"

100	Α	rea (sf)	CN	Description						
		4,143	55	Woods, Go	od, HSG B					
*		1,737	98	Impervious	mpervious					
		4,151	61	>75% Gras	s cover, Go	ood, HSG B				
		10,031	65	Weighted A	ighted Average					
		8,294		82.68% Per	vious Area					
		1,737		17.32% lmp	ervious Ar	ea				
	_		01	17-1	0	D				
	Tc	Length	Slope	•	Capacity	Description				
<u>(r</u>	nin)	(feet)	(ft/ft	(ft/sec)	(cfs)					
	5.0			Direct Entry,						

Subcatchment 2S: YARD AREA



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Summary for Subcatchment 3S: Wooded area

[49] Hint: Tc<2dt may require smaller dt

Runoff =

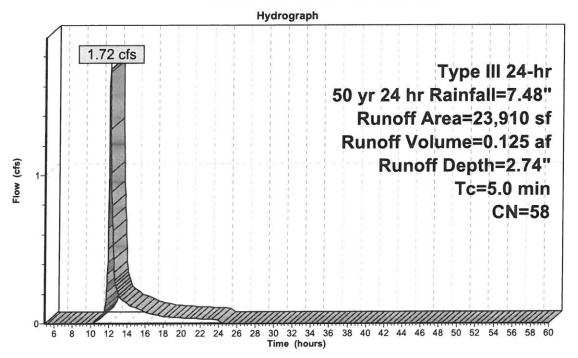
1.72 cfs @ 12.08 hrs, Volume=

0.125 af, Depth= 2.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs Type III 24-hr 50 yr 24 hr Rainfall=7.48"

	Α	rea (sf)	CN	Description					
		17,990	55	Woods, Go	od, HSG B	3			
*		1,210	98	Impervious					
		4,710	61	>75% Gras	s cover, Go	ood, HSG B			
		23,910	58	Neighted A	verage				
		22,700			vious Area	a			
		1,210		5.06% Impe	ervious Area	ea			
				•					
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	5.0					Direct Entry,			

Subcatchment 3S: Wooded area



Runoff

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Summary for Reach 100R: design point

[40] Hint: Not Described (Outflow=Inflow)

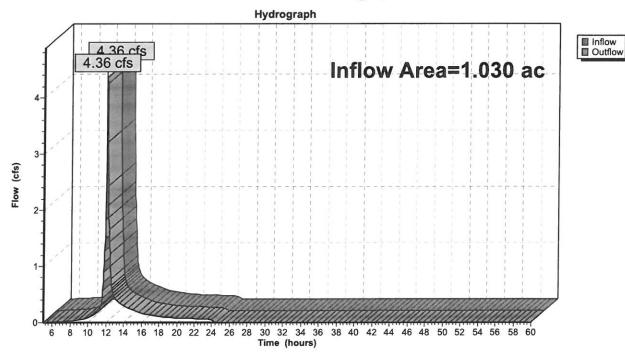
1.030 ac, 25.31% Impervious, Inflow Depth > 3.74" for 50 yr 24 hr event Inflow Area =

0.321 af Inflow

4.36 cfs @ 12.08 hrs, Volume= 4.36 cfs @ 12.08 hrs, Volume= Outflow 0.321 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs

Reach 100R: design point



Type III 24-hr 50 yr 24 hr Rainfall=7.48"

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Summary for Pond 1P: Ex. CB

[82] Warning: Early inflow requires earlier time span [81] Warning: Exceeded Pond 2-1P by 2.12' @ 12.05 hrs

Inflow Area = 0.481 ac, 48.41% Impervious, Inflow Depth > 4.88" for 50 yr 24 hr event

Inflow = 2.66 cfs @ 12.07 hrs, Volume= 0.195 af

Outflow = 2.66 cfs @ 12.07 hrs, Volume= 0.195 af, Atten= 0%, Lag= 0.0 min

Primary = 2.66 cfs @ 12.07 hrs, Volume= 0.195 af

Routing by Stor-Ind method, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs / 3

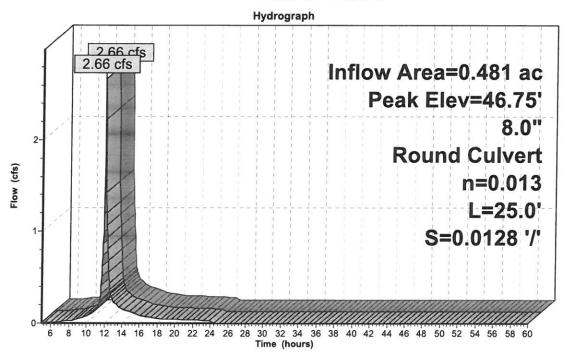
Peak Elev= 46.75' @ 12.07 hrs

Flood Elev= 47.82'

Device	Routing	Invert	Outlet Devices
#1	Primary	43.86'	8.0" Round Culvert L= 25.0' Ke= 0.500 Inlet / Outlet Invert= 43.86' / 43.54' S= 0.0128 '/' Cc= 0.900 n= 0.013. Flow Area= 0.35 sf

Primary OutFlow Max=2.57 cfs @ 12.07 hrs HW=46.60' (Free Discharge)
1=Culvert (Barrel Controls 2.57 cfs @ 7.35 fps)

Pond 1P: Ex. CB





Type III 24-hr 50 yr 24 hr Rainfall=7.48"

Ex drainage rev 7-5-21

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Summary for Pond 2-1P: Ex CB

0.230 ac, 17.32% Impervious, Inflow Depth = 3.48" for 50 yr 24 hr event Inflow Area =

0.94 cfs @ 12.08 hrs, Volume= 0.067 af Inflow

0.94 cfs @ 12.08 hrs, Volume= 0.94 cfs @ 12.08 hrs, Volume= 0.067 af, Atten= 0%, Lag= 0.0 min Outflow

0.067 af Primary

Routing by Stor-Ind method, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs / 3

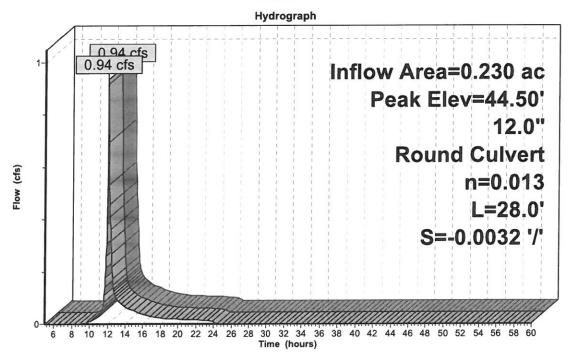
Peak Elev= 44.50' @ 12.08 hrs

Flood Elev= 48.09'

Device Routing Invert **Outlet Devices** 12.0" Round Culvert L= 28.0' Ke= 0.500 #1 43.88' Primary Inlet / Outlet Invert= 43.79' / 43.88' S= -0.0032 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.91 cfs @ 12.08 hrs HW=44.49' (Free Discharge) 1=Culvert (Barrel Controls 0.91 cfs @ 2.18 fps)

Pond 2-1P: Ex CB





Type III 24-hr First flush Rainfall=1.00"

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Time span=5.00-60.00 hrs, dt=0.05 hrs, 1101 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Pro. pavement & lot areaRunoff Area=10,927 sf 76.95% Impervious Runoff Depth=0.28"

Tc=5.0 min CN=89 Runoff=0.08 cfs 0.006 af

Subcatchment 2S: YARD AREA Runoff Area=10,031 sf 17.32% Impervious Runoff Depth=0.00"

Tc=5.0 min CN=65 Runoff=0.00 cfs 0.000 af

Subcatchment 3S: Wooded area Runoff Area=23,910 sf 5.06% Impervious Runoff Depth=0.00"

Tc=5.0 min CN=58 Runoff=0.00 cfs 0.000 af

Reach 100R: design point Inflow=0.08 cfs 0.006 af

Outflow=0.08 cfs 0.006 af

Pond 1P: Ex. CB Peak Elev=44.01' Inflow=0.08 cfs 0.006 af

8.0" Round Culvert n=0.013 L=25.0' S=0.0128'/' Outflow=0.08 cfs 0.006 af

Pond 2-1P: Ex CB Peak Elev=43.88' Inflow=0.00 cfs 0.000 af

12.0" Round Culvert n=0.013 L=28.0' S=-0.0032 '/' Outflow=0.00 cfs 0.000 af

Total Runoff Area = 1.030 ac Runoff Volume = 0.006 af Average Runoff Depth = 0.07" 74.69% Pervious = 0.769 ac 25.31% Impervious = 0.261 ac

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Summary for Subcatchment 1S: Pro. pavement & lot area

[49] Hint: Tc<2dt may require smaller dt

Runoff =

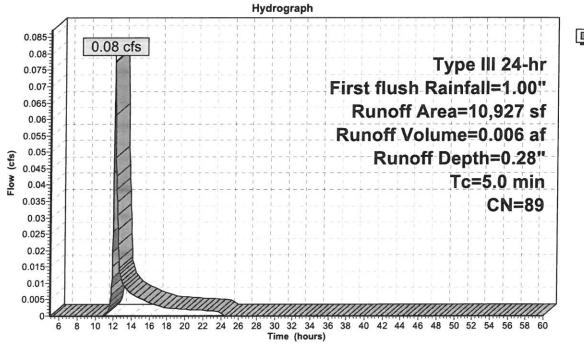
0.08 cfs @ 12.09 hrs, Volume=

0.006 af, Depth= 0.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs Type III 24-hr First flush Rainfall=1.00"

	A	rea (sf)	CN	Description						
*		8,408	98	Impervious						
		2,519	61	>75% Gras	s cover, Go	lood, HSG B				
		10,927	89	Weighted A	eighted Average					
		2,519		23.05% Per	vious Area	a				
		8,408		76.95% Imp	ervious Ar	rea				
	Тс	Length	Slope	•	Capacity	•				
_	(min)	(feet)	(ft/ft	(ft/sec)	(cfs)					
	5.0					Direct Entry, 1				

Subcatchment 1S: Pro. pavement & lot area



Runoff

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Summary for Subcatchment 2S: YARD AREA

[49] Hint: Tc<2dt may require smaller dt

[45] Hint: Runoff=Zero

Runoff

0.00 cfs @

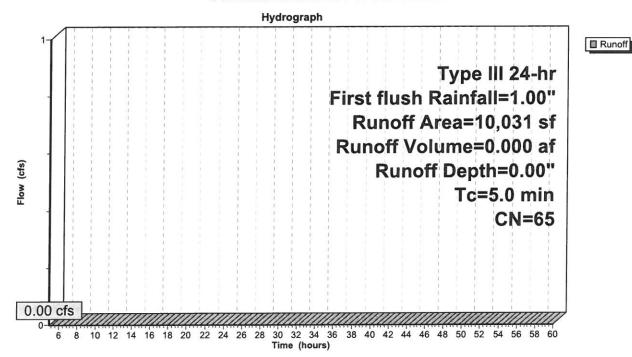
5.00 hrs, Volume=

0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs Type III 24-hr First flush Rainfall=1.00"

9224	A	rea (sf)	CN	Description					
		4,143	55	Woods, Go	od, HSG B	3			
*		1,737	98	Impervious					
		4,151	61	>75% Gras	s cover, Go	Good, HSG B			
-		10,031	65	Weighted Average					
		8,294		32.68% Per	rvious Area	a			
		1,737		17.32% lmp	pervious Ar	rea			
	Tc	Length	Slope	,	Capacity	•			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	5.0			Direct Entry,					

Subcatchment 2S: YARD AREA



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Summary for Subcatchment 3S: Wooded area

[49] Hint: Tc<2dt may require smaller dt

[45] Hint: Runoff=Zero

Runoff

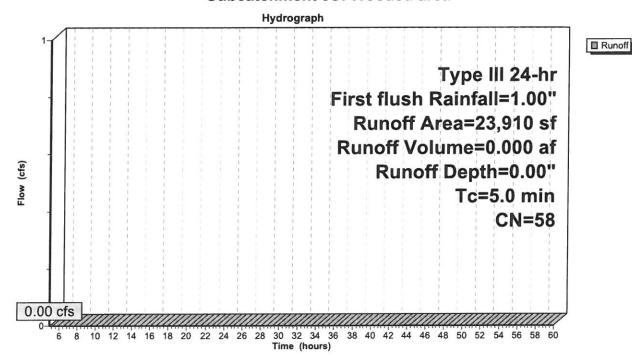
0.00 cfs @ 5.00 hrs, Volume=

0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs Type III 24-hr First flush Rainfall=1.00"

0.000	Д	rea (sf)	CN	Description					
		17,990	55	Noods, Go	od, HSG B	3			
*		1,210	98	Impervious					
		4,710	61	>75% Grass cover, Good, HSG B					
		23,910	58	Weighted A	verage				
		22,700	9	94.94% Per	vious Area	a			
		1,210		5.06% Impe	ervious Area	ea			
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	5.0					Direct Entry,			

Subcatchment 3S: Wooded area



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Summary for Reach 100R: design point

[40] Hint: Not Described (Outflow=Inflow)

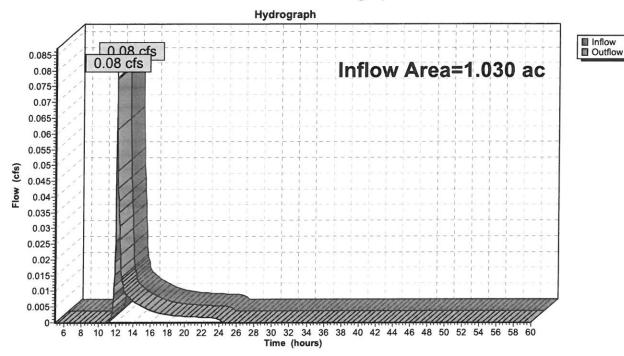
1.030 ac, 25.31% Impervious, Inflow Depth = 0.07" for First flush event Inflow Area =

0.08 cfs @ 12.09 hrs, Volume= 0.006 af Inflow

0.08 cfs @ 12.09 hrs, Volume= 0.006 af, Atten= 0%, Lag= 0.0 min Outflow

Routing by Stor-Ind+Trans method, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs

Reach 100R: design point



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Summary for Pond 1P: Ex. CB

[81] Warning: Exceeded Pond 2-1P by 0.13' @ 12.10 hrs

Inflow Area = 0.481 ac, 48.41% Impervious, Inflow Depth = 0.15" for First flush event

Inflow = 0.08 cfs @ 12.09 hrs, Volume= 0.006 af

Outflow = 0.08 cfs @ 12.09 hrs, Volume= 0.006 af, Atten= 0%, Lag= 0.0 min

Primary = 0.08 cfs @ 12.09 hrs, Volume= 0.006 af

Routing by Stor-Ind method, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs / 3

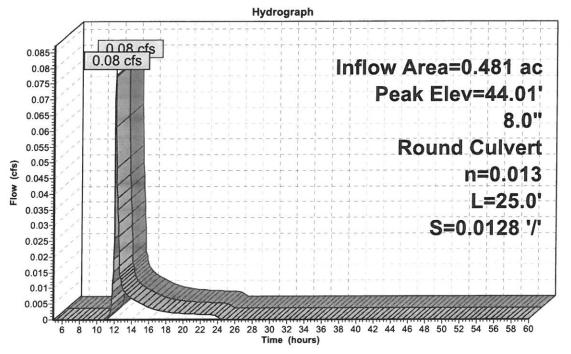
Peak Elev= 44.01' @ 12.09 hrs

Flood Elev= 47.82'

Device	Routing	Invert	Outlet Devices
#1	Primary	43.86'	8.0" Round Culvert L= 25.0' Ke= 0.500 Inlet / Outlet Invert= 43.86' / 43.54' S= 0.0128 '/' Cc= 0.900 n= 0.013, Flow Area= 0.35 sf

Primary OutFlow Max=0.08 cfs @ 12.09 hrs HW=44.01' (Free Discharge) 1=Culvert (Barrel Controls 0.08 cfs @ 1.97 fps)

Pond 1P: Ex. CB





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Summary for Pond 2-1P: Ex CB

Inflow Area = 0.230 ac, 17.32% Impervious, Inflow Depth = 0.00" for First flush event

Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs / 3

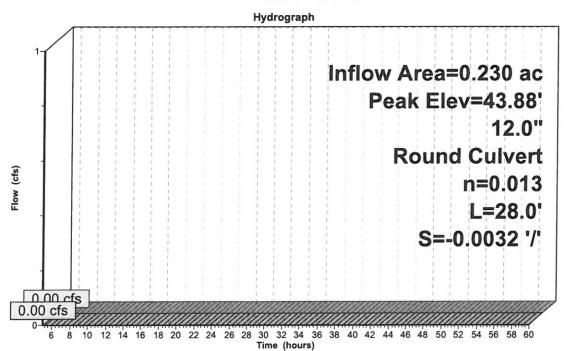
Peak Elev= 43.88' @ 5.00 hrs

Flood Elev= 48.09'

Device	Routing	Invert	Outlet Devices
#1	Primary	43.88'	12.0" Round Culvert L= 28.0' Ke= 0.500 Inlet / Outlet Invert= 43.79' / 43.88' S= -0.0032 '/' Cc= 0.900 n= 0.013. Flow Area= 0.79 sf

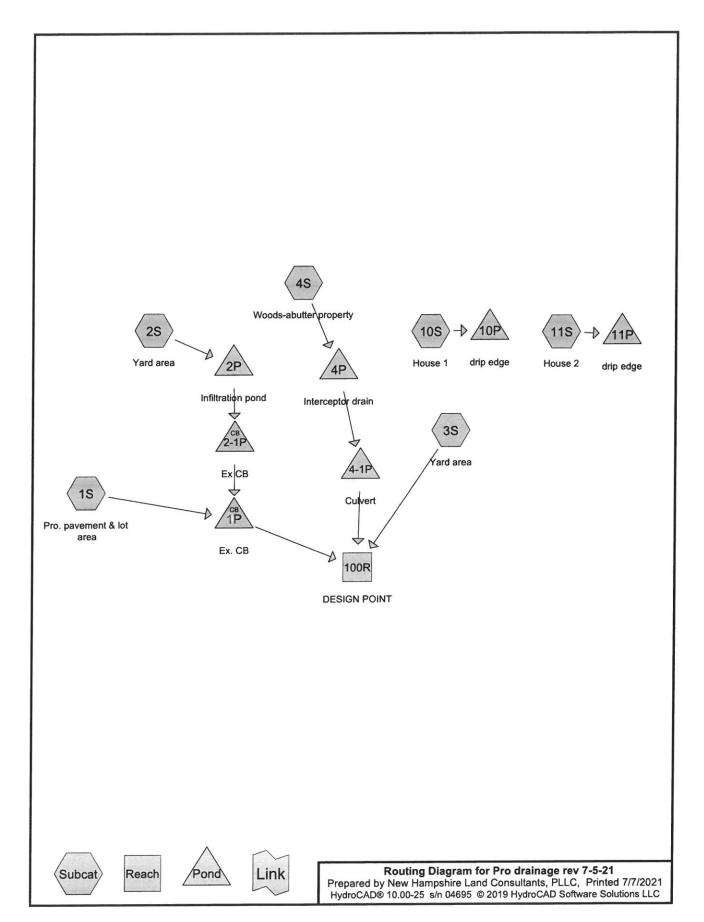
Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=43.88' (Free Discharge) 1=Culvert (Controls 0.00 cfs)

Pond 2-1P: Ex CB





Pro-Conditions Drainage Analysis Full summary 50 YR – 24 HR rainfall = 7.48"



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Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.612	61	>75% Grass cover, Good, HSG B (1S, 2S, 3S, 4S)
0.372	98	Impervious (1S, 3S, 4S, 10S, 11S)
0.045	55	Woods, Good, HSG B (3S)
1.030	74	TOTAL AREA

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Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.658	HSG B	1S, 2S, 3S, 4S
0.000	HSG C	
0.000	HSG D	
0.372	Other	1S, 3S, 4S, 10S, 11S
1.030		TOTAL AREA

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Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.612	0.000	0.000	0.000	0.612	>75% Grass cover, Good	1S, 2S,
							3S, 4S
0.000	0.000	0.000	0.000	0.372	0.372	Impervious	1S, 3S,
							4S, 10S,
							11S
0.000	0.045	0.000	0.000	0.000	0.045	Woods, Good	3S
0.000	0.658	0.000	0.000	0.372	1.030	TOTAL AREA	

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Pipe Listing (all nodes)

l	Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
	1	1P	43.86	43.54	25.0	0.0128	0.013	8.0	0.0	0.0
	2	2-1P	43.79	43.88	28.0	-0.0032	0.013	12.0	0.0	0.0
	3	2P	45.34	44.00	28.0	0.0479	0.012	4.0	0.0	0.0
	4	4-1P	53.30	52.80	19.0	0.0263	0.012	12.0	0.0	0.0
	5	4P	54.84	54.00	7.0	0.1200	0.012	12.0	0.0	0.0

Type III 24-hr 10 yr 24 hr Rainfall=4.92"

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Time span=5.00-60.00 hrs, dt=0.05 hrs, 1101 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Pro. pavement & lot area Runoff Area=14,639 sf 76.60% Impervious Runoff Depth>3.69"

Tc=5.0 min CN=89 Runoff=1.42 cfs 0.103 af

Subcatchment 2S: Yard area Runoff Area=2,923 sf 0.00% Impervious Runoff Depth=1.32"

Tc=5.0 min CN=61 Runoff=0.10 cfs 0.007 af

Subcatchment 3S: Yard area Runoff Area=5,765 sf 0.95% Impervious Runoff Depth=1.19"

Tc=5.0 min CN=59 Runoff=0.16 cfs 0.013 af

Subcatchment 4S: Woods-abutter property Runoff Area=18,317 sf 9.43% Impervious Runoff Depth=1.53"

Tc=5.0 min CN=64 Runoff=0.71 cfs 0.054 af

Subcatchment 10S: House 1 Runoff Area=1,680 sf 100.00% Impervious Runoff Depth>4.56"

Tc=5.0 min CN=98 Runoff=0.19 cfs 0.015 af

Subcatchment 11S: House 2 Runoff Area=1,524 sf 100.00% Impervious Runoff Depth>4.56"

Tc=5.0 min CN=98 Runoff=0.17 cfs 0.013 af

Reach 100R: DESIGN POINT Inflow=2.20 cfs 0.151 af

Outflow=2.20 cfs 0.151 af

Pond 1P: Ex. CB Peak Elev=44.94' Inflow=1.43 cfs 0.111 af

8.0" Round Culvert n=0.013 L=25.0' S=0.0128 '/' Outflow=1.43 cfs 0.111 af

Pond 2-1P: Ex CB Peak Elev=43.94' Inflow=0.02 cfs 0.007 af

12.0" Round Culvert n=0.013 L=28.0' S=-0.0032 '/' Outflow=0.02 cfs 0.007 af

Pond 2P: Infiltration pond Peak Elev=45.42' Storage=113 cf Inflow=0.10 cfs 0.007 af

4.0" Round Culvert n=0.012 L=28.0' S=0.0479 '/' Outflow=0.02 cfs 0.007 af

Pond 4-1P: Culvert Peak Elev=53.71' Storage=8 cf Inflow=0.66 cfs 0.027 af

12.0" Round Culvert n=0.012 L=19.0' S=0.0263 '/' Outflow=0.66 cfs 0.027 af

Pond 4P: Interceptor drain Peak Elev=55.25' Storage=191 cf Inflow=0.71 cfs 0.054 af

Discarded=0.03 cfs 0.027 af Primary=0.66 cfs 0.027 af Outflow=0.69 cfs 0.054 af

Pond 10P: drip edge Peak Elev=55.45' Storage=208 cf Inflow=0.19 cfs 0.015 af

Outflow=0.02 cfs 0.015 af

Pond 11P: drip edge Peak Elev=58.37' Storage=114 cf Inflow=0.17 cfs 0.013 af

Outflow=0.04 cfs 0.013 af

Total Runoff Area = 1.030 ac Runoff Volume = 0.206 af Average Runoff Depth = 2.40" 63.88% Pervious = 0.658 ac 36.12% Impervious = 0.372 ac

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Summary for Subcatchment 1S: Pro. pavement & lot area

[49] Hint: Tc<2dt may require smaller dt

Runoff =

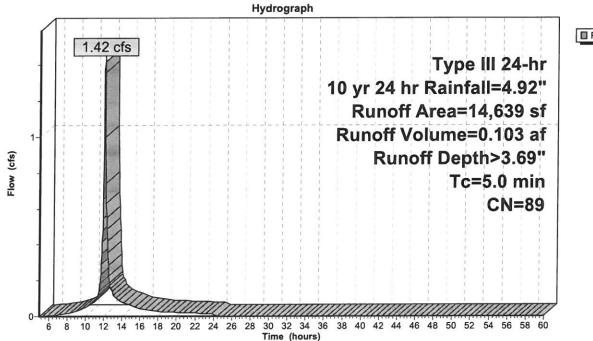
1.42 cfs @ 12.07 hrs, Volume=

0.103 af, Depth> 3.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs Type III 24-hr 10 yr 24 hr Rainfall=4.92"

400	Α	rea (sf)	CN	Description					
*		11,213	98	Impervious					
		3,426	61	>75% Gras	s cover, Go	ood, HSG B			
		14,639	89	Weighted A	Veighted Average				
		3,426		23.40% Per	23.40% Pervious Area				
		11,213		76.60% Imp	ervious Ar	rea			
	Tc (min)	Length (feet)	Slope (ft/ft	•	Capacity (cfs)	Description			
	5.0					Direct Entry, 1			

Subcatchment 1S: Pro. pavement & lot area



Runoff

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Summary for Subcatchment 2S: Yard area

[49] Hint: Tc<2dt may require smaller dt

Runoff =

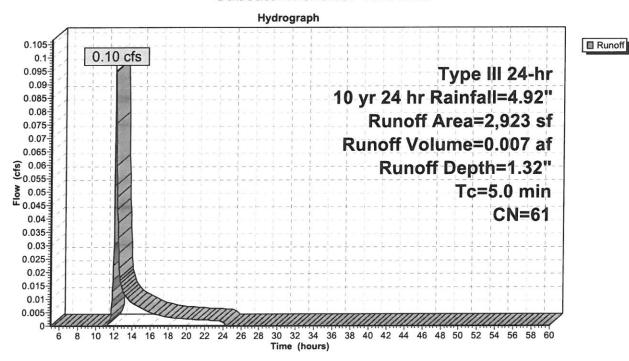
0.10 cfs @ 12.09 hrs, Volume=

0.007 af, Depth= 1.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs Type III 24-hr 10 yr 24 hr Rainfall=4.92"

Α	rea (sf)	CN I	Description						
	2,923	61	>75% Gras	75% Grass cover, Good, HSG B					
	2,923	н.	100.00% Pe	ervious Are	a				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.0					Direct Entry, 1				

Subcatchment 2S: Yard area



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Summary for Subcatchment 3S: Yard area

[49] Hint: Tc<2dt may require smaller dt

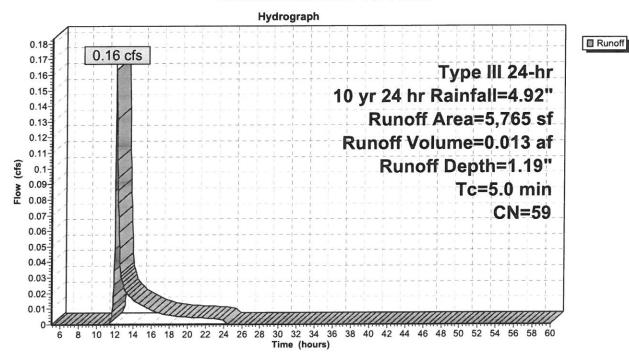
Runoff = 0.16 cfs @ 12.09 hrs, Volume=

0.013 af, Depth= 1.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs Type III 24-hr 10 yr 24 hr Rainfall=4.92"

	Α	rea (sf)	CN	Description							
		1,970	55	Woods, Go	Woods, Good, HSG B						
		3,740	61	>75% Grass cover, Good, HSG B							
*		55	98	Impervious							
		5,765	59	Weighted Average							
		5,710		99.05% Pervious Area							
		55		0.95% Impe	ervious Are	ea					
	Tc	Length	Slope		Capacity						
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	5.0					Direct Entry, 1					

Subcatchment 3S: Yard area



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Summary for Subcatchment 4S: Woods-abutter property

[49] Hint: Tc<2dt may require smaller dt

Runoff =

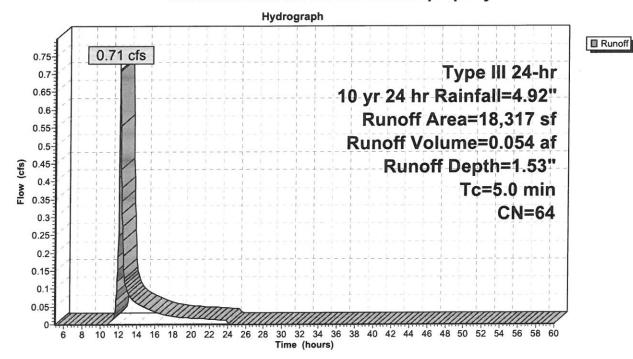
0.71 cfs @ 12.09 hrs, Volume=

0.054 af, Depth= 1.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs Type III 24-hr 10 yr 24 hr Rainfall=4.92"

-	А	rea (sf)	CN	Description					
_		16,590	61	>75% Gras	75% Grass cover, Good, HSG B				
*		1,727	98	Impervious					
		18,317 16,590 1,727		Weighted A 90.57% Per 9.43% Impe	rvious Area				
	Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description			
	5.0					Direct Entry,			

Subcatchment 4S: Woods-abutter property



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Summary for Subcatchment 10S: House 1

[49] Hint: Tc<2dt may require smaller dt

Runoff

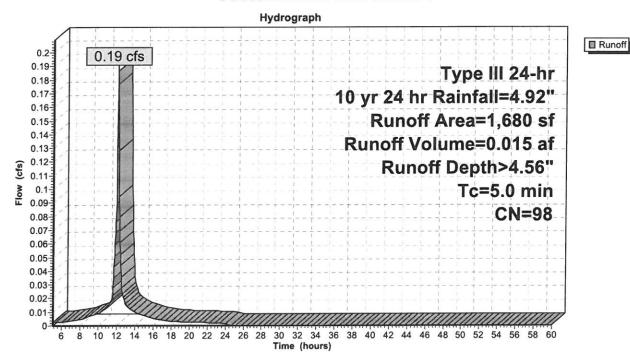
0.19 cfs @ 12.07 hrs, Volume=

0.015 af, Depth> 4.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs Type III 24-hr 10 yr 24 hr Rainfall=4.92"

	Α	rea (sf)	CN	Description		
*		1,680	98	mpervious		
		1,680	9	100.00% Im	pervious A	Area
	Tc (min)	Length	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	5.0	(feet)	(IVIL)	(IUSEC)	(CIS)	Direct Entry. 1

Subcatchment 10S: House 1



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Summary for Subcatchment 11S: House 2

[49] Hint: Tc<2dt may require smaller dt

Runoff

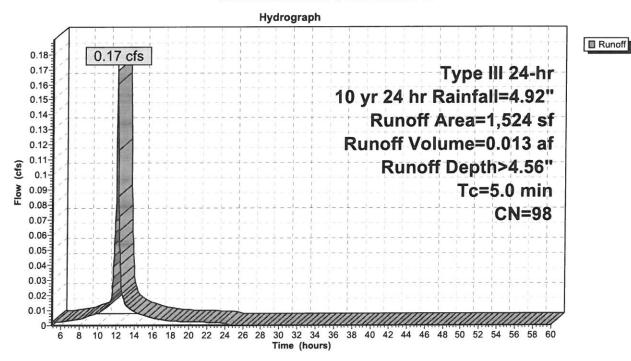
0.17 cfs @ 12.07 hrs, Volume=

0.013 af, Depth> 4.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs Type III 24-hr 10 yr 24 hr Rainfall=4.92"

	Α	rea (sf)	CN	Description			
*		1,524	98	mpervious			_
		1,524		100.00% Im	pervious A	Area	
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description	
_	5.0	(iect)	(1011)	(10300)	(013)	Direct Entry, 1	_

Subcatchment 11S: House 2



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Summary for Reach 100R: DESIGN POINT

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =

Outflow

0.956 ac, 31.20% Impervious, Inflow Depth > 1.90" for 10 yr 24 hr event

Inflow

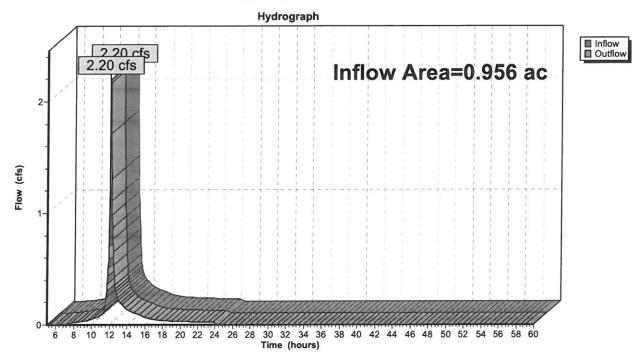
2.20 cfs @ 12.09 hrs, Volume= 2.20 cfs @ 12.09 hrs, Volume=

0.151 af

0.151 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs

Reach 100R: DESIGN POINT



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Summary for Pond 1P: Ex. CB

[81] Warning: Exceeded Pond 2-1P by 1.01' @ 12.05 hrs

Inflow Area = 0.403 ac, 63.85% Impervious, Inflow Depth > 3.30" for 10 yr 24 hr event

Inflow = 1.43 cfs @ 12.07 hrs, Volume= 0.111 af

Outflow = 1.43 cfs @ 12.07 hrs, Volume= 0.111 af, Atten= 0%, Lag= 0.0 min

Primary = 1.43 cfs @ 12.07 hrs, Volume= 0.111 af

Routing by Stor-Ind method, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs / 3

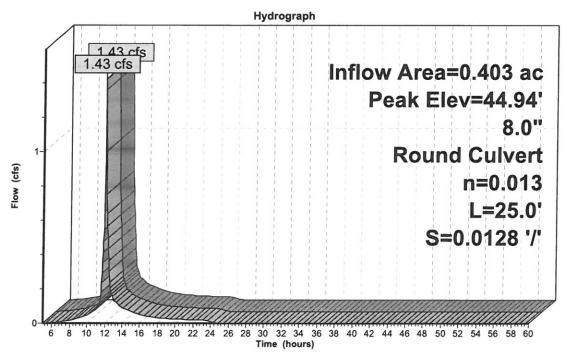
Peak Elev= 44.94' @ 12.07 hrs

Flood Elev= 47.82'

Device	Routing	Invert	Outlet Devices
#1	Primary	43.86'	8.0" Round Culvert L= 25.0' Ke= 0.500 Inlet / Outlet Invert= 43.86' / 43.54' S= 0.0128 '/' Cc= 0.900 n= 0.013, Flow Area= 0.35 sf

Primary OutFlow Max=1.37 cfs @ 12.07 hrs HW=44.89' (Free Discharge) 1=Culvert (Barrel Controls 1.37 cfs @ 3.93 fps)

Pond 1P: Ex. CB





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Summary for Pond 2-1P: Ex CB

Inflow Area = 0.067 ac, 0.00% Impervious, Inflow Depth > 1.32" for 10 yr 24 hr event Inflow = 0.02 cfs @ 12.67 hrs, Volume= 0.007 af

Outflow = 0.02 cfs @ 12.67 hrs, Volume= 0.007 af, Atten= 0%, Lag= 0.0 min

Primary = 0.02 cfs @ 12.67 hrs, Volume= 0.007 af

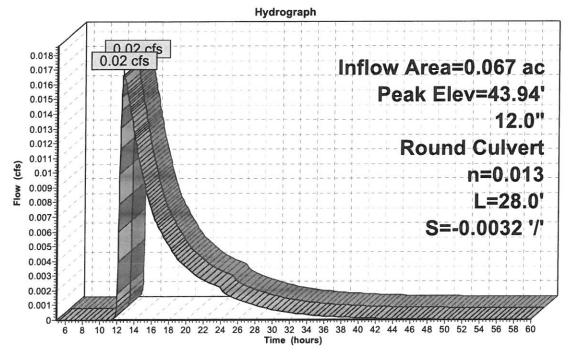
Routing by Stor-Ind method, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 43.94' @ 12.67 hrs

Flood Elev= 48.09'

Device	Routing	Invert	Outlet Devices
#1	Primary	43.88'	12.0" Round Culvert L= 28.0' Ke= 0.500 Inlet / Outlet Invert= 43.79' / 43.88' S= -0.0032 '/' Cc= 0.900 n= 0.013 Flow Area= 0.79 sf

Primary OutFlow Max=0.01 cfs @ 12.67 hrs HW=43.94' (Free Discharge) —1=Culvert (Inlet Controls 0.01 cfs @ 0.82 fps)

Pond 2-1P: Ex CB





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Summary for Pond 2P: Infiltration pond

Inflow Area = 0.067 ac, 0.00% Impervious, Inflow Depth = 1.32" for 10 yr 24 hr event 0.10 cfs @ 12.09 hrs, Volume= 0.007 af 0.02 cfs @ 12.67 hrs, Volume= 0.007 af, Atten= 83%, Lag= 35.0 min

Primary = 0.02 cfs @ 12.67 hrs, Volume= 0.007 af

Routing by Stor-Ind method, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 45.42' @ 12.67 hrs Surf.Area= 3,214 sf Storage= 113 cf Flood Elev= 48.00' Surf.Area= 327 sf Storage= 851 cf

Plug-Flow detention time= 190.5 min calculated for 0.007 af (100% of inflow)

Center-of-Mass det. time= 191.8 min (1,064.4 - 872.6)

Volume	Invert	Avail.Storage	Storage Description
#1	45.34'	849 cf	Custom Stage Data (Prismatic)Listed below (Recalc)
#2	45.84'	2 cf	4.0" Round Pipe Storage
			L= 26.0'

851 cf Total Available Storage

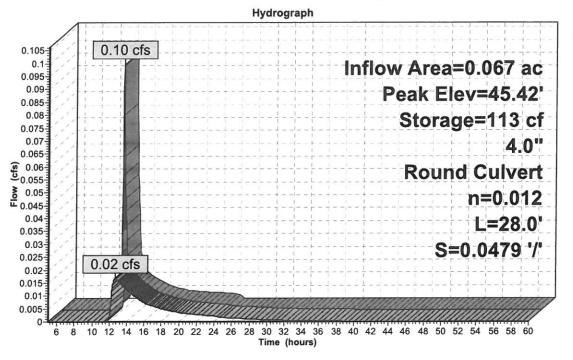
Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
45.34	3,627	0.0	0	0
46.00	327	40.0	522	522
47.00	327	100.0	327	849

Device	Routing	Invert	Outlet Devices
#1	Primary	45.34'	4.0" Round Culvert L= 28.0' Ke= 0.500 Inlet / Outlet Invert= 45.34' / 44.00' S= 0.0479'/' Cc= 0.900 n= 0.012 Flow Area= 0.09 sf

Primary OutFlow Max=0.02 cfs @ 12.67 hrs HW=45.42' (Free Discharge)
—1=Culvert (Inlet Controls 0.02 cfs @ 0.98 fps)

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Pond 2P: Infiltration pond





Type III 24-hr 10 yr 24 hr Rainfall=4.92"

Pro drainage rev 7-5-21

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Summary for Pond 4-1P: Culvert

Inflow Area = 0.421 ac, 9.43% Impervious, Inflow Depth = 0.77" for 10 yr 24 hr event 10 lnflow = 0.66 cfs @ 12.11 hrs, Volume= 0.027 af 0.66 cfs @ 12.11 hrs, Volume= 0.027 af, Atten= 1%, Lag= 0.2 min 0.66 cfs @ 12.11 hrs, Volume= 0.027 af

Routing by Stor-Ind method, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 53.71' @ 12.11 hrs Surf.Area= 29 sf Storage= 8 cf Flood Elev= 55.00' Surf.Area= 42 sf Storage= 18 cf

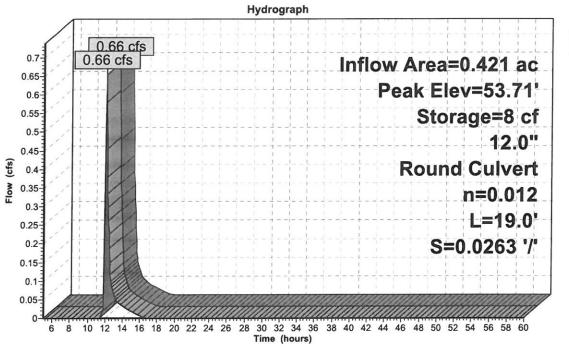
Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 0.3 min (765.5 - 765.2)

Volume	Inv	ert Avail.Sto	orage	Storage	Description		_
#1	53.	30'	18 cf	Custom	Stage Data (Pr	rismatic)Listed below (Recalc)	
Elevatio	et)	Surf.Area (sq-ft)		Store c-feet)	Cum.Store (cubic-feet)		
53.3 54.0		10 42		0 18	0 18		
Device	Routing	Invert	Outle	et Devices			
#1	Primary	53.30'	12.0' Inlet	' Round / Outlet In	Culvert L= 19.	2.80' S= 0.0263 '/' Cc= 0.900	

Primary OutFlow Max=0.64 cfs @ 12.11 hrs HW=53.70' (Free Discharge)
—1=Culvert (Inlet Controls 0.64 cfs @ 2.16 fps)

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Pond 4-1P: Culvert





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Summary for Pond 4P: Interceptor drain

Inflow Area =	0.421 ac,	9.43% Impervious, Inflow I	Depth = 1.53"	for 10 yr 24 hr event
Inflow =	0.71 cfs @	12.09 hrs, Volume=	0.054 af	
Outflow =	0.69 cfs @	12.11 hrs, Volume=	0.054 af, At	ten= 3%, Lag= 1.1 min
Discarded =	0.03 cfs @	11.60 hrs, Volume=	0.027 af	
Primary =	0.66 cfs @	12.11 hrs, Volume=	0.027 af	

Routing by Stor-Ind method, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 55.25' @ 12.11 hrs Surf.Area= 425 sf Storage= 191 cf Flood Elev= 56.00' Surf.Area= 425 sf Storage= 318 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 24.6 min (888.3 - 863.7)

Volume	Invert	Avail.Storag	e Storage	Description			
#1	54.24'	286			smatic)Listed below		
#2	54.50'	32 (of 4.0" Re	748 cf Overall - 32 cf Embedded = 716 cf x 40.0% Voids 4.0" Round Pipe Storage Inside #1 L= 366.0'			
		318	of Total Av	vailable Storage			
Elevatio			Inc.Store ubic-feet)	Cum.Store (cubic-feet)			
54.2	24	425	0	0			
56.0	00	425	748	748			
Device	Routing	Invert O	utlet Device	s			
#1	Primary			Culvert L= 7.0'	Ke= 0.500 .00' S= 0.1200 '/'	Co= 0 000	
#2	Discarded	n=	0.012, Flo	ow Area= 0.79 sf xfiltration over S		0.900	

Discarded OutFlow Max=0.03 cfs @ 11.60 hrs HW=54.26' (Free Discharge)

—2=Exfiltration (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=0.65 cfs @ 12.11 hrs HW=55.25' (Free Discharge)
—1=Culvert (Inlet Controls 0.65 cfs @ 2.17 fps)

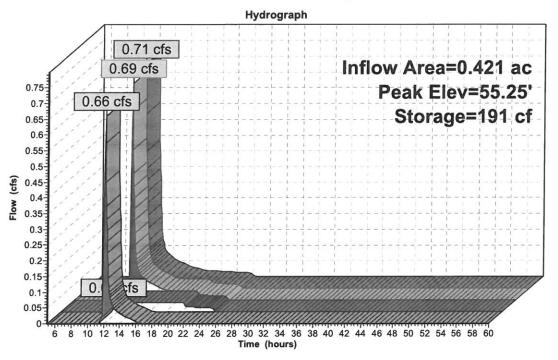
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Inflow

Outflow
Discarded

Primary

Pond 4P: Interceptor drain



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Summary for Pond 10P: drip edge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 0.039 ac,100.00% Impervious, Inflow Depth > 4.56" for 10 yr 24 hr event

Inflow = 0.19 cfs @ 12.07 hrs, Volume= 0.015 af

Outflow = 0.02 cfs @ 11.50 hrs, Volume= 0.015 af, Atten= 89%, Lag= 0.0 min

Discarded = 0.02 cfs @ 11.50 hrs, Volume= 0.015 af

Routing by Stor-Ind method, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 55.45' @ 12.73 hrs Surf.Area= 144 sf Storage= 208 cf

Flood Elev= 56.00' Surf.Area= 144 sf Storage= 288 cf

Plug-Flow detention time= 69.2 min calculated for 0.015 af (100% of inflow)

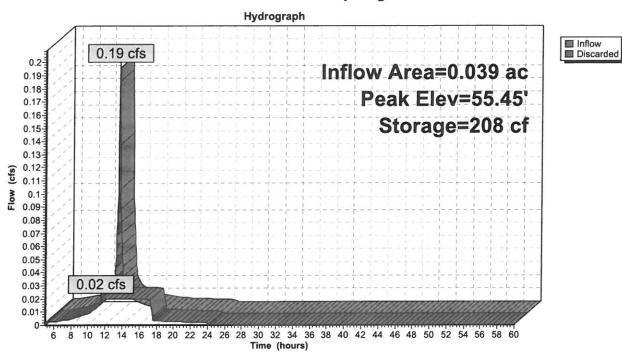
Center-of-Mass det. time= 68.7 min (830.5 - 761.8)

Volume	In	vert	Avail.Sto	rage	Storage	Description	
#1	54	.00'	2	88 cf	Custom	Stage Data (P	rismatic)Listed below (Recalc)
Elevation	on	Surf.	Area	Inc	Store	Cum.Store	
(fee	et)	(s	q-ft)	(cubic	c-feet)	(cubic-feet)	
54.0	00		144		0	0	
56.0	00		144		288	288	
Device	Routing	3	Invert	Outle	et Devices	;	
#1	Discard	led	54.00'	6.000	in/hr Ex	filtration over	Surface area

Discarded OutFlow Max=0.02 cfs @ 11.50 hrs HW=54.02' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.02 cfs)

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Pond 10P: drip edge



Type III 24-hr 10 yr 24 hr Rainfall=4.92"

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Summary for Pond 11P: drip edge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 0.035 ac,100.00% Impervious, Inflow Depth > 4.56" for 10 yr 24 hr event

Inflow = 0.17 cfs @ 12.07 hrs, Volume= 0.013 af

Outflow = 0.04 cfs @ 11.80 hrs, Volume= 0.013 af, Atten= 75%, Lag= 0.0 min

Discarded = 0.04 cfs @ 11.80 hrs, Volume= 0.013 af

Routing by Stor-Ind method, Time Span= 5.00-60.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 58.37' @ 12.43 hrs Surf.Area= 306 sf Storage= 114 cf

Flood Elev= 60.00' Surf.Area= 306 sf Storage= 612 cf

Plug-Flow detention time= 14.3 min calculated for 0.013 af (100% of inflow)

Center-of-Mass det. time= 13.6 min (775.4 - 761.8)

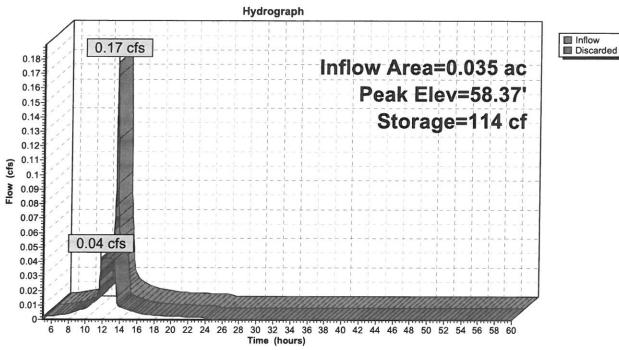
		7 (Vall. Oto	rage	Storage L	escription	
#1	58.00'	6	12 cf	Custom S	Stage Data (P	rismatic)Listed below (Recalc)
Elevation (feet)	Surf.	Area sq-ft)	Inc.	Store -feet)	Cum.Store (cubic-feet)	
58.00 60.00		306 306		0 612	0 612	
Device Rou #1 Disc	iting carded	Invert 58.00'	-	t Devices	iltration over	Curface area

Discarded OutFlow Max=0.04 cfs @ 11.80 hrs HW=58.02' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.04 cfs)

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Pond 11P: drip edge





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INSPECTION AND MAINTENANCE MANUAL

RAINFALL CHARACTERISTICS

This drainage report includes proposed conditions analysis for the site. The model was constructed using the USDA SCS TR-20 Method within the HydroCAD Stormwater Modeling System. The curve numbers were developed using the SCS TR-55 Runoff Curve numbers for Urban Areas. A Type III SCS 24-hour rainfall distribution was utilized in analyzing the data for a 50 Yr - 24 Hr (7.48") storm-event, to assure the adequacy of the proposed structure.

RAINFALL CHARACTERISTICS

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SEDIMENT & EROSION CONTROL PLANS BEST MANAGEMENT PRACTICES (BMP's)

Reference: Sheet - Proposed Conditions Plan

General Details

The proposed site development is protected from erosion and the roadways and abutting properties are protected from sediment by the use of Best Management Practices as outlined in the Stormwater Management & Erosion & Sediment Control Handbook for Urban & Developing Areas in New Hampshire. Any area disturbed by construction will be re-stabilized within 45 days and abutting properties and wetlands will not be adversely affected by this development. All swales and drainage structures will be constructed and stabilized prior to having run-off directed to them.

1 Filtrexx sock/Construction Fence

The plan set demonstrates the location of filtrexx sock for sediment control. In areas where the limits of construction need to be emphasized to operators, construction fence for added visibility will be installed. The Erosion and Sediment Control Details, has the specifications for installation and maintenance of the silt fence. Orange construction fence will be VISI Perimeter Fence by Conwed Plastic Fencing, or equal. The four-foot fencing is to be installed using six-foot posts at least two feet in the ground with six to eight feet spacing.

2 Drainage Swales / Stormwater Conveyance Channels

Drainage swales will be stabilized with vegetation for long term cover as outlined below, and using seed mixture C. As a general rule, velocities in the swale should not exceed 3.0 feet per second for a vegetated swale although velocities as high as 4.5 FPS are allowed under certain soil conditions. The use of jute matting will aid in the stabilization of vegetation.

3 Vegetated Stabilization

All areas that are disturbed during construction will be stabilized with vegetated material within 45 days of breaking ground. Construction will be managed in such a manner that erosion is prevented and that no abutter's property will be subjected to any siltation, unless otherwise permitted. All areas to be planted with grass for long-term cover will follow the specification and on Sheet E-1 using seeding mixture C, as follows:

Mixture	Pounds	Pounds per
	per Acre	1,000 Sq. Ft.
Tall Fescue	20	0.45
Creeping Red Fescue	20	0.45
Birdsfoot Trefoil	8	0.20
Total	48	1.10

4 Stabilized Construction Entrance

A temporary gravel construction entrance provides an area where mud can be dislodged from tires before the vehicle leaves the construction site to reduce the amount of mud and sediment transported onto paved municipal and state roads. The stone size for the pad should be between 1 and 2-inch coarse aggregate, and the pad itself constructed to a minimum length of 50' for the full width of the access road. The aggregate should be placed at least six inches thick. A plan view and profile are shown on Sheet E1 - Sediment and Erosion Control Detail Plan.

5 Environmental Dust Control

Dust will be controlled on the site by the use of multiple Best Management Practices. Mulching and temporary seeding will be the first line of protection to be utilized where problems occur. If dust problems are not solved by these applications, the use of water and calcium chloride can be applied. Calcium chloride will be applied at a rate that will keep the surface moist but not cause pollution.

7 Construction Sequence

- 1. Cut and remove trees and pavement in construction areas as directed or required.
- 2. Construct and/or install temporary and permanent sediment erosion and detention control facilities, as required (swales, berms, level spreaders, etc. Erosion, sediment and detention control facilities shall be installed and stabilized prior to any earth moving operation, and prior to directing run-off to them.
- 3. Clear, cut, grub, and dispose of debris in approved facilities.
- 4. Excavate and stockpile topsoil / loam. All disturbed areas shall be stabilized immediately after grading.
- 5. Begin permanent and temporary seeding and mulching. All cut and fill slopes and disturbed areas shall be seeded and mulched as required, or directed.

- Daily, or as required, construct temporary berms, drainage ditches, check dams, sediment traps, etc. to prevent erosion on the site and prevent any siltation of abutting waters or property.
- 7. Inspect and maintain all erosion and sediment control measures during construction.
- 8. Complete permanent seeding and landscaping.
- 9. Remove temporary erosion control measures after seeding areas have established themselves and site improvements are complete. Smooth and re-vegetate all disturbed areas.
- 10. All drainage structures will be constructed and stabilized prior to having run-off being directed to them.

9 Temporary Erosion Control Measures

- 1. The smallest practical area of land shall be exposed at any one time.
- 2. Erosion, sediment and detention measures shall be installed as shown on the plans and at locations as required, or directed by the engineer.
- 3. All disturbed areas shall be returned to original grades and elevations. Disturbed areas shall be loamed with a minimum of 4" of loam and seeded with not less than 1.10 pound of seed per 1,000 square feet (48 pounds per acre) of area.
- 4. Silt fences and other barriers shall be inspected periodically and after every rainstorm during the life of the project. All damaged areas shall be repaired, sediment deposits shall periodically be removed and properly disposed of.
- 5. After all disturbed areas have been stabilized, the temporary erosion control measures are to be removed and the area disturbed by the removal smoothed and revegetated.
- 6. Areas must be seeded and mulched within 5 days of final grading, permanently stabilized within 15 days of final grading, or temporarily stabilized within 45 days of initial disturbance of soil.

10 Inspection and Maintenance Schedule

Fencing will be inspected during and after storm events to ensure that the fence still has integrity and is not allowing sediment to pass. Sediment build-up will be removed if it is deeper than six inches.

CONCLUSION

Pre vs Pro comparison Discharge Point 1R

Storm Yr/24 hr 50

Existing CFS 4.36

Proposed CFS 4.30 Difference

Conclusion

The intent of this report is to evaluate the re-construction of Patricia Drive and the improvement to two proposed parcels. We have evaluated the watershed areas on the property. We have determined the best course of action would be to leave the existing two catch basins in place and add a stormwater treatment insert into the catch basin further downstream in the drainage system. These two catch basins would catch all of the road and driveway run-off. In addition to improving the two catch basins we plan on adding an inceptor swale with an underdrain along the slope to divert the overland runoff away from the road run-off and an infiltration pond at the bottom of the slope to collect the small amount of overland flow that was not collected in the interceptor swale. The infiltration pond will be equipped with an underdrain to remove any water that is not infiltrated.

A Site Specific, Terrain Alteration Permit (RSA 485: A-17) is **not** required for this site plan due to the area of disturbance is less than 100,000 square feet for AOT and a SWPPP is **not** required as the disturbance is less than 1 acre.

Respectfully Submitted,

New Hampshire Land Consultants, PLLC

Scott R Frankiewicz, LLS Project Manager

Jeff Burd, PE Project Engineer

PRE & POST WATERSHED PLANS

