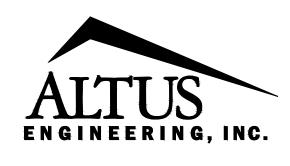
# **Residential Development Plans**

# *Owner/Applicant:*

HAPPY MOUNTAIN HOLDINGS, LLC 901 N. MARKET STREET SUITE 705 WILMINGTON, DE 19801

# Civil Engineer:



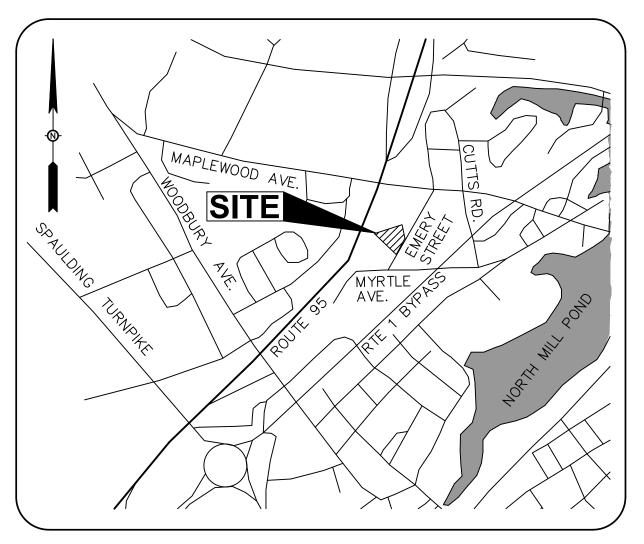
(603) 433–2335 www.ALTUS–ENG.com

133 COURT STREET PORTSMOUTH, NH 0380

# Assessor's Parcel 220-87-2 74 EMERY STREET 8 Assessor's Parcel 220-87-3 64 EMERY STREET Portsmouth, New Hampshire

# Issued:

OCTOBER 11, 2018 -SEPTEMBER 14, 2018PLANNING BOARD SUBMISSION TAC Submission



LOCUS MAP 1" = 1,000 FEET +/- Sheet Index Title

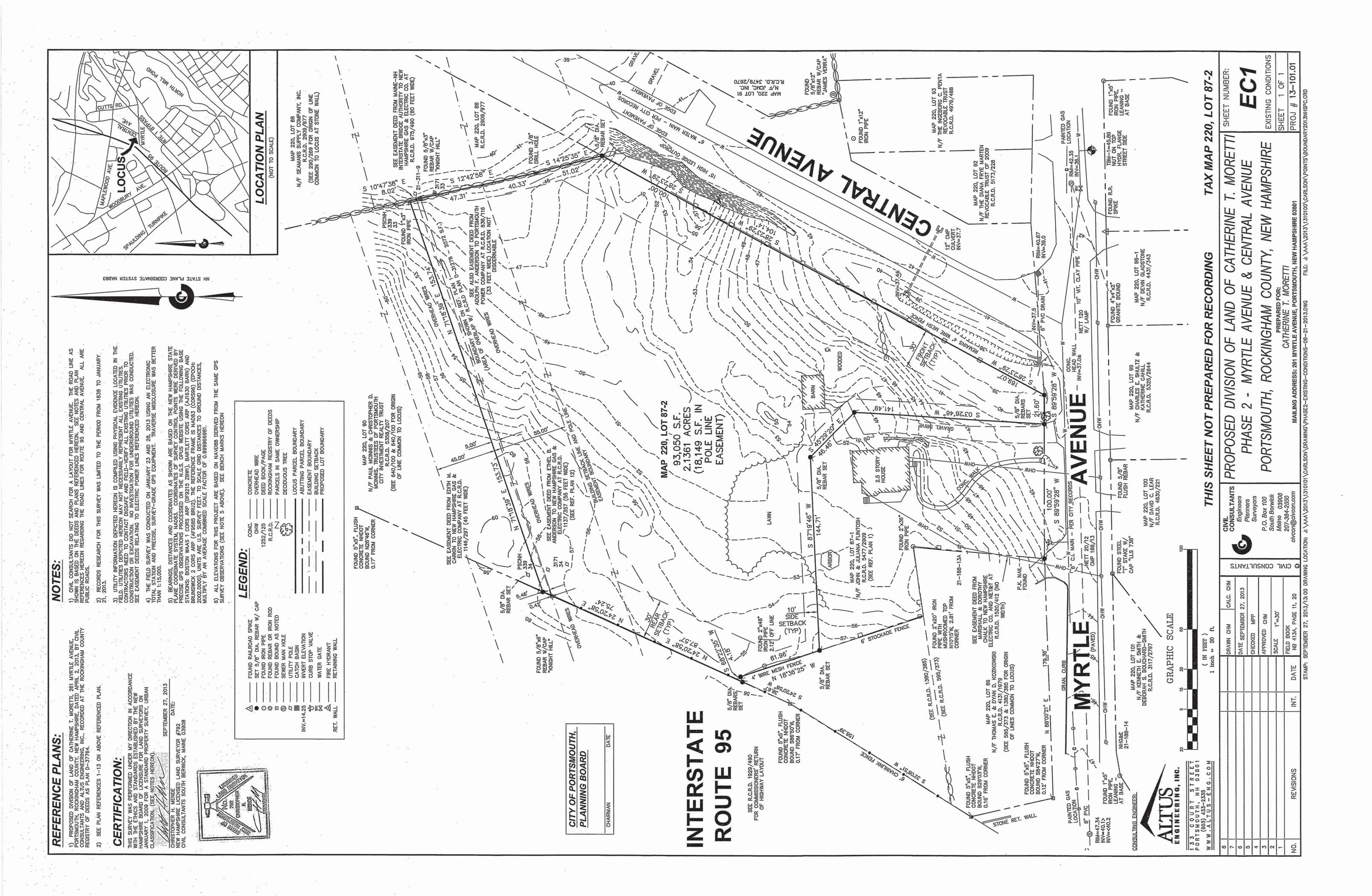
Existing Conditions Plan (by C Site Plan Grading Plan Utilities Plan General Notes & Sitework Dete Sitework Details

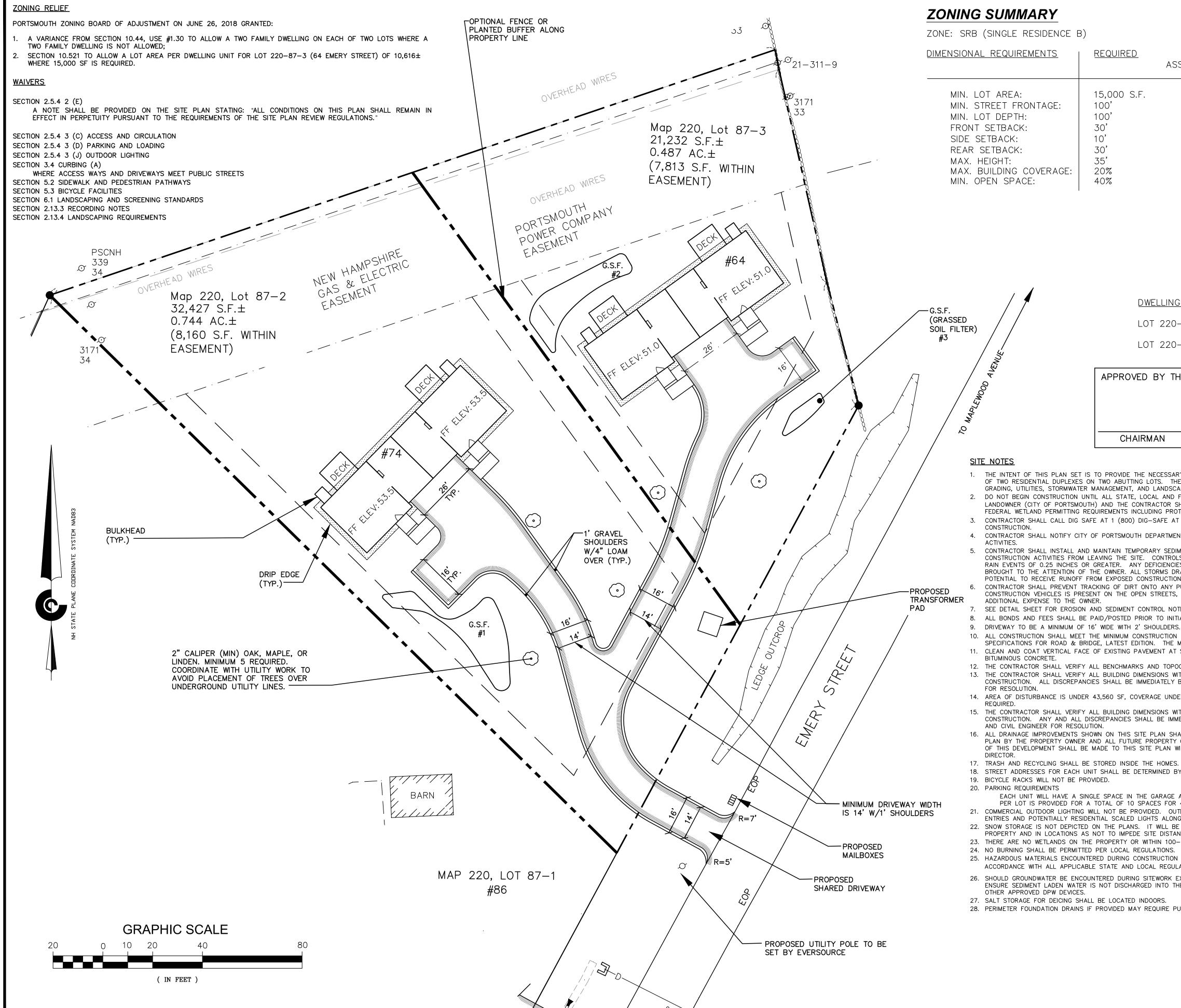
APPROVED BY THE PORTSMOUTH PLANNING BOARD

CHAIRMAN

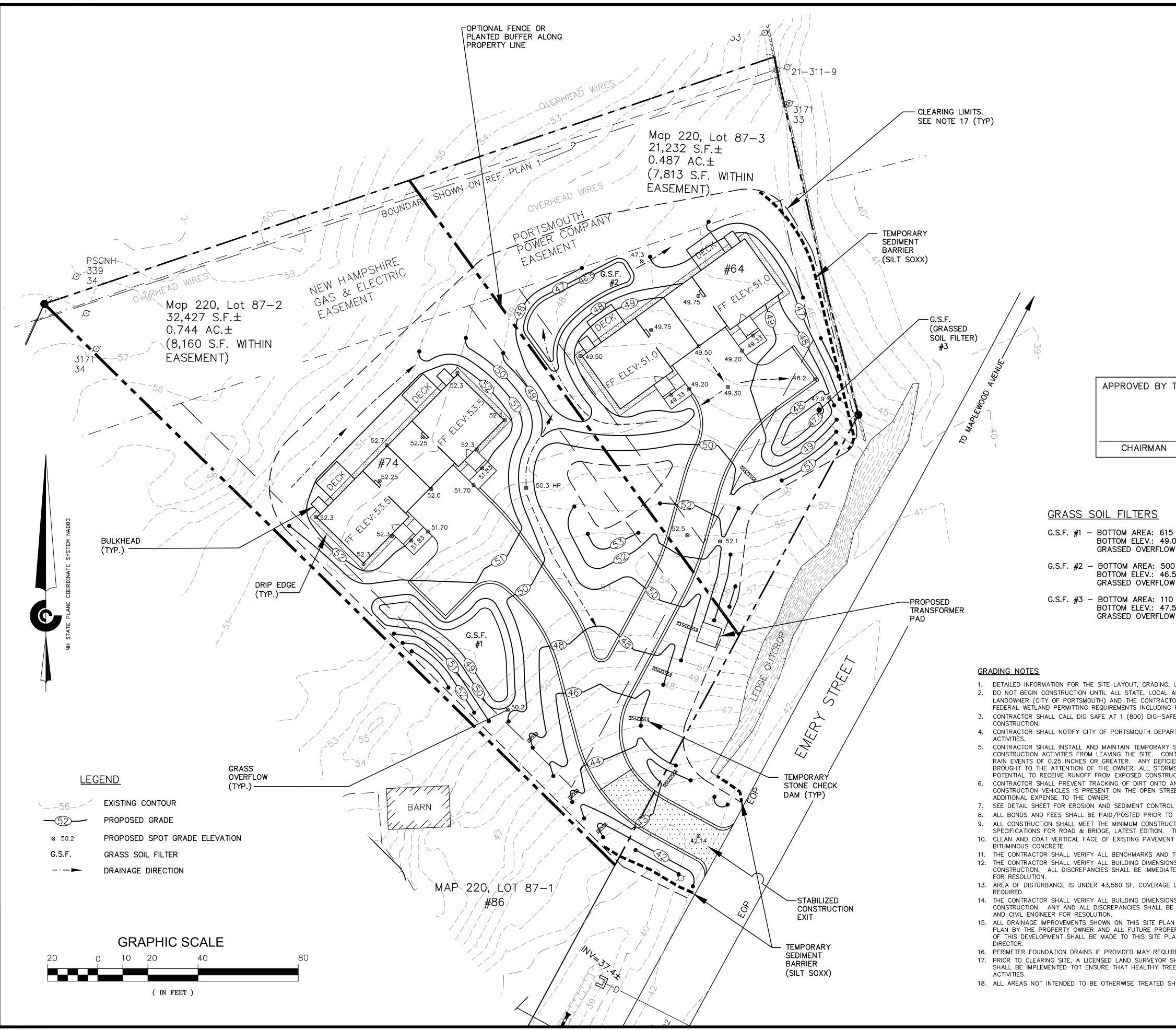
DATE

	Sheet No.:	Rev.	Date
Civil Consultants, Inc.)	EC-1	0	09/27/13
	C-1	2	10/11/18
	C-2	2	10/11/18
tails	C-3	2	10/11/18
	C-4	1	10/11/18
	C-5	0	09/14/18





<u>PROVIDED</u> SESSOR'S PARCEL ASSESSOR'S PARCEL 220-87-2 220-87-3	ALTUS ENGINEERING, INC.
$32,427$ SF $21,232$ S.F. $104'\pm$ $100'$ $224'\pm$ $146'\pm$ $125'\pm$ $56'\pm$ $14'\pm$ $14'\pm$	133 COURT STREET PORTSMOUTH, NH 03801 (603) 433–2335 www.ALTUS–ENG.com
$75' \pm$ $57' \pm$ $<35'$ $<35'$ $8.2\% \pm$ $12.5\% \pm$ $71\% \pm$ $74\% \pm$	
<u>- DENSITY PER LOT:</u> -87–2: 16,213.5 SF/DWELLING UNIT -87–3: 10,616 SF/DWELLING UNIT	ISSUED FOR:         PLANNING BOARD APPROVAL         ISSUE DATE:         OCTOBER 11, 2018         REVISIONS         NO. DESCRIPTION         BY       DATE         0       INITIAL SUBMISSION
E PORTSMOUTH PLANNING BOARD	1 PER TAC COMMENTS EDW 09/14/18 2 PB APPROVAL EDW 10/11/18
Y INFORMATION FOR THE REVIEW, PERMITTING AND DEVELOPMENT ISE PLANS PROVIDE DETAILED INFORMATION FOR THE SITE LAYOUT, IPE IMPROVEMENTS. TEDERAL PERMITS HAVE BEEN APPLIED FOR AND RECEIVED. THE HALL BE RESPONSIBLE FOR COMPLYING WITH LOCAL, STATE AND IECTION OF NATURAL RESOURCES AND THEIR BUFFERS. LEAST SEVENTY-TWO (72) HOURS PRIOR TO COMMENCING INT OF PUBLIC WORKS PRIOR TO COMMENCING CONSTRUCTION MENT AND EROSION CONTROL ITEMS TO PREVENT SEDIMENT FROM S SHALL BE INSPECTED ON A REGULAR BASIS AND AFTER ALL S IN THE CONTROLS SHALL BE ADDRESSED IMMEDIATELY AND ANIS WITHIN OR ADJACENT TO THE WORK AREA, WITH THE A AREAS, SHALL RECEIVE STORM DRAIN INLET PROTECTION. UBLIC OR PRIVATE ROADWAYS. IF TRACKING OF DIRT FROM CONTRACTOR WILL BE REQUIRED TO SWEEP THE ROADWAY AT NO ES AND DETAILS. ATING CONSTRUCTION. SHOULDERS SHALL BE PLOWED. STANDARDS OF THE CITY OF PORTSMOUTH & NHDOT'S STANDARD AGRE STRINGENT SPECIFICATION SHALL GOVERN. SAWCUT LINE WITH RS-1 IMMEDIATELY PRIOR TO PLACING NEW GRAPHY IN THE FIELD PRIOR TO CONSTRUCTION. IN THE ARCHITECTURAL AND STRUCTURAL PLANS PRIOR TO BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER IF. EPA NPDES PHASE II CONSTRUCTION GENERAL PERMIT IS NOT IN THE ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO EDIATELY BROUGHT TO THE ATTENTION OF BOTH THE ARCHITECT ALL BE CONSTRUCTED AND MAINTAINED IN ACCORDANCE WITH THE OWNERS. NO CHANGES TO THE STORMWATER MANAGMENT ASPECTS ITHOUT THE EXPRESS APPROVAL OF THE PORTSMOUTH PLANNING	DRAWN BY:       RLH         APPROVED BY:       EDW         DRAWING FILE:       4916 SITE.DWG         SCALE:       11"x17": 1" = 40'         22"x 34": 1" = 20'         APPLICANT/OWNER:         HAPPY MOUNTAIN HOLDINGS,         LLC         901 N. MARKET STREET         SUITE 705         WILMINGTON, DE 19801         PROJECT:         RESIDENTIAL         DEVELOPMENT         ASSESSOR'S PARCEL         220-87-2         74 EMERY STREET         &         ASSESSOR'S PARCEL         220-87-3         64 EMERY STREET
AND SPACE STACKED BEHIND THE GARAGE. ONE ADDITONAL SPACE 4 RESIDENTIAL UNITS. DOOR LIGHTING WILL BE LIMITED TO BUILDING MOUNTED LIGHTS AT 5 THE DRIVEWAY. ALL LIGHTS WILL BE DARK SKY FRIENDLY. STORED ALONG THE EDGE OF THE DRIVEWAY ON PRIVATE ICE AT THE DRIVEWAY. FEET OF ANY PROPOSED SITE DISTRUBANCES.	64 EMERY STREET         PORTSMOUTH,         NEW HAMPSHIRE
ACTIVITIES SHALL BE ABATED IN STRICT ATIONS. XCAVATION, BEST MANAGEMENT PRACTICES SHALL BE EMPLOYED TO E CITY DRAINAGE SYSTEM. CONTRACTOR SHALL USE SILT BAGS OR JMPING TO DAYLIGHT.	SITE PLAN
P4916	<u>SHEET NUMBER:</u> C - 1

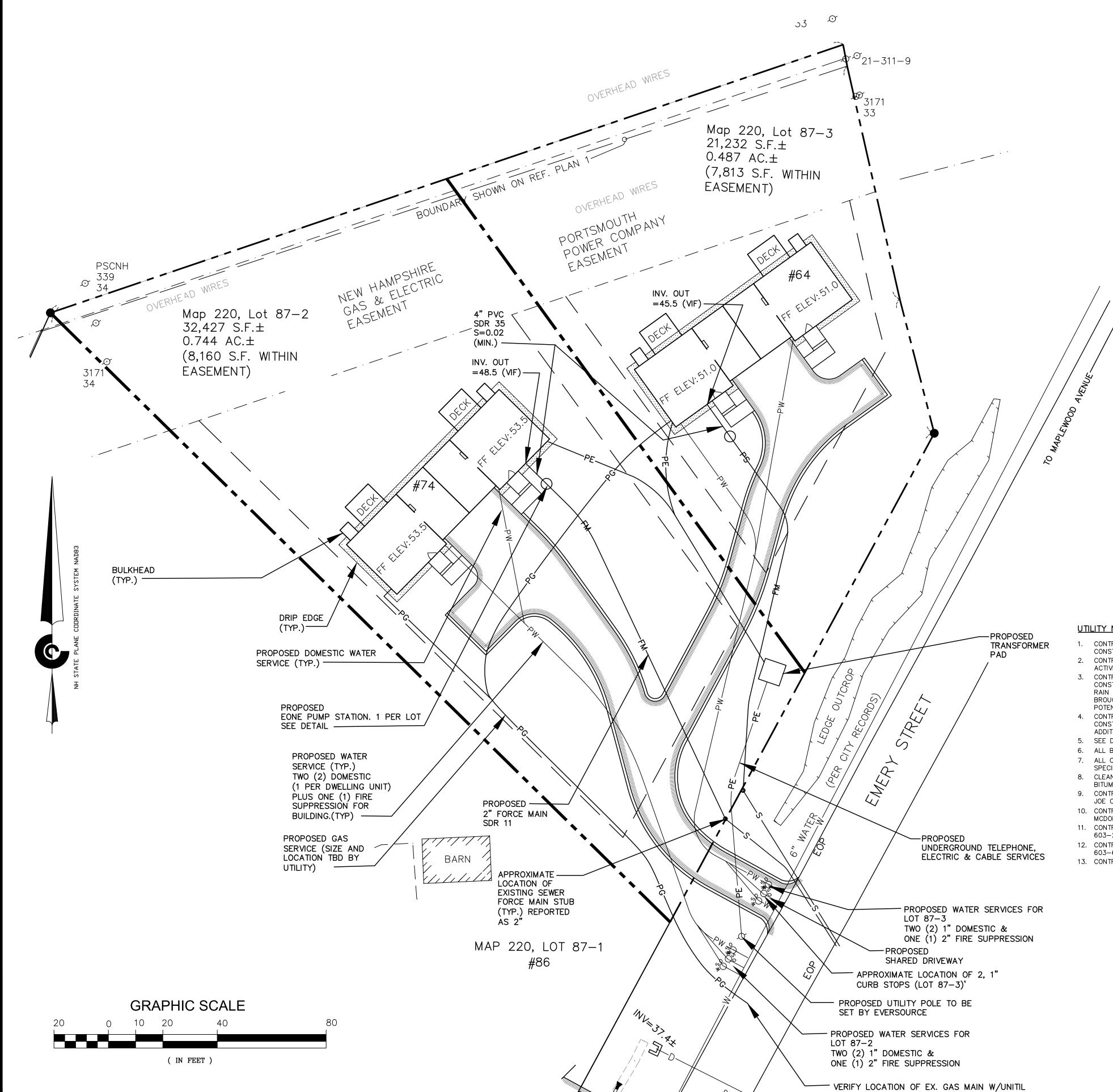


Y THE PORTSMOUTH PLANNING BOARD	ISSUED FOR: PLANNING BOARD APPROVA ISSUE DATE: OCTOBER 11, 2018 REVISIONS NO. DESCRIPTION BY DATE 0 INITIAL SUBMISSION EDW 08/07/ 1 PER TAC COMMENTS EDW 09/14/ 2 PB APPROVAL EDW 10/11/
N DATE 015 S.F. 49.0 LOW WEIR: 50.2	DRAWN BY:       RLH         APPROVED BY:       EDW         DRAWING FILE:       4916 SITE.DWG         SCALE:       11"x17": 1" = 40         22"x 34": 1" = 20
500 S.F. 46.5 LOW WEIR: 47.3 110 S.F. 47.5 LOW WEIR: 47.9	APPLICANT/OWNER: HAPPY MOUNTAIN HOLDINGS LLC 901 N. MARKET STREET SUITE 705 WILMINGTON, DE 19801
AG, UTILITIES, STORMWATER MANAGEMENT, AND LANDSCAPE IMPROVEMENTS. AL AND FEDERAL PERMITS HAVE BEEN APPLIED FOR AND RECEIVED. THE ACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH LOCAL, STATE AND NG PROTECTION OF NATURAL RESOURCES AND THEIR BUFFERS. SAFE AT LEAST SEVENTY-TWO (72) HOURS PRIOR TO COMMENCING PARTMENT OF PUBLIC WORKS PRIOR TO COMMENCING CONSTRUCTION RY SEDIMENT AND EROSION CONTROL ITEMS TO PREVENT SEDIMENT FROM CONTROLS SHALL BE INSPECTED ON A REGULAR BASIS AND AFTER ALL FICIENCIES IN THE CONTROLS SHALL BE ADDRESSED IMMEDIATELY AND DRMS DRAINS WITHIN OR ADJACENT TO THE WORK AREA, WITH THE TRUCTION AREAS, SHALL RECEIVE STORM DRAIN INLET PROTECTION. O ANY PUBLIC OR PRIVATE ROADWAYS. IF TRACKING OF DIRT FROM STREETS, CONTRACTOR WILL BE REQUIRED TO SWEEP THE ROADWAY AT NO ROL NOTES AND DETAILS. TO INITIATING CONSTRUCTION. RUCTION STANDARDS OF THE CITY OF PORTSMOUTH & NHDOT'S STANDARD . THE MORE STRINGENT SPECIFICATION SHALL GOVERN. ENT AT SAWCUT LINE WITH RS-1 IMMEDIATELY PRIOR TO PLACING NEW	PROJECT: RESIDENTIAL DEVELOPMENT ASSESSOR'S PARCEL 220-87-2 74 EMERY STREET & ASSESSOR'S PARCEL 220-87-3 64 EMERY STREET PORTSMOUTH, NEW HAMPSHIRE
ND TOPOGRAPHY IN THE FIELD PRIOR TO CONSTRUCTION. SIONS WITH THE ARCHITECTURAL AND STRUCTURAL PLANS PRIOR TO DIATELY BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER GE UNDER EPA NPDES PHASE II CONSTRUCTION GENERAL PERMIT IS NOT SIONS WITH THE ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO BE IMMEDIATELY BROUGHT TO THE ATTENTION OF BOTH THE ARCHITECT "LAN SHALL BE CONSTRUCTED AND MAINTAINED IN ACCORDANCE WITH THE OPERTY OWNERS. NO CHANGES TO THE STORMWATER MANAGMENT ASPECTS PLAN WITHOUT THE EXPRESS APPROVAL OF THE PORTSMOUTH PLANNING QUIRE PUMPING TO DAYLIGHT. R SHALL BE ENGAGED TO STAKE THE CLEARING LIMITS TREE PROTECTION TREES SCHEDULED TO REMAIN ARE NOT DISTURBED BY TEH CONSTRUCTION O SHALL RECEIVE 6" LOAM, SEED, FERTILIZER AND MULCH.	TITLE: GRADING PLAN SHEET NUMBER: C - 2

ENGINEERING, INC. 133 COURT STREET (603) 433–2335

PORTSMOUTH, NH 03801 www.ALTUS-ENG.com

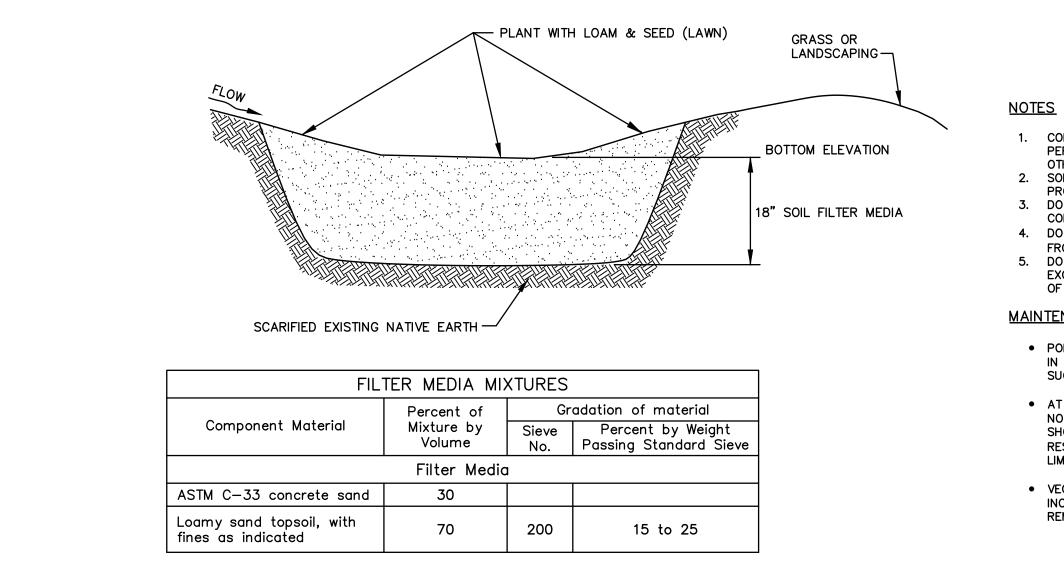
G



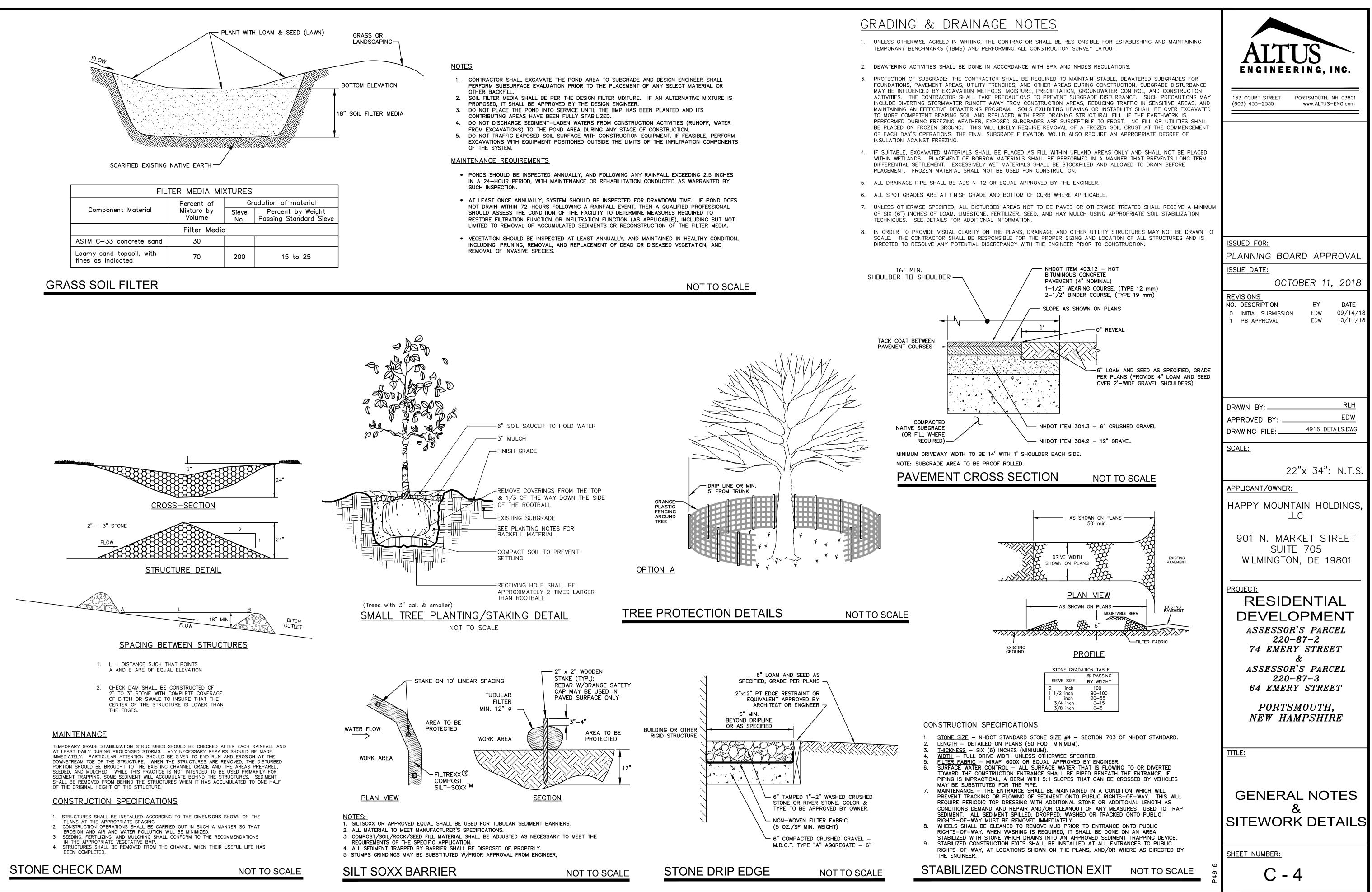
# <u>UTILITY NOTES</u>

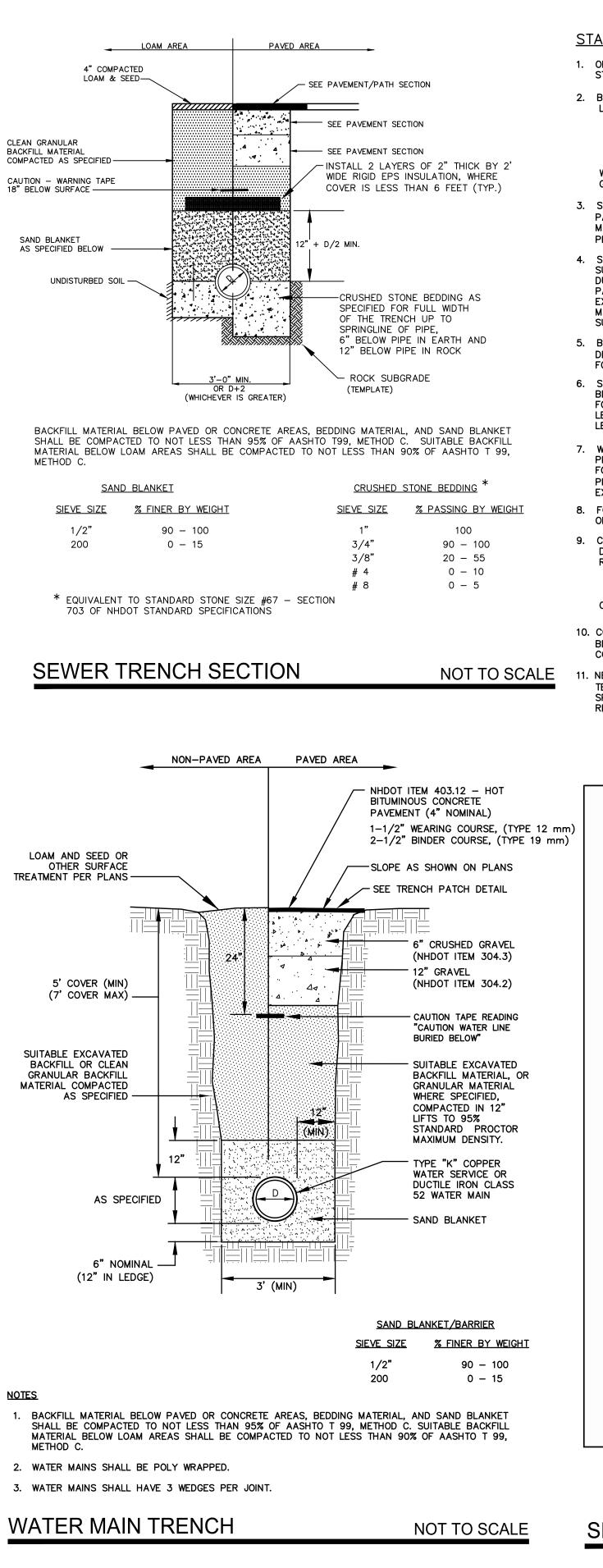
- 1. CONTRACTOR SHALL CALL DIG SAFE AT 1 (800) DIG-SAFE AT LEAST SEVENTY-TWO (72) HOURS CONSTRUCTION.
- 2. CONTRACTOR SHALL NOTIFY CITY OF PORTSMOUTH DEPARTMENT OF PUBLIC WORKS PRIOR TO COM ACTIVITIES.
- 3. CONTRACTOR SHALL INSTALL AND MAINTAIN TEMPORARY SEDIMENT AND EROSION CONTROL ITEMS CONSTRUCTION ACTIVITIES FROM LEAVING THE SITE. CONTROLS SHALL BE INSPECTED ON A REGU RAIN EVENTS OF 0.25 INCHES OR GREATER. ANY DEFICIENCIES IN THE CONTROLS SHALL BE ADD BROUGHT TO THE ATTENTION OF THE OWNER. ALL STORMS DRAINS WITHIN OR ADJACENT TO THE
- POTENTIAL TO RECEIVE RUNOFF FROM EXPOSED CONSTRUCTION AREAS, SHALL RECEIVE STORM DF 4. CONTRACTOR SHALL PREVENT TRACKING OF DIRT ONTO ANY PUBLIC OR PRIVATE ROADWAYS. IF CONSTRUCTION VEHICLES IS PRESENT ON THE OPEN STREETS, CONTRACTOR WILL BE REQUIRED I
- ADDITIONAL EXPENSE TO THE OWNER. 5. SEE DETAIL SHEET FOR EROSION AND SEDIMENT CONTROL NOTES AND DETAILS.
- 6. ALL BONDS AND FEES SHALL BE PAID/POSTED PRIOR TO INITIATING CONSTRUCTION.
- 7. ALL CONSTRUCTION SHALL MEET THE MINIMUM CONSTRUCTION STANDARDS OF THE CITY OF PORTS SPECIFICATIONS FOR ROAD & BRIDGE, LATEST EDITION. THE MORE STRINGENT SPECIFICATION SHA
- 8. CLEAN AND COAT VERTICAL FACE OF EXISTING PAVEMENT AT SAWCUT LINE WITH RS-1 IMMEDIATE
- BITUMINOUS CONCRETE.
- 9. CONTRACTOR SHALL COORDINATE ALL TELE-COMMUNICATION DISCONNECTIONS AND INSTALLATION JOE CONSIDINE @ 603-427-5525.
- 10. CONTRACTOR SHALL COORDINATE ALL ELECTRICAL CONNECTIONS/INSTALLATIONS WITH EVERSOURC
- MCDONNELL @ 603-436-7708 EXT 5555641. 11. CONTRACTOR SHALL COORDINATE ALL NATURAL GAS INSTALLATIONS WITH UNITIL CORPORATION.
- 603-294-5174 12. CONTRACTOR SHALL COORDINATE ALL CABLE CONNECTIONS/INSTALLATIONS WITH COMCAST. CONT
- 603-679-5695, EXT. 1037
- 13. CONTRACTOR SHALL LOCATE EXISTING SEWER SERVICE. CONFIRM SIZE & INVERT ELEVATION WITH &

	ALTUS ENGINEERING, INC.
	133 COURT STREETPORTSMOUTH, NH 03801(603) 433-2335www.ALTUS-ENG.com
	ISSUED FOR: PLANNING BOARD APPROVAL ISSUE DATE:
	OCTOBER 11, 2018         REVISIONS         NO. DESCRIPTION       BY       DATE         0       INITIAL SUBMISSION       EDW       08/07/18         1       PER TAC COMMENTS       EDW       09/14/18         2       PB APPROVL       EDW       10/11/18
	DRAWN BY: RLH APPROVED BY: EDW DRAWING FILE:
	11"x 17": 1" = 40' 22"x 34": 1" = 20' <u>APPLICANT/OWNER:</u> HAPPY MOUNTAIN HOLDINGS,
S PRIOR TO COMMENCING	LLC 901 N. MARKET STREET SUITE 705
S TO PREVENT SEDIMENT FROM ULAR BASIS AND AFTER ALL DRESSED IMMEDIATELY AND WORK AREA, WITH THE WAIN INLET PROTECTION.	WILMINGTON, DE 19801
TO SWEEP THE ROADWAY AT NO TSMOUTH & NHDOT'S STANDARD	RESIDENTIAL DEVELOPMENT
IALL GOVERN. TELY PRIOR TO PLACING NEW WITH CONSOLIDATED. CONTACT	ASSESSOR'S PARCEL 220–87–2 74 EMERY STREET &
CE. CONTACT: CASEY CONTACT: JANET OLIVER @ FACT: MIKE COLLINS @	ASSESSOR'S PARCEL 220–87–3 64 EMERY STREET
ENGINEER.	PORTSMOUTH, NEW HAMPSHIRE
	<u>TITLE:</u>
	UTILITIES
	PLAN
P4916	<u>Sheet Number:</u> <b>C - 3</b>



# **GRASS SOIL FILTER**

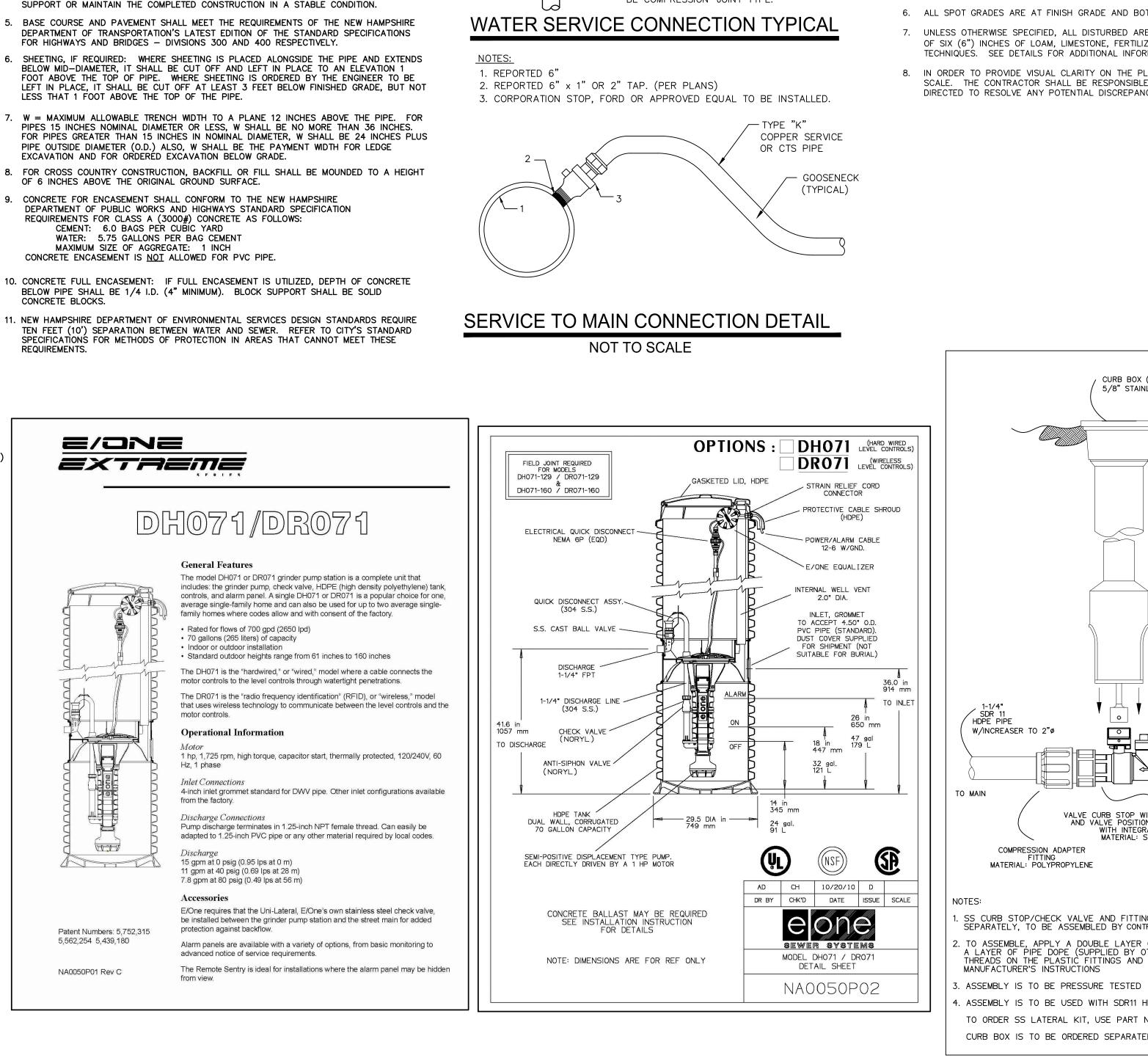




STANDARD TRENCH NOTES:

ORDERED EXCAVATION OF UNSUITABLE MATERIAL BELOW GRADE: BACKFILL AS STATED IN THE TECHNICAL SPECIFICATIONS OR AS SHOWN OF THE DRAWING.

- 2. BEDDING: SCREENED GRAVEL AND/OR CRUSHED STONE FREE FROM CLAY, LOAM, ORGANIC MATTER AND MEETING ASTM C33, STONE SIZE NO. 67. PASSING 1 INCH SCREEN 100% 90 - 100% PASSING 3/4 INCH SCREEN 20 - 55% PASSING 3/8 INCH SCREEN PASSING #4 SIEVE 0-10% 0-5% PASSING #8 SIEVE WHERE ORDERED BY THE ENGINEER TO STABILIZE THE BASE, SCREENED
- GRAVEL OR CRUSHED STONE 1-1/2 INCH TO 1/2 INCH SHALL BE USED. 3. SAND BLANKET: CLEAN SAND FREE FROM ORGANIC MATTER, SO GRADED THAT 90 - 100% PASSES 1/2 INCH SIEVE AND NOT MORE THAN 15% WILL PASS A #200 SIEVE. BLANKET MAY BE ÓMITTED FOR CAST-IRON, DUCTILE IRON, AND REINFORCED CONCRETE PIPE
- PROVIDED HOWEVER. THAT NO STONE LARGER THAN 2" IS IN CONTACT WITH THE PIPE. 4. SUITABLE MATERIAL: IN ROADS, ROAD SHOULDERS, WALKWAYS AND TRAVELED WAYS, SUITABLE MATERIAL FOR TRENCH BACKFILL SHALL BE THE NATURAL MATERIAL EXCAVATED DURING THE COURSE OF CONSTRUCTION, BUT SHALL EXCLUDE DEBRIS; PIECES OF PAVEMENT; ORGANIC MATTER; TOP SOIL; ALL WET OR SOFT MUCK, PEAT, OR CLAY; ALL EXCAVATED LEDGE MATERIAL; ALL ROCKS OVER 6 INCHES IN LARGEST DIMENSION; AND ANY MATERIAL WHICH, AS DETERMINED BY THE ENGINEER, WILL NOT PROVIDE SUFFICIENT
- LESS THAT 1 FOOT ABOVE THE TOP OF THE PIPE.
- EXCAVATION AND FOR ORDERED EXCAVATION BELOW GRADE.
- OF 6 INCHES ABOVE THE ORIGINAL GROUND SURFACE.
- REQUIREMENTS FOR CLASS A (3000#) CONCRETE AS FOLLOWS: CEMENT: 6.0 BAGS PER CUBIC YARD WATER: 5.75 GALLONS PER BAG CEMENT MAXIMUM SIZE OF AGGREGATE: 1 INCH CONCRETE ENCASEMENT IS NOT ALLOWED FOR PVC PIPE.
- CONCRETE BLOCKS.
- REQUIREMENTS.



Patent Numbers: 5,752,315 5,562,254 5,439,180

NA0050P01 Rev C

SEWER PUMP STATION (E-ONE) DETAILS

NOT TO SCALE

GRADING & DRAINAGE

- 1. UNLESS OTHERWISE AGREED IN WRITING, THE CONT TEMPORARY BENCHMARKS (TBMS) AND PERFORMING
- 2. DEWATERING ACTIVITIES SHALL BE DONE IN ACCORI
- 3. PROTECTION OF SUBGRADE: THE CONTRACTOR SHA FOUNDATIONS, PAVEMENT AREAS, UTILITY TRENCHE MAY BE INFLUENCED BY EXCAVATION METHODS, M ACTIVITIES. THE CONTRACTOR SHALL TAKE PRECA INCLUDE DIVERTING STORMWATER RUNOFF AWAY F MAINTAINING AN EFFECTIVE DEWATERING PROGRAM. TO MORE COMPETENT BEARING SOIL AND REPLACE PERFORMED DURING FREEZING WEATHER, EXPOSED BE PLACED ON FROZEN GROUND. THIS WILL LIKEL OF EACH DAY'S OPERATIONS. THE FINAL SUBGRADE INSULATION AGAINST FREEZING.
- 4. IF SUITABLE, EXCAVATED MATERIALS SHALL BE PL WITHIN WETLANDS. PLACEMENT OF BORROW MATE DIFFERENTIAL SETTLEMENT. EXCESSIVELY WET MA PLACEMENT. FROZEN MATERIAL SHALL NOT BE US
- 5. ALL DRAINAGE PIPE SHALL BE ADS N-12 OR EQU.
- 6. ALL SPOT GRADES ARE AT FINISH GRADE AND BOT
- 7. UNLESS OTHERWISE SPECIFIED, ALL DISTURBED ARI OF SIX (6") INCHES OF LOAM, LIMESTONE, FERTILIZ TECHNIQUES. SEE DETAILS FOR ADDITIONAL INFOR
- 8. IN ORDER TO PROVIDE VISUAL CLARITY ON THE PL SCALE. THE CONTRACTOR SHALL BE RESPONSIBLE DIRECTED TO RESOLVE ANY POTENTIAL DISCREPANO

RIGHT OF WAY

-6"ø VALVE

BOX COVER

OVER CURB STOP

IN PAVED AREAS

WATER MAIN

CORP. STOP FORD

-1" TYPE "K"

OR APPROVED EQUAL

VALVE BOX (TYP.)

1'-0" MIN.

COPPER SERVICE LINE, SEE PLAN.

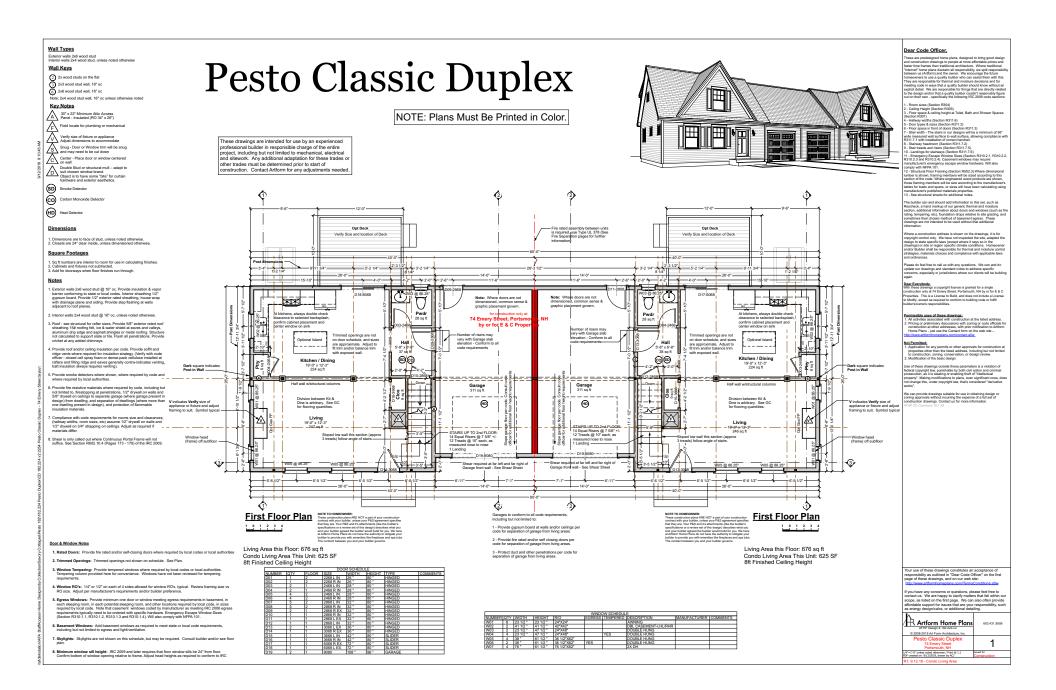
(FORD OR APPROVED EQUAL) ----

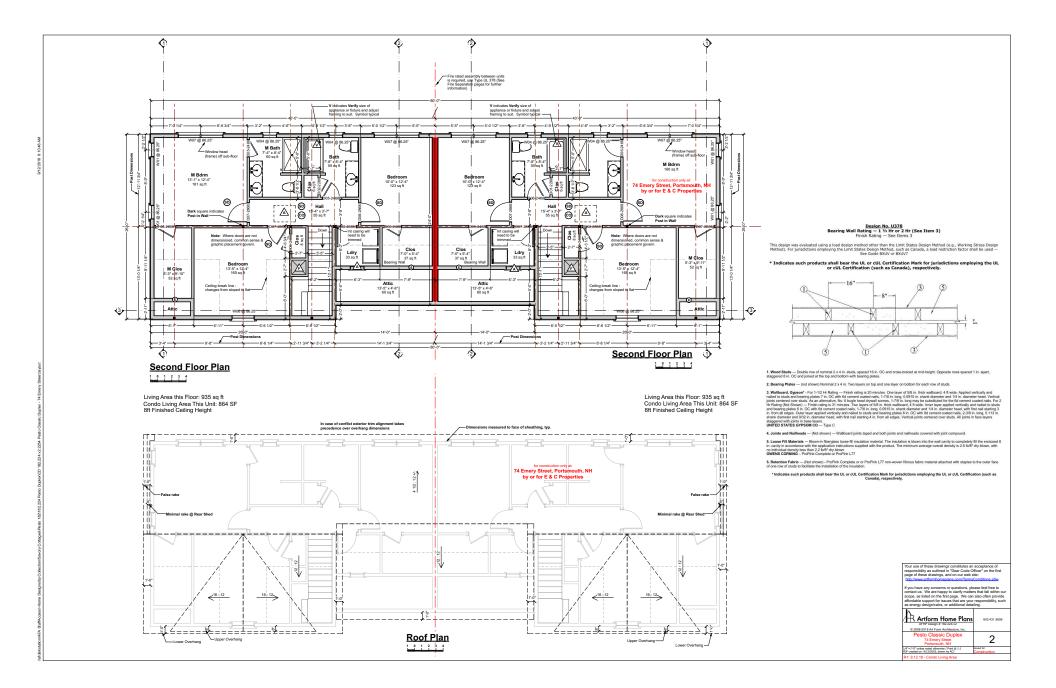
NOTE: ALL CURB AND CORP. STOPS TO

BE COMPRESSION-JOINT TYPE.

CURB STOP W/2-1/2" C.I.

RAINAGE NOTES	
N WRITING, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ESTABLISHING AND MAINTAINING 3MS) AND PERFORMING ALL CONSTRUCTION SURVEY LAYOUT.	
BE DONE IN ACCORDANCE WITH EPA AND NHDES REGULATIONS.	ALTUS ENGINEERING, INC.
THE CONTRACTOR SHALL BE REQUIRED TO MAINTAIN STABLE, DEWATERED SUBGRADES FOR EAS, UTILITY TRENCHES, AND OTHER AREAS DURING CONSTRUCTION. SUBGRADE DISTURBANCE AVATION METHODS, MOISTURE, PRECIPITATION, GROUNDWATER CONTROL, AND CONSTRUCTION	
R SHALL TAKE PRECAUTIONS TO PREVENT SUBGRADE DISTURBANCE. SUCH PRECAUTIONS MAY TER RUNOFF AWAY FROM CONSTRUCTION AREAS, REDUCING TRAFFIC IN SENSITIVE AREAS, AND EWATERING PROGRAM. SOILS EXHIBITING HEAVING OR INSTABILITY SHALL BE OVER EXCAVATED	133 COURT STREETPORTSMOUTH, NH 03801(603) 433-2335www.ALTUS-ENG.com
G SOIL AND REPLACED WITH FREE DRAINING STRUCTURAL FILL. IF THE EARTHWORK IS WEATHER, EXPOSED SUBGRADES ARE SUSCEPTIBLE TO FROST. NO FILL OR UTILITIES SHALL IND. THIS WILL LIKELY REQUIRE REMOVAL OF A FROZEN SOIL CRUST AT THE COMMENCEMENT	
THE FINAL SUBGRADE ELEVATION WOULD ALSO REQUIRE AN APPROPRIATE DEGREE OF G.	
ERIALS SHALL BE PLACED AS FILL WITHIN UPLAND AREAS ONLY AND SHALL NOT BE PLACED NT OF BORROW MATERIALS SHALL BE PERFORMED IN A MANNER THAT PREVENTS LONG TERM EXCESSIVELY WET MATERIALS SHALL BE STOCKPILED AND ALLOWED TO DRAIN BEFORE AL SHALL NOT BE USED FOR CONSTRUCTION.	
E ADS N-12 OR EQUAL APPROVED BY THE ENGINEER.	
NISH GRADE AND BOTTOM OF CURB WHERE APPLICABLE.	
, LIMESTONE, FERTILIZER, SEED, AND HAY MULCH USING APPROPRIATE SOIL STABILIZATION OR ADDITIONAL INFORMATION.	
L CLARITY ON THE PLANS, DRAINAGE AND OTHER UTILITY STRUCTURES MAY NOT BE DRAWN TO HALL BE RESPONSIBLE FOR THE PROPER SIZING AND LOCATION OF ALL STRUCTURES AND IS POTENTIAL DISCREPANCY WITH THE ENGINEER PRIOR TO CONSTRUCTION.	ISSUED FOR:
	PLANNING BOARD APPROVAL ISSUE DATE:
	SEPTEMBER 14, 2018
	REVISIONS NO. DESCRIPTION BY DATE
	0 INITIAL SUBMISSION EDW 09/14/18
	DRAWN BY:
CURB BOX (ERIE STYLE W/ LATERAL KIT 5/8" STAINLESS STEEL ROD)	APPROVED BY:EDW
SDR 11 HDPE PIPE	DRAWING FILE: 4916 DETAILS.DWG
	<u>SCALE:</u>
	22"x 34": N.T.S.
	APPLICANT/OWNER:
	LLC
	901 N. MARKET STREET
	SUITE 705 WILMINGTON, DE 19801
COMPRESSION ADAPTER FITTING MATERIAL: POLYPROPYLENE	RESIDENTIAL
	DEVELOPMENT
	ASSESSOR'S PARCEL 220–87–2
	74 EMERY STREET &
	ASSESSOR'S PARCEL 220–87–3
VALVE CURB STOP WITH FEMALE PIPE THREADS AND VALVE POSITION STOPS (OPEN/CLOSED) WITH INTEGRAL CHECK VALVE MATERIAL: STAINLESS STEEL	64 EMERY STREET
ADAPTER (CONTRACTOR)	PORTSMOUTH, NEW HAMPSHIRE
CK VALVE AND FITTINGS ARE PROVIDED ASSEMBLED BY CONTRACTOR	<u>TITLE:</u>
A DOUBLE LAYER OF TEFLON TAPE, AND OPE (SUPPLIED BY OTHERS) TO THE ASTIC FITTINGS AND INSTALL PER THE KIT PARTS ARE NOT ASSEMBLED	
E PRESSURE TESTED	
RAL KIT, USE PART NUMBER NC0193G01 SEWER SYSTEMS	SITEWORK DETAILS
E ORDERED SEPARATELY, SEE ABOVE 1-1/4" SDR 11 HDPE PIPE	SHEVVORK DETAILS
STAINLESS STEEL LATERAL KIT	SHEET NUMBER:
<u>- 1 1/4" SDR 11 HDPE PIPE</u> NOT TO SCALE	C - 5





### Structural General Notes:

1. Builder shall consult and follow the building code and othe suider shall consult and follow the building code and other regulations in effect for the building site for all construction details not shown in these drawings. Requirements described here are specific to this design and/or are provided as reference. Additional building code or local requirements may apply.

2. Builder shall m intain a safe worksite, including b to, provision of temporary supports where appropriate and adherence to applicable safety standards.

Design is based on the snow load listed on the framing plans, 90 mph basic wind speed, Exposure type B, soil bearing capacity of 2000 psf, and Seismic Categooy C, unless otherwise noted on the framing plans. Builder shall promptly inform Artform Home Plans of differing conditions.

Foundations

1. No footing shall be poured on loose or unsuitable soils, in water or on frozen ground.

2. All exterior footings to conform to all applicable code requirements for frost protection.

3. All concrete shall have a minimum compre least 3000 PSI at 28 days.

- Constantion characge to comply with IRC 2009 Section Re30.15, it that is constant of minimum size. 1/2" dementer sendors Re30.15, it that is constant of minimum size. 1/2" dementer sendors have stantistic and "Lo for more that have softnes, max of 1/2" form each conser, min of 2 bots per wall. Anchor toot shall extend 7 manore that a group under may be counted by ny uncodes afford as a story. Additional anchorage may be required at brance with a group. Additional anchorage may be required at brance with a group. Additional anchorage may be required at brance within a constant of the soft anchorage may be required at brance within the soft and the soft and the soft and the soft and and the soft and and the soft and the

Wood Framing

All structural wood shall be identified by a grade mark or certificate of inspection by a recognized inspection agency.

2. Structural wood shall be Spruce-Pine-Fir (SPF) #2 or better

When used, LVL or PSL indicate Laminated Veneer Lumber or Parallel Strand Lumber, respectively. Products used shall equal or exceed the strangh properties for the size indicated as manufacturered by TrusJoist.

4. When used, AUS indicates wood I-joists as manufactured by Boise Cascade. Products of alternate manufacturers may be substituted provided they meet or exceed the strength properties for the member specified.

All floor joists shall have bridging installed at mid-span or at 8:0° or maximum

Floor systems are designed for performance with subfloor glued and screwed.

At posts, provide solid framing/blocking to supports below. Provide minimum 1 1/2\* bearing length for all beams and headers, unless noted otherwise.

All wood permanently exposed to the weather, in conta concrete or in contact with the ground shall meet code requirements for wood in these environments.

Deck ledgers shall be securely attached to the structure and/ or independently supported, including against lateral movement, per building code requirements and best practices. Unless otherwise noted, decks shall have solid 444 pt posts up to 6 th above grade, and solid 8x8 for heights above that.

Wherever beams are noted as Flush framed, install joist hangers at all joists, sized appropriately for the members being connected.

Support the lower end of roof beams via minimum 2<sup>e</sup> horizontal bearing on a post, ledger or via an appropriately sized and configured hanger.

Where multiple beams are supported on one post, provide min 2<sup>n</sup> bearing for each, via either appropriately sized post cap or additional post(s).

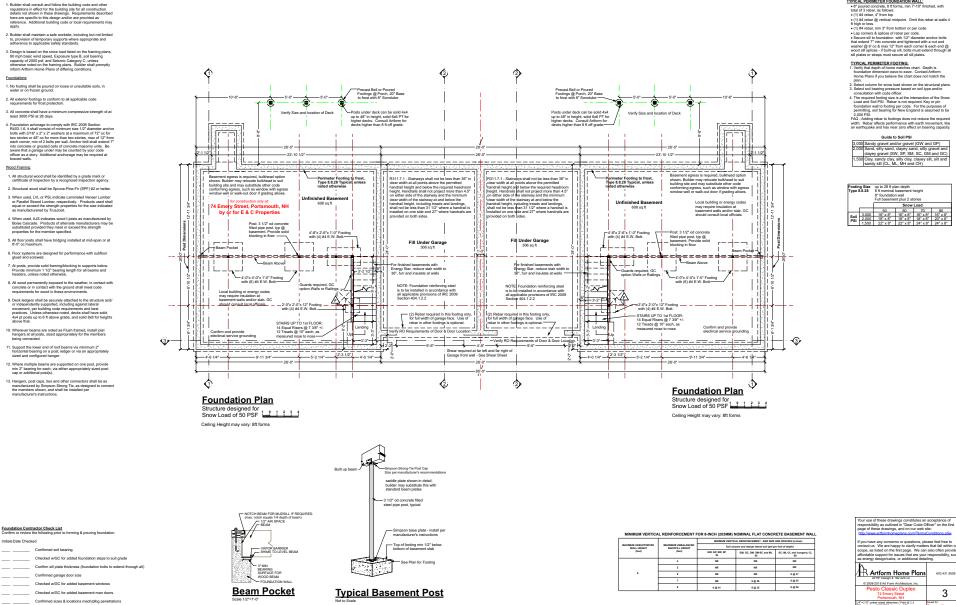
Hangers, post caps, lies and other connectors shall be as manufactured by Simpson Strong Tie, as designed to connect the members shown, and shall be installed per manufacturer's instructions.

ned soil bearing

Confirmed garage door size

Confirmed sizes and locations of beams w/GC, added or adjusted beam pockets

nitials Date Checked



TYPICAL PERIMETER FOUNDATION WALL:

603.431.9559

3

1'-0" unless noted otherwise / Print @ 1:1 created on: 9/12/2018, drawn by ACJ





4"=1"-0" unless noted offwreide Print (9:1:1 M" created on: 9/12/2018, deaven by AC2 11: 9.12.18 - Condo Living Area



Civil Site Planning Environmental Engineering

133 Court Street Portsmouth, NH 03801-4413

October 9, 2018

Juliet T. H. Walker, AICP, Planning Director City of Portsmouth Municipal Complex Planning Department 1 Junkins Avenue Portsmouth, New Hampshire 03801

Re: Application for Site Plan Review - Planning Board Approval Assessor's Map 220, Lots 87-2 and 87-3 74 and 64 Emery Street Altus Project P4916

### Dear Juliet:

On behalf of Corey Cawthorn and Happy Mountain Holdings, LLC, Altus Engineering, Inc. (Altus) respectfully submits the revised Site Plan Review application package for the properties located at 64 and 74 Emery Street. The project proposes to construct one two-unit residential building on each lot, each building to be 2-stories with a 2,080 s.f. footprint and a 3,000 s.f. Gross Floor Area, with related paving, lighting, utilities, landscaping, drainage and associated site improvements. Although the project is residential in nature, the development requires Site Plan Approval as four dwelling units will be constructed. As such, we believe many of the "standard" Site Plan Review requirements are not relevant and merit obtaining waivers. The waiver requests are included in this submittal along with the additional supporting documentation.

On June 26, 2018, Happy Mountain Holdings obtained Zoning relief from Section 10.44 and Section 10.521 of the City of Portsmouth Zoning Ordinance to allow the construction of two multi-family dwellings (duplex) where only single family residential units are allowed and a variance from Section 10.521 for Lot 87-3 for lot size of 21,232 SF where 30,000 SF is required. The applicant met with the Technical Advisory Committee (TAC) work session on August 14, 2018 and after addressing initial comments from the work session, the project was considered at the October 2, 2018 TAC Meeting where the Committee voted to **recommend** Site Plan approval to the Planning Board.

Juliet T. H. Walker, AICP, Planning Director October 9, 2018 Page 2

The following stipulations were noted by TAC and are addressed below:

1. Applicant shall show outline of proposed stormwater areas on the site plan so that it is clear to future homeowners.

Response: The stormwater areas have been added to the site plan, sheet C-1.

2. Stormwater features must be maintained by the owners in perpetuity. Stormwater system maintenance and enforcement oversight by City of Portsmouth shall be documented in a deed restriction. The deed restriction for stormwater maintenance shall be recorded and include language that notes any changes shall require review and approval by the Planning Director.

Response: Agreed. The deed restriction will be provided prior to CO.

3. An easement shall be required between the two properties to allow stormwater to drain across lot lines.

Response: Agreed. All easements will be provided prior to CO.

- A Conditional Use Permit shall be required from the Planning Board to comply with the new Highway Noise Overlay District.
   Response: The Owner is currently working with Planning Department to address this new zoning regulation.
- 5. The applicant may reduce the overall driveway width to 14' driveway as previously proposed.

Response: The driveway width has been reduced to 14' indicated.

- Applicant shall provide documentation of utility and driveway access easements prior to the issuance of a Certificate of Occupancy for either property. Response: Agreed. Easements will be provided prior to CO.
- 7. Applicant shall provide a landscaping plan that includes limits of clearing, loaming and seeding.

Response: As discussed at TAC, a minimum of 5 trees are required and are shown on the site plan. All other disturbed areas will be lawn.

8. Temporary check dams shall be placed during construction to address any impact to abutting property.

Response: Temporary check dams are shown on the grading plan.

 Drainage and grading shall be updated and clarified to address TAC comments and approved by Planning and DPW staff prior to Planning Board review.
 Response: The grading plan has been updated and is submitted for final approval. Juliet T. H. Walker, AICP, Planning Director October 9, 2018 Page 3

The Applicant will submit twelve (12) copies of the following items for consideration at the October 18<sup>th</sup> Planning Board Meeting:

- Site Plan Review Application and Checklist
- Site Plans (two full size (22" x 34") and ten half size (11" x 17"))
- Zoning Board of Adjustment decision letter, dated June 29, 2018.
- Waiver Requests
- Sitework Cost Estimate
- Autoturn Truck Turning Movements
- Drainage Study (two full copies and ten summaries)
- CD with pdf copies of the complete application package.

As always, Altus looks to working with the Planning Department on this development project. Please call me if you have any questions or need any additional information.

Sincerely,

ALTUS ENGINEERING, INC.

Eric D. Weinrieb, PE President

wde/4916-PB cvr ltr\_101918

Enclosure

Ecopy: Corey Cawthorn



Civil Site Planning Environmental Engineering

133 Court Street Portsmouth, NH 03801-4413

October 9, 2018

Juliet T. H. Walker, AICP, Planning Director City of Portsmouth Municipal Complex Planning Department 1 Junkins Avenue Portsmouth, New Hampshire 03801

Re: Application for Site Plan Review - Planning Board Approval Assessor's Map 220, Lots 87-2 and 87-3 74 and 64 Emery Street Altus Project P4916

Dear Juliet:

On behalf of Corey Cawthron and Happy Mountain Holdings, LLC, Altus Engineering, Inc. (Altus) respectfully submits the revised Site Plan Review application package for the properties located at 64 and 74 Emery Street. The project proposes to construct one two-unit residential building on each lot, each building to be 2-stories with a 2,080 s.f. footprint and a 3,000 s.f. Gross Floor Area, with related paving, lighting, utilities, landscaping, drainage and associated site improvements. Although the project is residential in nature, the development requires Site Plan Approval as four dwelling units will be constructed. As such, we believe many of the "standard" Site Plan Review requirements are not relevant and merit obtaining waivers. The waiver requests are included in this submittal along with the additional supporting documentation.

On June 26, 2018, Happy Mountain Holdings obtained Zoning relief from Section 10.44 and Section 10.521 of the City of Portsmouth Zoning Ordinance to allow the construction of two multi-family dwellings (duplex) where only single family residential units are allowed and a variance from Section 10.521 for Lot 87-3 for lot size of 21,232 SF where 30,000 SF is required. The applicant met with the Technical Advisory Committee (TAC) work session on August 14, 2018 and after addressing initial comments from the work session, the project was considered at the October 2, 2018 TAC Meeting where the Committee voted to **recommend** Site Plan approval to the Planning Board.

Juliet T. H. Walker, AICP, Planning Director October 9, 2018 Page 2

The following stipulations were noted by TAC and are addressed below:

- Applicant shall show outline of proposed stormwater areas on the site plan so that it is clear to future homeowners.
   Response: The stormwater areas have been added to the site plan, sheet C-1.
- 2. Stormwater features must be maintained by the owners in perpetuity. Stormwater system maintenance and enforcement oversight by City of Portsmouth shall be documented in a deed restriction. The deed restriction for stormwater maintenance shall be recorded and include language that notes any changes shall require review and approval by the Planning Director.

Response: Agreed. The deed restriction will be provided prior to CO.

3. An easement shall be required between the two properties to allow stormwater to drain across lot lines.

Response: Agreed. All easements will be provided prior to CO.

- A Conditional Use Permit shall be required from the Planning Board to comply with the new Highway Noise Overlay District.
   Response: The Owner is currently working with Planning Department to address this new zoning regulation.
- 5. The applicant may reduce the overall driveway width to 14' driveway as previously proposed.

Response: The driveway width has been reduced to 14' indicated.

- Applicant shall provide documentation of utility and driveway access easements prior to the issuance of a Certificate of Occupancy for either property. Response: Agreed. Easements will be provided prior to CO.
- Applicant shall provide a landscaping plan that includes limits of clearing, loaming and seeding.
   Response: As discussed at TAC, a minimum of 5 trees are required and are shown on the site plan.

Response: As discussed at TAC, a minimum of 5 trees are required and are shown on the site plan. All other disturbed areas will be lawn.

8. Temporary check dams shall be placed during construction to address any impact to abutting property.

Response: Temporary check dams are shown on the grading plan.

 Drainage and grading shall be updated and clarified to address TAC comments and approved by Planning and DPW staff prior to Planning Board review.
 Response: The grading plan has been updated and is submitted for final approval. Juliet T. H. Walker, AICP, Planning Director October 9, 2018 Page 3

The Applicant will submit twelve (12) copies of the following items for consideration at the October 18<sup>th</sup> Planning Board Meeting:

- Site Plan Review Application and Checklist
- Site Plans (two full size (22" x 34") and ten half size (11" x 17"))
- Zoning Board of Adjustment decision letter, dated June 29, 2018.
- Waiver Requests
- Sitework Cost Estimate
- Autoturn Truck Turning Movements
- Drainage Study (two full copies and ten summaries)
- CD with pdf copies of the complete application package.

As always, Altus looks to working with the Planning Department on this development project. Please call me if you have any questions or need any additional information.

Sincerely,

ALTUS ENGINEERING, INC.

Eric D. Weinrieb, PE

Eric D. Weinrieb, PE President

wde/4916-PB cvr ltr\_101918

Enclosure

Ecopy: Corey Cawthorn

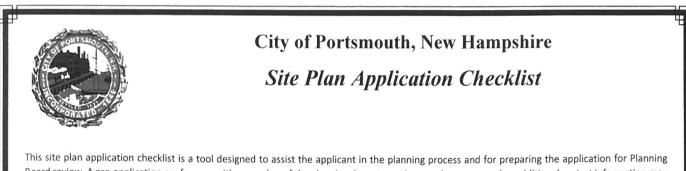
<b>CITY OF PORTSMOUTH</b>	<b>SITE REVIEW</b>
NEW HAMPSHIRE	APPLICATION
Building Permit Application Number 30387/89	Case Number
	Fee
Map <u>220</u> Lot <u>87-3/3</u> Zone <u>SRB</u>	Wetlands: Inland 1/4 Coastal Lot Area 53,579
Dat	e of Approvals (Indicate if Pending)
Conservation Commission	_ Conditional Use Board of Adjustment _ 6 26 18
Historic District Commission	Other
Street Address 64&74 EMER	y St.
Description of Project including all use(s)	ONSTRUCTION OF ONE CONDEX STYLE
	CONTAINING TWO 1500 SQ FT.
CONDO UNITS.	
	Gross Floor Area <u>3000 x Z</u> #of Stories <u>Z</u>
# of Dwelling Units Number o	f Parking Spaces: Existing Proposed
Property Owner's Name HAPPY MD	Print Information Below UN TAIN HOLDINGS LLC
	Suire City/Town WILMINGTON State DE Zip 19801
	Fax # Email Address
Telephone # Cell Phone #	Fax # Email Address
	Print Information Below
Applicant's / Developer's Name CAINTHR	ION BUILDERS LLC
Street Address 27 SPINE ST.	City/Town DOVER State NH Zip 03820
<u>603 - 731 - 8156</u> Telephone # Cell Phone #	Fax # Email Address
	ow (Include Additional Contact Information on Next Page) Engineer M Surveyor T Other I If other, state relationship
	RIEB - ALMS ENGINEERING
	City/Town <u>PORTSMOUTH</u> State <u>NH</u> Zip <u>0380</u>
<u>603-433-2335</u> Telephone # Cell Phone #	Fax # Email Address
I hereby apply for Site Review and acknowledge that I will of City of Portsmouth in the development and construction of t	
Owner's Signature	DEFF     BISHOP     9/11/18       Print Owner's Name     Date
Clery Churt	COREY     CAWTHEON     9/11/18       Print Applicant's/Developer's Name     Date
Applicant's/Developer's Signature	Print Applicant's/Developer's Name / Date

		Print Inf	ormation Below			
Check One: Owner's Attorney	Applicant's Attorney 🗆	Engineer	Surveyor 🛙	Other 🗌	If other, state relationship	)
Representative's Name						
Street Address			City/Town		State	Zip
Telephone #	Cell Phone #	5		Fax #		Email Address
		Print Info	ormation Below			
Check One: Owner's Attorney 🗆	Applicant's Attorney 🗆	Engineer 🗆	Surveyor 🛙	Other 🗆	If other, state relationship	
Representative's Name						
Street Address						
Telephone #	Cell Phone #	Ł		Fax #		Email Address
		Print Info	ormation Below			
Check One: Owner's Attorney	Applicant's Attorney 🛛	Engineer 🗆	Surveyor 🗆	Other 🗆	If other, state relationship	
Representative's Name						
Street Address						Zip
Telephone #	Cell Phone #	Ł		Fax #		Email Address

# Attachments

The following materials must be submitted to the Planning Department along with the completed Application Form:

Site Plan Application Checklist
Ten (10) stamped and folded copies of the site plan – four (4) full-size (22" x 34") and six (6) reduced (11" x 17")
Digital copy of any plans and/or exhibits (in PDF format)
Application Fee
Any required State or Federal Permits



Board review. A pre-application checklist is a tool designed to assist the applicant in the planning process and for preparing the application for Planning Board review. A pre-application conference with a member of the planning department is strongly encouraged as additional project information may be required depending on the size and scope. The applicant is cautioned that this checklist is only a guide and is not intended to be a complete list of all site plan review requirements. Please refer to the Site Plan review regulations for full details.

Applicant Responsibilities (Section 2.5.2): Applicable fees are due upon application submittal along with required attachments. The application shall be complete as submitted and provide adequate information for evaluation of the proposed site development. <u>Waiver requests must be submitted</u> in writing with appropriate justification.

Name of Owner/Applicant:	Date Submitted: 9/17/2018
	E-mail: ccawthron@kw.com
Site Address: 64 & 74 Emery Street	

Zoning District: Single Residence B Lot area: 53579 sq. ft.

	Application Requirements					
Ø	Required Items for Submittal	Item Location (e.g. Page or Plan Sheet/Note #)	Waiver Requested			
~	Fully executed and signed Application form. (2.5.2.3)	Application Package	N/A			
~	All application documents, plans, supporting documentation and other materials provided in digital Portable Document Format (PDF) on compact disc, DVD or flash drive. (2.5.2.8)	Application Package	N/A			

	Site Plan Review Application Required Information				
M	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested		
~	Statement that lists and describes "green" building components and systems. (2.5.3.1A)	Application Package			
~	Gross floor area and dimensions of all buildings and statement of uses and floor area for each floor. (2.5.3.1B)	Construction Set	N/A		
~	Tax map and lot number, and current zoning of all parcels under Site Plan Review. (2.5.3.1C)	Site Plan C-1	N/A		
~	Owner's name, address, telephone number, and signature. Name, address, and telephone number of applicant if different from owner. (2.5.3.1D)	Application Package	N/A		

Site Plan Application Checklist/December 2017

Page 1 of 7

	Site Plan Review Application Required Inf	ormation	
Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested
	Names and addresses (including Tax Map and Lot number and zoning districts) of all direct abutting property owners (including properties located across abutting streets) and holders of existing conservation, preservation or agricultural preservation restrictions affecting the subject property. (2.5.3.1E)	Site Plan EC-1	N/A
~	Names, addresses and telephone numbers of all professionals involved in the site plan design. (2.5.3.1F)	Cover Sheet	N/A
~	List of reference plans. (2.5.3.1G)	Site Plan EC-1	N/A
~		Utility Plan C-3 Notes #9-#12	N/A

	Site Plan Specifications					
Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested			
<ul> <li></li> </ul>	Full size plans shall not be larger than 22 inches by 34 inches with match lines as required, unless approved by the Planning Director. Submittals shall be a minimum of 11 inches by 17 inches as specified by Planning Dept. staff. (2.5.4.1A)	Required on all plan sheets	N/A			
<b>~</b>	Scale: Not less than 1 inch = 60 feet and a graphic bar scale shall be included on all plans. (2.5.4.1B)	Required on all plan sheets	N/A			
~	GIS data should be referenced to the coordinate system New Hampshire State Plane, NAD83 (1996), with units in feet. (2.5.4.1C)	Required on all plan sheets	N/A			
~	Plans shall be drawn to scale. (2.5.4.1D)	Required on all plan sheets	N/A			
~	Plans shall be prepared and stamped by a NH licensed civil engineer. (2.5.4.1D)	Required on all plan sheets	N/A			
~	Wetlands shall be delineated by a NH certified wetlands scientist. (2.5.4.1E)	N/A	N/A			
~	Title (name of development project), north point, scale, legend. (2.5.4.2A)	Required on all plan sheets	N/A			
~	Date plans first submitted, date and explanation of revisions. (2.5.4.2B)	Required on all plan sheets	N/A			
V	Individual plan sheet title that clearly describes the information that is displayed. (2.5.4.2C)	Required on all plan sheets	N/A			

Page 2 of 7

	Site Plan Specifications		
Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested
~	Source and date of data displayed on the plan. (2.5.4.2D)	Required on all plan sheets	N/A
~	A note shall be provided on the Site Plan stating: "All conditions on this Plan shall remain in effect in perpetuity pursuant to the requirements of the Site Plan Review Regulations." (2.5.4.2E)	Required on all plan sheets WAIVER REQUESTED	N/A
~	<ul> <li>Plan sheets submitted for recording shall include the following notes: <ul> <li>a. "This Site Plan shall be recorded in the Rockingham County Registry of Deeds."</li> <li>b. "All improvements shown on this Site Plan shall be constructed and maintained in accordance with the Plan by the property owner and all future property owners. No changes shall be made to this Site Plan without the express approval of the Portsmouth Planning Director."</li> </ul> </li> <li>(2.13.3)</li> </ul>	Waiver Requested	N/A
V	<ul> <li>Plan sheets showing landscaping and screening shall also include the following additional notes: <ul> <li>a. "The property owner and all future property owners shall be responsible for the maintenance, repair and replacement of all required screening and landscape materials."</li> <li>b. "All required plant materials shall be tended and maintained in a healthy growing condition, replaced when necessary, and kept free of refuse and debris. All required fences and walls shall be maintained in good repair."</li> <li>c. "The property owner shall be responsible to remove and replace dead or diseased plant materials immediately with the same type, size and quantity of plant materials as originally installed, unless alternative plantings are requested, justified and approved by the Planning Board or Planning Director."</li> </ul> </li> </ul>	Waiver Requested	N/A

V		Site Plan Specifications – Required Exhibit Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested
	1.	Existing Conditions: (2.5.4.3A)		
V	a.	Surveyed plan of site showing existing natural and built features;	EC-1	
~	b.	Zoning boundaries;	EC-1	
V	с.	Dimensional Regulations;	EC-1	
~	d.	Wetland delineation, wetland function and value assessment;	No Wetlands	
V	e.	SFHA, 100-year flood elevation line and BFE data.	N/A	
	2.	Buildings and Structures: (2.5.4.3B)		
~	a.	elevation;	Construction Set	
~	b.	façade treatments;	Construction Set	
V	с.		Construction Set	
~	d.	Number of Usable Floors;	Construction Set	
~	e.	Gross floor area by floor and use.	Construction Set	
	3.	Access and Circulation: (2.5.4.3C)		
~	a.	Location/width of access ways within site;		V
~	b.	Location of curbing, right of ways, edge of pavement and sidewalks;		~
~	c.	markings);		~
~	d.	Names/layout of existing abutting streets;		V
~	e.	Driveway curb cuts for abutting prop. and public roads;		V
~	f.	If subdivision; Names of all roads, right of way lines and easements noted;		<ul> <li>Image: A start of the start of</li></ul>
~	g.	AASHTO truck turning templates, description of minimum vehicle allowed being a WB-50 (unless otherwise approved by TAC).		~
	4.	Parking and Loading: (2.5.4.3D)		
~	a.	Location of off street parking/loading areas, landscaped areas/buffers;		~
<u> </u>	b.			~
	5.	Water Infrastructure: (2.5.4.3E)		
~	a.	Engineering data;	Utility Plan C-3	
~	b.		N/A	
	6.			
~	a.	data.	Utility Plan C-3	
	7.	Utilities: (2.5.4.3G)		
V	a.	The size, type and location of all above & below ground utilities;	Utility Plan C-3	
	b.	Size type and location of generator pads, transformers and other	Utility Plan C-3	

Page 4 of 7

$\mathbf{\nabla}$	Required Items for Submittal	Item Location	Waiver
		(e.g. Page/line or Plan Sheet/Note #)	Requested
V	8. Solid Waste Facilities: (2.5.4.3H)		
~	a. The size, type and location of solid waste facilities.	C-1 Note #17	
	9. Storm water Management: (2.5.4.3I)		
~	a. The location, elevation and layout of all storm-water drainage.	Grading Plan C-2	
	10. Outdoor Lighting: (2.5.4.3J)		
~	<ul> <li>a. Type and placement of all lighting (exterior of building, parking lot and any other areas of the site) and;</li> <li>b. photometric plan.</li> </ul>		~
~	<ol> <li>Indicate where dark sky friendly lighting measures have been implemented. (10.1)</li> </ol>		~
	12. Landscaping: (2.5.4.3K)		
~	<ul> <li>Identify all undisturbed area, existing vegetation and that which is to be retained;</li> </ul>		~
~	b. Location of any irrigation system and water source.		V
	13. Contours and Elevation: (2.5.4.3L)		
~	<ul> <li>Existing/Proposed contours (2 foot minimum) and finished grade elevations.</li> </ul>	Grading Plan C-2	
	14. Open Space: (2.5.4.3M)		
V	a. Type, extent and location of all existing/proposed open space.	Site Plan C-1	
~	<ol> <li>All easements, deed restrictions and non-public rights of ways. (2.5.4.3N)</li> </ol>	Application Package	
~	<ol> <li>Location of snow storage areas and/or off-site snow removal. (2.5.4.30)</li> </ol>	C-1 Note #22	
~	<ol> <li>Character/Civic District (All following information shall be included): (2.5.4.3Q)</li> </ol>	N/A	
	a. Applicable Building Height (10.5A21.20 & 10.5A43.30);		
	b. Applicable Special Requirements (10.5A21.30);		
	c. Proposed building form/type (10.5A43);		
	d. Proposed community space (10.5A46).		

Other Required Information			
Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested
~	Traffic Impact Study or Trip Generation Report, as required. (Four (4) hardcopies of the full study/report and Six (6) summaries to be submitted with the Site Plan Application) <b>(3.2.1-2)</b>	N/A	
~	Indicate where Low Impact Development Design practices have been incorporated. (7.1)	Grading Plan C-2	
~	Indicate whether the proposed development is located in a wellhead protection or aquifer protection area. Such determination shall be approved by the Director of the Dept. of Public Works. (7.3.1)	N/A	
~	Indicate where measures to minimize impervious surfaces have been implemented. (7.4.3)	No Other Practical Alternative	
~	Calculation of the maximum effective impervious surface as a percentage of the site. (7.4.3.2)	Site Plan C-1	
~	Stormwater Management and Erosion Control Plan. (Four (4) hardcopies of the full plan/report and Six (6) summaries to be submitted with the Site Plan Application) <b>(7.4.4.1)</b>	Grading Plan C-2 and Application Package	

Final Site Plan Approval Required Information			
M	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested
(2.	local approvals, permits, easements and licenses required, luding but not limited to: a. Waivers; b. Driveway permits; c. Special exceptions; d. Variances granted; e. Easements; f. Licenses. 5.3.2A)	Site Plan C-1 and Application Package	
pa	<ul> <li>hibits, data, reports or studies that may have been required as rt of the approval process, including but not limited to: <ul> <li>a. Calculations relating to stormwater runoff;</li> <li>b. Information on composition and quantity of water demand and wastewater generated;</li> <li>c. Information on air, water or land pollutants to be discharged, including standards, quantity, treatment and/or controls;</li> <li>d. Estimates of traffic generation and counts pre- and post-construction;</li> <li>e. Estimates of noise generation;</li> <li>f. A Stormwater Management and Erosion Control Plan;</li> <li>g. Endangered species and archaeological / historical studies;</li> <li>h. Wetland and water body (coastal and inland) delineations;</li> <li>i. Environmental impact studies.</li> </ul> </li> </ul>	<ul> <li>a. Applcation Package</li> <li>b. Domestic Water</li> <li>c. N/A</li> <li>d. N/A</li> <li>e. N/A</li> <li>f. C-2 Grading Plan</li> <li>g. N/A</li> <li>h. N/A</li> </ul>	

	Final Site Plan Approval Required Information			
	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested	
~	A document from each of the required private utility service providers indicating approval of the proposed site plan and indicating an ability to provide all required private utilities to the site. (2.5.3.2D)	Application Package		
~	A list of any required state and federal permit applications required for the project and the status of same. <b>(2.5.3.2E)</b>	N/A		
Appli	icant's Signature: Date:	9/17/18		

Page 7 of 7

4



# HAPPY MOUNTAIN HOLDINGS LLC

"Statement of Green Building Components and Systems"

64 & 74 Emery Street Portsmouth, NH

# SECTION 2.5.3.1A

The Condominium units will be constructed using quality building products and will be certified under the Energy Star Home Program. Building products and techniques are as follows:

- Energy Star Certified exterior doors and windows
- James Hardie Fiber Cement Siding
- York Energy Star Certified 90+% AFUE Gas Furnaces
- Programmable Thermostats
- Code Compliant Energy Star Insulating for Climate Zone 5
- Energy Star Certified Appliances
- LED Lighting and Energy Star Light Bulbs
- Low Flow toilets and faucets
- LID Elements including rain gardens for Stormwater Management
- Avoiding large ledge outcrop to minimize site work disturbance area with shared impervious driveway.



Electric Service Support Center PO Box 330 Manchester, NH 03105 1-800-362-7764

07/31/2018

Corey Cawthron 750 Lafayette Rd. Suite 201 Portsmouth, NH 03801

Re: 64 Emery Street Portsmouth, NH 03801

Dear Corey:

Eversource Energy agrees to provide electric service to the above site in accordance with the Tariff for Electric Service on file with the New Hampshire Public Utilities Commission (NHPUC), subject to the applicable NHPUC rules and regulations, as well as Eversource's "Requirements for Electric Service Connections".

Please keep in mind that all requirements for providing electric service, such as, but not limited to, contracts, licenses, fees, payments, easements and inspections must be provided to Eversource prior to the construction of the electric facilities.

Should you have any questions or concerns, please call us at 1-800-362-7764

Sincerely,

Tom Eger Electric Service Support Center PO Box 330 Manchester, NH 03105-9989



Electric Service Support Center PO Box 330 Manchester, NH 03105 1-800-362-7764

07/31/2018

Corey Cawthron 750 Lafayette Rd. Suite 201 Portsmouth, NH 03801

Re: 74 Emery Street Portsmouth, NH 03801

Dear Corey:

Eversource Energy agrees to provide electric service to the above site in accordance with the Tariff for Electric Service on file with the New Hampshire Public Utilities Commission (NHPUC), subject to the applicable NHPUC rules and regulations, as well as Eversource's "Requirements for Electric Service Connections".

Please keep in mind that all requirements for providing electric service, such as, but not limited to, contracts, licenses, fees, payments, easements and inspections must be provided to Eversource prior to the construction of the electric facilities.

Should you have any questions or concerns, please call us at 1-800-362-7764

Sincerely,

Tom Eger Electric Service Support Center PO Box 330 Manchester, NH 03105-9989



September 14, 2018

Corey Cawthron Happy Mountain Holdings LLC 91 N Market St Wilmington DE 19801

RE: Natural Gas Availability to 64 & 74 Emery St Portsmouth

Dear Corey

Unitil's natural gas division has reviewed the requested site for natural gas service.

Unitil hereby confirms natural gas service will be available to 64 & 74 Emery St Portsmouth. Installation is pending an authorized installation agreement with Happy Mountain Holdings LLC and street opening approval from the City of Portsmouth DPW

Let me know if you have any questions. You can email me at oliver@unitil.com. My phone number is 603-294-5174.

Sincerely,

Janet Oliver Business Development Representative

T 888.486.4845 www.unitil.com



September 14, 2018, 2018

RE: "Will Serve Letter for 64 and 74 Emery St. Portsmouth, NH.

Dear Mr. Cawthron,

Consolidated Communications has agreed to provide communications service to these locations subject to the Tariffs and terms of NHPUC No. 83, section 2.

Please note that a payment may be required from the customer requesting service as described in NH PUC Tariff No. 83, section 2.1.3.

You may review these documents at: http://www.puc.nh.gov/Regulatory/Tariffs/FairPoint\_83/FairPointLST.HTM

Subsequent to the customer responsibilities being satisfied, FairPoint will proceed with construction of the services requested.

Should you have any questions, please feel free to contact me at 603-427-5525

Joseph P. Considine Engineer Consolidated Communications



Civil Site Planning Environmental Engineering

133 Court Street Portsmouth, NH 03801-4413

WAIVER REQUESTS Assessor's Map 220 Lot 87-2 (74 Emery Street) & Lot 87-3 (64 Emery Street) Altus Project P4916 September 17, 2018

On behalf of Happy Mountain Holdings, LLC, Altus Engineering, Inc. request the following waivers from the City of Portsmouth, New Hampshire Site Plan Review Regulations.

Section 2.5.4 2 (E) A Note shall be provided on the plan stating, "All conditions on this plan shall remain in effect in perpetuity pursuant to the requirements of the site plan regulations." Section 2.5.4 3 (C) Access and circulation Section 2.5.4 3 (D) Parking and loading Section 2.5.4 3 (J) Outdoor lighting

Section 3.4 Curbing (A) where access ways and driveways meet public streets Section 5.2 Sidewalk and Pedestrian Pathways Section 5.3 Bicycle Facilities Section 6.1 Landscaping and Screening Standards. Section 2.13.3 Recording Notes

This project is unique in the fact that it is the development of two duplex homes on two abutting lots. Because four residential housing units are proposed, the project falls under the criteria for Site Plan Review Regulations. As such, the duplex homes do not require loading, outdoor lighting, curbing at the entrance, bicycle racks and other types of development features that normally are depicted on commercial site developments. We have combined all of the waiver requests with a single explanation.

As discussed at the TAC Workshop, it is understood that the general intent of the Technical Advisory Committee's Review and the concerns that would be of interest to the Planning Board include the design of the stormwater management system and the utility service design. The plans submitted for review and approval demonstrate that there will be no adverse impacts to abutting properties from runoff from the site. A detailed utility service design plan is included in

Tel. (602) /22 7225 E mail. Altur @altur and and

Waiver Requests Emery Street September 2018 Page 2

the plan set.

To require that all conditions on the plan to remain in effect in perpetuity is an overly burdensome requirement for the homes. This would require the homeowners to file an amendment to the Site Plans to install a shed, light post, swing set or any other feature that is normally constructed on a duplex lot without requiring Site Plan Approval. To require the Site Plan to be recorded is an excessive requirement for this development.

Wde/4916 waiver

9/11/2018



# City of Portsmouth Driveway Permit Public Works Department

680 Peverly Hill Road

Portsmouth, NH 03801 (603) 427-1530 Permit Number: 32320

Date of Issue: September 11, 2018

Site Address: 74 Emery Street Portsmouth, NH 03801 Main Address: 74 EMERY ST Portsmouth NH 03801 Property Owner: HAPPY MOUNTAIN HOLDINGS LLC

Applicant's Name: Corey Cawthron Phone: 6037318156 Email: ccawthron@kw.com

Description of Work: Shared driveway servicing 64 & amp; 74 Emery Street with one access point from public street.

New Drive: true

**Existing Drive:** 

City Staff Remarks & Comments:

PERMIT HOLDER has read this permit, permit application, DPW Driveway Rules & Procedures, conditions and comments, and agrees to perform the work authorized. The cost of all work shall be borne by the applicant / property owner.

An **EXCAVATION PERMIT** is required if cutting into any public way or public right-of-way. A **FLAGGING PERMIT** is required if any action would hinder free passage of vehicles on any street or right-of-way. Permits are issued by DPW. Applications can be found online: <u>http://www.cityofportsmouth.com/publicworks/permits-applications</u>

Call DIG SAFE at 811 for every project.

The City of Portsmouth reserves the right to deny any permits when:

Proposed driveway does not conform to the requirements of the Portsmouth Zoning Ordinance; Proposed driveway does not conform to the Driveway Specifications that are part of this permitting process; or Proposed driveway would present an unreasonable safety risk to the public.

# The Permit Card Shall Be Posted and Visible from the Street During Driveway Construction.

Contact Dave Desfosses @ (603) 766-1411 / djdesfosses@cityofportsmouth.com for a FINAL INSPECTION when work is completed.

**Department Director:** 

bolle

Peter H. Rice, P.E. Director of Public Works

This is an e-permit.





# CITY OF PORTSMOUTH

Community Development Department (603) 610-7281

Planning Department (603) 610-7216

# PLANNING DEPARTMENT

June 29, 2018

Happy Mountain Holdings LLC 901 N. Market St, Ste. 705 Wilmington, Delaware 19801

Re: Property at 64 & 74 Emery Street, Permit #30387 Assessor Plan 220, Lot 87-2&3

Dear Applicant:

The Board of Adjustment at its reconvened meeting on June 26, 2018 completed its consideration of your application described as follows:

Application:

Case 6-7	
Petitioner:	Happy Mountain Holdings LLC
Property:	64 and 74 Emery Street
Assessor Plan:	Map 220, Lots 87-2 and 87-3
Zoning District:	Single Residence B
Description:	Build a two-family dwelling on two lots
Requests:	Variances and/or Special Exceptions necessary to grant the required
	relief from the Zoning Ordinance including the following variances:
1.	from Section 10.440, Use #1.30 to allow a two family dwelling on
	each of two lots where a two family dwelling on a lot is not allowed; and
2.	from Section 10.521 to allow a lot area per dwelling unit for Lot 220-

# 87-3 (64 Emery Street) of 10,616±s.f. where 15,000 s.f. is required.

# Action:

The Board voted to grant the petition as presented and advertised.

1 Junkins Avenue Portsmouth, New Hampshire 03801 Fax (603) 427-1593 Happy Mountain Holdings LLC - Page Two June 29, 2018

#### Review Criteria:

The petition was granted for the following reasons:

- Granting the variances will not be contrary to the public interest and the spirit of the ordinance will be observed as the essential character of the neighborhood will not be altered, nor will the health, safety or welfare of the public be threatened. The project will fit appropriately within this neighborhood which is a mixture of commercial and residential uses.
- Substantial justice will be done as the loss to the applicant if the petition were denied and strict adherence to the ordinance enforced would not be outweighed by any gain to the general public.
- The value of surrounding properties will not be diminished. Most of the surrounding properties are either commercial, places of assembly or other residential properties, all of which will sustain their values.
- Literal enforcement of the ordinance would result in unnecessary hardship due to the special conditions of the property. These include the proximity of the properties to the highway and the bypass as well as its location in a single residence zone while surrounded on three sides by commercial uses or places of assembly. Due to the special conditions, there is no fair and substantial relationship between the purposes of the ordinance provision limiting a lot to a single residence and their specific application to these properties. A residential use in a residential zone is a reasonable use.

As provided for in NH RSA Chapter 677, the Board's decision may be appealed 30 days after the vote. Any action taken by the applicant pursuant to the Board's decision during this appeal period shall be at the applicant's risk. Please contact the Planning Department for more details about the appeals process. Construction drawings or sketches must be reviewed and approved by the Building Inspector prior to the issuance of a building permit. Approvals by other land use boards may also be required prior to the issuance of a building permit.

The minutes and tape recording of the meeting may be reviewed in the Planning Department.

Very truly yours, David Cheane

David Rheaume, Chairman Board of Adjustment

mek

c: Robert Marsilia, Chief Building Inspector Roseann Maurice-Lentz, City Assessor Douglas W. Macdonald, Esq.

#### WARRANTY DEED

KNOW ALL MEN BY THESE PRESENTS, that HAPPY MOUNTAIN HOLDINGS, LLC, a Delaware limited liability company with a business address at Delaware Corporate Service, Inc., 901 N. Market St., Suite 705, Wilmington, County of New Castle, Delaware, 19801, grant to\_\_\_\_\_\_, with WARRANTY COVENANTS, the following:

A certain parcel or tract of land situated in the City of Portsmouth, County of Rockingham, State of New Hampshire, located on the northwesterly side of Emery Street, (f/k/a Central Avenue), being Proposed Map 220, Lot 103, (also known as 64 Emery Street), shown on a plan by Civil Consultants and Altus Engineering, Inc., dated September 27, 2013, entitled "PROPOSED DIVISION OF LAND OF CATHERINE T. MORETTI - PHASE 2 - MYRTLE AVENUE & CENTRAL AVENUE, PORTSMOUTH, ROCKINGHAM COUNTY, NEW HAMPSHIRE", recorded at the Rockingham County Registry of Deeds on June 23, 2014 as Plan D-38286 and being more particularly described as follows:

BEGINNING at a point in the northwesterly line of Central Avenue at the southeasterly corner of the parcel herein described, marked by a set 5/8" diameter rebar;

thence N 35°56'34" W, 32.91 feet to a set 5/8" diameter rebar;

thence N 35°56'34" W, 22.09 feet to a set 5/8" diameter rebar;

thence N 35°56'34" W, 166.58 feet to a set 5/8" diameter rebar;

thence N 35°56'34" W, 57.58 feet to a 5/8" diameter rebar set;

thence N 71°18'39" E, 153.74 feet to a found 1" diameter, 3" tall iron pipe in a stone wall;

thence S 10°47'38" E, along said stone wall, 8.02 feet to a found a 5/8" diameter rebar;

thence S 12°42'58" E, along said stone wall, 47.31 feet to a set 5/8" diameter rebar;

thence S 12°42'58" E, along said stone wall, 40.33 feet to a found 3/8" diameter drill hole;

thence S 14°25'35" E, along said stone wall, 51.02 feet to a 5/8" diameter rebar set in the northwesterly line of Central Avenue;

thence S 28°23'29" W, by the northwesterly line of Central Avenue, 100.00 feet to the POINT OF BEGINNING;

containing 21,232 square feet;

Subject to an easement as granted by Ethel B. Anderson to New Hampshire Gas & Electric Company as described in easement deed recorded at book 1137, page 357 of the Rockingham County Registry of Deeds.

Subject to an easement as granted by Aldolph F. Anderson to Potsmouth Power Company as described in easement deed recorded at book 836, page 116 of the Rockingham County Registry of Deeds.

Subject to an easement as granted by Marshall H. and Dorothy A. Chalk to New Hampshire Electric Company as described in easement deed recorded at book 1520, page 412 of the Rockingham County Registry of Deeds.

Subject to a Declaration of Easement Imposed by Catherine T. Moretti, for a Subdivision Located at Myrtle Avenue and Central Avenue, recorded at book 5539, page 730 of the Rockingham County Registry of Deeds.

Subject to all other restrictions, rights, easements, rights-of-way, and anything else as shown on said above described plan.

Meaning and intending to describe and convey the premises conveyed to the Grantor herein by fiduciary Deed dated April 16,, 2018 and recorded at the Rockingham County Registry of Deeds at Book 5905, Page 2549.

IN WITNESS WHEREOF, I have executed this deed on this this \_\_\_\_ day of \_\_\_\_, 201 .

Witness:

#### HAPPY MOUNTAIN HOLDINGS, LLC,

By: Its: \_\_\_\_\_, Duly Authorized

## STATE OF NEW HAMPSHIRE COUNTY OF ROCKINGHAM, SS:

. . . . L .

This instrument was acknowledged before me on this \_\_\_\_\_\_ day of \_\_\_\_\_, 201\_, by \_\_\_\_\_\_\_ in his capacity as \_\_\_\_\_\_ of Happy Mountain Holdings, LLC.

#### NOTARY PUBLIC

Name: My Commission Expires:

ह स्थलको प्रायमकेल हैं। यहाँ है। होता हरता अव्योधने स्थल व्याहन कर तेत्र है। यहां हता हता है। होता हर होती । इस्टलको प्रायमकेक क्रमण न्यूकिंग गया यही। त्यांग केले का क्रियन होने क्रिकिंग गया। होता कहा है। यह नहहता होने क

المركز المحمد بها المحمد من المركز المركز المركز المركز المركز المركز المركز المركز المركز المحمد المحم المحمد المحم المحمد المحم المحمد المحمد المحمد المحمد المحمد المحمد المحمد المحمد محمد المحمد المحم المحمد المحم المحمد المحم المحمد المحم المحمد المحم المحمد المحمد

MAIL '	ropd
--------	------

Return to: City of Portsmouth Legal Department Planning Division City Hall – 1 Junkins Ave. Portsmouth, NH 03801

#### DECLARATION OF EASEMENT IMPOSED BY CATHERINE T. MORETTI, FOR A SUBDIVISION LOCATED AT MYRTLE AVENUE AND CENTRAL AVENUE AS SHOWN ON THE SUBDIVISION PLAN ENTITLED "PROPOSED DIVISION OF LAND OF CATHERINE T. MORETTI PHASE 2 – MYRTLE AVENUE & CENTRAL AVENUE PORTSMOUTH, ROCKINGHAM COUNTY, NEW HAMPSHIRE"

THIS DECLARATION is made by CATHERINE T. MORETTI, with a mailing address of 9 Prince Lane, Raymond, NH 03077, (hereinafter "Declarant") and is made for the benefit of Lot 104 as shown on a plan of land entitled "Proposed Division of Land of Catherine T. Moretti Phase 2 - Myrtle Avenue & Central Avenue Portsmouth, Rockingham County, New Hampshire", dated \_\_\_\_\_\_\_, /0, 2014 and prepared by Civil Consultants and Altus Engineering, Inc. (hereinafter "Subdivision Plan") to be recorded herewith in the Rockingham County Registry of Deeds.  $0 - 35 \ge 56$ 

Acceptance of a deed by any person of either Lot 103 or Lot 104 shall constitute acceptance of these easements, regardless of whether said deed is expressly made subject thereto.

1. <u>COMMON ACCESS EASEMENT</u>. Lot 103 is hereby burdened by, and Lot 104 is hereby benefited by, a common access easement on Lot 103 for access from Central Avenue to each respective lot over the area shown on the Subdivision Plan as "Proposed Driveway Easement Appurtenant to Lot 104 4,219 S.F." (hereinafter "Common Access Easement Area"). The owners of Lot 103 and Lot 104 shall equally share in the ongoing costs and maintenance of and repair to said Common Access Easement Area, including the cost of snow removal. Repairs and maintenance of the Common Access Easement Area shall be performed from time to time by agreement of the then owners of Lot 103 and Lot 104. There shall be no parking upon or the obstruction of the Common Access Easement Area by any person entitled to use the same.

2. <u>SEWER & WATER LINE EASEMENT</u>. Lot 103 is hereby burdened by and Lot 104 is hereby benefited by, a sewer and water line easements on Lot 103 in the area depicted on the Subdivision Plan as "Proposed Sewer and Water Line Easement Appurtenant to Lot 104 2,072 S.F." (hereinafter "Sewer & Water Line Easement Area"). The owner of Lot 104 is hereby granted an easement over Lot 103, in the Sewer & Water Line Easement Area, for the installation, maintenance and repair of underground water and sewer lines.

3. **ENFORCEMENT.** Enforcement of these Declarations of Easements shall be by a proceeding at law or in equity against any person or persons violating or attempting to violate any easement, either to restrain violation of, or to recover damages, and failure by any owner to

ROCKINGHAM COUNTY REGISTRY OF DEEDS enforce any easement or other rights listed herein shall in no event be deemed a waiver of a right to do so thereafter.

4. <u>AMENDMENT, MODIFICATION OR TERMINATION</u>. This Declaration of Easements may only be amended, modified or terminated by an instrument signed by the then owners of both Lot 103 and Lot 104 and the Planning Director for the City of Portsmouth (or similar official authorized by the City Manager).

5. **TITLE REFERENCE**. Being a portion of the lands of Grantor described in a deed dated April 28, 2005 and recorded at the Rockingham County Registry of Deeds in Book 4471, Page 2618, being Lot 2 shown on a plan by Civil Consultants and Altus Engineering, Inc., dated April 2, 2013, entitled "PROPOSED DIVISION OF LAND OF CATHERINE T. MORETTI, 261 MYRTLE AVENUE, PORTSMOUTH, ROCKINGHAM, NEW HAMPSHIRE", recorded at the Rockingham County Registry of Deeds as Plan D-37764.

Executed this  $\cancel{10}$  day of 20 / Signature: Catherine T. Moretti

STATE OF NEW HAMPSHIRE COUNTY OF ROCKINGHAM

The foregoing instrument was acknowledged before me this  $10^{-1}$  day of  $5^{-2}$  and 2014 by the above-named Catherine T. Moretti.

Notary Public / Justice of the Peace

Print Name: 0 Commission Expires:

SAMANTHA L. GARLAND Notary Public, State of New Hampshire My Commission Expires Aug. 10, 2016





# **CITY OF PORTSMOUTH**

Community Development Department (603) 610-7281

Planning Department (603) 610-7216

# PLANNING DEPARTMENT

June 29, 2018

Happy Mountain Holdings LLC 901 N. Market St, Ste. 705 Wilmington, Delaware 19801

Re: Property at 64 & 74 Emery Street, Permit #30387 Assessor Plan 220, Lot 87-2&3

Dear Applicant:

The Board of Adjustment at its reconvened meeting on June 26, 2018 completed its consideration of your application described as follows:

Application:

Case 6-7	
Petitioner:	Happy Mountain Holdings LLC
Property:	64 and 74 Emery Street
Assessor Plan:	Map 220, Lots 87-2 and 87-3
Zoning District:	Single Residence B
Description:	Build a two-family dwelling on two lots
Requests:	Variances and/or Special Exceptions necessary to grant the required
	relief from the Zoning Ordinance including the following variances:
1.	from Section 10.440, Use #1.30 to allow a two family dwelling on
	each of two lots where a two family dwelling on a lot is not allowed;
	and
2.	from Section 10.521 to allow a lot area per dwelling unit for Lot 220- 87-3 (64 Emery Street) of 10,616±s.f. where 15,000 s.f. is required.

#### Action:

The Board voted to grant the petition as presented and advertised.

Happy Mountain Holdings LLC - Page Two June 29, 2018

#### Review Criteria:

The petition was granted for the following reasons:

- Granting the variances will not be contrary to the public interest and the spirit of the ordinance will be observed as the essential character of the neighborhood will not be altered, nor will the health, safety or welfare of the public be threatened. The project will fit appropriately within this neighborhood which is a mixture of commercial and residential uses.
- Substantial justice will be done as the loss to the applicant if the petition were denied and strict adherence to the ordinance enforced would not be outweighed by any gain to the general public.
- The value of surrounding properties will not be diminished. Most of the surrounding properties are either commercial, places of assembly or other residential properties, all of which will sustain their values.
- Literal enforcement of the ordinance would result in unnecessary hardship due to the special conditions of the property. These include the proximity of the properties to the highway and the bypass as well as its location in a single residence zone while surrounded on three sides by commercial uses or places of assembly. Due to the special conditions, there is no fair and substantial relationship between the purposes of the ordinance provision limiting a lot to a single residence and their specific application to these properties. A residential use in a residential zone is a reasonable use.

As provided for in NH RSA Chapter 677, the Board's decision may be appealed 30 days after the vote. Any action taken by the applicant pursuant to the Board's decision during this appeal period shall be at the applicant's risk. Please contact the Planning Department for more details about the appeals process. Construction drawings or sketches must be reviewed and approved by the Building Inspector prior to the issuance of a building permit. Approvals by other land use boards may also be required prior to the issuance of a building permit.

The minutes and tape recording of the meeting may be reviewed in the Planning Department.

Very truly yours, D. Rheave

David Rheaume, Chairman Board of Adjustment

mek

c: Robert Marsilia, Chief Building Inspector Roseann Maurice-Lentz, City Assessor Douglas W. Macdonald, Esq.



Civil Site Planning Environmental Engineering

133 Court Street Portsmouth, NH 03801-4413

WAIVER REQUESTS Assessor's Map 220 Lot 87-2 (74 Emery Street) & Lot 87-3 (64 Emery Street) Altus Project P4916 September 17, 2018

On behalf of Happy Mountain Holdings, LLC, Altus Engineering, Inc. request the following waivers from the City of Portsmouth, New Hampshire Site Plan Review Regulations.

Section 2.5.4 2 (E) A Note shall be provided on the plan stating, "All conditions on this plan shall remain in effect in perpetuity pursuant to the requirements of the site plan regulations." Section 2.5.4 3 (C) Access and circulation Section 2.5.4 3 (D) Parking and loading Section 2.5.4 3 (J) Outdoor lighting Section 2.5.4.3 (K) Landscaping Section 3.4 Curbing (A) where access ways and driveways meet public streets Section 5.2 Sidewalk and Pedestrian Pathways Section 5.3 Bicycle Facilities Section 6.1 Landscaping and Screening Standards. Section 2.13.3 Recording Notes Section 2.13.4 Landscaping requirements

This project is unique in the fact that it is the development of two duplex homes on two abutting lots. Because four residential housing units are proposed, the project falls under the criteria for Site Plan Review Regulations. As such, the duplex homes do not require loading, outdoor lighting, curbing at the entrance, bicycle racks and other types of development features that normally are depicted on commercial site developments. We have combined all of the waiver requests with a single explanation.

As discussed at the TAC Workshop, it is understood that the general intent of the Technical Advisory Committee's Review and the concerns that would be of interest to the Planning Board include the design of the stormwater management system and the utility service design. The plans submitted for review and approval demonstrate that there will be no adverse impacts to abutting properties from runoff from the site. A detailed utility service design plan is included in

Waiver Requests Emery Street September 2018 Page 2

the plan set.

To require that all conditions on the plan to remain in effect in perpetuity is an overly burdensome requirement for the homes. This would require the homeowners to file an amendment to the Site Plans to install a shed, light post, swing set or any other feature that is normally constructed on a duplex lot without requiring Site Plan Approval. To require the Site Plan to be recorded is an excessive requirement for this development.

Wde/4916 waiver



Civil Site Planning Environmental Engineering

133 Court Street Portsmouth, NH 03801-4413

# HAPPY MOUNTAIN HOLDINGS, LCC

#### 64 & 74 EMERY STREET Portsmouth, NH PRELIMINARY OPINION OF SITEWORK COST

DATE: 14-Sep-18 PROJECT: 4916

ITEM DESCRIPTION	QUANTITY	UNIT	UNIT	TOTAL
SITEWORK DEMOLITION	and the second			
MOBILIZATI	ON 1	LS	\$2,000.00	\$2,000.00
		10	\$2,000.00	\$2,000.00
CLEARING AND GRUBBING				
TREE AND VEGETATION REMOV	AL 1	LS	\$3,000.00	\$3,000.00
SEWER SERVICE				
LOW PRESSURE FORCE M	AIN 300	LF	\$34.00	\$10,200
WATER SERVICE 2-INCH FIRE SUPPRESSION WATER SERVIC	F0 400			
1-INCH DOMESTIC WATER SERVIC		LF LF	\$36.00 \$32.00	\$15,120
WATER TAPS AND CURB STO		EA	\$32.00	\$26,560 \$2,000
		E.A.	\$500.00	32,000
GAS SERVICE				
GAS SERVIC	ES 335	LF	\$26.00	\$8,710
ELECTRIC/PHONE/CABLE SERVICES				
UNDERGROUND ELECTRIC AND TELE-COMMUNICATION CONDU	TS 260	LF	\$30.00	\$7,800
TRANSFORMER AND P		EA	\$4.000.00	\$4,000
			\$1,000.00	04,000
STORM DRAINAGE SYSTEM				
EROSION CONTROL RIPRAP AND DRIP ED	GE 1	LS	\$1,000.00	\$1,000
SEDIMENT AND EROSION CONTROL				
TEMPORARY EROSION CONTR	DL 1	LS	\$1,500.00	\$1,500
		20	01,000.00	\$1,500
AGGREGATE BASE COURSES				
12" GRAVEL (NHDOT 304 6" CRUSHED GRAVEL (NHDOT 304	1.2) 312 1.3) 156	CY	\$18.00	\$5,616
CUTS AND FIL		CY	\$22.00 \$12.00	\$3,432
0010 845 14	200	C1	\$12.00	\$3,000
HOT BITUMINOUS PAVEMENT				
2.5" BASE COUR		TONS	\$85.00	\$9,520
1.5" WEARING COUR	SE 68	TONS	\$85.00	\$5,780
LANDSCAPING				
LOAM AND SEED - TURF ESTABLISHME	NT 1	LS	\$6,000.00	\$6,000
LIGHTING				
LIGHTING	NIC			
	NIC			

SUBTOTAL

\$115,238

TOTAL: \$115,238

EXCLUSIONS:

ITEMS EXCLUDED FROM THIS ESTIMATE INCLUDE, BUT ARE NOT LIMITED TO, THOSE ITEMS SPECIFIED ABOVE AS BEING NOT INCLUDED IN THIS ESTIMATE AND THE FOLLOWING: LEDGE REMOVAL, TAPPING FEES, INSPECTIONS, UTILITY SERVICE FEES

Emery St. Portsmouth, NH

# **DRAINAGE STUDY**

#### EXECUTIVE SUMMARY

Happy Mountain Holdings, LLC and Corey Cawthron are planning to develop two residential lots that have recently been approved by the Board of Adjustment to allow each to have a duplex housing. The project involves just lot development as the utility services are available in the public right-of-way. There are no wetlands on the lot or within 100-feet of the lot lines. The lots are encumbered with a utility easement at the rear of the site. No site improvements are proposed in the easement. However, it is expected that the homeowners will mow and maintain the easement areas. The two lots that will be developed are:

Assessor's Parcel	Lot Area
220-87-2	32,427 SF
2220-87-3	21,232 SF

The two lots are 53,659 square feet in size (1.23 acres) and are predominantly wooded lots. The lots were created in 2013. At that time, the City approved the development with a shared driveway and utility cross easements to allow Lot 87-3 to be developed without impacting the ledge outcrop in the Emery Street right-of-way. The two lots are approved developable lots that could be developed with up to 60% impervious area based on zoning regulations, which would allow over 30,000 sf of impervious area. The proposed development will provide approximately 12,000 sf of combined impervious areas as well as three stormwater management ponds to reduce peak flows and provide stormwater treatment.

The proposed project will include the two duplexes, a shared driveway, new utility services and associated site improvements, including; site grading, drainage improvements, and utility service connections. Stormwater ponds will be constructed on each of the two residential lots to manage the storm water flow and provide treatment. The ponds will consist of a depressed lawn area with a loamy-sand material that will promote infiltration, drainage, and provide treatment.

#### DRAINAGE ANALYSIS

This drainage study is intended to show that the proposed development will manage and treat the stormwater to improve the existing site conditions and minimize impacts from the development. The project was analyzed to compare the  $\frac{1}{2}$ ", 2, 10, 25, and 50 year storm events. As a conservative design approach, which exceeds the city Site Plan Review Regulations, Altus has designed the site following the NHDES Alteration of Terrain rainfall criteria by adding 15-percent to the 24-hour rainfall precipitation for each storm event modeled.

The pre-development subcatchments were modeled and input into HydroCAD for analysis. The "Pre-Development Watershed Plan" illustrates the subcatchments that were modeled for the existing stormwater system. The existing site drains towards the Emery Street right-of-way with a high point near the proposed driveway that directs a portion of the flow to the south towards Myrtle Ave and a portion of the flow to the north towards Maplewood Avenue.

The "Post-Development Watershed Plan" illustrates the proposed stormwater management system. The original subcatchments have been divided into smaller areas to emulate the proposed grading and stormwater management system proposed for construction. The post-development conditions were analyzed at the same primary discharge points examined in the pre-development modeling.

For existing soil conditions the NRCS Web Soil Survey tool was used to determine the existing hydrologic soil groups. The entire site is listed as a type 799 soil series, urban land -canton complex. Hydrologic Soil Group (HSG) Type B was used for the entire as a conservative approach, as much of the site appears to be HSG Type C based on field observations. Referencing the Ksat Values for NH Soils, an infiltration rate of 0.6 was used for the design of the grassed soil filter ponds. The low C Ksat value for Canton is 6.0, but due to the disturbed soil, the design rate used was 1/10 of low Ksat instead of 1/2 (3.0 in/hr) which is a typical design application.

The following Stormwater Modelling Summary compares pre-development and postdevelopment peak rates of runoff for all analyzed storm events:

#### Stormwater Modeling Summary

The Stormwater Modeling Summary Table below shows the results for the peak flow rates for stormwater discharge for the <sup>1</sup>/<sub>2</sub>" Inch, 2 year, 10 year, 25 year, and 50 year storm events:

	1/2"- Storm (0.5 inch)	2-Yr Storm (3.69 inch)	10-Yr Storm (5.60 inch)	25-Yr Storm (7.10 inch)	50-Yr Storm (8.50 inch)
POA #1					
Pre	0.00	0.27	1.46	2.82	4.28
Post	0.00	0.28	1.22	1.99	3.78
Change	0.0	+0.01	-0.24	-0.83	-0.50
POA #2					
Pre	0.00	0.05	0.26	0.49	0.72
Post	0.00	0.17	0.36	1.19	2.12
Change	0.0	+0.12	+0.10	+0.70	+1.40
Net Change	0.0	+0.13	-0.14	-0.13	+0.90

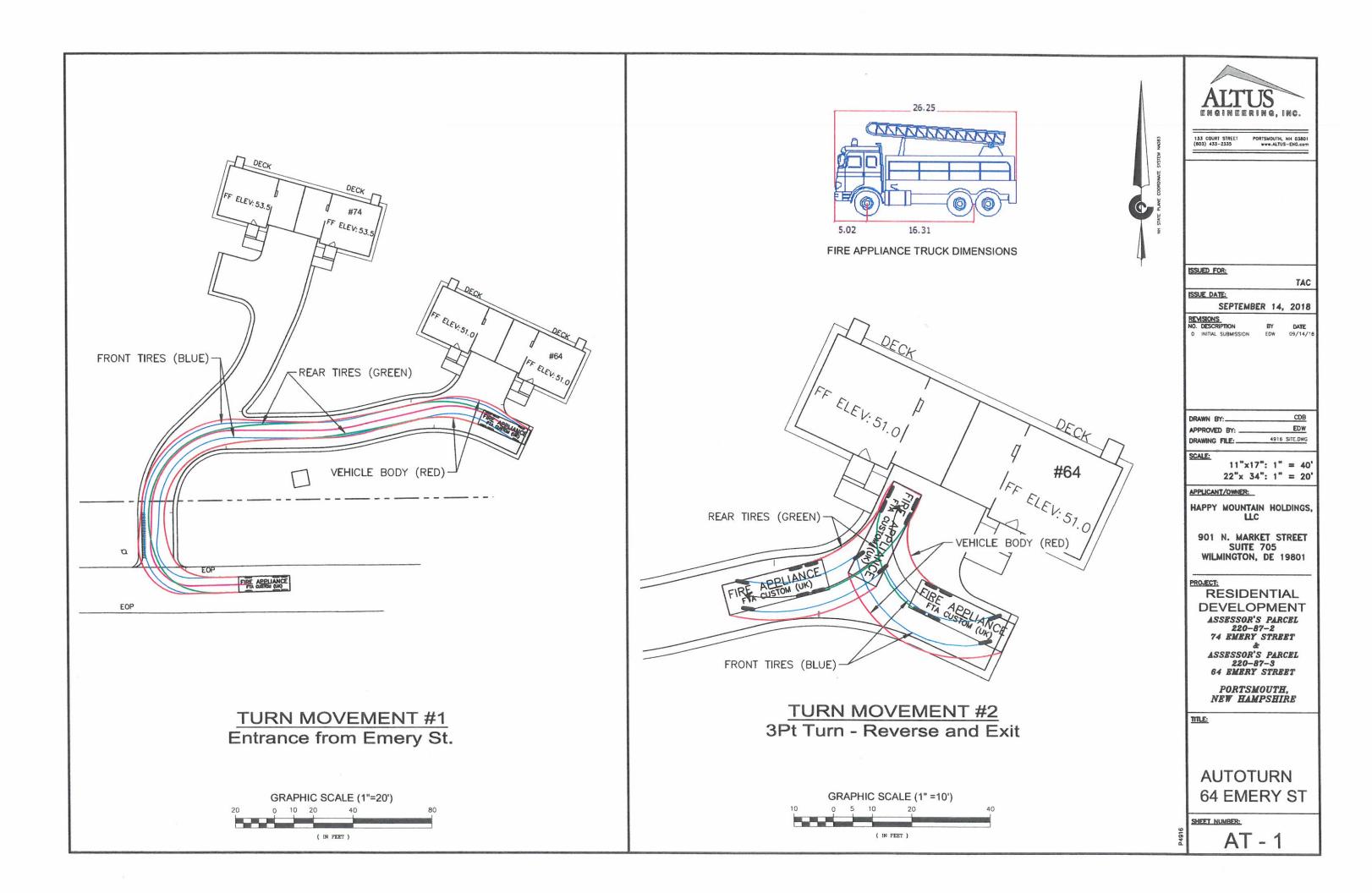
#### Stormwater Modeling Summary Table (Pre vs. Post-Development Stormwater Peak Runoff Rates)

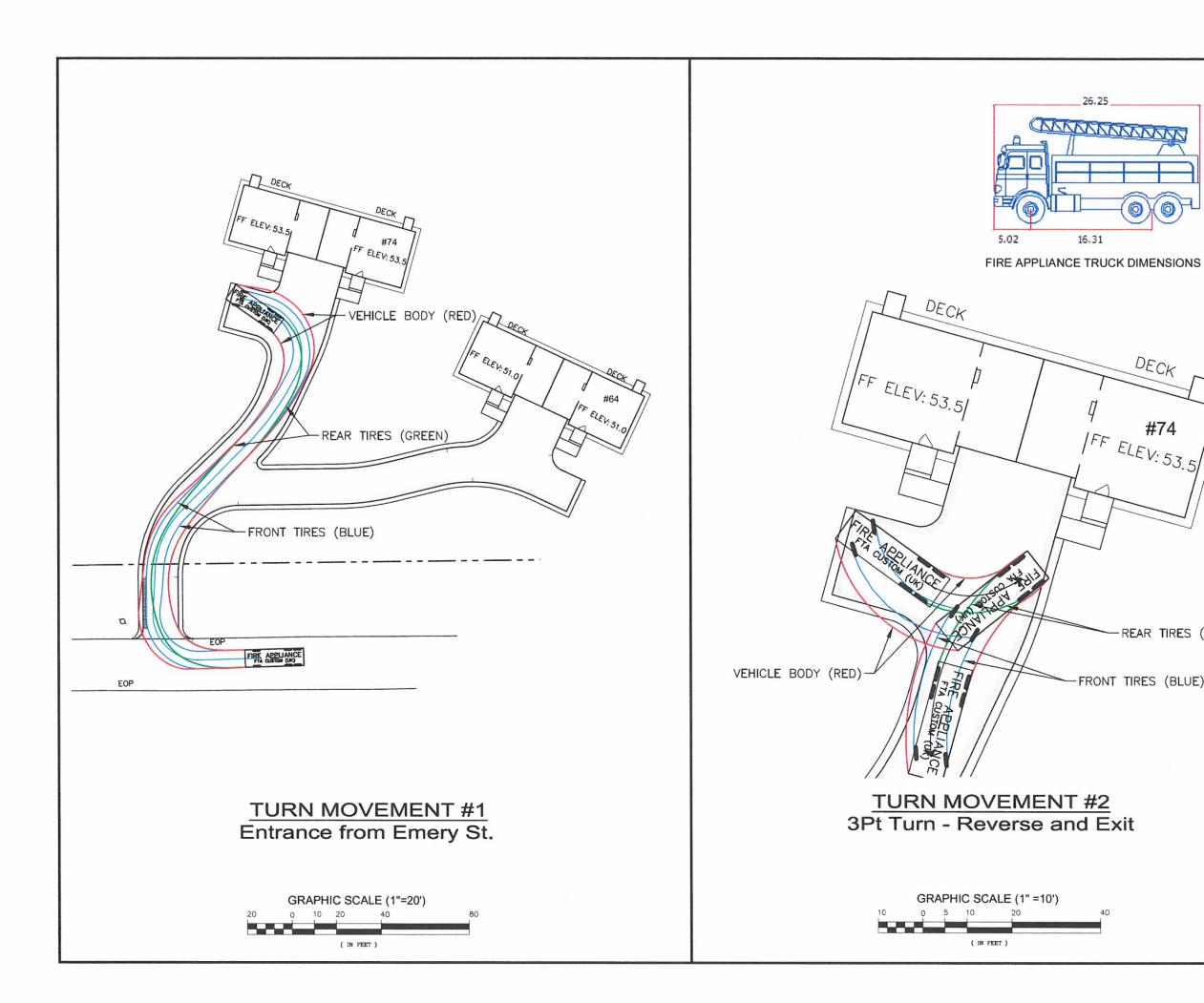
As the Stormwater Modeling Summaries demonstrate, the proposed project will manage the stormwater runoff to mitigate impacts to the surrounding areas. The peak flow rates are managed to replicate the existing conditions, with a variance of 0.1 cfs +/- for the 1/2" storm through the 25 year storm event, which is the design intent for low impact development.

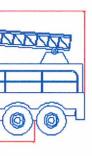
# **CONCLUSION**

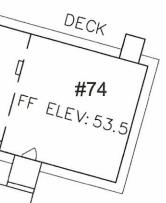
The proposed project will not have an adverse effect on abutting properties and infrastructure as a result of stormwater runoff. As the stormwater summary indicates, the peak flow rates discharging from the site will be managed to minimize impacts to the surrounding areas. Three grass soil filter ponds will be constructed to provide retention and treatment of stormwater on site prior discharging to the Emery Street drainage. As noted in the drainage report, the stormwater model utilizes a number of conservative design approaches. The estimated soil type and infiltration rates were conservative based on the Soil and Ksat values. Although not a City of Portsmouth requirement, a 15-percent increase was added to each rainfall event, similar to the requirements of NHDES Alteration of Terrain permitting. Additionally, there are proposed stormwater management features such as roof drip edges and grasses swales that were not incorporated in to the design model. It is expected that the drip edges will infiltrate all of the roof flows directed to them in all but the largest storm events. With this conservative approach, the post development stormwater model still indicates a variance of 0.1 cfs +/- for the 1/2" storm through the 25 year storm event, which is within the design modeling tolerance, and illustrates that the project is managing the stormwater flows.

In addition to the permanent stormwater management practices, appropriate steps will be taken to properly mitigate erosion and sedimentation during construction through the use of temporary Best Management Practices for sediment and erosion control. In summary, the proposed development will manage stormwater runoff during construction and post development so that there is no adverse impact to the surrounding area as a result of this development.



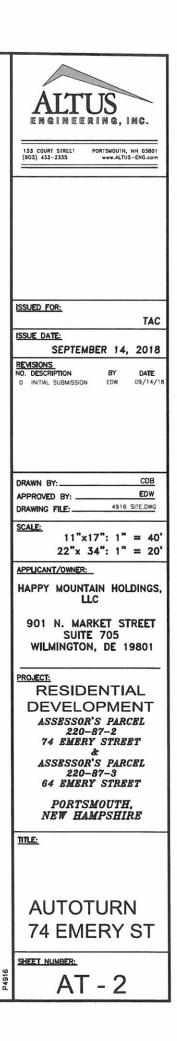






-REAR TIRES (GREEN)

-FRONT TIRES (BLUE)



# **Two Residential Duplexes**

64 & 74 Emery Street Portsmouth, NH Assessor's Map 220, Lots 87-2 & 87-3

# **DRAINAGE STUDY**

# OCTOBER 2018 SEPTEMBER 2018

Prepared For:

HAPPY MOUNTAIN HOLDINGS, LLC 901 N. MARKET STREET SUITE 705 WILMINGTON, DE 19801 C/O: Corey Cawthron 750 Lafayette Road Portsmouth, NH 03801

Prepared By:

ALTUS ENGINEERING, INC. 133 Court Street

> Portsmouth, NH 03801 Phone: (603) 433-2335



# **DRAINAGE STUDY**

#### EXECUTIVE SUMMARY

Happy Mountain Holdings, LLC and Corey Cawthron are planning to develop two residential lots that have recently been approved by the Board of Adjustment to allow each to have a duplex housing. The project involves just lot development as the utility services are available in the public right-of-way. There are no wetlands on the lot or within 100-feet of the lot lines. The lots are encumbered with a utility easement at the rear of the site. No site improvements are proposed in the easement. However, it is expected that the homeowners will mow and maintain the easement areas. The two lots that will be developed are:

Assessor's Parcel	Lot Area
220-87-2	32,427 SF
2220-87-3	21,232 SF

The two lots are 53,659 square feet in size (1.23 acres) and are predominantly wooded lots. The lots were created in 2013. At that time, the City approved the development with a shared driveway and utility cross easements to allow Lot 87-3 to be developed without impacting the ledge outcrop in the Emery Street right-of-way. The two lots are approved developable lots that could be developed with up to 60% impervious area based on zoning regulations, which would allow over 30,000 sf of impervious area. The proposed development will provide approximately 12,000 sf of combined impervious areas as well as three stormwater management ponds to reduce peak flows and provide stormwater treatment.

The proposed project will include the two duplexes, a shared driveway, new utility services and associated site improvements, including; site grading, drainage improvements, and utility service connections. Stormwater ponds will be constructed on each of the two residential lots to manage the storm water flow and provide treatment. The ponds will consist of a depressed lawn area with a loamy-sand material that will promote infiltration, drainage, and provide treatment.

#### DRAINAGE ANALYSIS

This drainage study is intended to show that the proposed development will manage and treat the stormwater to improve the existing site conditions and minimize impacts from the development. The project was analyzed to compare the  $\frac{1}{2}$ ", 2, 10, 25, and 50 year storm events. As a conservative design approach, which exceeds the city Site Plan Review Regulations, Altus has designed the site following the NHDES Alteration of Terrain rainfall criteria by adding 15-percent to the 24-hour rainfall precipitation for each storm event modeled.

The pre-development subcatchments were modeled and input into HydroCAD for analysis. The "Pre-Development Watershed Plan" illustrates the subcatchments that were modeled for the existing stormwater system. The existing site drains towards the Emery Street right-of-way with a high point near the proposed driveway that directs a portion of the flow to the south towards Myrtle Ave and a portion of the flow to the north towards Maplewood Avenue .

The "Post-Development Watershed Plan" illustrates the proposed stormwater management system. The original subcatchments have been divided into smaller areas to emulate the proposed grading and stormwater management system proposed for construction. The post-development conditions were analyzed at the same primary discharge points examined in the pre-development modeling.

For existing soil conditions the NRCS Web Soil Survey tool was used to determine the existing hydrologic soil groups. The entire site is listed as a type 799 soil series, urban land -canton complex. Hydrologic Soil Group (HSG) Type B was used for the entire as a conservative approach, as much of the site appears to be HSG Type C based on field observations. Referencing the Ksat Values for NH Soils, an infiltration rate of 0.6 was used for the design of the grassed soil filter ponds. The low C Ksat value for Canton is 6.0, but due to the disturbed soil, the design rate used was 1/10 of low Ksat instead of 1/2 (3.0 in/hr) which is a typical design application.

The following Stormwater Modelling Summary compares pre-development and postdevelopment peak rates of runoff for all analyzed storm events:

#### Stormwater Modeling Summary

The Stormwater Modeling Summary Table below shows the results for the peak flow rates for stormwater discharge for the <sup>1</sup>/<sub>2</sub>" Inch, 2 year, 10 year, 25 year, and 50 year storm events:

	1/2"- Storm (0.5 inch)	2-Yr Storm (3.69 inch)	10-Yr Storm (5.60 inch)	25-Yr Storm (7.10 inch)	50-Yr Storm (8.50 inch)
POA #1		()		()	(512 5 - 52)
Pre	0.00	0.27	1.46	2.82	4.28
Post	0.00	0.28	1.22	1.99	3.78
Change	0.0	+0.01	-0.24	-0.83	-0.50
POA #2					
Pre	0.00	0.05	0.26	0.49	0.72
Post	0.00	0.17	0.36	1.19	2.12
Change	0.0	+0.12	+0.10	+0.70	+1.40
Net Change	0.0	+0.13	-0.14	-0.13	+0.90

#### Stormwater Modeling Summary Table (Pre vs. Post-Development Stormwater Peak Runoff Rates)

As the Stormwater Modeling Summaries demonstrate, the proposed project will manage the stormwater runoff to mitigate impacts to the surrounding areas. The peak flow rates are managed to replicate the existing conditions, with a variance of 0.1 cfs +/- for the 1/2" storm through the 25 year storm event, which is the design intent for low impact development.

# CONCLUSION

The proposed project will not have an adverse effect on abutting properties and infrastructure as a result of stormwater runoff. As the stormwater summary indicates, the peak flow rates discharging from the site will be managed to minimize impacts to the surrounding areas. Three grass soil filter ponds will be constructed to provide retention and treatment of stormwater on site prior discharging to the Emery Street drainage. As noted in the drainage report, the stormwater model utilizes a number of conservative design approaches. The estimated soil type and infiltration rates were conservative based on the Soil and Ksat values. Although not a City of Portsmouth requirement, a 15-percent increase was added to each rainfall event, similar to the requirements of NHDES Alteration of Terrain permitting. Additionally, there are proposed stormwater management features such as roof drip edges and grasses swales that were not incorporated in to the design model. It is expected that the drip edges will infiltrate all of the roof flows directed to them in all but the largest storm events. With this conservative approach, the post development stormwater model still indicates a variance of 0.1 cfs +/- for the 1/2" storm through the 25 year storm event, which is within the design modeling tolerance, and illustrates that the project is managing the stormwater flows.

In addition to the permanent stormwater management practices, appropriate steps will be taken to properly mitigate erosion and sedimentation during construction through the use of temporary Best Management Practices for sediment and erosion control. In summary, the proposed development will manage stormwater runoff during construction and post development so that there is no adverse impact to the surrounding area as a result of this development.

#### Methodology

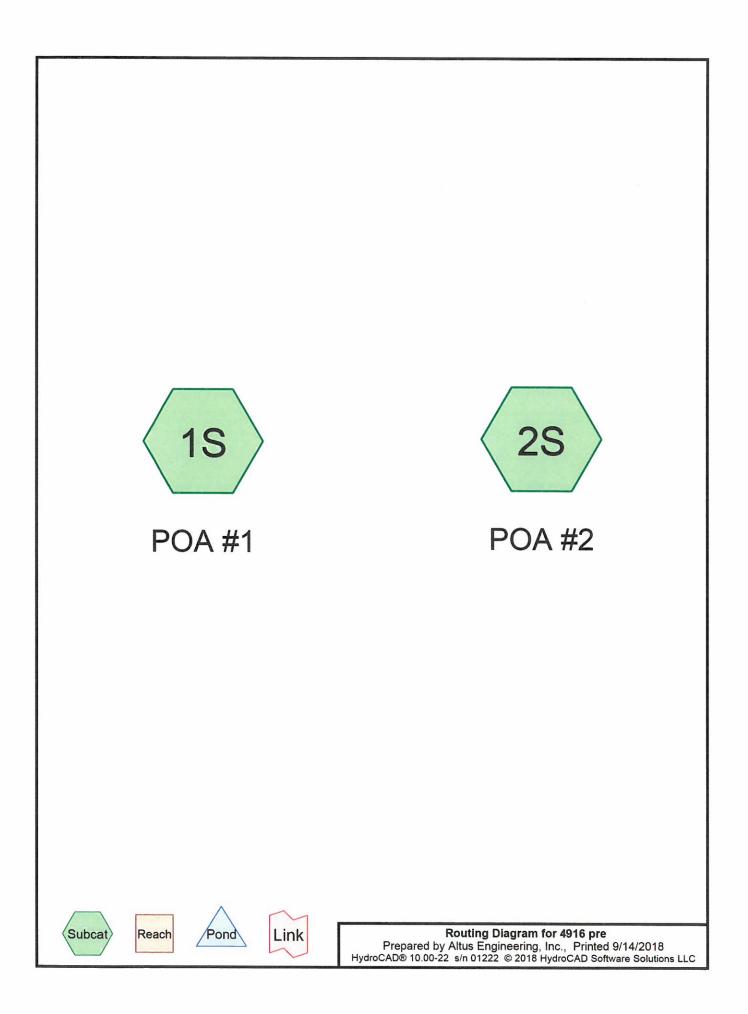
The drainage study was completed using the USDA SCS TR-20 Method within the HydroCAD Stormwater Modeling System. Reservoir routing was performed with the Dynamic Storage Indication method which automates the calculation of Tailwater conditions. A Type III 24-hour rainfall distribution was utilized in analyzing the data for the 0.5", 2, 10, 25, and 50 year 24-hour storm events using rainfall data obtained from the Northeast Regional Climate Center (NRCC) Extreme Precipitation Tables. As a conservative measure, 15-percent has been added to each rainfall mimic the requirements of NHDES Alteration of Terrain Permitting requirements. Site topography, existing features, proposed site improvements, proposed grading, drainage and erosion control measures are shown on the accompanying plans. Recommended erosion control measures are based upon the December 2008 edition of the "New Hampshire Stormwater Manual Volumes 1 through 3" prepared by NHDES and Comprehensive Environmental, Inc. as amended.

#### Stormwater Modeling Disclaimer

Altus Engineering, Inc. notes that stormwater modeling is limited in its capacity to precisely predict peak rates of runoff and flood elevations. Results should not be considered to represent actual storm events due to the number of variables and assumptions involved in the modeling effort. Surface roughness coefficients (n), entrance loss coefficients (ke), velocity factors (kv) and times of concentration (Tc) are based on subjective field observations and engineering judgment using available data. For design purposes, curve numbers (Cn) describe the average conditions. However, curve numbers will vary from storm to storm depending on the antecedent runoff conditions (ARC) including saturation and frozen ground. Also, higher water elevations than predicted by modeling could occur if drainage channels, closed drain systems or culverts are not maintained and/or become blocked by debris before and/or during a storm event as this will impact flow capacity of the structures. Structures should be re-evaluated if future changes occur within relevant drainage areas in order to assess any required design modification.

# APPENDIX

- A. Site Maps
  - a. USGS Map
  - b. Aerial Image
- B. HydroCAD Modeling Results
  - a. Extreme Precipitation Table
  - b. Pre-Development (2, 10, 25, & 50 Year Storms)
  - c. Post Development (2, 10, 25, & 50 Year Storms)
- C. Web Soil Survey
- D. Plans
  - Project Site Plans (Separate Submittal)
  - Pre-Development Watershed Plan
  - Post-Development Watershed Plan



# Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.634	48	Brush, Good, HSG B (1S)
1.629	55	Woods, Good, HSG B (1S, 2S)
2.263	53	TOTAL AREA

## **4916 pre** Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

# Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
2.263	HSG B	1S, 2S
0.000	HSG C	
0.000	HSG D	
0.000	Other	
2.263		TOTAL AREA

4916 pre
Prepared by Altus Engineering, Inc.
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

				· · · ·	,		
HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
 0.000	0.634	0.000	0.000	0.000	0.634	Brush, Good	1S
0.000	1.629	0.000	0.000	0.000	1.629	Woods, Good	1S, 2S
0.000	2.263	0.000	0.000	0.000	2.263	TOTAL	
						AREA	

# Ground Covers (all nodes)

4916 pre	Type III 24-hr	0.5 Inch storm Rainfall=0.50"
Prepared by Altus Engineering, Inc.		Printed 9/14/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software	Solutions LLC	Page 5

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: POA #1Runoff Area=88,216 sf 0.00% Impervious Runoff Depth=0.00"<br/>Flow Length=425' Tc=21.5 min CN=53 Runoff=0.00 cfs 0.000 afSubcatchment 2S: POA #2Runoff Area=10,360 sf 0.00% Impervious Runoff Depth=0.00"

Total Runoff Area = 2.263 ac Runoff Volume = 0.000 af Average Runoff Depth = 0.00"

100.00% Pervious = 2.263 ac 0.00% Impervious = 0.000 ac

Flow Length=100' Slope=0.1000 '/' Tc=10.5 min CN=55 Runoff=0.00 cfs 0.000 af

<b>4916 pre</b> <i>Ty</i>	be III 24-hr 0.5 Inch storm Rainfall=0.50"
Prepared by Altus Engineering, Inc.	Printed 9/14/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solu	itions LLC Page 6

# Summary for Subcatchment 1S: POA #1

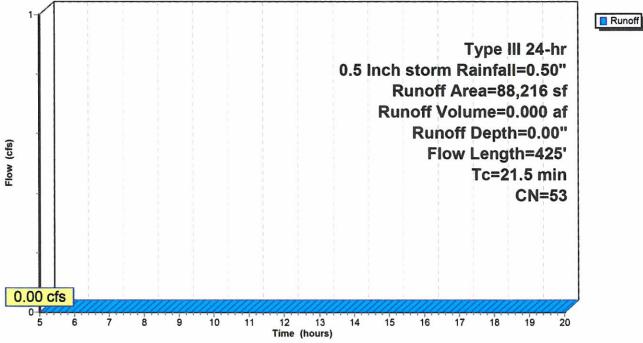
Runoff =	:	0.00 cfs @	5.00 hrs,	Volume=	0.000 af,	Depth= 0.00"
----------	---	------------	-----------	---------	-----------	--------------

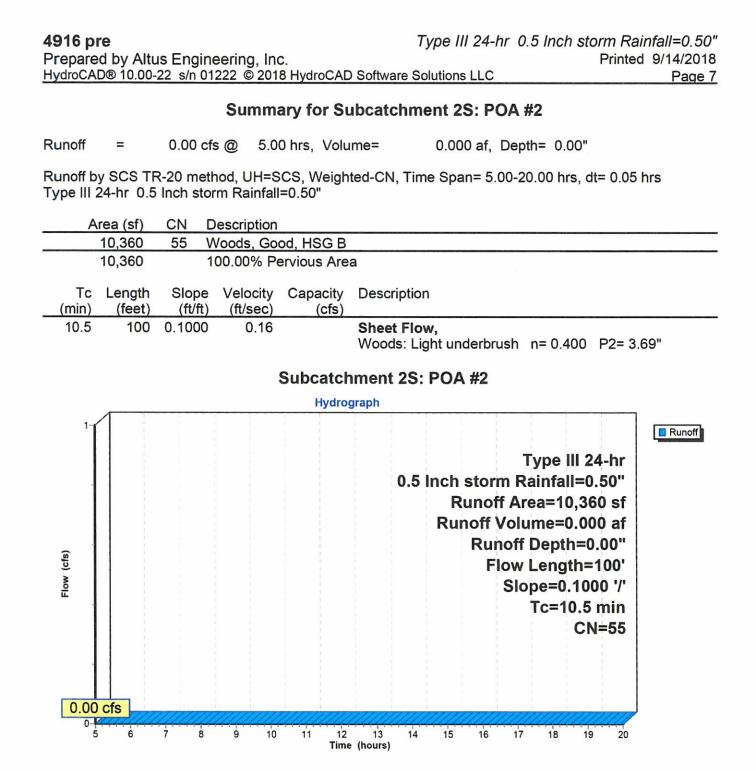
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

A	Area (sf)	CN D	escription		
	27,600		rush, Goo		
	60,616	55 V	Voods, Goo	od, HSG B	
	88,216	53 V	Veighted A	verage	
	88,216	1	00.00% Pe	ervious Are	а
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
15.2	100	0.0400	0.11		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
6.3	325	0.0300	0.87		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
21.5	425	Total			

# Subcatchment 1S: POA #1







4916 pre Type III	24-hr 2-yr storm Rainfall=3.69"
Prepared by Altus Engineering, Inc.	Printed 9/14/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC	Page 8

#### Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: POA #1Runoff Area=88,216 sf 0.00% Impervious Runoff Depth>0.29"Flow Length=425'Tc=21.5 minCN=53Runoff=0.27 cfs 0.048 af

Subcatchment 2S: POA #2 Runoff Area=10,360 sf 0.00% Impervious Runoff Depth>0.35" Flow Length=100' Slope=0.1000 '/' Tc=10.5 min CN=55 Runoff=0.05 cfs 0.007 af

> Total Runoff Area = 2.263 ac Runoff Volume = 0.055 af Average Runoff Depth = 0.29" 100.00% Pervious = 2.263 ac 0.00% Impervious = 0.000 ac

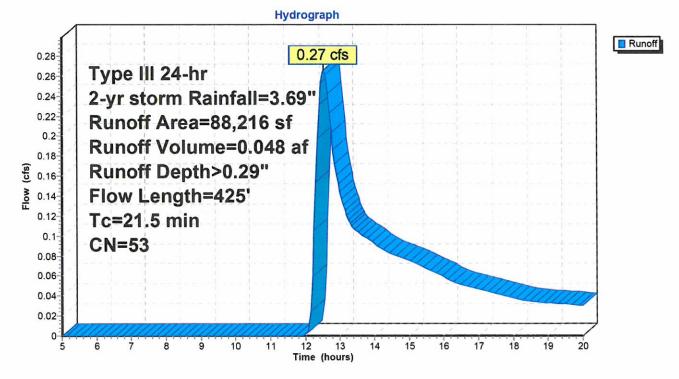
#### Summary for Subcatchment 1S: POA #1

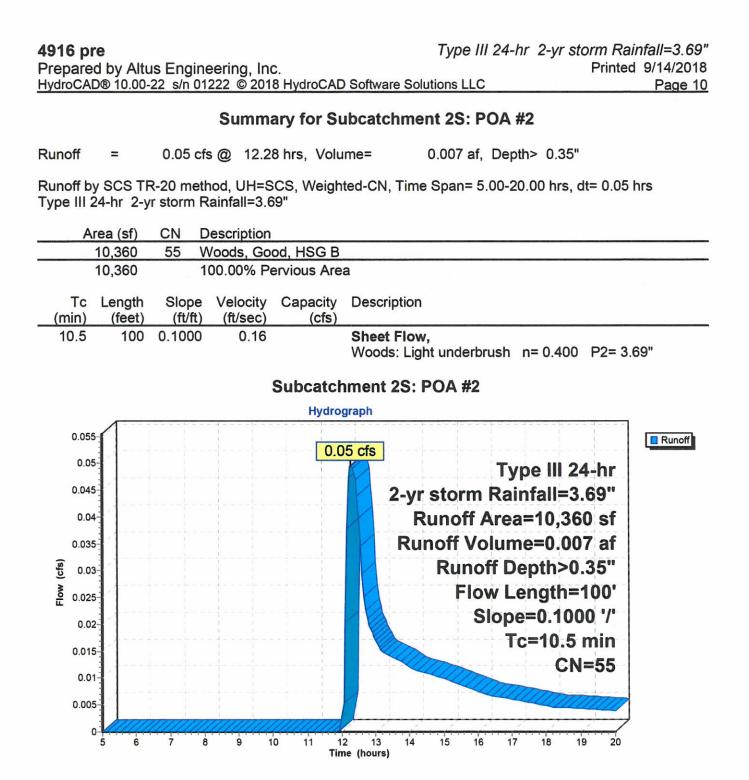
Runoff =	0.27 cfs @	12.53 hrs,	Volume=	0.048 af,	Depth> 0.29"
----------	------------	------------	---------	-----------	--------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

A	rea (sf)	CN D	escription			
	27,600	48 E	rush, Goo	d, HSG B		
	60,616	55 V	Voods, Goo	od, HSG B		
	88,216	53 V	Veighted A	verage		
	88,216	1	00.00% Pe	ervious Are	а	
Тс	Length	Slope	Velocity	Capacity	Description	
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)		
15.2	100	0.0400	0.11		Sheet Flow,	
					Woods: Light underbrush n= 0.400	P2= 3.69"
6.3	325	0.0300	0.87		Shallow Concentrated Flow,	
					Woodland Kv= 5.0 fps	
21.5	425	Total				

#### Subcatchment 1S: POA #1





4916 pre	Type III 24-hr	10-yr storm Rainfall=5.60"
Prepared by Altus Engineering, Inc.		Printed 9/14/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solu	utions LLC	Page 11

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

 Subcatchment 1S: POA #1
 Runoff Area=88,216 sf
 0.00% Impervious
 Runoff Depth>1.02"

 Flow Length=425'
 Tc=21.5 min
 CN=53
 Runoff = 1.46 cfs
 0.172 af

 Subcatchment 2S: POA #2
 Runoff Area=10,360 sf
 0.00% Impervious
 Runoff Depth>1.16"

 Flow Length=100'
 Slope=0.1000 '/'
 Tc=10.5 min
 CN=55
 Runoff = 0.26 cfs
 0.023 af

Total Runoff Area = 2.263 ac Runoff Volume = 0.195 af Average Runoff Depth = 1.04" 100.00% Pervious = 2.263 ac 0.00% Impervious = 0.000 ac

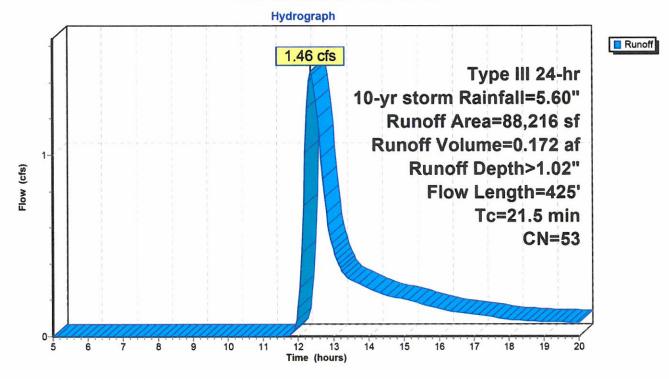
#### Summary for Subcatchment 1S: POA #1

Runoff	=	1.46 cfs @	12.36 hrs,	Volume=	0.172 af,	Depth> 1.02"	
--------	---	------------	------------	---------	-----------	--------------	--

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

 A	rea (sf)	CN [	Description			
	27,600	48 E	Brush, Goo	d, HSG B		
	60,616	55 V	Voods, Go	od, HSG B		
	88,216	53 V	Veighted A	verage		
	88,216	1	00.00% Pe	ervious Are	а	
Тс	Length	Slope	Velocity	Capacity	Description	
 (min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
15.2	100	0.0400	0.11		Sheet Flow,	
					Woods: Light underbrush n= 0.400 P2	2= 3.69"
6.3	325	0.0300	0.87		Shallow Concentrated Flow,	
					Woodland Kv= 5.0 fps	
21.5	425	Total				

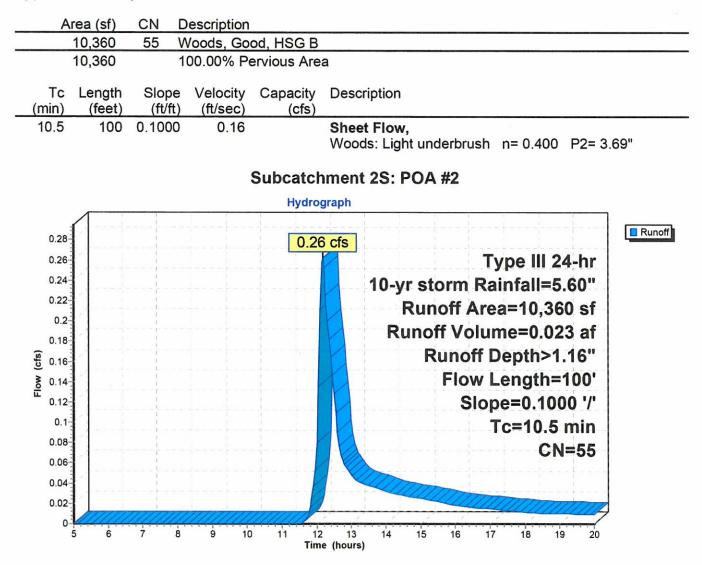
## Subcatchment 1S: POA #1



#### Summary for Subcatchment 2S: POA #2

Runoff = 0.26 cfs @ 12.17 hrs, Volume= 0.023 af, Depth> 1.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"



4916 pre	Type III 24-hr 25-yr storm Rainfall=7.10"
Prepared by Altus Engineering, Inc.	Printed 9/14/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Sc	olutions LLC Page 14

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: POA #1Runoff Area=88,216 sf 0.00% Impervious Runoff Depth>1.80"Flow Length=425'Tc=21.5 minCN=53Runoff=2.82 cfs 0.303 af

Subcatchment 2S: POA #2 Runoff Area=10,360 sf 0.00% Impervious Runoff Depth>1.99" Flow Length=100' Slope=0.1000 '/' Tc=10.5 min CN=55 Runoff=0.49 cfs 0.039 af

> Total Runoff Area = 2.263 ac Runoff Volume = 0.343 af Average Runoff Depth = 1.82" 100.00% Pervious = 2.263 ac 0.00% Impervious = 0.000 ac

4916 pre	Type III 24-hr 25-yr storm Rainfall=7.10"
Prepared by Altus Engineering, Inc.	Printed 9/14/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Sc	blutions LLC Page 15

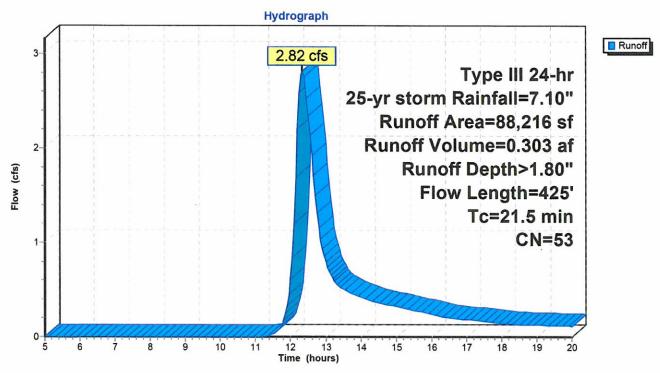
# Summary for Subcatchment 1S: POA #1

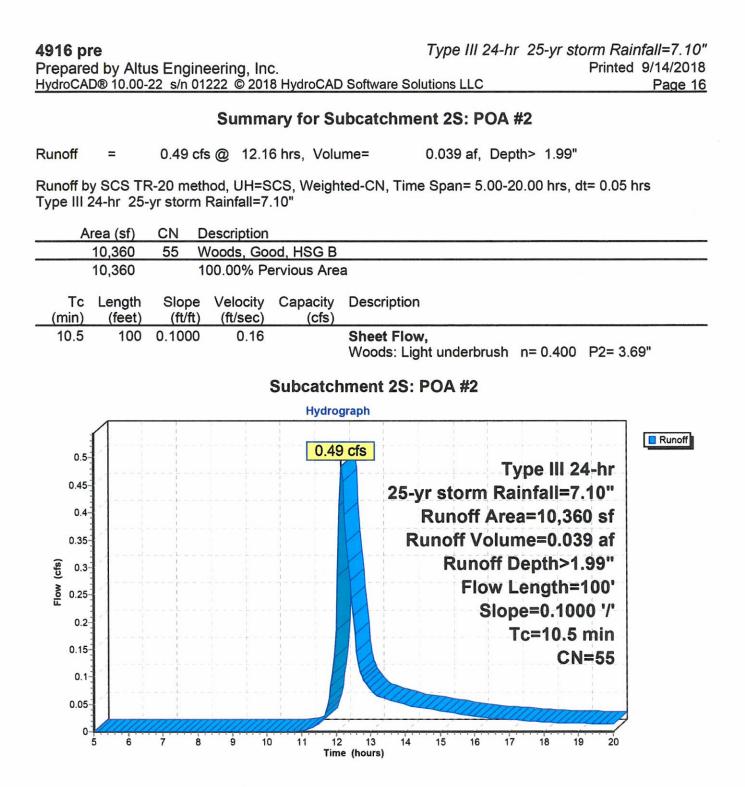
Runoff = $2.82$ cfs (a) 12.33 nrs, Volume= $0.303$ at, Depth>	unoff	.82 cfs @ 12.33 hrs, Volume=	0.303 af, Depth> 1.80"
---	-------	------------------------------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

-	Area (sf)	CN	Description		
	27,600	48	Brush, Goo	d, HSG B	
	60,616	55	Woods, Go	od, HSG B	
	88,216	53	Weighted A	verage	
	88,216			ervious Are	a
	c Length			Capacity	Description
(mir	n) (feet	) (ft/	ft) (ft/sec)	(cfs)	
15.	2 100	0.040	0.11		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
6.	3 325	5 0.030	0.87		Shallow Concentrated Flow,
Children and Child					Woodland Kv= 5.0 fps
21.	5 425	5 Total			

# Subcatchment 1S: POA #1





<b>4916 pre</b> Type III 24-	hr 50-yr storm Rainfall=8.50"
Prepared by Altus Engineering, Inc.	Printed 9/14/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC	Page 17

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: POA #1	Runoff Area=88,216 sf 0.00% Impervious Runoff Depth>2.63" Flow Length=425' Tc=21.5 min CN=53 Runoff=4.28 cfs 0.445 af
Subcatchment 2S: POA #2	Runoff Area=10,360 sf 0.00% Impervious Runoff Depth>2.87"
Flow Length=100'	Slope=0.1000 '/' Tc=10.5 min CN=55 Runoff=0.72 cfs 0.057 af

Total Runoff Area = 2.263 ac Runoff Volume = 0.501 af Average Runoff Depth = 2.66" 100.00% Pervious = 2.263 ac 0.00% Impervious = 0.000 ac

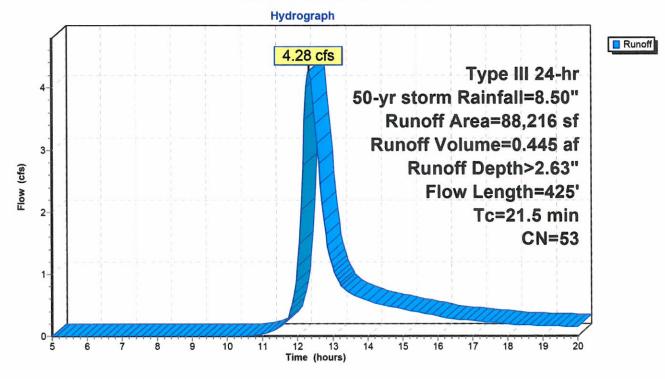
#### Summary for Subcatchment 1S: POA #1

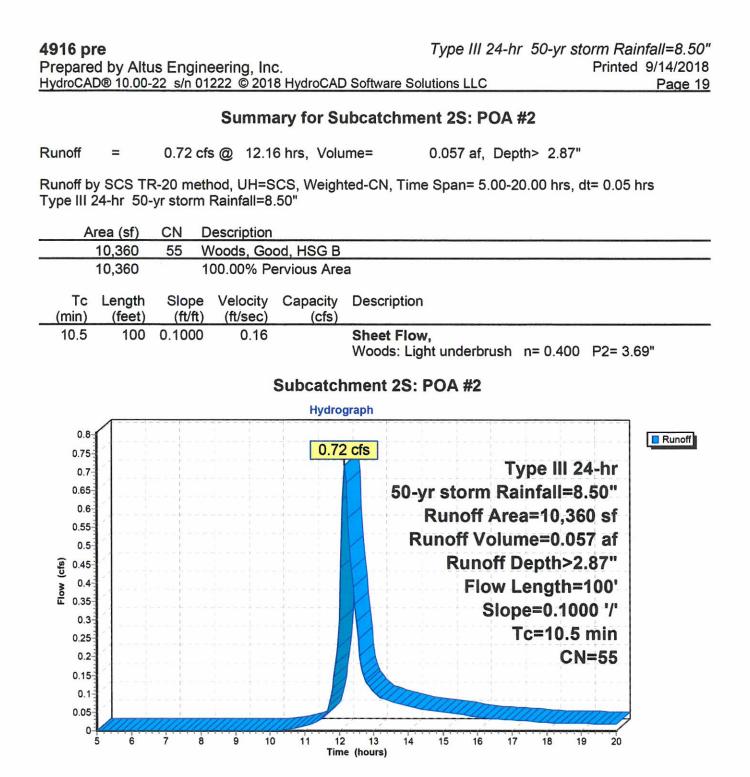
Runoff = 4.28 cfs @ 12.32 hrs, Volume= 0.445 af, Depth> 2.63"

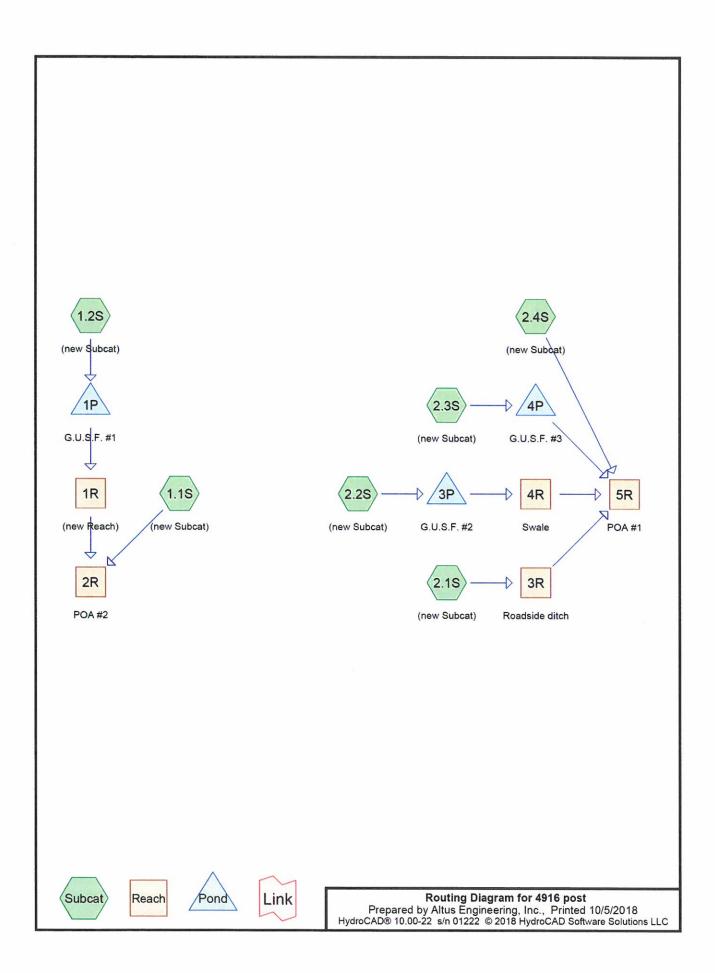
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

	A	rea (sf)	CN E	Description		
		27,600		Brush, Goo		
_		60,616	55 V	Voods, Goo	od, HSG B	
		88,216	53 V	Veighted A	verage	
		88,216	1	00.00% Pe	ervious Are	a
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	15.2	100	0.0400	0.11		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.69"
	6.3	325	0.0300	0.87		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	21.5	425	Total			

# Subcatchment 1S: POA #1







**4916 post** Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Printed 10/5/2018 Page 2

# Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.434	61	>75% Grass cover, Good, HSG B (1.1S, 1.2S, 2.1S, 2.2S, 2.3S, 2.4S)
0.634	48	Brush, Good, HSG B (1.2S, 2.2S, 2.4S)
0.166	98	Paved parking, HSG B (1.1S, 1.2S, 2.1S, 2.2S, 2.3S, 2.4S)
0.025	98	Roofs, HSG B (1.2S, 2.2S, 2.4S)
0.066	98	Unconnected roofs, HSG B (1.2S, 2.2S, 2.4S)
0.939	55	Woods, Good, HSG B (1.1S, 1.2S, 2.1S, 2.2S, 2.3S, 2.4S)
2.263	59	TOTAL AREA

**4916 post** Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

# Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
2.263	HSG B	1.1S, 1.2S, 2.1S, 2.2S, 2.3S, 2.4S
0.000	HSG C	
0.000	HSG D	
0.000	Other	
2.263		TOTAL AREA

Prepared by HydroCAD® 10				Software So	olutions LLC		10/5/2018 Page 4
			Ground C	Covers (all	nodes)		
HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.434	0.000	0.000	0.000	0.434	>75% Grass cover, Good	1.1S, 1.2S, 2.1S, 2.2S, 2.3S, 2.4S
0.000	0.634	0.000	0.000	0.000	0.634	Brush, Good	1.2S, 2.2S, 2.4S
0.000	0.166	0.000	0.000	0.000	0.166	Paved parking	1.1S, 1.2S, 2.1S, 2.2S, 2.3S, 2.4S
0.000	0.025	0.000	0.000	0.000	0.025	Roofs	1.2S, 2.2S, 2.4S
0.000	0.066	0.000	0.000	0.000	0.066	Unconnected roofs	1.2S, 2.2S, 2.4S
0.000	0.939	0.000	0.000	0.000	0.939	Woods, Good	1.1S, 1.2S, 2.1S, 2.2S, 2.3S, 2.4S
0.000	2.263	0.000	0.000	0.000	2.263	TOTAL AREA	

4916 post

4916 post	Type III 24-h
Prepared by Altus Engineering, Inc.	
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Softwar	e Solutions LLC

# Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1.1S: (new Sub Flow	w Length=130' Runoff Area=4,731 sf 37.16% Impervious Runoff D v Length=130' Slope=0.0750 '/' Tc=6.0 min CN=74 Runoff=0.00 c	
Subcatchment1.2S: (new Sub	Runoff Area=41,050 sf 2.04% Impervious Runoff D Flow Length=230' Tc=18.1 min CN=55 Runoff=0.00 c	
Subcatchment2.1S: (new Sub	Cocat) Runoff Area=8,038 sf 36.17% Impervious Runoff D Flow Length=85' Tc=6.0 min CN=73 Runoff=0.00 c	
Subcatchment2.2S: (new Sub	Cat) Runoff Area=18,932 sf 16.87% Impervious Runoff D Flow Length=190' Tc=11.9 min UI Adjusted CN=60 Runoff=0.00 c	
Subcatchment2.3S: (new Sub	Cocat) Runoff Area=3,654 sf 31.53% Impervious Runoff D Flow Length=65' Tc=6.0 min CN=71 Runoff=0.00 c	
Subcatchment2.4S: (new Sub	Flow Length=160' Tc=10.4 min UI Adjusted CN=52 Runoff=0.00 c	
Reach 1R: (new Reach)	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 c n=0.022 L=100.0' S=0.1020 '/' Capacity=6.31 cfs Outflow=0.00 c	
Reach 2R: POA #2	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 c n=0.022 L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=0.00 c	
Reach 3R: Roadside ditch	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 c n=0.022 L=160.0' S=0.0125 '/' Capacity=2.21 cfs Outflow=0.00 c	
Reach 4R: Swale	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 c n=0.022 L=85.0' S=0.0624 '/' Capacity=4.93 cfs Outflow=0.00 c	
Reach 5R: POA #1	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 c n=0.022 L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=0.00 c	
Pond 1P: G.U.S.F. #1 Disc	Peak Elev=46.17' Storage=0 cf Inflow=0.00 c carded=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af Outflow=0.00 c	
Pond 3P: G.U.S.F. #2 Disc	Peak Elev=44.17' Storage=0 cf Inflow=0.00 c carded=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af Outflow=0.00 c	
Pond 4P: G.U.S.F. #3 Disc	Peak Elev=45.17' Storage=0 cf Inflow=0.00 c carded=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af Outflow=0.00 c	
Total Runoff Ar	rea = 2.263 ac Runoff Volume = 0.000 af Average Runoff D 88.66% Pervious = 2.006 ac 11.34% Impervious	

4916 post	Type III 24-hr 0.5 Inch storm Rainfall=0.50"	
Prepared by Altus Engineering, Inc.	Printed 10/5/2018	
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software	re Solutions LLC Page 6	

# Summary for Subcatchment 1.1S: (new Subcat)

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af,	Depth= 0.00"
---	--------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

A	rea (sf)	CN [	Description		
	1,758	98 F	Paved park	ing, HSG B	3
	2,373	61 >	75% Gras	s cover, Go	ood, HSG B
	600	55 V	Voods, Go	od, HSG B	
	4,731	74 V	Veighted A	verage	
	2,973	6	52.84% Per	vious Area	
	1,758	3	37.16% Imp	pervious Ar	ea
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
0.5	130	0.0750	4.11		Shallow Concentrated Flow,
-					Grassed Waterway Kv= 15.0 fps
0.5	130	Total, I	ncreased t	o minimum	n Tc = 6.0 min

# Summary for Subcatchment 1.2S: (new Subcat)

Runoff	=	0.00 cfs @	5.00 hrs.	Volume=	0.000 af,	Depth=	0.00"
--------	---	------------	-----------	---------	-----------	--------	-------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

A	rea (sf)	CN C	escription		
	138	98 F	Roofs, HSG	B	
	214	98 L	Inconnecte	ed roofs, HS	SG B
	486	98 F	aved park	ing, HSG B	
	3,131	61 >	75% Grass	s cover, Go	ood, HSG B
	4,903	48 E	Brush, Goo	d, HSG B	
	32,178	55 V	Voods, Go	od, HSG B	
	41,050	55 V	Veighted A	verage	
	40,212	9	7.96% Per	vious Area	L
	838	2	.04% Impe	ervious Area	а
	214	2	5.54% Un	connected	
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
15.2	100	0.0400	0.11		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
2.7	100	0.0150	0.61		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.2	30	0.0200	2.12		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
18.1	230	Total			

# Summary for Subcatchment 2.1S: (new Subcat)

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

A	rea (sf)	CN E	Description		
	2,907	98 F	Paved park	ing, HSG B	3
	3,899	61 >	75% Gras	s cover, Go	ood, HSG B
	1,232	<u>55 V</u>	Voods, Go	od, HSG B	
	8,038	73 V	Veighted A	verage	
	5,131	6	3.83% Per	vious Area	
	2,907	3	86.17% Imp	pervious Ar	ea
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
4.6	50	0.2000	0.18		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.2	35	0.0600	3.67		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
4.8	85	Total, I	ncreased t	o minimum	1 Tc = 6.0 min

#### Summary for Subcatchment 2.2S: (new Subcat)

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000	af, Depth= 0.00"
---	------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

	Area (sf)	CN /	Adj Desc	cription				
	626	98	Roof	Roofs, HSG B				
	1,794	98	Unco	onnected ro	ofs, HSG B			
	774	98		d parking,				
	5,940	61	>75%	6 Grass co	ver, Good, HSG B			
	4,945	48		Brush, Good, HSG B				
	4,853	55	Woo	ds, Good, I	ISG B			
	18,932	62	60 Weig	hted Avera	age, UI Adjusted			
	15,738		83.1	3% Perviou	is Area			
	3,194		16.8	7% Impervi	ous Area			
	1,794		56.1	7% Unconr	nected			
Т		Slope	Velocity	Capacity	Description			
(min	6	(ft/ft)	(ft/sec)	(cfs)				
11.5	5 100	0.0800	0.15		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.69"			
0.4	4 90	0.0500	3.35		Shallow Concentrated Flow,			
					Grassed Waterway Kv= 15.0 fps			
11.9	9 190	Total						

# Summary for Subcatchment 2.3S: (new Subcat)

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.0	000 af, Depth= 0.00"
---	----------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

_	A	rea (sf)	CN I	Description		
		1,152	98 I	Paved parki	ng, HSG B	
		1,622	61 3	>75% Grass	s cover, Go	ood, HSG B
_		880	55 \	Woods, Goo	od, HSG B	
		3,654	71 \	Weighted A	verage	
		2,502	(	58.47% Per	vious Area	
		1,152	:	31.53% Imp	ervious Are	ea
	Тс	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	4.3	40	0.1500	0.16		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.69"
	0.1	25	0.1800	6.36		Shallow Concentrated Flow,
()						Grassed Waterway Kv= 15.0 fps
	4.4	65	Total,	Increased t	o minimum	Tc = 6.0 min

#### Summary for Subcatchment 2.4S: (new Subcat)

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af,	Depth= 0.0	00"
---	------------	-----

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

	A	rea (sf)	CN A	Adj Desc	ription	
		317	98	Roof	s, HSG B	
		872	98	Unco	nnected ro	ofs, HSG B
		145	98	Pave	d parking,	HSG B
		1,928	61	>75%	6 Grass co	ver, Good, HSG B
		17,752	48	Brus	h, Good, H	SG B
-		1,157	55	Woo	ds, Good, H	HSG B
		22,171	53			age, UI Adjusted
		20,837		93.9	8% Perviou	is Area
		1,334			% Impervio	
		872		65.3	7% Unconn	nected
					-	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	9.3	80	0.0875	0.14		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.69"
	1.1	80	0.0625	1.25		Shallow Concentrated Flow,
					10. 200 / 200 / 200 / 200 / 200	Woodland Kv= 5.0 fps
	10.4	160	Total			

**4916 post** Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

#### Summary for Reach 1R: (new Reach)

 Inflow Area =
 0.942 ac,
 2.04% Impervious, Inflow Depth =
 0.00" for
 0.5 Inch storm event

 Inflow =
 0.00 cfs @
 5.00 hrs, Volume=
 0.000 af

 Outflow =
 0.00 cfs @
 5.00 hrs, Volume=
 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.31 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 100.0' Slope= 0.1020 '/' Inlet Invert= 50.20', Outlet Invert= 40.00'



Summary for Reach 2R: POA #2

Inflow Area	a =	1.051 ac,	5.67% Impervious, Inflow	Depth = 0.00"	for 0.5 Inch storm event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0.000 af	
Outflow	=	0.00 cfs @	5.00 hrs, Volume=	0.000 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'

4916 postType III 24-hr0.5 Inch storm Rainfall=0.50"Prepared by Altus Engineering, Inc.Printed10/5/2018HydroCAD® 10.00-22 s/n 01222© 2018 HydroCAD Software Solutions LLCPage 10

#### Summary for Reach 3R: Roadside ditch

 Inflow Area =
 0.185 ac, 36.17% Impervious, Inflow Depth =
 0.00" for 0.5 Inch storm event

 Inflow =
 0.00 cfs @
 5.00 hrs, Volume=
 0.000 af

 Outflow =
 0.00 cfs @
 5.00 hrs, Volume=
 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 2.21 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 160.0' Slope= 0.0125 '/' Inlet Invert= 42.00', Outlet Invert= 40.00'

Summary for Reach 4R: Swale

Inflow Area	a =	0.435 ac, 16	6.87% Impervious, Inflow	Depth = 0.00"	for 0.5 Inch storm event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0.000 af	
Outflow	=	0.00 cfs @	5.00 hrs, Volume=	0.000 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 4.93 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 85.0' Slope= 0.0624 '/' Inlet Invert= 47.30', Outlet Invert= 42.00'

4916 post Type III 24-hr 0.5 Inch storm Rainfall=0.50" Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

#### Summary for Reach 5R: POA #1

Printed 10/5/2018

Page 11

Inflow Area = 1.212 ac, 16.26% Impervious, Inflow Depth = 0.00" for 0.5 Inch storm event Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af Outflow 5.00 hrs, Volume= 0.00 cfs @ = 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



Summary for Pond 1P: G.U.S.F. #1

Inflow Area	=	0.942 ac,	2.04% Impervious, Ir	nflow Depth = 0.00"	for 0.5 Inch storm event
Inflow	=		5.00 hrs, Volume=		
Outflow	=	0.00 cfs @	5.00 hrs, Volume=	0.000 af, Atte	en= 0%, Lag= 0.0 min
Discarded	=	0.00 cfs @	5.00 hrs, Volume=	0.000 af	
Primary	=	0.00 cfs @	5.00 hrs, Volume=	0.000 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 46.17' @ 5.00 hrs Surf.Area= 615 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no inflow)

Volume	Invert	Ava	il.Storage	Storage Description				
<b>#1</b>	46.17'		2,283 cf	Custom Stage	Data (Prismatic)Li	sted below		
Elevation (feet)		f.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)			
46.17		615	0.0	0	0			
47.17		615	40.0	246	246			
48.67		615	20.0	185	431			
49.00		615	100.0	203	633			
50.00		1,115	100.0	865	1,498			
50.60		1,500	100.0	785	2,283			

4916 post	Type III 24-hr	0.5 Inch storm Rainfall=0.50"
Prepared by Altus Engineering, Inc.		Printed 10/5/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Softwar	e Solutions LLC	Page 12

Device	Routing	Invert	Outlet Devices				
#1	Discarded	46.17'	0.600 in/hr Exfiltration over Surface area				
#2	Primary	50.20'	4.0' long x 8.0' breadth Broad-Crested Rectangular Weir				
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00				
			2.50 3.00 3.50 4.00 4.50 5.00 5.50				
			Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64				
			2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74				
Discarded OutFlow Max=0.00 cfs @ 5.00 hrs HW=46.17' (Free Discharge)							
T-1=Exfiltration (Passes 0.00 cfs of 0.01 cfs potential flow)							

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=46.17' (Free Discharge)

# Summary for Pond 3P: G.U.S.F. #2

Inflow Area =	0.435 ac, 16.87% Impervious, Inflow I	Depth = 0.00" for 0.5 Inch storm event
Inflow =	0.00 cfs @ 5.00 hrs, Volume=	0.000 af
Outflow =	0.00 cfs @ 5.00 hrs, Volume=	0.000 af, Atten= 0%, Lag= 0.0 min
Discarded =	0.00 cfs @ 5.00 hrs, Volume=	0.000 af
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 44.17' @ 5.00 hrs Surf.Area= 500 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no inflow)

Volume	Inve	ert Ava	il.Stora	brage Storage Description				
#1	44.1	7'	1,351	cf Custom Stage	e Data (Prismatio	:)Listed below		
Elevatio	et)	Surf.Area (sq-ft)	Voids (%)	) (cubic-feet)	Cum.Store (cubic-feet)			
44.1		500	0.0		0			
44.6 46.1		500	40.0		100			
		500	20.0		250			
46.5		500	100.0		415			
47.0		697	100.0		714			
47.7	75	1,000	100.0	636	1,351			
Device	Routing	Ir	vert	Outlet Devices				
#1	Discarde	d 44	1.17'	0.600 in/hr Exfiltra	tion over Surface	e area		
#2	Primary	47	7.30'	4.0' long x 8.0' bre	adth Broad-Cres	ted Rectangular Weir		
				00 1.20 1.40 1.60 1.80 2.00				
	2.50 3.00 3.50 4.00 4.50 5.00 5.50							
				Coef. (English) 2.4	3 2.54 2.70 2.69	2.68 2.68 2.66 2.64 2.64		
				2.64 2.65 2.65 2.6				

**Discarded OutFlow** Max=0.00 cfs @ 5.00 hrs HW=44.17' (Free Discharge) **1=Exfiltration** (Passes 0.00 cfs of 0.01 cfs potential flow)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=44.17' (Free Discharge)

#### Summary for Pond 4P: G.U.S.F. #3

Inflow Area =	0.084 ac, 31	.53% Impervious, Inflow [	Depth = 0.00"	for 0.5 Inch storm event
Inflow =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	
Outflow =	0.00 cfs @	5.00 hrs, Volume=	0.000 af, Atte	en= 0%, Lag= 0.0 min
Discarded =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	
Primary =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 45.17' @ 5.00 hrs Surf.Area= 110 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no inflow)

Volume	Inve	rt Ava	il.Storage	age Storage Description				
#1	45.17	7'	372 cf	Custom Stage	e Data (Prismatic	)Listed below		
Elevatior	n (	Surf Aree	Voido	In a Chara	Ourse Otherse			
		Surf.Area	Voids	Inc.Store	Cum.Store			
(feet	t)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)			
45.17	7	110	0.0	0	0			
45.67	7	110	40.0	22	22			
47.17	7	110	20.0	33	55			
47.50	0	110	100.0	36	91			
48.00	0	302	100.0	103	194			
48.50	0	410	100.0	178	372			
Device	Routing	In	vert Ou	tlet Devices				
#1	Discarded	45	.17' 0.6	00 in/hr Exfiltrat	ion over Surface	area		
#2	Primary	47	.90' 4.0	long x 8.0' bre	adth Broad-Cres	ted Rectangular Weir		
				Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00				
				2.50 3.00 3.50 4.00 4.50 5.00 5.50				
						2.68 2.68 2.66 2.64 2.64		
					6 2.66 2.68 2.70			

**Discarded OutFlow** Max=0.00 cfs @ 5.00 hrs HW=45.17' (Free Discharge) **1=Exfiltration** (Passes 0.00 cfs of 0.00 cfs potential flow)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=45.17' (Free Discharge)

<b>4916 post</b> Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Sol	Type III 24-hr 2-yr storm Rainfall=3.69" Printed 10/5/2018 utions LLC Page 14
Time span=5.00-20.00 hrs, dt=0.0 Runoff by SCS TR-20 method, UH= Reach routing by Stor-Ind+Trans method - Po	SCS, Weighted-CN
	731 sf 37.16% Impervious Runoff Depth>1.26" Tc=6.0 min CN=74 Runoff=0.17 cfs 0.011 af
	1,050 sf 2.04% Impervious Runoff Depth>0.35" Tc=18.1 min CN=55 Runoff=0.18 cfs 0.028 af
	038 sf 36.17% Impervious Runoff Depth>1.20" Tc=6.0 min CN=73 Runoff=0.27 cfs 0.018 af
	932 sf 16.87% Impervious Runoff Depth>0.54" n UI Adjusted CN=60 Runoff=0.18 cfs 0.020 af
	,654 sf 31.53% Impervious Runoff Depth>1.08" Tc=6.0 min CN=71 Runoff=0.11 cfs 0.008 af
	2,171 sf 6.02% Impervious Runoff Depth>0.26" n UI Adjusted CN=52 Runoff=0.06 cfs 0.011 af
	.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af //' Capacity=6.31 cfs Outflow=0.00 cfs 0.000 af
	.09' Max Vel=3.23 fps Inflow=0.17 cfs 0.011 af '/' Capacity=6.24 cfs Outflow=0.17 cfs 0.011 af
	.18' Max Vel=1.78 fps Inflow=0.27 cfs 0.018 af '/' Capacity=2.21 cfs Outflow=0.25 cfs 0.018 af
	.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af '/' Capacity=4.93 cfs Outflow=0.00 cfs 0.000 af
	.12' Max Vel=3.76 fps Inflow=0.28 cfs 0.031 af '/' Capacity=6.24 cfs Outflow=0.28 cfs 0.031 af
	49.34' Storage=930 cf Inflow=0.18 cfs 0.028 af ary=0.00 cfs 0.000 af Outflow=0.01 cfs 0.006 af
Pond 3P: G.U.S.F. #2 Peak Elev=	-46.84' Storage=621 cf Inflow=0.18 cfs 0.020 af ary=0.00 cfs 0.000 af Outflow=0.01 cfs 0.005 af
Pond 4P: G.U.S.F. #3 Peak Elev=	47.91' Storage=175 cf Inflow=0.11 cfs 0.008 af ary=0.01 cfs 0.001 af Outflow=0.01 cfs 0.004 af

Total Runoff Area = 2.263 acRunoff Volume = 0.096 afAverage Runoff Depth = 0.51"88.66% Pervious = 2.006 ac11.34% Impervious = 0.257 ac

# Summary for Subcatchment 1.1S: (new Subcat)

Runoff = 0.17 cfs @ 12.10 hrs, Volume= 0.011 af, Depth> 1.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

_	A	rea (sf)	CN I	Description							
		1,758	98	Paved parking, HSG B							
		2,373	61 :	>75% Grass	s cover, Go	od, HSG B					
_		600	55	Noods, Goo	od, HSG B						
		4,731	74	Weighted Average							
		2,973	(	52.84% Per	vious Area						
		1,758	:	37.16% Imp	ervious Are	ea					
	0.0000										
	Tc	Length	Slope		Capacity	Description					
-	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	0.5	130	0.0750	4.11		Shallow Concentrated Flow,					
						Grassed Waterway Kv= 15.0 fps					
	0.5	130	Total,	Increased to	o minimum	Tc = 6.0 min					

#### Summary for Subcatchment 1.2S: (new Subcat)

Runoff = 0.18 cfs @ 12.43 hrs, Volume= 0.028 af, Depth> 0.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

A	rea (sf)	CN E	Description							
	138	98 F	98 Roofs, HSG B							
	214	98 L	Inconnecte	ed roofs, HS	SG B					
	486	98 F	Paved park	ing, HSG B	3					
	3,131	61 >	75% Gras	s cover, Go	bod, HSG B					
	4,903	48 E	Brush, Goo	d, HSG B						
	32,178	55 V	Voods, Go	od, HSG B						
	41,050	55 V	Veighted A	verage						
	40,212	9	7.96% Per	vious Area						
	838	2	.04% Impe	ervious Area	а					
	214	2	5.54% Un	connected						
Tc	Length	Slope	Velocity	Capacity	Description					
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)						
15.2	100	0.0400	0.11		Sheet Flow,					
1 an 1 an 1					Woods: Light underbrush n= 0.400 P2= 3.69"					
2.7	100	0.0150	0.61		Shallow Concentrated Flow,					
					Woodland Kv= 5.0 fps					
0.2	30	0.0200	2.12		Shallow Concentrated Flow,					
					Grassed Waterway Kv= 15.0 fps					
18.1	230	Total								

# Summary for Subcatchment 2.1S: (new Subcat)

Runoff	=	0.27 cfs @	12.10 hrs,	Volume=	0.018 af,	Depth>	1.20"	
--------	---	------------	------------	---------	-----------	--------	-------	--

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

A	rea (sf)	CN [	Description		
	2,907	98 F	Paved parki	ing, HSG B	1
	3,899	61 >	75% Grass	s cover, Go	ood, HSG B
	1,232	55 N	Voods, Goo	od, HSG B	,
	8,038	73 \	Veighted A	verage	
	5,131	e	53.83% Per	vious Area	
	2,907	3	36.17% Imp	pervious Ar	ea
Tc	Length	Slope		Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
4.6	50	0.2000	0.18		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.2	35	0.0600	3.67		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
4.8	85	Total,	Increased t	o minimum	1 Tc = 6.0 min

# Summary for Subcatchment 2.2S: (new Subcat)

Runoff	=	0.18 cfs @	12.21 hrs,	Volume=	0.020 af,	Depth> 0.54"
--------	---	------------	------------	---------	-----------	--------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

A	rea (sf)	CN A	Adj Desc	ription	
	626	98	Roof	s, HSG B	
	1,794	98	Unco	nnected ro	ofs, HSG B
	774	98		d parking,	
	5,940	61	>75%	6 Grass co	ver, Good, HSG B
	4,945	48	Brus	h, Good, H	SG B
	4,853	55	Woo	ds, Good, I	ISG B
	18,932	62	60 Weig	hted Avera	ige, UI Adjusted
	15,738		83.13	3% Perviou	is Area
	3,194		16.8	7% Impervi	ous Area
	1,794		56.1	7% Unconr	ected
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
11.5	100	0.0800	0.15		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.4	90	0.0500	3.35		Shallow Concentrated Flow,
				-	Grassed Waterway Kv= 15.0 fps
11.9	190	Total			

### Summary for Subcatchment 2.3S: (new Subcat)

Runoff = 0.11 cfs @ 12.10 hrs, Volume= 0.008 af, Depth> 1.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

A	rea (sf)	CN [	Description		
	1,152	98 F	Paved park	ing, HSG B	3
	1,622	61 >	>75% Gras	s cover, Go	bod, HSG B
	880	55 \	Noods, Go	od, HSG B	
	3,654	71 \	Neighted A	verage	
	2,502	e	68.47% Per	vious Area	
	1,152	3	31.53% Imp	pervious Ar	ea
Тс	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
4.3	40	0.1500	0.16		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.1	25	0.1800	6.36		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
4.4	65	Total	Increased t	o minimum	Tc = 6.0 min

Summary for Subcatchment 2.4S: (new Subcat)

#### .

Runoff = 0.06 cfs @ 12.39 hrs, Volume= 0.011 af, Depth> 0.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

A	rea (sf)	CN /	Adj Desc	cription	
	317	98	Roof	s, HSG B	
	872	98	Unco	onnected ro	ofs, HSG B
	145	98	Pave	d parking,	HSG B
	1,928	61	>75%	6 Grass co	ver, Good, HSG B
	17,752	48	Brus	h, Good, H	SG B
	1,157	55	Woo	ds, Good, H	HSG B
	22,171	53	52 Weig	hted Avera	ige, UI Adjusted
	20,837		93.9	8% Perviou	is Area
	1,334		6.02	% Impervio	us Area
	872		65.3	7% Unconn	nected
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
9.3	80	0.0875	0.14		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
1.1	80	0.0625	1.25		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
10.4	160	Total			

4916 postType III 24-hr2-yr storm Rainfall=3.69"Prepared by Altus Engineering, Inc.Printed 10/5/2018HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLCPage 18

#### Summary for Reach 1R: (new Reach)

 Inflow Area =
 0.942 ac,
 2.04% Impervious, Inflow Depth =
 0.00" for 2-yr storm event

 Inflow =
 0.00 cfs @
 5.00 hrs, Volume=
 0.000 af

 Outflow =
 0.00 cfs @
 5.00 hrs, Volume=
 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.31 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 100.0' Slope= 0.1020 '/' Inlet Invert= 50.20', Outlet Invert= 40.00'



Summary for Reach 2R: POA #2

Inflow Area	a =	1.051 ac,	5.67% Impervious,	Inflow Depth >	0.13"	for 2-yr storm event
Inflow	=	0.17 cfs @	12.10 hrs, Volume	= 0.011 a	af	
Outflow	=	0.17 cfs @	12.10 hrs, Volume	= 0.011 a	af, Atter	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 3.23 fps, Min. Travel Time= 0.0 min Avg. Velocity = 1.33 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.10 hrs Average Depth at Peak Storage= 0.09' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'

**4916 post** Type III Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

#### Summary for Reach 3R: Roadside ditch

 Inflow Area =
 0.185 ac, 36.17% Impervious, Inflow Depth >
 1.20" for 2-yr storm event

 Inflow =
 0.27 cfs @
 12.10 hrs, Volume=
 0.018 af

 Outflow =
 0.25 cfs @
 12.15 hrs, Volume=
 0.018 af, Atten= 6%, Lag= 2.9 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 1.78 fps, Min. Travel Time= 1.5 min Avg. Velocity = 0.75 fps, Avg. Travel Time= 3.6 min

Peak Storage= 23 cf @ 12.12 hrs Average Depth at Peak Storage= 0.18' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 2.21 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 160.0' Slope= 0.0125 '/' Inlet Invert= 42.00', Outlet Invert= 40.00'



#### Summary for Reach 4R: Swale

Inflow Area	a =	0.435 ac, 1	6.87% Impervious,	Inflow Depth = 0.0	00" for 2-yr storm event
Inflow	=	0.00 cfs @	5.00 hrs, Volume	= 0.000 af	<ul> <li>A REAL CONTRACTOR CONTRACTOR CONTRACTOR</li> </ul>
Outflow	=	0.00 cfs @	5.00 hrs, Volume	= 0.000 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 4.93 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 85.0' Slope= 0.0624 '/' Inlet Invert= 47.30', Outlet Invert= 42.00'

4916 postType III 24-hr2-yr storm Rainfall=3.69"Prepared by Altus Engineering, Inc.Printed10/5/2018HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLCPage 20

#### Summary for Reach 5R: POA #1

 Inflow Area =
 1.212 ac, 16.26% Impervious, Inflow Depth > 0.30" for 2-yr storm event

 Inflow =
 0.28 cfs @
 12.16 hrs, Volume=
 0.031 af

 Outflow =
 0.28 cfs @
 12.16 hrs, Volume=
 0.031 af

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 3.76 fps, Min. Travel Time= 0.0 min Avg. Velocity = 1.87 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.16 hrs Average Depth at Peak Storage= 0.12' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



#### Summary for Pond 1P: G.U.S.F. #1

Inflow Area =	0.942 ac,	2.04% Impervious, Inflow Depth	> 0.35" for 2-yr storm event
Inflow =	0.18 cfs @		28 af
Outflow =	0.01 cfs @	20.00 hrs, Volume= 0.0	06 af, Atten= 94%, Lag= 454.0 min
Discarded =	0.01 cfs @	20.00 hrs, Volume= 0.0	06 af
Primary =	0.00 cfs @	5.00 hrs, Volume= 0.0	000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 49.34' @ 20.00 hrs Surf.Area= 787 sf Storage= 930 cf

Plug-Flow detention time= 232.7 min calculated for 0.006 af (23% of inflow) Center-of-Mass det. time= 101.2 min (977.3 - 876.0)

Volume	Invert	Ava	il.Storage	Storage Descrip	tion	
#1	46.17'		2,283 cf	Custom Stage	Data (Prismatic)∟	isted below
Elevation (feet)		Area sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
46.17		615	0.0	0	0	
47.17		615	40.0	246	246	
48.67		615	20.0	185	431	
49.00		615	100.0	203	633	
50.00		1,115	100.0	865	1,498	
50.60	-	1,500	100.0	785	2,283	

4916 post

Type III 24-hr 2-yr storm Rainfall=3.69" Printed 10/5/2018 Page 21

Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices						
#1 #2	Discarded Primary	46.17' 50.20'	0.600 in/hr Exfiltration over Surface area 4.0' long x 8.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74						
		v Max=0.01 cfs Exfiltration Con	a @ 20.00 hrs HW=49.34' (Free Discharge) trols 0.01 cfs)						
Primary 12=Br	OutFlow Noad-Creste	/lax=0.00 cfs ( <b>d Rectangula</b>	0 5.00 hrs HW=46.17' (Free Discharge) Weir (Controls 0.00 cfs)						
		S	ummary for Pond 3P: G.U.S.F. #2						
Inflow A Inflow Outflow Discarde Primary Routing	= = ed = =	0.18 cfs @ 12 0.01 cfs @ 20 0.01 cfs @ 20 0.00 cfs @ 3	87% Impervious, Inflow Depth > 0.54" for 2-yr storm event         8.21 hrs, Volume=       0.020 af         0.00 hrs, Volume=       0.005 af, Atten= 95%, Lag= 467.1 min         0.00 hrs, Volume=       0.005 af         0.00 hrs, Volume=       0.000 af         0.00 hrs, Volume=       0.000 af         0.00 hrs, Volume=       0.000 af						
Peak Ele Plug-Flo	Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 46.84' @ 20.00 hrs Surf.Area= 636 sf Storage= 621 cf Plug-Flow detention time= 236.6 min calculated for 0.005 af (27% of inflow) Center-of-Mass det. time= 120.0 min (972.4 - 852.3)								
Volume	Inver		age Storage Description						
#1	44.17	1,3	1 cf Custom Stage Data (Prismatic)Listed below						
Elevatio	et)	urf.Area Void (sq-ft) (%	b) (cubic-feet) (cubic-feet)						
44.1 44.6 46.1 46.5 47.0 47.7	67 17 50 00	500 0 500 40 500 20 500 100 697 100 1,000 100	0 150 250 0 165 415 0 299 714						
Device	Routing	Invert	Outlet Devices						
#1 #2	Discarded Primary	44.17' 47.30'	0.600 in/hr Exfiltration over Surface area 4.0' long x 8.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74						

4916 post	Type III 24-hr	2-yr storm Rainfall=3.69"
Prepared by Altus Engineering, Inc.		Printed 10/5/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solu	tions LLC	Page 22

**Discarded OutFlow** Max=0.01 cfs @ 20.00 hrs HW=46.84' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=44.17' (Free Discharge)

#### Summary for Pond 4P: G.U.S.F. #3

Inflow Area =	0.084 ac, 31.53% Impervious, Inflow D	Depth > 1.08" for 2-yr storm event
Inflow =	0.11 cfs @ 12.10 hrs, Volume=	0.008 af
Outflow =	0.01 cfs @ 13.13 hrs, Volume=	0.004 af, Atten= 89%, Lag= 61.9 min
Discarded =	0.00 cfs @ 13.13 hrs, Volume=	0.002 af
Primary =	0.01 cfs @ 13.13 hrs, Volume=	0.001 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 47.91' @ 13.13 hrs Surf.Area= 266 sf Storage= 175 cf

Plug-Flow detention time= 195.4 min calculated for 0.004 af (49% of inflow) Center-of-Mass det. time= 104.2 min ( 922.9 - 818.7 )

Volume	Inver	t Ava	il.Storag	ge Storage Description				
#1	45.17	71	372	cf Custom Stage	e Data (Prismatic	)Listed below		
Elevetie			Maida	las Olars	Ourse Others			
Elevatio		Surf.Area	Voids	Inc.Store	Cum.Store			
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)			
45.1	7	110	0.0	0	0			
45.6	57	110	40.0	22	22			
47.1	7	110	20.0	33	55			
47.5	50	110	100.0	36	91			
48.0	00	302	100.0	103	194			
48.5	50	410	100.0	178	372			
Device	Routing	In	vert C	Dutlet Devices				
#1	Discarded	45	5.17' O	.600 in/hr Exfiltra	tion over Surface	area		
#2	Primary			4.0' long x 8.0' breadth Broad-Crested Rectangular Weir				
						00 1.20 1.40 1.60 1.80 2.00		
				2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.				
			2	.64 2.65 2.65 2.6	2.00 2.08 2.70	J 2.14		

**Discarded OutFlow** Max=0.00 cfs @ 13.13 hrs HW=47.91' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.00 cfs @ 13.13 hrs HW=47.91' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 0.00 cfs @ 0.18 fps)

<b>4916 post</b> Prepared by Altus Engineering, Inc. <u>HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solu</u>	Type III 24-hr 10-yr storm Rainfall=5.60" Printed 10/5/2018 utions LLC Page 23											
Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method , Pond routing by Stor-Ind method												
	731 sf 37.16% Impervious Runoff Depth>2.65" Tc=6.0 min CN=74 Runoff=0.36 cfs 0.024 af											
	,050 sf 2.04% Impervious Runoff Depth>1.15" Tc=18.1 min CN=55 Runoff=0.85 cfs 0.091 af											
	038 sf 36.17% Impervious Runoff Depth>2.56" Tc=6.0 min CN=73 Runoff=0.58 cfs 0.039 af											
	932 sf 16.87% Impervious Runoff Depth>1.51" UI Adjusted CN=60 Runoff=0.64 cfs 0.055 af											
	654 sf 31.53% Impervious Runoff Depth>2.39" Tc=6.0 min CN=71 Runoff=0.25 cfs 0.017 af											
	,171 sf 6.02% Impervious Runoff Depth>0.96" UI Adjusted CN=52 Runoff=0.43 cfs 0.041 af											
	09' Max Vel=3.27 fps Inflow=0.17 cfs 0.039 af ' Capacity=6.31 cfs Outflow=0.17 cfs 0.038 af											
Reach 2R: POA #2 Avg. Flow Depth=0.7 n=0.022 L=1.0' S=0.1000 '/'	13' Max Vel=4.05 fps Inflow=0.36 cfs 0.062 af ' Capacity=6.24 cfs Outflow=0.36 cfs 0.062 af											
	26' Max Vel=2.25 fps Inflow=0.58 cfs 0.039 af ' Capacity=2.21 cfs Outflow=0.55 cfs 0.039 af											
Reach 4R: Swale         Avg. Flow Depth=0.7           n=0.022         L=85.0'         S=0.0624 '/'	10' Max Vel=2.77 fps Inflow=0.18 cfs 0.024 af ' Capacity=4.93 cfs Outflow=0.17 cfs 0.024 af											
	23' Max Vel=5.84 fps Inflow=1.22 cfs 0.115 af ' Capacity=6.24 cfs Outflow=1.22 cfs 0.115 af											
	.26' Storage=1,845 cf Inflow=0.85 cfs 0.091 af ry=0.17 cfs 0.039 af Outflow=0.19 cfs 0.050 af											
	.37' Storage=1,027 cf Inflow=0.64 cfs 0.055 af ry=0.18 cfs 0.024 af Outflow=0.19 cfs 0.032 af											
Pond 4P: G.U.S.F. #3 Peak Elev=4 Discarded=0.00 cfs 0.003 af Primar	47.99' Storage=192 cf Inflow=0.25 cfs 0.017 af ry=0.25 cfs 0.010 af Outflow=0.26 cfs 0.013 af											
Total Runoff Area = 2.263 ac Runoff Volum												

Runoff Area = 2.263 ac Runoff Volume = 0.266 af Average Runoff Depth = 1.41" 88.66% Pervious = 2.006 ac 11.34% Impervious = 0.257 ac

#### Summary for Subcatchment 1.1S: (new Subcat)

0.36 cfs @ 12.09 hrs, Volume= Runoff = 0.024 af, Depth> 2.65"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

A	rea (sf)	CN E	escription		· · · · · · · · · · · · · · · · · · ·					
	1,758	98 F	98 Paved parking, HSG B							
	2,373	61 >	61 >75% Grass cover, Good, HSG B							
	600	55 V	Voods, Go	od, HSG B						
	4,731	74 V	Veighted A	verage						
	2,973	6	2.84% Per	vious Area						
	1,758	3	7.16% Imp	pervious Ar	ea					
Тс	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
0.5	130	0.0750	4.11		Shallow Concentrated Flow,					
					Grassed Waterway Kv= 15.0 fps					
0.5	130	Total, I	ncreased t	o minimum	1 Tc = 6.0 min					

creased to minimul

#### Summary for Subcatchment 1.2S: (new Subcat)

Runoff	=	0.85 cfs @	12.29 hrs.	Volume=	0.091 af,	Depth>	1.15"
--------	---	------------	------------	---------	-----------	--------	-------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

A	rea (sf)	CN D	escription		
	138	98 F	loofs, HSG	B	
	214	98 L	Inconnecte	d roofs, HS	SG B
	486	98 P	aved parki	ing, HSG B	
	3,131	61 >	75% Grass	s cover, Go	ood, HSG B
	4,903	48 B	rush, Goo	d, HSG B	
	32,178	55 V	Voods, Go	od, HSG B	
	41,050	55 V	Veighted A	verage	
	40,212	9	7.96% Per	vious Area	
	838	2	.04% Impe	ervious Area	а
	214	2	5.54% Und	connected	
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
15.2	100	0.0400	0.11		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
2.7	100	0.0150	0.61		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.2	30	0.0200	2.12		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
18.1	230	Total			

#### Summary for Subcatchment 2.1S: (new Subcat)

Runoff = 0.58 cfs @ 12.09 hrs, Volume= 0.039 af, Depth> 2.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

_	A	rea (sf)	CN I	Description							
		2,907	98	98 Paved parking, HSG B							
		3,899	61 :	>75% Gras	s cover, Go	ood, HSG B					
		1,232	55 \	Woods, Go	od, HSG B						
		8,038	73	Weighted A	verage						
		5,131	(	53.83% Per	vious Area						
		2,907	:	36.17% Imp	pervious Ar	ea					
	Tc	Length	Slope		Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	4.6	50	0.2000	0.18		Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 3.69"					
	0.2	35	0.0600	3.67		Shallow Concentrated Flow,					
_						Grassed Waterway Kv= 15.0 fps					
	4.8	85	Total.	Increased t	o minimum	Tc = 6.0 min					

8 85 Total, increased to minimum Tc = 6.0 min

### Summary for Subcatchment 2.2S: (new Subcat)

Runoff = 0.64 cfs @ 12.18 hrs, Volume= 0.055 af, Depth> 1.51"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

A	rea (sf)	CN A	Adj Desc	ription	
	626	98	Roof	s, HSG B	
	1,794	98	Unco	onnected ro	oofs, HSG B
	774	98		ed parking,	
	5,940	61	>75%	6 Grass co	ver, Good, HSG B
	4,945	48	Brus	h, Good, H	SG B
	4,853	55	Woo	ds, Good, I	HSG B
	18,932	62	60 Weig	hted Avera	age, UI Adjusted
	15,738		83.1	3% Perviou	is Area
	3,194		16.8	7% Impervi	ous Area
	1,794		56.1	7% Unconr	nected
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
11.5	100	0.0800	0.15		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.4	90	0.0500	3.35		Shallow Concentrated Flow,
-					Grassed Waterway Kv= 15.0 fps
11.9	190	Total			

4916 post	Type III 24-hr	10-yr storm Rainfall=5.60"
Prepared by Altus Engineering, Inc.		Printed 10/5/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software So	olutions LLC	Page 26

# Summary for Subcatchment 2.3S: (new Subcat)

Runoff	=	0.25 cfs @	12.10 hrs,	Volume=	0.017 af,	Depth> 2	2.39"	
--------	---	------------	------------	---------	-----------	----------	-------	--

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

-	A	rea (sf)	CN [	Description						
		1,152	98 F	98 Paved parking, HSG B						
		1,622	61 >	>75% Gras	s cover, Go	ood, HSG B				
		880	55 \	Noods, Go	od, HSG B					
		3,654	71 \	Neighted A	verage					
		2,502	6	58.47% Per	vious Area					
		1,152	3	31.53% Imp	pervious Ar	ea				
	Тс	Length	Slope		Capacity	Description				
0	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	4.3	40	0.1500	0.16		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 3.69"				
	0.1	25	0.1800	6.36		Shallow Concentrated Flow,				
_						Grassed Waterway Kv= 15.0 fps				
	4.4	65	Total,	Increased t	o minimum	Tc = 6.0 min				

# Summary for Subcatchment 2.4S: (new Subcat)

Runoff	=	0.43 cfs @	12.18 hrs,	Volume=	0.041 af,	Depth> 0.96'	
--------	---	------------	------------	---------	-----------	--------------	--

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

A	rea (sf)	CN A	Adj Desc	ription			
	317	98	Roof	Roofs, HSG B			
	872	98	Unco	onnected ro	ofs, HSG B		
	145	98	Pave	d parking,	HSG B		
	1,928	61	>75%	6 Grass co	ver, Good, HSG B		
	17,752	48	Brus	h, Good, H	SG B		
	1,157	55	Woo	ds, Good, H	ISG B		
	22,171	53	52 Weig	hted Avera	age, UI Adjusted		
	20,837 93.98% Pervious Area						
	1,334 6.02% Impervious Area						
	872 65.37% Unconnected						
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
9.3	80	0.0875	0.14		Sheet Flow,		
					Woods: Light underbrush n= 0.400 P2= 3.69"		
1.1	80	0.0625	1.25		Shallow Concentrated Flow,		
				and the state of the second	Woodland Kv= 5.0 fps		
10.4	160	Total					

**4916 post** Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

#### Summary for Reach 1R: (new Reach)

 Inflow Area =
 0.942 ac,
 2.04% Impervious, Inflow Depth >
 0.49" for 10-yr storm event

 Inflow =
 0.17 cfs @
 13.18 hrs, Volume=
 0.039 af

 Outflow =
 0.17 cfs @
 13.20 hrs, Volume=
 0.038 af, Atten= 0%, Lag= 1.3 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 3.27 fps, Min. Travel Time= 0.5 min Avg. Velocity = 2.35 fps, Avg. Travel Time= 0.7 min

Peak Storage= 5 cf @ 13.20 hrs Average Depth at Peak Storage= 0.09' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.31 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 100.0' Slope= 0.1020 '/' Inlet Invert= 50.20', Outlet Invert= 40.00'

Summary for Reach 2R: POA #2

Inflow Area =		1.051 ac,	5.67% Impervious,	Inflow Depth > 0	0.71"	for 10-yr storm event
Inflow	=	0.36 cfs @	12.09 hrs, Volume	= 0.062 a	f	n en dezen en son - Franken son 🔎 en en son en
Outflow	=	0.36 cfs @	12.09 hrs, Volume	= 0.062 a	f, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 4.05 fps, Min. Travel Time= 0.0 min Avg. Velocity = 2.17 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.09 hrs Average Depth at Peak Storage= 0.13' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'

4916 postType III 24-hr 10-yr storm Rainfall=5.60"Prepared by Altus Engineering, Inc.Printed 10/5/2018HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLCPage 28

#### Summary for Reach 3R: Roadside ditch

 Inflow Area =
 0.185 ac, 36.17% Impervious, Inflow Depth > 2.56" for 10-yr storm event

 Inflow =
 0.58 cfs @ 12.09 hrs, Volume=
 0.039 af

 Outflow =
 0.55 cfs @ 12.13 hrs, Volume=
 0.039 af, Atten= 6%, Lag= 2.2 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 2.25 fps, Min. Travel Time= 1.2 min Avg. Velocity = 0.88 fps, Avg. Travel Time= 3.0 min

Peak Storage= 41 cf @ 12.11 hrs Average Depth at Peak Storage= 0.26' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 2.21 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 160.0' Slope= 0.0125 '/' Inlet Invert= 42.00', Outlet Invert= 40.00'



#### Summary for Reach 4R: Swale

 Inflow Area =
 0.435 ac, 16.87% Impervious, Inflow Depth > 0.67" for 10-yr storm event

 Inflow =
 0.18 cfs @ 12.67 hrs, Volume=
 0.024 af

 Outflow =
 0.17 cfs @ 12.69 hrs, Volume=
 0.024 af, Atten= 3%, Lag= 1.2 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 2.77 fps, Min. Travel Time= 0.5 min Avg. Velocity = 1.65 fps, Avg. Travel Time= 0.9 min

Peak Storage= 5 cf @ 12.67 hrs Average Depth at Peak Storage= 0.10' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 4.93 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 85.0' Slope= 0.0624 '/' Inlet Invert= 47.30', Outlet Invert= 42.00'

**4916 post** Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Type III 24-hr 10-yr storm Rainfall=5.60" Printed 10/5/2018 utions LLC Page 29

#### Summary for Reach 5R: POA #1

 Inflow Area =
 1.212 ac, 16.26% Impervious, Inflow Depth >
 1.13" for 10-yr storm event

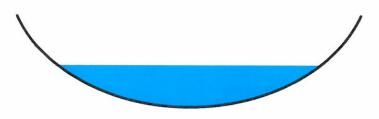
 Inflow =
 1.22 cfs @
 12.15 hrs, Volume=
 0.115 af

 Outflow =
 1.22 cfs @
 12.15 hrs, Volume=
 0.115 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 5.84 fps, Min. Travel Time= 0.0 min Avg. Velocity = 2.52 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.15 hrs Average Depth at Peak Storage= 0.23' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



#### Summary for Pond 1P: G.U.S.F. #1

Inflow Area =	0.942 ac,	2.04% Impervious, Inflow De	epth > 1.15" for 10-yr storm event
Inflow =	0.85 cfs @	12.29 hrs, Volume=	0.091 af
Outflow =	0.19 cfs @	13.18 hrs, Volume=	0.050 af, Atten= 78%, Lag= 53.2 min
Discarded =	0.02 cfs @	13.18 hrs, Volume=	0.011 af
Primary =	0.17 cfs @	13.18 hrs, Volume=	0.039 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.26' @ 13.18 hrs Surf.Area= 1,285 sf Storage= 1,845 cf

Plug-Flow detention time= 175.7 min calculated for 0.050 af (55% of inflow) Center-of-Mass det. time= 85.0 min (927.3 - 842.3)

Volume	Invert	Ava	il.Storage	Storage Descrip	tion	
#1	46.17'		2,283 cf	Custom Stage	Data (Prismatic)Listed	pelow
Elevation (feet)		Area sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
46.17		615	0.0	0	0	
47.17		615	40.0	246	246	
48.67		615	20.0	185	431	
49.00		615	100.0	203	633	
50.00	1	,115	100.0	865	1,498	
50.60	1	,500	100.0	785	2,283	

Type III 24-hr 10-yr storm Rainfall=5.60" 4916 post Prepared by Altus Engineering, Inc. Printed 10/5/2018 HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

2.00
64

Page 30

**Discarded OutFlow** Max=0.02 cfs @ 13.18 hrs HW=50.26' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.16 cfs @ 13.18 hrs HW=50.26' (Free Discharge) —2=Broad-Crested Rectangular Weir (Weir Controls 0.16 cfs @ 0.62 fps)

#### Summary for Pond 3P: G.U.S.F. #2

Inflow Area =	0.435 ac, 16.87% Impervious, Inflow De	epth > 1.51" for 10-yr storm event
Inflow =	0.64 cfs @ 12.18 hrs, Volume=	0.055 af
Outflow =	0.19 cfs @ 12.67 hrs, Volume=	0.032 af, Atten= 70%, Lag= 29.0 min
Discarded =	0.01 cfs @ 12.67 hrs, Volume=	0.008 af
Primary =	0.18 cfs @ 12.67 hrs, Volume=	0.024 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 47.37' @ 12.67 hrs Surf.Area= 846 sf Storage= 1,027 cf

Plug-Flow detention time= 154.1 min calculated for 0.032 af (59% of inflow) Center-of-Mass det. time= 70.0 min ( 896.6 - 826.7 )

Volume	Inve	rt Ava	il.Stora	ge Storage Descr	iption	
#1	44.1	7'	1,351	cf Custom Stage	e Data (Prismatic	:)Listed below
Elevatio (fee 44.1 44.6 46.1 46.5	et) 17 57 17 50	Surf.Area (sq-ft) 500 500 500 500	Voids (%) 0.0 40.0 20.0 100.0	(cubic-feet) 0 100 150 165	Cum.Store (cubic-feet) 0 100 250 415 714	-
47.0 47.7		697 1,000	100.0 100.0		1,351	
Device	Routing	In	vert (	Outlet Devices		
#1 #2	Discarde Primary		7.30' 4	Head (feet) 0.20 0. 2.50 3.00 3.50 4.0	adth Broad-Cres 40 0.60 0.80 1.0 00 4.50 5.00 5.50 3 2.54 2.70 2.69	<b>Sted Rectangular Weir</b> 00 1.20 1.40 1.60 1.80 2.00 0 9 2.68 2.68 2.66 2.64 2.64

<b>4916 post</b> 7	ype III 24-hr	10-yr storm Rair	nfall=5.60"
Prepared by Altus Engineering, Inc.		Printed	10/5/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Soluti	ons LLC		Page 31

**Discarded OutFlow** Max=0.01 cfs @ 12.67 hrs HW=47.37' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.17 cfs @ 12.67 hrs HW=47.37' (Free Discharge) -2=Broad-Crested Rectangular Weir (Weir Controls 0.17 cfs @ 0.63 fps)

### Summary for Pond 4P: G.U.S.F. #3

Inflow Area =	0.084 ac, 31.53% Impervious, Inflow De	epth > 2.39" for 10-yr storm event
Inflow =	0.25 cfs @ 12.10 hrs, Volume=	0.017 af
Outflow =	0.26 cfs @ 12.16 hrs, Volume=	0.013 af, Atten= 0%, Lag= 3.6 min
Discarded =	0.00 cfs @ 12.16 hrs, Volume=	0.003 af
Primary =	0.25 cfs @ 12.16 hrs, Volume=	0.010 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 47.99' @ 12.16 hrs Surf.Area= 297 sf Storage= 192 cf

Plug-Flow detention time= 90.7 min calculated for 0.013 af (76% of inflow) Center-of-Mass det. time= 31.3 min (832.3 - 801.0)

Volume	Inve	ert Ava	il.Storage	Storage Descr	ription	
#1	45.1	7'	372 c	f Custom Stage	e Data (Prismatic	:)Listed below
Elevatio	on	Surf.Area	Voids	Inc.Store	Cum.Store	
(fee		(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
45.1		110	0.0	0	0	
45.6	67	110	40.0	22	22	
47.1	17	110	20.0	33	55	
47.5	50	110	100.0	36	91	
48.0		302	100.0	103	194	
48.5	50	410	100.0	178	372	
Device	Routing	In	vert Ou	Itlet Devices		
#1	Discarde				tion over Surface	
#2	Primary	47				ted Rectangular Weir
						00 1.20 1.40 1.60 1.80 2.00
					0 4.50 5.00 5.5	
						2.68 2.68 2.66 2.64 2.64
			2.6	64 2.65 2.65 2.6	6 2.66 2.68 2.7	0 2.74

**Discarded OutFlow** Max=0.00 cfs @ 12.16 hrs HW=47.98' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.24 cfs @ 12.16 hrs HW=47.98' (Free Discharge)

<b>4916 post</b> Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroC	Type III 24-hr 25-yr storm Rainfall=7.10" Printed 10/5/2018 AD Software Solutions LLC Page 32
Runoff by SCS TR-2	0.00 hrs, dt=0.05 hrs, 301 points 0 method, UH=SCS, Weighted-CN is method - Pond routing by Stor-Ind method
Subcatchment 1.1S: (new Subcat) Flow Length=130'	Runoff Area=4,731 sf 37.16% Impervious Runoff Depth>3.86" Slope=0.0750 '/' Tc=6.0 min CN=74 Runoff=0.52 cfs 0.035 af
Subcatchment 1.2S: (new Subcat)	Runoff Area=41,050 sf 2.04% Impervious Runoff Depth>1.98" w Length=230' Tc=18.1 min CN=55 Runoff=1.58 cfs 0.155 af
Subcatchment2.1S: (new Subcat)	Runoff Area=8,038 sf 36.17% Impervious Runoff Depth>3.75" Flow Length=85' Tc=6.0 min CN=73 Runoff=0.85 cfs 0.058 af
	Runoff Area=18,932 sf 16.87% Impervious Runoff Depth>2.45" 90' Tc=11.9 min UI Adjusted CN=60 Runoff=1.08 cfs 0.089 af
Subcatchment2.3S: (new Subcat)	Runoff Area=3,654 sf 31.53% Impervious Runoff Depth>3.55" Flow Length=65' Tc=6.0 min CN=71 Runoff=0.37 cfs 0.025 af
Subcatchment2.4S: (new Subcat) Flow Length=1	Runoff Area=22,171 sf 6.02% Impervious Runoff Depth>1.72" 60' Tc=10.4 min UI Adjusted CN=52 Runoff=0.88 cfs 0.073 af
	g. Flow Depth=0.22' Max Vel=5.67 fps Inflow=1.09 cfs 0.102 af 00.0' S=0.1020 '/' Capacity=6.31 cfs Outflow=1.08 cfs 0.102 af
	g. Flow Depth=0.23' Max Vel=5.78 fps Inflow=1.19 cfs 0.137 af =1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=1.19 cfs 0.137 af
Reach 3R: Roadside ditch Av n=0.022 L=16	g. Flow Depth=0.32' Max Vel=2.52 fps Inflow=0.85 cfs 0.058 af 60.0' S=0.0125 '/' Capacity=2.21 cfs Outflow=0.81 cfs 0.058 af
	g. Flow Depth=0.21' Max Vel=4.34 fps Inflow=0.80 cfs 0.058 af 35.0' S=0.0624 '/' Capacity=4.93 cfs Outflow=0.75 cfs 0.058 af
	g. Flow Depth=0.29' Max Vel=6.73 fps Inflow=1.99 cfs 0.206 af =1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=1.99 cfs 0.206 af
Pond 1P: G.U.S.F. #1 Discarded=0.02 cfs	Peak Elev=50.43' Storage=2,062 cf Inflow=1.58 cfs 0.155 af 0.012 af Primary=1.09 cfs 0.102 af Outflow=1.11 cfs 0.114 af
Pond 3P: G.U.S.F. #2 Discarded=0.01 cfs	Peak Elev=47.49' Storage=1,129 cf Inflow=1.08 cfs 0.089 af 0.008 af Primary=0.80 cfs 0.058 af Outflow=0.81 cfs 0.066 af
Pond 4P: G.U.S.F. #3 Discarded=0.00 cfs	Peak Elev=48.01' Storage=198 cf Inflow=0.37 cfs 0.025 af 0.003 af Primary=0.35 cfs 0.018 af Outflow=0.36 cfs 0.021 af

Total Runoff Area = 2.263 acRunoff Volume = 0.435 afAverage Runoff Depth = 2.30"88.66% Pervious = 2.006 ac11.34% Impervious = 0.257 ac

# Summary for Subcatchment 1.1S: (new Subcat)

Runoff = 0.52 cfs @ 12.09 hrs, Volume= 0.035 af, Depth> 3.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

_	A	rea (sf)	CN I	Description		
		1,758	98 I	Paved parki	ng, HSG B	
		2,373	61 3	>75% Grass	s cover, Go	od, HSG B
-		600	55 \	Woods, Goo	od, HSG B	
		4,731	74	Weighted A	verage	
		2,973	6	52.84% Per	vious Area	
		1,758	3	37.16% Imp	ervious Are	a
	Tc	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	0.5	130	0.0750	4.11		Shallow Concentrated Flow,
-		1				Grassed Waterway Kv= 15.0 fps
	0.5	130	Total,	Increased to	o minimum	Tc = 6.0 min

#### Summary for Subcatchment 1.2S: (new Subcat)

Runoff = 1.58 cfs @ 12.27 hrs, Volume= 0.155 af, Depth> 1.98"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

A	rea (sf)	CN E	Description		
	138	98 F	Roofs, HSG	B	
	214	98 L	Inconnecte	ed roofs, HS	SG B
	486	98 F	aved park	ing, HSG B	
	3,131	61 >	75% Gras	s cover, Go	ood, HSG B
	4,903		Brush, Goo		
	32,178	55 V	Voods, Go	od, HSG B	
	41,050	55 V	Veighted A	verage	
	40,212	9	7.96% Per	vious Area	
	838	2	.04% Impe	ervious Area	a
	214	2	5.54% Un	connected	
Тс	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
15.2	100	0.0400	0.11		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
2.7	100	0.0150	0.61		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.2	30	0.0200	2.12		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
18.1	230	Total			

#### Summary for Subcatchment 2.1S: (new Subcat)

Runoff = 0.85 cfs @ 12.09 hrs, Volume= 0.058 af, Depth> 3.75"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

_	A	rea (sf)	CN [	Description		
		2,907	98 F	aved parki	ng, HSG B	
		3,899	61 >	75% Grass	s cover, Go	ood, HSG B
		1,232	55 V	Voods, Goo	od, HSG B	
		8,038	73 V	Veighted A	verage	
		5,131	e	3.83% Per	vious Area	
		2,907	3	6.17% Imp	ervious Are	ea
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	4.6	50	0.2000	0.18		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.69"
	0.2	35	0.0600	3.67		Shallow Concentrated Flow,
						Grassed Waterway Kv= 15.0 fps
	4.8	85	Total,	ncreased t	o minimum	Tc = 6.0 min

#### Summary for Subcatchment 2.2S: (new Subcat)

Runoff = 1.08 cfs @ 12.17 hrs, Volume= 0.089 af, Depth> 2.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

A	rea (sf)	CN A	Adj Desc	ription		
	626	98	Roof	s, HSG B		
	1,794	98	Unco	onnected ro	ofs, HSG B	
	774	98	Pave	d parking,	HSG B	
	5,940	61	>75%	6 Grass co	ver, Good, HSG B	
	4,945	48	Brus	h, Good, H	SG B	
	4,853	55	Woo	ds, Good, I	HSG B	
	18,932	62	60 Weig	hted Avera	ige, UI Adjusted	
	15,738		83.1	3% Perviou	is Area	
	3,194		16.8	7% Impervi	ous Area	
	1,794		56.1	7% Unconn	nected	
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
11.5	100	0.0800	0.15		Sheet Flow,	
					Woods: Light underbrush n= 0.400	P2= 3.69"
0.4	90	0.0500	3.35		Shallow Concentrated Flow,	
					Grassed Waterway Kv= 15.0 fps	
11.9	190	Total				

#### Summary for Subcatchment 2.3S: (new Subcat)

Runoff = 0.37 cfs @ 12.09 hrs, Volume= 0.025 af, Depth> 3.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

	Area (sf)	CN I	Description		
	1,152	98 I	Paved park	ing, HSG B	3
	1,622	61 3	>75% Gras	s cover, Go	bod, HSG B
	880	55 \	Noods, Go	od, HSG B	
	3,654	71	<b>Neighted A</b>	verage	
	2,502	6	58.47% Per	vious Area	L
	1,152	:	31.53% Imp	pervious Ar	ea
T	c Length	Slope	Velocity	Capacity	Description
(min	) (feet)	(ft/ft)	(ft/sec)	(cfs)	
4.3	3 40	0.1500	0.16		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.1	1 25	0.1800	6.36		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
4.4	4 65	Total,	Increased t	o minimum	1 Tc = 6.0 min

.

# Summary for Subcatchment 2.4S: (new Subcat)

Runoff = 0.88 cfs @ 12.16 hrs, Volume= 0.073 af, Depth> 1.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

A	rea (sf)	CN /	Adj Desc	ription	
	317	98	Roof	s, HSG B	
	872	98	Unco	onnected ro	ofs, HSG B
	145	98		d parking,	
	1,928	61	>75%	6 Grass co	ver, Good, HSG B
	17,752	48	Brus	h, Good, H	SG B
	1,157	55	Woo	ds, Good, H	ISG B
	22,171	53	52 Weig	hted Avera	ige, UI Adjusted
	20,837			3% Perviou	
	1,334		6.029	% Impervio	us Area
	872		65.3	7% Unconn	ected
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
9.3	80	0.0875	0.14		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
1.1	80	0.0625	1.25		Shallow Concentrated Flow,
-					Woodland Kv= 5.0 fps
10.4	160	Total			

4916 postType III 24-hr25-yr storm Rainfall=7.10"Prepared by Altus Engineering, Inc.Printed10/5/2018HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLCPage 36

#### Summary for Reach 1R: (new Reach)

 Inflow Area =
 0.942 ac,
 2.04% Impervious, Inflow Depth >
 1.30" for 25-yr storm event

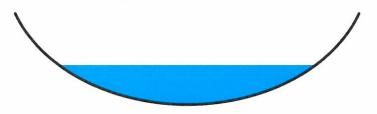
 Inflow =
 1.09 cfs @
 12.52 hrs, Volume=
 0.102 af

 Outflow =
 1.08 cfs @
 12.53 hrs, Volume=
 0.102 af, Atten= 1%, Lag= 1.1 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 5.67 fps, Min. Travel Time= 0.3 min Avg. Velocity = 2.96 fps, Avg. Travel Time= 0.6 min

Peak Storage= 19 cf @ 12.53 hrs Average Depth at Peak Storage= 0.22' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.31 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 100.0' Slope= 0.1020 '/' Inlet Invert= 50.20', Outlet Invert= 40.00'



#### Summary for Reach 2R: POA #2

 Inflow Area =
 1.051 ac,
 5.67% Impervious, Inflow Depth >
 1.57" for 25-yr storm event

 Inflow =
 1.19 cfs @
 12.52 hrs, Volume=
 0.137 af

 Outflow =
 1.19 cfs @
 12.52 hrs, Volume=
 0.137 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 5.78 fps, Min. Travel Time= 0.0 min Avg. Velocity = 2.50 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.52 hrs Average Depth at Peak Storage= 0.23' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



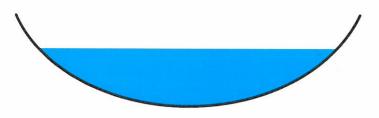
4916 post Type III 24-hr 25-yr storm Rainfall=7.10" Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

#### Summary for Reach 3R: Roadside ditch

0.185 ac, 36.17% Impervious, Inflow Depth > 3.75" for 25-yr storm event Inflow Area = Inflow 0.85 cfs @ 12.09 hrs, Volume= = 0.058 af Outflow = 0.81 cfs @ 12.12 hrs, Volume= 0.058 af, Atten= 5%, Lag= 1.8 min Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 2.52 fps, Min. Travel Time= 1.1 min Avg. Velocity = 0.95 fps, Avg. Travel Time= 2.8 min

Peak Storage= 54 cf @ 12.11 hrs Average Depth at Peak Storage= 0.32' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 2.21 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 160.0' Slope= 0.0125 '/' Inlet Invert= 42.00', Outlet Invert= 40.00'



Summary for Reach 4R: Swale

Inflow Are	a =	0.435 ac, 16.87% Impervious, Inflow Depth > 1.59" for 25-yr storm event
Inflow	=	0.80 cfs @ 12.32 hrs, Volume= 0.058 af
Outflow	=	0.75 cfs @ 12.34 hrs, Volume= 0.058 af, Atten= 6%, Lag= 1.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 4.34 fps, Min. Travel Time= 0.3 min Avg. Velocity = 2.03 fps, Avg. Travel Time= 0.7 min

Peak Storage= 16 cf @ 12.32 hrs Average Depth at Peak Storage= 0.21' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 4.93 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 85.0' Slope= 0.0624 '/' Inlet Invert= 47.30', Outlet Invert= 42.00'

Page 37

4916 postType III 24-hr25-yr storm Rainfall=7.10"Prepared by Altus Engineering, Inc.Printed10/5/2018HydroCAD® 10.00-22 s/n 01222© 2018 HydroCAD Software Solutions LLCPage 38

#### Summary for Reach 5R: POA #1

 Inflow Area =
 1.212 ac, 16.26% Impervious, Inflow Depth > 2.04" for 25-yr storm event

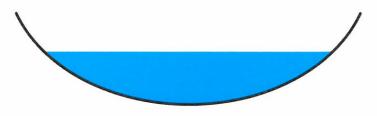
 Inflow =
 1.99 cfs @ 12.14 hrs, Volume=
 0.206 af

 Outflow =
 1.99 cfs @ 12.14 hrs, Volume=
 0.206 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 6.73 fps, Min. Travel Time= 0.0 min Avg. Velocity = 2.81 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.14 hrs Average Depth at Peak Storage= 0.29' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



#### Summary for Pond 1P: G.U.S.F. #1

Inflow Area =	0.942 ac,	2.04% Impervious, Inflow De	epth > 1.98"	for 25-yr storm event
Inflow =	1.58 cfs @	12.27 hrs, Volume=	0.155 af	
Outflow =	1.11 cfs @	12.52 hrs, Volume=	0.114 af, Atte	en= 30%, Lag= 14.5 min
Discarded =	0.02 cfs @	12.52 hrs, Volume=	0.012 af	
Primary =	1.09 cfs @	12.52 hrs, Volume=	0.102 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.43' @ 12.52 hrs Surf.Area= 1,392 sf Storage= 2,062 cf

Plug-Flow detention time= 105.2 min calculated for 0.114 af (74% of inflow) Center-of-Mass det. time= 40.6 min ( 870.3 - 829.8 )

Volume	Invert	Ava	il.Storage	Storage Descrip	tion	
#1	46.17'		2,283 cf	Custom Stage I	Data (Prismatic)Lis	sted below
Elevation (feet)		Area sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
46.17	11110	615	0.0	0	0	
47.17		615	40.0	246	246	
48.67		615	20.0	185	431	
49.00		615	100.0	203	633	
50.00		1,115	100.0	865	1,498	
50.60		1,500	100.0	785	2,283	

4916 post

Type III 24-hr 25-yr storm Rainfall=7.10" Printed 10/5/2018

Page 39

Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Discarded	46.17'	0.600 in/hr Exfiltration over Surface area
#2	Primary	50.20'	4.0' long x 8.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64
			2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74

**Discarded OutFlow** Max=0.02 cfs @ 12.52 hrs HW=50.43' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=1.07 cfs @ 12.52 hrs HW=50.43' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 1.07 cfs @ 1.17 fps)

#### Summary for Pond 3P: G.U.S.F. #2

Inflow Area =	0.435 ac, 16.87% Impervious, Inflow Depth > 2.45" for 25-yr storm event
Inflow =	1.08 cfs @ 12.17 hrs, Volume= 0.089 af
Outflow =	0.81 cfs @ 12.32 hrs, Volume= 0.066 af, Atten= 25%, Lag= 9.0 min
Discarded =	0.01 cfs @ 12.32 hrs, Volume= 0.008 af
Primary =	0.80 cfs @ 12.32 hrs, Volume= 0.058 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 47.49' @ 12.32 hrs Surf.Area= 895 sf Storage= 1,129 cf

Plug-Flow detention time= 99.2 min calculated for 0.066 af (75% of inflow) Center-of-Mass det. time= 36.4 min (852.3 - 815.8)

Volume	Inver	t Ava	il.Storag	ge Storage Description				
#1	44.17	•	1,351 c	of Custom Stage	Custom Stage Data (Prismatic)Listed below			
-								
Elevatio		Surf.Area	Voids					
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)			
44.1	7	500	0.0	0	0			
44.6	67	500	40.0	100	100			
46.1	17	500	20.0	150	250			
46.5	50	500	100.0	165	415			
47.0	00	697	100.0	299	714			
47.7	75	1,000	100.0	636	1,351			
Device	Routing	In	ivert O	utlet Devices				
#1	Discarded	44	.17' <b>0</b> .	600 in/hr Exfiltrat	tion over Surface	e area		
#2	Primary	47	.30' <b>4</b> .	30' 4.0' long x 8.0' breadth Broad-Crested Rectangular				
Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.					00 1.20 1.40 1.60 1.80 2.00			
				.50 3.00 3.50 4.0				
			С	oef. (English) 2.43	3 2.54 2.70 2.69	2.68 2.68 2.66 2.64 2.64		
				.64 2.65 2.65 2.6				

4916 post	Type III 24-hr	25-yr storm Rainfall=7.10"
Prepared by Altus Engineering, Inc.		Printed 10/5/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software S	olutions LLC	Page 40

**Discarded OutFlow** Max=0.01 cfs @ 12.32 hrs HW=47.48' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.75 cfs @ 12.32 hrs HW=47.48' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 0.75 cfs @ 1.04 fps)

# Summary for Pond 4P: G.U.S.F. #3

Inflow Area =	0.084 ac, 31.53% Impervious, Inflow De	epth > 3.55" for 25-yr storm event
Inflow =	0.37 cfs @ 12.09 hrs, Volume=	0.025 af
Outflow =	0.36 cfs @ 12.11 hrs, Volume=	0.021 af, Atten= 3%, Lag= 1.0 min
Discarded =	0.00 cfs @ 12.11 hrs, Volume=	0.003 af
Primary =	0.35 cfs @ 12.11 hrs, Volume=	0.018 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 48.01' @ 12.11 hrs Surf.Area= 304 sf Storage= 198 cf

Plug-Flow detention time= 68.6 min calculated for 0.021 af (84% of inflow) Center-of-Mass det. time= 22.3 min (814.4 - 792.0)

Volume	Inve	ert Ava	il.Stora	ge Storage Desci	Storage Description		
#1	45.1	<b>7</b> '	372	cf Custom Stag	e Data (Prismati	c)Listed below	
Elevatio	<b>N</b> D	Surf.Area	Voide	Inc.Store	Cum Store		
			Voids		Cum.Store		
(fee	et)	(sq-ft)	(%)	) (cubic-feet)	(cubic-feet)		
45.1	7	110	0.0	0	0		
45.6	57	110	40.0	22	22		
47.1	7	110	20.0	33	55		
47.5	50	110	100.0	36	91		
48.0	00	302	100.0	103	194		
48.5		410	100.0		372		
Device	Routing	In	ivert	Outlet Devices			
#1	Discarde	d 45	5.17'	0.600 in/hr Exfiltra	tion over Surfac	e area	
		4.0' long x 8.0' breadth Broad-Crested Rectangular Weir					
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00				
				2.50 3.00 3.50 4.0			
						9 2.68 2.68 2.66 2.64 2.64	
				2.64 2.65 2.65 2.6			
				2.04 2.05 2.05 2.0	2.00 2.00 2.1	0 2.14	

**Discarded OutFlow** Max=0.00 cfs @ 12.11 hrs HW=48.01' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.34 cfs @ 12.11 hrs HW=48.01' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 0.34 cfs @ 0.80 fps)

4916 postType III 24-hr50-yr storm Rainfall=8.50"Prepared by Altus Engineering, Inc.Printed 10/5/2018HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLCPage 41									
Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method , Pond routing by Stor-Ind method									
Subcatchment 1.1S: (new Subcat)Runoff Area=4,731 sf 37.16% ImperviousRunoff Depth>5.04"Flow Length=130'Slope=0.0750 '/' Tc=6.0 minCN=74Runoff=0.67 cfs 0.046 af									
Subcatchment 1.2S: (new Subcat)Runoff Area=41,050 sf 2.04% Impervious Runoff Depth>2.86"Flow Length=230'Tc=18.1 minCN=55Runoff=2.33 cfs 0.224 af									
Subcatchment 2.1S: (new Subcat)Runoff Area=8,038 sf 36.17% ImperviousRunoff Depth>4.92"Flow Length=85'Tc=6.0 minCN=73Runoff=1.11 cfs 0.076 af									
Subcatchment 2.2S: (new Subcat) Flow Length=190' Tc=11.9 min UI Adjusted CN=60 Runoff=1.53 cfs 0.124 af									
Subcatchment 2.3S: (new Subcat)Runoff Area=3,654 sf 31.53% ImperviousRunoff Depth>4.69"Flow Length=65'Tc=6.0 minCN=71Runoff=0.48 cfs 0.033 af									
Subcatchment 2.4S: (new Subcat) Flow Length=160' Tc=10.4 min UI Adjusted CN=52 Runoff=1.35 cfs 0.108 af									
Reach 1R: (new Reach)         Avg. Flow Depth=0.30'         Max Vel=6.89 fps         Inflow=2.12 cfs         0.170 af           n=0.022         L=100.0'         S=0.1020 '/'         Capacity=6.31 cfs         Outflow=2.11 cfs         0.170 af									
Reach 2R: POA #2         Avg. Flow Depth=0.31'         Max Vel=7.05 fps         Inflow=2.35 cfs         0.216 af           n=0.022         L=1.0'         S=0.1000 '/'         Capacity=6.24 cfs         Outflow=2.35 cfs         0.216 af									
Reach 3R: Roadside ditch         Avg. Flow Depth=0.36'         Max Vel=2.72 fps         Inflow=1.11 cfs         0.076 af           n=0.022         L=160.0'         S=0.0125 '/'         Capacity=2.21 cfs         Outflow=1.06 cfs         0.076 af									
Reach 4R: Swale         Avg. Flow Depth=0.29'         Max Vel=5.21 fps         Inflow=1.47 cfs         0.092 af           n=0.022         L=85.0'         S=0.0624 '/'         Capacity=4.93 cfs         Outflow=1.47 cfs         0.092 af									
Reach 5R: POA #1         Avg. Flow Depth=0.39'         Max Vel=8.12 fps         Inflow=3.78 cfs         0.301 af           n=0.022         L=1.0'         S=0.1000 '/'         Capacity=6.24 cfs         Outflow=3.78 cfs         0.301 af									
Pond 1P: G.U.S.F. #1         Peak Elev=50.55' Storage=2,223 cf Inflow=2.33 cfs 0.224 af           Discarded=0.02 cfs 0.013 af Primary=2.12 cfs 0.170 af Outflow=2.14 cfs 0.183 af									
Pond 3P: G.U.S.F. #2         Peak Elev=47.58' Storage=1,207 cf         Inflow=1.53 cfs         0.124 af           Discarded=0.01 cfs         0.009 af         Primary=1.47 cfs         0.092 af         Outflow=1.48 cfs         0.101 af									
Pond 4P: G.U.S.F. #3         Peak Elev=48.03' Storage=206 cf Inflow=0.48 cfs 0.033 af           Discarded=0.00 cfs 0.003 af Primary=0.46 cfs 0.026 af Outflow=0.47 cfs 0.029 af									
Total Runoff Area = 2.263 ac Runoff Volume = 0.610 af Average Runoff Depth = 3.23" 88.66% Pervious = 2.006 ac 11.34% Impervious = 0.257 ac									

4916 post	Type III 24-hr 50-yr storm Rainfall=8.50"
Prepared by Altus Engineering, Inc.	Printed 10/5/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software So	Solutions LLC Page 42

# Summary for Subcatchment 1.1S: (new Subcat)

Runoff = 0.67 cfs @ 12.09 hrs, Volume= 0.046 af, Depth> 5.04"	
---	--

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

	A	rea (sf)	CN [	Description						
		1,758		Paved parking, HSG B						
		2,373	61 >	>75% Grass cover, Good, HSG B						
_		600	55 \	Noods, Go	od, HSG B					
		4,731	74	<b>Veighted A</b>	verage					
		2,973	(	52.84% Per	vious Area					
		1,758	:	37.16% Impervious Area						
	Тс	Length	Slope	•	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			_		
	0.5	130	0.0750	4.11		Shallow Concentrated Flow,				
						Grassed Waterway Kv= 15.0 fps				
	0.5	130	Total,	Increased t	o minimum	Tc = 6.0 min				

# Summary for Subcatchment 1.2S: (new Subcat)

Runoff = 2.33 cfs @ 12.27 hrs, Volume= 0.224 af, Depth> 2.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

A	rea (sf)	CN D	escription					
98 - 1853 c	138	98 F	98 Roofs, HSG B					
	214	98 L	Inconnecte	ed roofs, HS	SG B			
	486			ing, HSG B				
	3,131	61 >	75% Grass	s cover, Go	ood, HSG B			
	4,903	48 E	rush, Goo	d, HSG B				
	32,178	55 V	Voods, Goo	od, HSG B				
	41,050		Veighted A					
	40,212	9	7.96% Per	vious Area				
	838	2	.04% Impe	ervious Area	a			
	214	2	5.54% Uno	connected				
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
15.2	100	0.0400	0.11		Sheet Flow,			
	10102103		627 ST778		Woods: Light underbrush n= 0.400 P2= 3.69"			
2.7	100	0.0150	0.61		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
0.2	30	0.0200	2.12		Shallow Concentrated Flow,			
<u></u>					Grassed Waterway Kv= 15.0 fps			
18.1	230	Total						

# Summary for Subcatchment 2.1S: (new Subcat)

Runoff = 1.11 cfs @ 12.09 hrs, Volume= 0.076 af, Depth> 4.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

A	rea (sf)	CN E	Description			
	2,907	98 F	Paved parking, HSG B			
	3,899	61 >	75% Gras	s cover, Go	bod, HSG B	
	1,232	55 V	Voods, Go	od, HSG B		
	8,038	73 V	Veighted A	verage		
	5,131	6	3.83% Per	vious Area		
	2,907	3	6.17% Imp	pervious Ar	ea	
_	2 22					
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
4.6	50	0.2000	0.18		Sheet Flow,	
					Woods: Light underbrush n= 0.400 P2= 3.69"	
0.2	35	0.0600	3.67		Shallow Concentrated Flow,	
					Grassed Waterway Kv= 15.0 fps	
4.8	85	Total, I	ncreased t	o minimum	Tc = 6.0 min	

#### Summary for Subcatchment 2.2S: (new Subcat)

Runoff = 1.53 cfs @ 12.17 hrs, Volume= 0.124 af, Depth> 3.42"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

<i>F</i>	Area (sf)	CN /	Adj Desc	cription	
	626	98	Roof	s, HSG B	
	1,794	98	Unco	onnected ro	oofs, HSG B
	774	98	Pave	ed parking,	HSG B
	5,940	61	>75%	6 Grass co	ver, Good, HSG B
	4,945	48	Brus	h, Good, H	SG B
-	4,853	55	Woo	ds, Good, H	HSG B
	18,932	62	60 Weig	hted Avera	age, UI Adjusted
	15,738		83.1	3% Perviou	is Area
	3,194		16.8	7% Impervi	ous Area
	1,794		56.1	7% Unconn	nected
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
11.5	100	0.0800	0.15		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.4	90	0.0500	3.35		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
11.9	190	Total			

4916 postType III 24-hr50-yr storm Rainfall=8.50"Prepared by Altus Engineering, Inc.Printed10/5/2018HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLCPage 44

#### Summary for Subcatchment 2.3S: (new Subcat)

Runoff = 0.48 cfs @ 12.09 hrs, Volume= 0.033 af, Depth> 4.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

_	A	rea (sf)	CN [	Description				
		1,152	98 F	Paved parking, HSG B				
		1,622	61 >	75% Gras	s cover, Go	ood, HSG B		
-		880	55 V	Voods, Go	od, HSG B			
		3,654	71 V	Weighted Average				
		2,502	e	8.47% Per	vious Area			
		1,152	3	31.53% Imp	pervious Ar	ea		
	Tc	Length	Slope		Capacity	Description		
-	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	4.3	40	0.1500	0.16		Sheet Flow,		
						Woods: Light underbrush n= 0.400 P2= 3.69"		
	0.1	25	0.1800	6.36		Shallow Concentrated Flow,		
-						Grassed Waterway Kv= 15.0 fps		
	4.4	65	Total,	Increased t	o minimum	Tc = 6.0 min		

#### Summary for Subcatchment 2.4S: (new Subcat)

Runoff = 1.35 cfs @ 12.16 hrs, Volume= 0.108 af, Depth> 2.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

A	rea (sf)	CN A	Adj Desc	ription			
	317	98	Roof	Roofs, HSG B			
	872	98	Unco	nnected ro	oofs, HSG B		
	145	98		d parking,			
	1,928	61	>75%	6 Grass co	ver, Good, HSG B		
	17,752	48	Brus	h, Good, H	SG B		
	1,157	55	Woo	ds, Good, I	HSG B		
	22,171	53			age, UI Adjusted		
	20,837		93.9	3% Perviou	is Area		
	1,334			% Impervio			
	872		65.3	7% Unconr	nected		
	11 - 11 - 11 - 11 - 11 - 11 - 11 - 11				-		
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
9.3	80	0.0875	0.14		Sheet Flow,		
					Woods: Light underbrush n= 0.400 P2= 3.69"		
1.1	80	0.0625	1.25		Shallow Concentrated Flow,		
		101			Woodland Kv= 5.0 fps		
10.4	160	Total					

**4916 post** Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

#### Summary for Reach 1R: (new Reach)

 Inflow Area =
 0.942 ac,
 2.04% Impervious, Inflow Depth >
 2.17" for 50-yr storm event

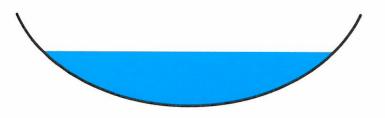
 Inflow =
 2.12 cfs @
 12.37 hrs, Volume=
 0.170 af

 Outflow =
 2.11 cfs @
 12.37 hrs, Volume=
 0.170 af, Atten= 0%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 6.89 fps, Min. Travel Time= 0.2 min Avg. Velocity = 3.34 fps, Avg. Travel Time= 0.5 min

Peak Storage= 31 cf @ 12.37 hrs Average Depth at Peak Storage= 0.30' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.31 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 100.0' Slope= 0.1020 '/' Inlet Invert= 50.20', Outlet Invert= 40.00'



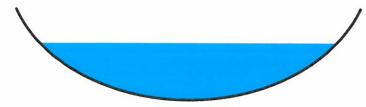
### Summary for Reach 2R: POA #2

Inflow Are	a =	1.051 ac,	5.67% Impervious,	Inflow Depth > 2.4	46" for 50-yr storm event
Inflow	-	2.35 cfs @	12.37 hrs, Volume	= 0.216 af	
Outflow	=	2.35 cfs @	12.37 hrs, Volume:	= 0.216 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 7.05 fps, Min. Travel Time= 0.0 min Avg. Velocity = 2.70 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.37 hrs Average Depth at Peak Storage= 0.31' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



4916 postType III 24-hr50-yr storm Rainfall=8.50"Prepared by Altus Engineering, Inc.Printed 10/5/2018HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLCPage 46

#### Summary for Reach 3R: Roadside ditch

 Inflow Area =
 0.185 ac, 36.17% Impervious, Inflow Depth > 4.92" for 50-yr storm event

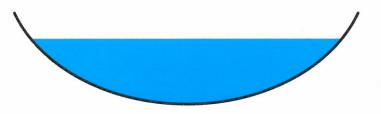
 Inflow =
 1.11 cfs @ 12.09 hrs, Volume=
 0.076 af

 Outflow =
 1.06 cfs @ 12.12 hrs, Volume=
 0.076 af, Atten= 5%, Lag= 1.6 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 2.72 fps, Min. Travel Time= 1.0 min Avg. Velocity = 1.01 fps, Avg. Travel Time= 2.7 min

Peak Storage= 65 cf @ 12.10 hrs Average Depth at Peak Storage= 0.36' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 2.21 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 160.0' Slope= 0.0125 '/' Inlet Invert= 42.00', Outlet Invert= 40.00'



#### Summary for Reach 4R: Swale

 Inflow Area =
 0.435 ac, 16.87% Impervious, Inflow Depth > 2.55" for 50-yr storm event

 Inflow =
 1.47 cfs @ 12.22 hrs, Volume=
 0.092 af

 Outflow =
 1.47 cfs @ 12.22 hrs, Volume=
 0.092 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 5.21 fps, Min. Travel Time= 0.3 min Avg. Velocity = 2.28 fps, Avg. Travel Time= 0.6 min

Peak Storage= 25 cf @ 12.22 hrs Average Depth at Peak Storage= 0.29' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 4.93 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 85.0' Slope= 0.0624 '/' Inlet Invert= 47.30', Outlet Invert= 42.00'

4916 post Type III 24-hr 50-yr storm Rainfall=8.50" Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

#### Summary for Reach 5R: POA #1

Printed 10/5/2018

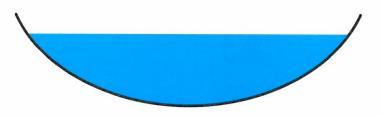
Page 47

Inflow Area = 1.212 ac, 16.26% Impervious, Inflow Depth > 2.98" for 50-yr storm event Inflow = 3.78 cfs @ 12.20 hrs, Volume= 0.301 af Outflow = 3.78 cfs @ 12.20 hrs, Volume= 0.301 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 8.12 fps, Min. Travel Time= 0.0 min Avg. Velocity = 3.01 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.20 hrs Average Depth at Peak Storage= 0.39' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



#### Summary for Pond 1P: G.U.S.F. #1

Inflow Area =	0.942 ac,	2.04% Impervious, Inflow D	epth > 2.86"	for 50-yr storm event
Inflow =	2.33 cfs @	12.27 hrs, Volume=	0.224 af	
Outflow =	2.14 cfs @	12.37 hrs, Volume=	0.183 af, Atte	en= 8%, Lag= 6.1 min
Discarded =	0.02 cfs @	12.37 hrs, Volume=	0.013 af	
Primary =	2.12 cfs @	12.37 hrs, Volume=	0.170 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.55' @ 12.37 hrs Surf.Area= 1,470 sf Storage= 2,223 cf

Plug-Flow detention time= 76.5 min calculated for 0.182 af (81% of inflow) Center-of-Mass det. time= 27.1 min (848.7 - 821.6)

Volume	Invert Av	ail.Storage	Storage Descrip	tion	
#1	46.17'	2,283 cf	Custom Stage	Data (Prismatic)Listed b	elow
Elevation (feet)	Surf.Area (sq-ft)		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
46.17 47.17	615 615		0 246	0 246	
48.67	615	20.0	185	431	
49.00 50.00	615 1,115	100.0	203 865	633 1,498	
50.60	1,500	100.0	785	2,283	

4916 post Type III 24-hr 50-yr storm Rainfall=8.50" Printed 10/5/2018 Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Discarded	46.17'	0.600 in/hr Exfiltration over Surface area
#2	Primary	50.20'	4.0' long x 8.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64
			2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74

Page 48

**Discarded OutFlow** Max=0.02 cfs @ 12.37 hrs HW=50.55' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=2.07 cfs @ 12.37 hrs HW=50.55' (Free Discharge) —2=Broad-Crested Rectangular Weir (Weir Controls 2.07 cfs @ 1.48 fps)

#### Summary for Pond 3P: G.U.S.F. #2

Inflow Area =	0.435 ac, 16.87% Impervious, Inflow D	Depth > 3.42" for 50-yr storm event
Inflow =	1.53 cfs @ 12.17 hrs, Volume=	0.124 af
Outflow =	1.48 cfs @ 12.22 hrs, Volume=	0.101 af, Atten= 3%, Lag= 3.1 min
Discarded =	0.01 cfs @ 12.22 hrs, Volume=	0.009 af
Primary =	1.47 cfs @ 12.22 hrs, Volume=	0.092 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 47.58' @ 12.22 hrs Surf.Area= 932 sf Storage= 1,207 cf

Plug-Flow detention time= 75.8 min calculated for 0.101 af (82% of inflow) Center-of-Mass det. time= 25.8 min (834.2 - 808.4)

Volume	Inve	ert Ava	il.Stora	ge Storage Descr	iption	
#1	44.1	7'	1,351	cf Custom Stag	e Data (Prismatic	:)Listed below
Elevatio (fee 44.1 44.6 46.1 46.5 47.0	et) 17 57 17 50	Surf.Area (sq-ft) 500 500 500 500 697	Voids (%) 0.0 40.0 20.0 100.0 100.0	lnc.Store (cubic-feet) 0 100 150 165	Cum.Store (cubic-feet) 0 100 250 415 714	
47.7		1,000	100.0		1,351	
Device	Routing	In	vert (	Outlet Devices		
#1 #2	Discarde Primary		7.30'	Head (feet) 0.20 0 2.50 3.00 3.50 4.0	adth Broad-Cres 40 0.60 0.80 1. 00 4.50 5.00 5.5 3 2.54 2.70 2.69	Sted Rectangular Weir           00         1.20         1.40         1.60         1.80         2.00           0         2.68         2.68         2.66         2.64         2.64

4916 post	Type III 24-hr	50-yr storm Rair	nfall=8.50"
Prepared by Altus Engineering, Inc.		Printed	10/5/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Sol	utions LLC		Page 49

**Discarded OutFlow** Max=0.01 cfs @ 12.22 hrs HW=47.57' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=1.40 cfs @ 12.22 hrs HW=47.57' (Free Discharge) —2=Broad-Crested Rectangular Weir (Weir Controls 1.40 cfs @ 1.29 fps)

#### Summary for Pond 4P: G.U.S.F. #3

Inflow Area =	0.084 ac, 31.53% Impervious, Inflow De	epth > 4.69" for 50-yr storm event
Inflow =	0.48 cfs @ 12.09 hrs, Volume=	0.033 af
Outflow =	0.47 cfs @ 12.11 hrs, Volume=	0.029 af, Atten= 3%, Lag= 1.1 min
Discarded =	0.00 cfs @ 12.11 hrs, Volume=	0.003 af
Primary =	0.46 cfs @ 12.11 hrs, Volume=	0.026 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 48.03' @ 12.11 hrs Surf.Area= 309 sf Storage= 206 cf

Plug-Flow detention time= 57.1 min calculated for 0.029 af (88% of inflow) Center-of-Mass det. time= 19.2 min ( 804.9 - 785.6 )

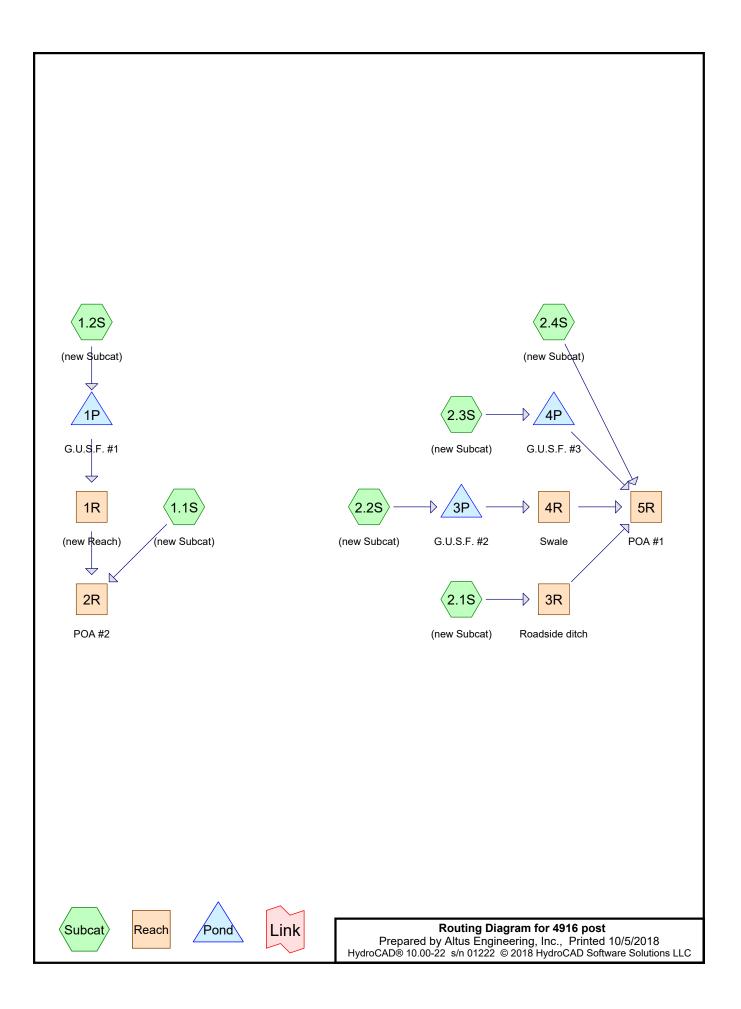
Volume	Inve	ert Ava	il.Storag	Storage Description				
#1	45.1	7'	372 0	of Custom Stag	Custom Stage Data (Prismatic)Listed below			
Elevatio	n	Surf.Area	Voids	Inc.Store	Cum.Store			
(fee		(sq-ft)	(%)	(cubic-feet)	(cubic-feet)			
45.1	17	110	0.0	0	0			
45.6	57	110	40.0	22	22			
47.1	17	110	20.0	33	55			
47.5	50	110	100.0	36	91			
48.0	00	302	100.0	103	194			
48.5	50	410	100.0	178	372			
Device	Routing	In	vert O	utlet Devices				
#1	Discarde	d 45	5.17' <b>O</b> .	600 in/hr Exfiltra	tion over Surface	area		
#2	Primary	47				ted Rectangular Weir		
	, , , , , ,							
				Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50				
						2.68 2.68 2.66 2.64 2.64		
			2.	64 2.65 2.65 2.6	66 2.66 2.68 2.7	0 2.74		

**Discarded OutFlow** Max=0.00 cfs @ 12.11 hrs HW=48.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.45 cfs @ 12.11 hrs HW=48.03' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 0.45 cfs @ 0.87 fps)







# Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.434	61	>75% Grass cover, Good, HSG B (1.1S, 1.2S, 2.1S, 2.2S, 2.3S, 2.4S)
0.634	48	Brush, Good, HSG B (1.2S, 2.2S, 2.4S)
0.166	98	Paved parking, HSG B (1.1S, 1.2S, 2.1S, 2.2S, 2.3S, 2.4S)
0.025	98	Roofs, HSG B (1.2S, 2.2S, 2.4S)
0.066	98	Unconnected roofs, HSG B (1.2S, 2.2S, 2.4S)
0.939	55	Woods, Good, HSG B (1.1S, 1.2S, 2.1S, 2.2S, 2.3S, 2.4S)
2.263	59	TOTAL AREA

# Soil Listing (all nodes)

Soil	Subcatchment
Group	Numbers
HSG A	
HSG B	1.1S, 1.2S, 2.1S, 2.2S, 2.3S, 2.4S
HSG C	
HSG D	
Other	
	TOTAL AREA
	Group HSG A HSG B HSG C HSG D

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchmen Numbers
0.000	0.434	0.000	0.000	0.000	0.434	>75% Grass cover, Good	1.1S,
0.000	0.434	0.000	0.000	0.000	0.434	>75% Glass cover, Good	
							1.2S, 2.1S,
							2.2S,
							2.3S,
0.000	0.004	0.000	0.000	0.000	0.004	Druch Cood	2.4S
0.000	0.634	0.000	0.000	0.000	0.634	Brush, Good	1.2S,
							2.2S,
0.000	0.400	0.000	0.000	0.000	0.400		2.4S
0.000	0.166	0.000	0.000	0.000	0.166	Paved parking	1.1S,
							1.2S,
							2.1S,
							2.2S,
							2.3S,
0.000	0.005	0.000	0.000	0.000	0.005		2.4S
0.000	0.025	0.000	0.000	0.000	0.025	Roofs	1.2S,
							2.2S,
							2.4S
0.000	0.066	0.000	0.000	0.000	0.066	Unconnected roofs	1.2S,
							2.2S,
							2.4S
0.000	0.939	0.000	0.000	0.000	0.939	Woods, Good	1.1S,
							1.2S,
							2.1S,
							2.2S,
							2.3S,
							2.4S
0.000	2.263	0.000	0.000	0.000	2.263	TOTAL AREA	

# Ground Covers (all nodes)

4916 post	Type III 24-hr 0.5 Inch storm Rainfall=0.50"
Prepared by Altus Engineering, Inc.	Printed 10/5/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software	e Solutions LLC Page 5

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1.1S: (new Subcat) Flow Length=130	Runoff Area=4,731 sf 37.16% Impervious Runoff Depth=0.00" Slope=0.0750 '/' Tc=6.0 min CN=74 Runoff=0.00 cfs 0.000 af
Subcatchment1.2S: (new Subcat)	Runoff Area=41,050 sf 2.04% Impervious Runoff Depth=0.00" Flow Length=230' Tc=18.1 min CN=55 Runoff=0.00 cfs 0.000 af
Subcatchment2.1S: (new Subcat)	Runoff Area=8,038 sf 36.17% Impervious Runoff Depth=0.00" Flow Length=85' Tc=6.0 min CN=73 Runoff=0.00 cfs 0.000 af
Subcatchment2.2S: (new Subcat) Flow Length	Runoff Area=18,932 sf 16.87% Impervious Runoff Depth=0.00" =190' Tc=11.9 min UI Adjusted CN=60 Runoff=0.00 cfs 0.000 af
Subcatchment2.3S: (new Subcat)	Runoff Area=3,654 sf 31.53% Impervious Runoff Depth=0.00" Flow Length=65' Tc=6.0 min CN=71 Runoff=0.00 cfs 0.000 af
Subcatchment 2.4S: (new Subcat) Flow Length	Runoff Area=22,171 sf   6.02% Impervious   Runoff Depth=0.00" =160'   Tc=10.4 min   UI Adjusted CN=52   Runoff=0.00 cfs   0.000 af
	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af 100.0' S=0.1020 '/' Capacity=6.31 cfs Outflow=0.00 cfs 0.000 af
	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=0.00 cfs 0.000 af
	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af 160.0' S=0.0125 '/' Capacity=2.21 cfs Outflow=0.00 cfs 0.000 af
	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af =85.0' S=0.0624 '/' Capacity=4.93 cfs Outflow=0.00 cfs 0.000 af
	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=0.00 cfs 0.000 af
Pond 1P: G.U.S.F.#1 Discarded=0.00 c	Peak Elev=46.17' Storage=0 cf Inflow=0.00 cfs 0.000 af fs 0.000 af Primary=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af
Pond 3P: G.U.S.F.#2 Discarded=0.00 c	Peak Elev=44.17' Storage=0 cf Inflow=0.00 cfs 0.000 af fs 0.000 af Primary=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af
Pond 4P: G.U.S.F.#3 Discarded=0.00 c	Peak Elev=45.17' Storage=0 cf Inflow=0.00 cfs 0.000 af fs 0.000 af Primary=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af
Total Runoff Area = 2.263 a	ac Runoff Volume = 0.000 af Average Runoff Depth = 0.00"

88.66% Pervious = 2.006 ac 11.34% Impervious = 0.257 ac

# Summary for Subcatchment 1.1S: (new Subcat)

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

 A	rea (sf)	CN [	Description						
	1,758	98 F	Paved parking, HSG B						
	2,373	61 >	75% Gras	s cover, Go	ood, HSG B				
	600	55 V	Voods, Go	od, HSG B					
	4,731	74 V	4 Weighted Average						
	2,973	6	62.84% Pervious Area						
	1,758	3	37.16% Impervious Area						
Тс	Length	Slope	Velocity	Capacity	Description				
 <u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
0.5	130	0.0750	4.11		Shallow Concentrated Flow,				
					Grassed Waterway Kv= 15.0 fps				
0.5	130	Total, I	tal, Increased to minimum Tc = 6.0 min						

# Summary for Subcatchment 1.2S: (new Subcat)

Runoff	=	0.00 cfs @	5.00 hrs, Volume=	0.000 af, Depth= 0.00"
rtanon		0.00 010 @		

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

A	rea (sf)	CN E	Description					
	138	98 F	98 Roofs, HSG B					
	214	98 l						
	486	98 F	Paved park	ing, HSG B	3			
	3,131	61 >	•75% Ġras	s cover, Go	bod, HSG B			
	4,903	48 E	Brush, Goo	d, HSG B				
	32,178	55 V	Voods, Go	od, HSG B				
	41,050	55 V	55 Weighted Average					
	40,212	ç	97.96% Pervious Area					
	838	2	2.04% Impervious Area					
	214	2	25.54% Un	connected				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
15.2	100	0.0400	0.11		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.69"			
2.7	100	0.0150	0.61		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
0.2	30	0.0200	2.12		Shallow Concentrated Flow,			
					Grassed Waterway Kv= 15.0 fps			
18.1	230	Total						

# Summary for Subcatchment 2.1S: (new Subcat)

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

A	rea (sf)	CN E	CN Description					
	2,907	98 F						
	3,899	61 >	>75% Grass cover, Good, HSG B					
	1,232	55 V	Woods, Good, HSG B					
	8,038	73 V	73 Weighted Average					
	5,131	6	63.83% Pervious Area					
	2,907	3	36.17% Impervious Area					
Тс	Length	Slope	Velocity	Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
4.6	50	0.2000	0.18		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.69"			
0.2	35	0.0600	3.67		Shallow Concentrated Flow,			
					Grassed Waterway Kv= 15.0 fps			
4.8	85	Tatal			Tc = 6.0 min			

# Summary for Subcatchment 2.2S: (new Subcat)

Runoff	=	0.00 cfs @	5.00 hrs, Volume=	0.000 af, Depth= 0.00"
--------	---	------------	-------------------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

A	rea (sf)	CN /	Adj Desc	ription	
	626	98	Roof	s, HSG B	
	1,794	98	Unco	onnected ro	oofs, HSG B
	774	98	Pave	d parking,	HSG B
	5,940	61	>75%	6 Grass co	ver, Good, HSG B
	4,945	48	Brus	h, Good, H	SG B
	4,853	55	Woo	ds, Good, I	HSG B
	18,932	62	60 Weig	hted Avera	age, UI Adjusted
	15,738		83.1	3% Perviou	is Area
	3,194		16.8	7% Impervi	ous Area
	1,794		56.1	7% Unconr	nected
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
11.5	100	0.0800	0.15		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.4	90	0.0500	3.35		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
11.9	190	Total			

# Summary for Subcatchment 2.3S: (new Subcat)

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

	Area (sf)	CN [	Description		
	1,152	98 F	Paved park	ing, HSG B	3
	1,622	61 >	>75% Gras	s cover, Go	bod, HSG B
	880	55 \	Noods, Go	od, HSG B	
	3,654	71 \	Neighted A	verage	
	2,502	6	58.47% Pei	vious Area	
	1,152	3	31.53% Imp	pervious Ar	ea
Т	c Length	Slope	,	Capacity	Description
(mii	n) (feet)	(ft/ft)	(ft/sec)	(cfs)	
4	3 40	0.1500	0.16		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.	1 25	0.1800	6.36		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
4	4 65	Total	Increased t	o minimum	1 Tc = 6.0 min

# Summary for Subcatchment 2.4S: (new Subcat)

Runoff	=	0.00 cfs @	5.00 hrs, Volume=	0.000 af, Depth= 0.00"
--------	---	------------	-------------------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 0.5 Inch storm Rainfall=0.50"

A	rea (sf)	CN /	Adj Desc	cription				
	317	98	Roof	Roofs, HSG B				
	872	98	Unco	onnected ro	oofs, HSG B			
	145	98	Pave	ed parking,	HSG B			
	1,928	61	>75%	% Grass co	ver, Good, HSG B			
	17,752	48	Brus	h, Good, H	SG B			
	1,157	55	Woo	ds, Good, I	HSG B			
	22,171	53	52 Weig	hted Avera	age, UI Adjusted			
	20,837		93.9	93.98% Pervious Area				
	1,334		6.029	% Impervio	us Area			
	872		65.3	7% Unconr	nected			
Tc	Length	Slope	Velocity	Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
9.3	80	0.0875	0.14		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.69"			
1.1	80	0.0625	1.25		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
10.4	160	Total						

# Summary for Reach 1R: (new Reach)

 Inflow Area =
 0.942 ac,
 2.04% Impervious, Inflow Depth =
 0.00" for
 0.5 Inch storm event

 Inflow =
 0.00 cfs @
 5.00 hrs, Volume=
 0.000 af

 Outflow =
 0.00 cfs @
 5.00 hrs, Volume=
 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.31 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 100.0' Slope= 0.1020 '/' Inlet Invert= 50.20', Outlet Invert= 40.00'



# Summary for Reach 2R: POA #2

Inflow Area	a =	1.051 ac,	5.67% Impervious, Inflow	Depth = $0.00"$	for 0.5 Inch storm event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0.000 af	
Outflow	=	0.00 cfs @	5.00 hrs, Volume=	0.000 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



# Summary for Reach 3R: Roadside ditch

 Inflow Area =
 0.185 ac, 36.17% Impervious, Inflow Depth =
 0.00" for 0.5 Inch storm event

 Inflow =
 0.00 cfs @
 5.00 hrs, Volume=
 0.000 af

 Outflow =
 0.00 cfs @
 5.00 hrs, Volume=
 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 2.21 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 160.0' Slope= 0.0125 '/' Inlet Invert= 42.00', Outlet Invert= 40.00'



# Summary for Reach 4R: Swale

Inflow Area	a =	0.435 ac, 16	6.87% Impervious,	Inflow Depth =	0.00"	for 0.5 Inch storm event
Inflow	=	0.00 cfs @	5.00 hrs, Volume	= 0.000	af	
Outflow	=	0.00 cfs @	5.00 hrs, Volume	= 0.000	af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 4.93 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 85.0' Slope= 0.0624 '/' Inlet Invert= 47.30', Outlet Invert= 42.00'



# Summary for Reach 5R: POA #1

 Inflow Area =
 1.212 ac, 16.26% Impervious, Inflow Depth =
 0.00" for 0.5 Inch storm event

 Inflow =
 0.00 cfs @
 5.00 hrs, Volume=
 0.000 af

 Outflow =
 0.00 cfs @
 5.00 hrs, Volume=
 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



# Summary for Pond 1P: G.U.S.F. #1

Inflow Area =	0.942 ac,	2.04% Impervious, Inflow De	epth = 0.00"	for 0.5 Inch storm event
Inflow =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	
Outflow =	0.00 cfs @	5.00 hrs, Volume=	0.000 af, Atte	en= 0%, Lag= 0.0 min
Discarded =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	-
Primary =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 46.17' @ 5.00 hrs Surf.Area= 615 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no inflow)

Volume	Invert A	vail.Storage	Storage Descrip	otion	
#1	46.17'	2,283 cf	Custom Stage	Data (Prismatic)Listed below	N
Elevation (feet)	Surf.Are (sq-		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
46.17 47.17 48.67	6	15 40.0 15 20.0	0 246 185	0 246 431	
49.00 50.00 50.60	6 <sup>:</sup> 1,1 <sup>:</sup> 1,5(	100.0	203 865 785	633 1,498 2,283	

4916 post

Type III 24-hr 0.5 Inch storm Rainfall=0.50"Printed 10/5/2018Solutions LLCPage 12

Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Discarded	46.17'	0.600 in/hr Exfiltration over Surface area
#2	Primary	50.20'	4.0' long x 8.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64
			2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74

**Discarded OutFlow** Max=0.00 cfs @ 5.00 hrs HW=46.17' (Free Discharge) **1=Exfiltration** (Passes 0.00 cfs of 0.01 cfs potential flow)

**Primary OutFlow** Max=0.00 cfs @ 5.00 hrs HW=46.17' (Free Discharge) **2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

# Summary for Pond 3P: G.U.S.F. #2

Inflow Area =	0.435 ac, 16	6.87% Impervious, Inflow D	epth = 0.00"	for 0.5 Inch storm event
Inflow =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	
Outflow =	0.00 cfs @	5.00 hrs, Volume=	0.000 af, Atte	en= 0%, Lag= 0.0 min
Discarded =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	-
Primary =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 44.17' @ 5.00 hrs Surf.Area= 500 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no inflow)

Volume	Invert	. Ava	il.Storaç	ge Storage Descri	iption	
#1	44.17'		1,351	cf Custom Stage	e Data (Prismatic	)Listed below
Elevatio (fee 44. 44.6	et) 17 67	urf.Area (sq-ft) 500 500	Voids (%) 0.0 40.0	Inc.Store (cubic-feet) 0 100 150	Cum.Store (cubic-feet) 0 100 250	
46.1 46.5 47.0	50 00	500 500 697	20.0 100.0 100.0	150 165 299	250 415 714	
47.7 <u>Device</u>	Routing	1,000 In	100.0 <u>vert C</u>	636 Dutlet Devices	1,351	
#1 #2	Discarded Primary		7.30' <b>4</b> ⊢ 2 C	lead (feet) 0.20 0. 2.50 3.00 3.50 4.0	adth Broad-Cres 40 0.60 0.80 1.0 0 4.50 5.00 5.50 3 2.54 2.70 2.69	ted Rectangular Weir 00 1.20 1.40 1.60 1.80 2.00 0 2.68 2.68 2.66 2.64 2.64

**Discarded OutFlow** Max=0.00 cfs @ 5.00 hrs HW=44.17' (Free Discharge) **1=Exfiltration** (Passes 0.00 cfs of 0.01 cfs potential flow)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=44.17' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

# Summary for Pond 4P: G.U.S.F. #3

Inflow Area =	0.084 ac, 31	1.53% Impervious, Inflow D	epth = 0.00"	for 0.5 Inch storm event
Inflow =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	
Outflow =	0.00 cfs @	5.00 hrs, Volume=	0.000 af, Atte	en= 0%, Lag= 0.0 min
Discarded =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	
Primary =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 45.17' @ 5.00 hrs Surf.Area= 110 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no inflow)

Volume	Inver	t Ava	il.Storage	Storage Descri	ption	
#1	45.17	71	372 cf	Custom Stage	Data (Prismatic	Listed below
Elevatio (fee 45.1 45.6 47.1 47.5 48.0	on S et) 17 57 17 50 00	Surf.Area (sq-ft) 110 110 110 110 302	Voids (%) 0.0 40.0 20.0 100.0 100.0	Inc.Store (cubic-feet) 0 22 33 36 103	Cum.Store (cubic-feet) 0 22 55 91 194	,
48.5	50	410	100.0	178	372	
Device #1 #2	Routing Discarded Primary	45	5.17' <b>0.6</b> '.90' <b>4.0</b> Hea 2.5 Coo	<b>long x 8.0' brea</b> ad (feet) 0.20 0.4 0 3.00 3.50 4.00 ef. (English) 2.43	40 0.60 0.80 1.0 0 4.50 5.00 5.50	ted Rectangular Weir 00 1.20 1.40 1.60 1.80 2.00 ) 2.68 2.68 2.66 2.64 2.64

**Discarded OutFlow** Max=0.00 cfs @ 5.00 hrs HW=45.17' (Free Discharge) **1=Exfiltration** (Passes 0.00 cfs of 0.00 cfs potential flow)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=45.17' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

4916 post	Type III 24-hr 2-yr storm Rainfall=3.69"
Prepared by Altus Engineering, Inc.	Printed 10/5/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solu	utions LLC Page 14

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1.1S: (new Subcat) Flow Length=130	Runoff Area=4,731 sf 37.16% Impervious Runoff Depth>1.26" Slope=0.0750 '/' Tc=6.0 min CN=74 Runoff=0.17 cfs 0.011 af
Subcatchment1.2S: (new Subcat)	Runoff Area=41,050 sf 2.04% Impervious Runoff Depth>0.35" Tow Length=230' Tc=18.1 min CN=55 Runoff=0.18 cfs 0.028 af
Subcatchment2.1S: (new Subcat)	Runoff Area=8,038 sf 36.17% Impervious Runoff Depth>1.20" Flow Length=85' Tc=6.0 min CN=73 Runoff=0.27 cfs 0.018 af
Subcatchment2.2S: (new Subcat) Flow Length=	Runoff Area=18,932 sf 16.87% Impervious Runoff Depth>0.54" 190' Tc=11.9 min UI Adjusted CN=60 Runoff=0.18 cfs 0.020 af
Subcatchment2.3S: (new Subcat)	Runoff Area=3,654 sf 31.53% Impervious Runoff Depth>1.08" Flow Length=65' Tc=6.0 min CN=71 Runoff=0.11 cfs 0.008 af
Subcatchment2.4S: (new Subcat) Flow Length=	Runoff Area=22,171 sf   6.02% Impervious   Runoff Depth>0.26" 160'   Tc=10.4 min   UI Adjusted CN=52   Runoff=0.06 cfs   0.011 af
	vg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af 100.0' S=0.1020 '/' Capacity=6.31 cfs Outflow=0.00 cfs 0.000 af
	vg. Flow Depth=0.09' Max Vel=3.23 fps Inflow=0.17 cfs 0.011 af .=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=0.17 cfs 0.011 af
	vg. Flow Depth=0.18' Max Vel=1.78 fps Inflow=0.27 cfs 0.018 af I60.0' S=0.0125 '/' Capacity=2.21 cfs Outflow=0.25 cfs 0.018 af
	vg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af =85.0' S=0.0624 '/' Capacity=4.93 cfs Outflow=0.00 cfs 0.000 af
	vg. Flow Depth=0.12' Max Vel=3.76 fps Inflow=0.28 cfs 0.031 af _=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=0.28 cfs 0.031 af
Pond 1P: G.U.S.F. #1 Discarded=0.01 ct	Peak Elev=49.34' Storage=930 cf Inflow=0.18 cfs 0.028 af s 0.006 af Primary=0.00 cfs 0.000 af Outflow=0.01 cfs 0.006 af
Pond 3P: G.U.S.F. #2 Discarded=0.01 ct	Peak Elev=46.84' Storage=621 cf Inflow=0.18 cfs 0.020 af s 0.005 af Primary=0.00 cfs 0.000 af Outflow=0.01 cfs 0.005 af
Pond 4P: G.U.S.F. #3 Discarded=0.00 ct	Peak Elev=47.91' Storage=175 cf Inflow=0.11 cfs 0.008 af s 0.002 af Primary=0.01 cfs 0.001 af Outflow=0.01 cfs 0.004 af
Total Runoff Area = 2.263 a	c Runoff Volume = 0.096 af Average Runoff Depth = 0.51'

88.66% Pervious = 2.006 ac 11.34% Impervious = 0.257 ac

..

# Summary for Subcatchment 1.1S: (new Subcat)

Runoff = 0.17 cfs @ 12.10 hrs, Volume= 0.011 af, Depth> 1.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

A	rea (sf)	CN Description				
	1,758	98 I	Paved parking, HSG B			
	2,373	61 >	>75% Grass cover, Good, HSG B			
	600	55 \	Woods, Good, HSG B			
	4,731	74 \	74 Weighted Average			
	2,973	6	62.84% Pervious Area			
	1,758	3	37.16% Impervious Area			
_						
Tc	Length	Slope		Capacity	Description	
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)		
0.5	130	0.0750	4.11		Shallow Concentrated Flow,	
					Grassed Waterway Kv= 15.0 fps	
0.5	130	Total,	Increased t	o minimum	Tc = 6.0 min	
<u>(min)</u> 0.5	(feet) 130	(ft/ft) 0.0750	(ft/sec) 4.11	(cfs)	Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps	

# Summary for Subcatchment 1.2S: (new Subcat)

Runoff	=	0.18 cfs @	12.43 hrs,	Volume=	0.028 af,Depth> 0.35"
--------	---	------------	------------	---------	-----------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

A	rea (sf)	CN E	<b>Description</b>				
	138	98 Roofs, HSG B					
	214	98 L					
	486	98 F					
	3,131	61 >	61 >75% Grass cover, Good, HSG B				
	4,903	48 E	Brush, Goo	d, HSG B			
	32,178	55 V	55 Woods, Good, HSG B				
	41,050	55 Weighted Average					
	40,212	97.96% Pervious Area					
	838	2.04% Impervious Area					
	214	2	25.54% Unconnected				
Тс	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
15.2	100	0.0400	0.11		Sheet Flow,		
					Woods: Light underbrush n= 0.400 P2= 3.69"		
2.7	100	0.0150	0.61		Shallow Concentrated Flow,		
					Woodland Kv= 5.0 fps		
0.2	30	0.0200	2.12		Shallow Concentrated Flow,		
					Grassed Waterway Kv= 15.0 fps		
18.1	230	Total					

## Summary for Subcatchment 2.1S: (new Subcat)

Runoff = 0.27 cfs @ 12.10 hrs, Volume= 0.018 af, Depth> 1.20"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

A	rea (sf)	CN E	Description		
	2,907	98 F	aved park	ing, HSG B	
	3,899	61 >	75% Gras	s cover, Go	ood, HSG B
	1,232	55 V	Voods, Go	od, HSG B	
	8,038	73 V	Veighted A	verage	
	5,131	6	3.83% Per	vious Area	
	2,907	3	6.17% Imp	pervious Ar	ea
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
4.6	50	0.2000	0.18		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.2	35	0.0600	3.67		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
4.8					

### Summary for Subcatchment 2.2S: (new Subcat)

Runoff	=	0.18 cfs @	12.21 hrs, Vol	ume= 0.	.020 af, Depth>	0.54"
rtanon		0.10 010 @	12.21110, 101	unio 0.	oco al, Dopar	0.01

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

A	rea (sf)	CN A	Adj Desc	ription	
	626	98	Roof	s, HSG B	
	1,794	98	Unco	onnected ro	oofs, HSG B
	774	98	Pave	ed parking,	HSG B
	5,940	61	>75%	6 Grass co	ver, Good, HSG B
	4,945	48	Brus	h, Good, H	SG B
	4,853	55	Woo	ds, Good, I	HSG B
	18,932	62	60 Weig	hted Avera	age, UI Adjusted
	15,738		83.1	3% Perviou	is Area
	3,194		16.8	7% Impervi	ous Area
	1,794		56.1	7% Unconr	nected
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
11.5	100	0.0800	0.15		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.4	90	0.0500	3.35		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
11.9	190	Total			

## Summary for Subcatchment 2.3S: (new Subcat)

Runoff = 0.11 cfs @ 12.10 hrs, Volume= 0.008 af, Depth> 1.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

	Area (sf)	CN E	Description		
	1,152	98 F	aved park	ing, HSG B	5
	1,622	61 >	75% Gras	s cover, Go	bod, HSG B
	880	55 V	Voods, Go	od, HSG B	
	3,654	71 V	Veighted A	verage	
	2,502	6	8.47% Per	vious Area	
	1,152	3	1.53% Imp	ervious Ar	ea
Т	c Length	Slope	Velocity	Capacity	Description
(min	) (feet)	(ft/ft)	(ft/sec)	(cfs)	
4.3	3 40	0.1500	0.16		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.	1 25	0.1800	6.36		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
4.4	4 65	Total, I	ncreased t	o minimum	Tc = 6.0 min

## Summary for Subcatchment 2.4S: (new Subcat)

Runoff	=	0.06 cfs @	12.39 hrs, Volume	e= 0.011 af, Depth> 0.26"
--------	---	------------	-------------------	---------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr storm Rainfall=3.69"

A	rea (sf)	CN A	Adj Desc	cription	
	317	98	Roof	s, HSG B	
	872	98	Unco	onnected ro	oofs, HSG B
	145	98	Pave	ed parking,	HSG B
	1,928	61	>75%	% Grass co	ver, Good, HSG B
	17,752	48	Brus	h, Good, H	SG B
	1,157	55	Woo	ds, Good, I	HSG B
	22,171	53	52 Weig	hted Avera	age, UI Adjusted
	20,837		93.9	8% Perviou	is Area
	1,334		6.02	% Impervio	us Area
	872		65.3	7% Unconr	nected
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
9.3	80	0.0875	0.14		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
1.1	80	0.0625	1.25		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
10.4	160	Total			

### Summary for Reach 1R: (new Reach)

 Inflow Area =
 0.942 ac,
 2.04% Impervious, Inflow Depth =
 0.00" for 2-yr storm event

 Inflow =
 0.00 cfs @
 5.00 hrs, Volume=
 0.000 af

 Outflow =
 0.00 cfs @
 5.00 hrs, Volume=
 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.31 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 100.0' Slope= 0.1020 '/' Inlet Invert= 50.20', Outlet Invert= 40.00'



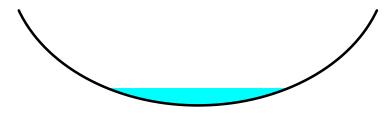
# Summary for Reach 2R: POA #2

Inflow Area	a =	1.051 ac,	5.67% Impervious,	Inflow Depth > 0.	13" for 2-yr storm event
Inflow	=	0.17 cfs @	12.10 hrs, Volume	e= 0.011 af	-
Outflow	=	0.17 cfs @	12.10 hrs, Volume	e= 0.011 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 3.23 fps, Min. Travel Time= 0.0 min Avg. Velocity = 1.33 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.10 hrs Average Depth at Peak Storage= 0.09' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



### Summary for Reach 3R: Roadside ditch

 Inflow Area =
 0.185 ac, 36.17% Impervious, Inflow Depth > 1.20" for 2-yr storm event

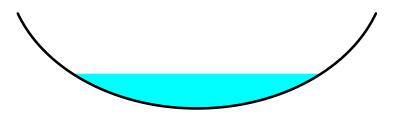
 Inflow =
 0.27 cfs @ 12.10 hrs, Volume=
 0.018 af

 Outflow =
 0.25 cfs @ 12.15 hrs, Volume=
 0.018 af, Atten= 6%, Lag= 2.9 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 1.78 fps, Min. Travel Time= 1.5 min Avg. Velocity = 0.75 fps, Avg. Travel Time= 3.6 min

Peak Storage= 23 cf @ 12.12 hrs Average Depth at Peak Storage= 0.18' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 2.21 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 160.0' Slope= 0.0125 '/' Inlet Invert= 42.00', Outlet Invert= 40.00'



Summary for Reach 4R: Swale

Inflow Area	a =	0.435 ac, 16	6.87% Impervious,	Inflow Depth = 0	0.00" for 2-yr storm event
Inflow	=	0.00 cfs @	5.00 hrs, Volume	e= 0.000 a	f
Outflow	=	0.00 cfs @	5.00 hrs, Volume	e= 0.000 a	f, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 4.93 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 85.0' Slope= 0.0624 '/' Inlet Invert= 47.30', Outlet Invert= 42.00'



# Summary for Reach 5R: POA #1

 Inflow Area =
 1.212 ac, 16.26% Impervious, Inflow Depth > 0.30" for 2-yr storm event

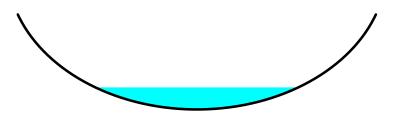
 Inflow =
 0.28 cfs @ 12.16 hrs, Volume=
 0.031 af

 Outflow =
 0.28 cfs @ 12.16 hrs, Volume=
 0.031 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 3.76 fps, Min. Travel Time= 0.0 min Avg. Velocity = 1.87 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.16 hrs Average Depth at Peak Storage= 0.12' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



# Summary for Pond 1P: G.U.S.F. #1

Inflow Area =	0.942 ac,	2.04% Impervious, Inflow De	epth > 0.35" for 2-yr storm event
Inflow =	0.18 cfs @	12.43 hrs, Volume=	0.028 af
Outflow =	0.01 cfs @	20.00 hrs, Volume=	0.006 af, Atten= 94%, Lag= 454.0 min
Discarded =	0.01 cfs @	20.00 hrs, Volume=	0.006 af
Primary =	0.00 cfs @	5.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 49.34' @ 20.00 hrs Surf.Area= 787 sf Storage= 930 cf

Plug-Flow detention time= 232.7 min calculated for 0.006 af (23% of inflow) Center-of-Mass det. time= 101.2 min (977.3 - 876.0)

Volume	Invert	Avail.Storage	Storage Descri	otion	
#1	46.17'	2,283 cf	Custom Stage	Data (Prismatic)List	ted below
Elevation (feet)	Surf.Ar (sq		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
46.17	6	15 0.0	0	0	
47.17	6	15 40.0	246	246	
48.67	6	15 20.0	185	431	
49.00	6	15 100.0	203	633	
50.00	1,1	15 100.0	865	1,498	
50.60	1,5	00 100.0	785	2,283	

4916 post

Type III 24-hr 2-yr storm Rainfall=3.69" Printed 10/5/2018

Page 21

Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Discarded	46.17'	0.600 in/hr Exfiltration over Surface area
#2	Primary	50.20'	4.0' long x 8.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64
			2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74

**Discarded OutFlow** Max=0.01 cfs @ 20.00 hrs HW=49.34' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

**Primary OutFlow** Max=0.00 cfs @ 5.00 hrs HW=46.17' (Free Discharge) **2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

# Summary for Pond 3P: G.U.S.F. #2

Inflow Area =	0.435 ac, 16.87% Impervious, Inflow D	epth > 0.54" for 2-yr storm event
Inflow =	0.18 cfs @ 12.21 hrs, Volume=	0.020 af
Outflow =	0.01 cfs @ 20.00 hrs, Volume=	0.005 af, Atten= 95%, Lag= 467.1 min
Discarded =	0.01 cfs @ 20.00 hrs, Volume=	0.005 af
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 46.84' @ 20.00 hrs Surf.Area= 636 sf Storage= 621 cf

Plug-Flow detention time= 236.6 min calculated for 0.005 af (27% of inflow) Center-of-Mass det. time= 120.0 min (972.4 - 852.3)

Volume	Invert	Ava	il.Storag	e Storage Description			
#1	44.17'		1,351	cf Custom Stage	e Data (Prismatic	)Listed below	
Elevatio (fee 44.1	et)	urf.Area (sq-ft) 500	Voids (%) 0.0	Inc.Store (cubic-feet) 0	Cum.Store (cubic-feet) 0		
44.6	67	500	40.0	100	100		
46.1		500	20.0	150	250		
46.5	50	500	100.0	165	415		
47.0	47.00 697 100.0		299	714			
47.7	75	1,000	100.0	636	1,351		
Device #1 #2	Routing Discarded Primary	44	.17' <b>0</b> . .30' <b>4</b> . H 2. C	lead (feet) 0.20 0. .50 3.00 3.50 4.0	adth Broad-Cres 40 0.60 0.80 1.0 0 4.50 5.00 5.50 3 2.54 2.70 2.69	ted Rectangular Weir 00 1.20 1.40 1.60 1.80 2.00 0 2.68 2.68 2.66 2.64 2.64	

**Discarded OutFlow** Max=0.01 cfs @ 20.00 hrs HW=46.84' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=44.17' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

## Summary for Pond 4P: G.U.S.F. #3

Inflow Area =	0.084 ac, 31.53% Impervious, Inflow E	Depth > 1.08" for 2-yr storm event
Inflow =	0.11 cfs @ 12.10 hrs, Volume=	0.008 af
Outflow =	0.01 cfs @ 13.13 hrs, Volume=	0.004 af, Atten= 89%, Lag= 61.9 min
Discarded =	0.00 cfs @ 13.13 hrs, Volume=	0.002 af
Primary =	0.01 cfs @ 13.13 hrs, Volume=	0.001 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 47.91' @ 13.13 hrs Surf.Area= 266 sf Storage= 175 cf

Plug-Flow detention time= 195.4 min calculated for 0.004 af (49% of inflow) Center-of-Mass det. time= 104.2 min ( 922.9 - 818.7 )

Volume	Invert	Ava	il.Storage	e Storage Description		
#1	45.17'		372 cf	Custom Stage Data (Prismatic)Listed below		sted below
Elevatio (fee 45.1 45.6 47.1 47.5 48.0 48.5	et) 17 57 17 50 00	urf.Area (sq-ft) 110 110 110 110 302 410	Voids (%) 0.0 40.0 20.0 100.0 100.0 100.0	Inc.Store (cubic-feet) 0 22 33 36 103 178	Cum.Store (cubic-feet) 0 22 55 91 194 372	
48.5 <u>Device</u> #1 #2	50 Routing Discarded Primary	<u>In</u> 45	vert Out 5.17' <b>0.6</b> 7.90' <b>4.0</b> Hea 2.5 Coe	tlet Devices <b>00 in/hr Exfiltrati</b> <b>' long x 8.0' brea</b> ad (feet) 0.20 0.4 0 3.00 3.50 4.00 ef. (English) 2.43	ion over Surface an adth Broad-Crested 40 0.60 0.80 1.00 0 4.50 5.00 5.50	d Rectangular Weir 1.20 1.40 1.60 1.80 2.00 .68 2.68 2.66 2.64 2.64

**Discarded OutFlow** Max=0.00 cfs @ 13.13 hrs HW=47.91' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.00 cfs @ 13.13 hrs HW=47.91' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Weir Controls 0.00 cfs @ 0.18 fps)

4916 post	Type III 24-hr	10-yr storm Rainfall=5.60"
Prepared by Altus Engineering, Inc.		Printed 10/5/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Sol	utions LLC	Page 23

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1.1S: (new Subcat) Flow Length=1	Runoff Area=4,731 sf 37.16% Impervious Runoff Depth>2.65" 30' Slope=0.0750 '/' Tc=6.0 min CN=74 Runoff=0.36 cfs 0.024 af
Subcatchment1.2S: (new Subcat)	Runoff Area=41,050 sf 2.04% Impervious Runoff Depth>1.15" Flow Length=230' Tc=18.1 min CN=55 Runoff=0.85 cfs 0.091 af
Subcatchment2.1S: (new Subcat)	Runoff Area=8,038 sf 36.17% Impervious Runoff Depth>2.56" Flow Length=85' Tc=6.0 min CN=73 Runoff=0.58 cfs 0.039 af
Subcatchment2.2S: (new Subcat) Flow Leng	Runoff Area=18,932 sf 16.87% Impervious Runoff Depth>1.51" th=190' Tc=11.9 min UI Adjusted CN=60 Runoff=0.64 cfs 0.055 af
Subcatchment2.3S: (new Subcat)	Runoff Area=3,654 sf 31.53% Impervious Runoff Depth>2.39" Flow Length=65' Tc=6.0 min CN=71 Runoff=0.25 cfs 0.017 af
Subcatchment2.4S: (new Subcat) Flow Leng	Runoff Area=22,171 sf   6.02% Impervious   Runoff Depth>0.96" th=160'   Tc=10.4 min   UI Adjusted CN=52   Runoff=0.43 cfs   0.041 af
Reach 1R: (new Reach) n=0.022	Avg. Flow Depth=0.09' Max Vel=3.27 fps Inflow=0.17 cfs 0.039 af L=100.0' S=0.1020 '/' Capacity=6.31 cfs Outflow=0.17 cfs 0.038 af
Reach 2R: POA #2 n=0.022	Avg. Flow Depth=0.13' Max Vel=4.05 fps Inflow=0.36 cfs 0.062 af L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=0.36 cfs 0.062 af
Reach 3R: Roadside ditch	Avg. Flow Depth=0.26' Max Vel=2.25 fps Inflow=0.58 cfs 0.039 af L=160.0' S=0.0125 '/' Capacity=2.21 cfs Outflow=0.55 cfs 0.039 af
Reach 4R: Swale n=0.022	Avg. Flow Depth=0.10' Max Vel=2.77 fps Inflow=0.18 cfs 0.024 af L=85.0' S=0.0624 '/' Capacity=4.93 cfs Outflow=0.17 cfs 0.024 af
Reach 5R: POA #1 n=0.022	Avg. Flow Depth=0.23' Max Vel=5.84 fps Inflow=1.22 cfs 0.115 af L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=1.22 cfs 0.115 af
Pond 1P: G.U.S.F.#1 Discarded=0.02	Peak Elev=50.26' Storage=1,845 cf Inflow=0.85 cfs 0.091 af 2 cfs 0.011 af Primary=0.17 cfs 0.039 af Outflow=0.19 cfs 0.050 af
Pond 3P: G.U.S.F.#2 Discarded=0.07	Peak Elev=47.37' Storage=1,027 cf Inflow=0.64 cfs 0.055 af 1 cfs 0.008 af Primary=0.18 cfs 0.024 af Outflow=0.19 cfs 0.032 af
Pond 4P: G.U.S.F.#3 Discarded=0.00	Peak Elev=47.99' Storage=192 cf Inflow=0.25 cfs 0.017 af cfs 0.003 af Primary=0.25 cfs 0.010 af Outflow=0.26 cfs 0.013 af
Total Runoff Area = 2.263	3 ac Runoff Volume = 0.266 af Average Runoff Depth = 1.41"

88.66% Pervious = 2.006 ac 11.34% Impervious = 0.257 ac

## Summary for Subcatchment 1.1S: (new Subcat)

Runoff = 0.36 cfs @ 12.09 hrs, Volume= 0.024 af, Depth> 2.65"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

^	rea (sf)	CN L	CN Description						
	1,758	98 F							
	2,373	61 >	>75% Grass cover, Good, HSG B						
	600	55 V	Woods, Good, HSG B						
	4,731	74 V	5 5						
	2,973	6	62.84% Pervious Area						
	1,758	3	37.16% Impervious Area						
_									
Tc	Length	Slope	Velocity	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
0.5	130	0.0750	4.11		Shallow Concentrated Flow,				
					Grassed Waterway Kv= 15.0 fps				
0.5	130	Total, Increased to minimum Tc = 6.0 min							
<u>(min)</u> 0.5	(feet) 130	(ft/ft) 0.0750	(ft/sec) 4.11	(cfs)	Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps				

### Summary for Subcatchment 1.2S: (new Subcat)

Runoff =	0.85 cfs @	12.29 hrs, Volume=	e 0.091 af, Depth> 1.1	5"
----------	------------	--------------------	------------------------	----

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

A	rea (sf)	CN E	Description					
	138	98 F	Roofs, HSG	ЪВ				
	214	98 l						
	486	98 F	Paved parking, HSG B					
	3,131	61 >	>75% Grass cover, Good, HSG B					
	4,903	48 E	Brush, Goo	d, HSG B				
	32,178	55 V	Voods, Go	od, HSG B				
	41,050	55 V	55 Weighted Average					
	40,212	ç	97.96% Pei	vious Area				
	838	2	2.04% Impe	ervious Are	а			
	214	2	25.54% Un	connected				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
15.2	100	0.0400	0.11		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.69"			
2.7	100	0.0150	0.61		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
0.2	30	0.0200	2.12		Shallow Concentrated Flow,			
					Grassed Waterway Kv= 15.0 fps			
18.1	230	Total						

## Summary for Subcatchment 2.1S: (new Subcat)

Runoff = 0.58 cfs @ 12.09 hrs, Volume= 0.039 af, Depth> 2.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

	Ar	ea (sf)	CN E	CN Description					
		2,907	98 F	aved park	ing, HSG B				
		3,899	61 >	>75% Grass cover, Good, HSG B					
		1,232	55 V	Woods, Good, HSG B					
		8,038	73 V	- <u> </u>					
		5,131	6	3.83% Per	vious Area				
		2,907	3	6.17% Imp	pervious Are	ea			
	Тс	Length	Slope	Velocity	Capacity	Description			
(m	in)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
4	1.6	50	0.2000	0.18		Sheet Flow,			
						Woods: Light underbrush n= 0.400 P2= 3.69"			
C	).2	35	0.0600	3.67		Shallow Concentrated Flow,			
						Grassed Waterway Kv= 15.0 fps			
4	1.8	85	Total, I	ncreased t	o minimum	Tc = 6.0 min			

## Summary for Subcatchment 2.2S: (new Subcat)

Runoff =	0.64 cfs @	12.18 hrs,	Volume=	0.055 af, Depth> 1.51"
----------	------------	------------	---------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

A	rea (sf)	CN A	Adj Desc	Description				
	626	98	Roof	s, HSG B				
	1,794	98	Unco	onnected ro	ofs, HSG B			
	774	98	Pave	ed parking,	HSG B			
	5,940	61	>75%	>75% Grass cover, Good, HSG B				
	4,945	48	Brus	Brush, Good, HSG B				
	4,853	55	Woo	Woods, Good, HSG B				
	18,932	62	60 Weig	hted Avera	age, UI Adjusted			
	15,738		83.13% Pervious Area					
	3,194		16.8	16.87% Impervious Area				
	1,794		56.1	56.17% Unconnected				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
11.5	100	0.0800	0.15		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.69"			
0.4	90	0.0500	3.35		Shallow Concentrated Flow,			
					Grassed Waterway Kv= 15.0 fps			
11.9	190	Total						

## Summary for Subcatchment 2.3S: (new Subcat)

Runoff = 0.25 cfs @ 12.10 hrs, Volume= 0.017 af, Depth> 2.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

	Area (sf)	CN [	CN Description					
	1,152	98 F	Paved park	ing, HSG B	3			
	1,622	61 >	>75% Grass cover, Good, HSG B					
	880	55 \	Woods, Good, HSG B					
	3,654	71 \	5 5					
	2,502	6	68.47% Pervious Area					
	1,152	3	31.53% Imp	pervious Ar	ea			
Т	c Length	Slope	,	Capacity	Description			
(mii	n) (feet)	(ft/ft)	(ft/sec)	(cfs)				
4	3 40	0.1500	0.16		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.69"			
0.	1 25	0.1800	6.36		Shallow Concentrated Flow,			
					Grassed Waterway Kv= 15.0 fps			
4	4 65	Total	Increased t	o minimum	1 Tc = 6.0 min			

### Summary for Subcatchment 2.4S: (new Subcat)

Runoff	=	0.43 cfs @	12.18 hrs,	Volume=	0.041 af, Depth> 0.96"
--------	---	------------	------------	---------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr storm Rainfall=5.60"

A	rea (sf)	CN A	Adj Desc	cription		
	317	98	Roof	Roofs, HSG B		
	872	98	Unco	onnected ro	oofs, HSG B	
	145	98	Pave	ed parking,	HSG B	
	1,928	61	>75%	6 Grass co	ver, Good, HSG B	
	17,752	48	Brus	h, Good, H	SG B	
	1,157	55	Woo	ds, Good, I	HSG B	
	22,171	53	52 Weig	hted Avera	age, UI Adjusted	
	20,837		93.9	8% Perviou	is Area	
	1,334		6.02	% Impervio	us Area	
	872		65.3	7% Unconr	nected	
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
9.3	80	0.0875	0.14		Sheet Flow,	
					Woods: Light underbrush n= 0.400 P2= 3.69"	
1.1	80	0.0625	1.25		Shallow Concentrated Flow,	
					Woodland Kv= 5.0 fps	
10.4	160	Total				

### Summary for Reach 1R: (new Reach)

 Inflow Area =
 0.942 ac,
 2.04% Impervious, Inflow Depth >
 0.49" for 10-yr storm event

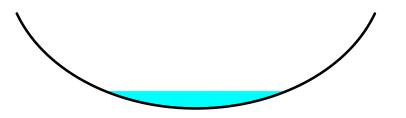
 Inflow =
 0.17 cfs @
 13.18 hrs, Volume=
 0.039 af

 Outflow =
 0.17 cfs @
 13.20 hrs, Volume=
 0.038 af, Atten= 0%, Lag= 1.3 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 3.27 fps, Min. Travel Time= 0.5 min Avg. Velocity = 2.35 fps, Avg. Travel Time= 0.7 min

Peak Storage= 5 cf @ 13.20 hrs Average Depth at Peak Storage= 0.09' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.31 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 100.0' Slope= 0.1020 '/' Inlet Invert= 50.20', Outlet Invert= 40.00'



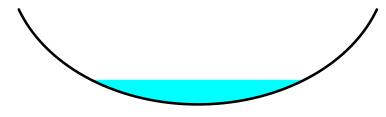
# Summary for Reach 2R: POA #2

Inflow Area	ı =	1.051 ac,	5.67% Impervious,	Inflow Depth >	0.71"	for 10-yr storm event
Inflow	=	0.36 cfs @	12.09 hrs, Volume	= 0.062	af	-
Outflow	=	0.36 cfs @	12.09 hrs, Volume	= 0.062	af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 4.05 fps, Min. Travel Time= 0.0 min Avg. Velocity = 2.17 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.09 hrs Average Depth at Peak Storage= 0.13' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



#### Summary for Reach 3R: Roadside ditch

 Inflow Area =
 0.185 ac, 36.17% Impervious, Inflow Depth > 2.56" for 10-yr storm event

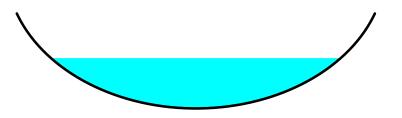
 Inflow =
 0.58 cfs @ 12.09 hrs, Volume=
 0.039 af

 Outflow =
 0.55 cfs @ 12.13 hrs, Volume=
 0.039 af, Atten= 6%, Lag= 2.2 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 2.25 fps, Min. Travel Time= 1.2 min Avg. Velocity = 0.88 fps, Avg. Travel Time= 3.0 min

Peak Storage= 41 cf @ 12.11 hrs Average Depth at Peak Storage= 0.26' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 2.21 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 160.0' Slope= 0.0125 '/' Inlet Invert= 42.00', Outlet Invert= 40.00'



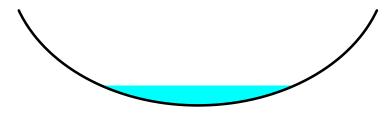
# Summary for Reach 4R: Swale

Inflow Area =	0.435 ac, 16.87% Impervious, In	flow Depth > 0.67" for	10-yr storm event
Inflow =	0.18 cfs @ 12.67 hrs, Volume=	0.024 af	•
Outflow =	0.17 cfs @ 12.69 hrs, Volume=	0.024 af, Atten=	3%, Lag= 1.2 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 2.77 fps, Min. Travel Time= 0.5 min Avg. Velocity = 1.65 fps, Avg. Travel Time= 0.9 min

Peak Storage= 5 cf @ 12.67 hrs Average Depth at Peak Storage= 0.10' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 4.93 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 85.0' Slope= 0.0624 '/' Inlet Invert= 47.30', Outlet Invert= 42.00'



# Summary for Reach 5R: POA #1

 Inflow Area =
 1.212 ac, 16.26% Impervious, Inflow Depth >
 1.13" for 10-yr storm event

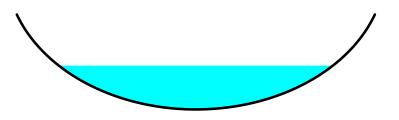
 Inflow =
 1.22 cfs @
 12.15 hrs, Volume=
 0.115 af

 Outflow =
 1.22 cfs @
 12.15 hrs, Volume=
 0.115 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 5.84 fps, Min. Travel Time= 0.0 min Avg. Velocity = 2.52 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.15 hrs Average Depth at Peak Storage= 0.23' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



# Summary for Pond 1P: G.U.S.F. #1

Inflow Area =	0.942 ac,	2.04% Impervious, Inflow De	epth > 1.15" for 10-yr storm event
Inflow =	0.85 cfs @	12.29 hrs, Volume=	0.091 af
Outflow =	0.19 cfs @	13.18 hrs, Volume=	0.050 af, Atten= 78%, Lag= 53.2 min
Discarded =	0.02 cfs @	13.18 hrs, Volume=	0.011 af
Primary =	0.17 cfs @	13.18 hrs, Volume=	0.039 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.26' @ 13.18 hrs Surf.Area= 1,285 sf Storage= 1,845 cf

Plug-Flow detention time= 175.7 min calculated for 0.050 af (55% of inflow) Center-of-Mass det. time= 85.0 min (927.3 - 842.3)

Volume	Invert A	vail.Storage	Storage Descrip	otion	
#1	46.17'	2,283 cf	Custom Stage	Data (Prismatic)	Listed below
Elevation (feet)	Surf.Are (sq-f		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
46.17	61	5 0.0	0	0	
47.17	61	5 40.0	246	246	
48.67	61	5 20.0	185	431	
49.00	61	5 100.0	203	633	
50.00	1,11	5 100.0	865	1,498	
50.60	1,50	0 100.0	785	2,283	

4916 post

Type III 24-hr 10-yr storm Rainfall=5.60" Printed 10/5/2018

Page 30

Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Discarded		0.600 in/hr Exfiltration over Surface area
#2	Primary	50.20'	4.0' long x 8.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64
			2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74

**Discarded OutFlow** Max=0.02 cfs @ 13.18 hrs HW=50.26' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

**Primary OutFlow** Max=0.16 cfs @ 13.18 hrs HW=50.26' (Free Discharge) **2=Broad-Crested Rectangular Weir** (Weir Controls 0.16 cfs @ 0.62 fps)

# Summary for Pond 3P: G.U.S.F. #2

Inflow Area =	0.435 ac, 16.87% Impervious, Inflow De	epth > 1.51" for 10-yr storm event
Inflow =	0.64 cfs @ 12.18 hrs, Volume=	0.055 af
Outflow =	0.19 cfs @ 12.67 hrs, Volume=	0.032 af, Atten= 70%, Lag= 29.0 min
Discarded =	0.01 cfs @12.67 hrs, Volume=	0.008 af
Primary =	0.18 cfs @ 12.67 hrs, Volume=	0.024 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 47.37' @ 12.67 hrs Surf.Area= 846 sf Storage= 1,027 cf

Plug-Flow detention time= 154.1 min calculated for 0.032 af (59% of inflow) Center-of-Mass det. time= 70.0 min ( 896.6 - 826.7 )

Volume	Invert	. Ava	il.Storaç	ge Storage Descri	iption	
#1	44.17'		1,351	cf Custom Stage	e Data (Prismatic	)Listed below
Elevatio (fee 44. 44.6	et) 17 67	urf.Area (sq-ft) 500 500	Voids (%) 0.0 40.0	Inc.Store (cubic-feet) 0 100 150	Cum.Store (cubic-feet) 0 100 250	
46.1 46.5 47.0	50 00	500 500 697	20.0 100.0 100.0	150 165 299	250 415 714	
47.7 <u>Device</u>	Routing	1,000 In	100.0 <u>vert C</u>	636 Dutlet Devices	1,351	
#1 #2	Discarded Primary		7.30' <b>4</b> ⊢ 2 C	lead (feet) 0.20 0. 2.50 3.00 3.50 4.0	adth Broad-Cres 40 0.60 0.80 1.0 0 4.50 5.00 5.50 3 2.54 2.70 2.69	ted Rectangular Weir 00 1.20 1.40 1.60 1.80 2.00 0 2.68 2.68 2.66 2.64 2.64

**Discarded OutFlow** Max=0.01 cfs @ 12.67 hrs HW=47.37' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

**Primary OutFlow** Max=0.17 cfs @ 12.67 hrs HW=47.37' (Free Discharge) **2=Broad-Crested Rectangular Weir** (Weir Controls 0.17 cfs @ 0.63 fps)

## Summary for Pond 4P: G.U.S.F. #3

Inflow Area =	0.084 ac, 31.53% Impervious, Inflow De	epth > 2.39" for 10-yr storm event
Inflow =	0.25 cfs @ 12.10 hrs, Volume=	0.017 af
Outflow =	0.26 cfs @ 12.16 hrs, Volume=	0.013 af, Atten= 0%, Lag= 3.6 min
Discarded =	0.00 cfs @ 12.16 hrs, Volume=	0.003 af
Primary =	0.25 cfs @ 12.16 hrs, Volume=	0.010 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 47.99' @ 12.16 hrs Surf.Area= 297 sf Storage= 192 cf

Plug-Flow detention time= 90.7 min calculated for 0.013 af (76% of inflow) Center-of-Mass det. time= 31.3 min ( 832.3 - 801.0 )

Volume	Invert	: Ava	il.Storage	e Storage Description		
#1	45.17'	I	372 c	Custom Stage	e Data (Prismatic)L	isted below
Elevatio (fee 45.0 45.0 47.1 47.5 48.0 48.5	et) 17 57 17 50 00	urf.Area (sq-ft) 110 110 110 110 302 410	Voids (%) 0.0 40.0 20.0 100.0 100.0 100.0	Inc.Store (cubic-feet) 0 22 33 36 103 178	Cum.Store (cubic-feet) 0 22 55 91 194 372	
48.5 <u>Device</u> #1 #2	50 Routing Discarded Primary	<u>In</u> 45	vert Ou 5.17' 0.6 7.90' 4.0 He 2.5 Co	tlet Devices <b>00 in/hr Exfiltrat</b> <b>' long x 8.0' bre</b> ad (feet) 0.20 0. 0 3.00 3.50 4.0 ef. (English) 2.43	ion over Surface a adth Broad-Creste 40 0.60 0.80 1.00 0 4.50 5.00 5.50	<b>d Rectangular Weir</b> 1.20 1.40 1.60 1.80 2.00 2.68 2.68 2.66 2.64 2.64

**Discarded OutFlow** Max=0.00 cfs @ 12.16 hrs HW=47.98' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.24 cfs @ 12.16 hrs HW=47.98' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Weir Controls 0.24 cfs @ 0.70 fps)

4916 post	Type III 24-hr 25-yr storm Rainfall=7.10"
Prepared by Altus Engineering, Inc.	Printed 10/5/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Sol	lutions LLC Page 32

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1.1S: (new Subcat) Flow Length=130	Runoff Area=4,731 sf 37.16% Impervious Runoff Depth>3.86" ' Slope=0.0750 '/' Tc=6.0 min CN=74 Runoff=0.52 cfs 0.035 af
Subcatchment1.2S: (new Subcat)	Runoff Area=41,050 sf 2.04% Impervious Runoff Depth>1.98" Flow Length=230' Tc=18.1 min CN=55 Runoff=1.58 cfs 0.155 af
Subcatchment2.1S: (new Subcat)	Runoff Area=8,038 sf 36.17% Impervious Runoff Depth>3.75" Flow Length=85' Tc=6.0 min CN=73 Runoff=0.85 cfs 0.058 af
Subcatchment2.2S: (new Subcat) Flow Length=	Runoff Area=18,932 sf 16.87% Impervious Runoff Depth>2.45" -190' Tc=11.9 min UI Adjusted CN=60 Runoff=1.08 cfs 0.089 af
Subcatchment2.3S: (new Subcat)	Runoff Area=3,654 sf 31.53% Impervious Runoff Depth>3.55" Flow Length=65' Tc=6.0 min CN=71 Runoff=0.37 cfs 0.025 af
Subcatchment2.4S: (new Subcat) Flow Length=	Runoff Area=22,171 sf   6.02% Impervious   Runoff Depth>1.72" 160'   Tc=10.4 min   UI Adjusted CN=52   Runoff=0.88 cfs   0.073 af
	Avg. Flow Depth=0.22' Max Vel=5.67 fps Inflow=1.09 cfs 0.102 af 100.0' S=0.1020 '/' Capacity=6.31 cfs Outflow=1.08 cfs 0.102 af
	Avg. Flow Depth=0.23' Max Vel=5.78 fps Inflow=1.19 cfs 0.137 af L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=1.19 cfs 0.137 af
	Avg. Flow Depth=0.32' Max Vel=2.52 fps Inflow=0.85 cfs 0.058 af 160.0' S=0.0125 '/' Capacity=2.21 cfs Outflow=0.81 cfs 0.058 af
	Avg. Flow Depth=0.21' Max Vel=4.34 fps Inflow=0.80 cfs 0.058 af =85.0' S=0.0624 '/' Capacity=4.93 cfs Outflow=0.75 cfs 0.058 af
	Avg. Flow Depth=0.29' Max Vel=6.73 fps Inflow=1.99 cfs 0.206 af L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=1.99 cfs 0.206 af
Pond 1P: G.U.S.F. #1 Discarded=0.02 c	Peak Elev=50.43' Storage=2,062 cf Inflow=1.58 cfs 0.155 af fs 0.012 af Primary=1.09 cfs 0.102 af Outflow=1.11 cfs 0.114 af
Pond 3P: G.U.S.F.#2 Discarded=0.01 c	Peak Elev=47.49' Storage=1,129 cf Inflow=1.08 cfs 0.089 af fs 0.008 af Primary=0.80 cfs 0.058 af Outflow=0.81 cfs 0.066 af
Pond 4P: G.U.S.F. #3 Discarded=0.00 c	Peak Elev=48.01' Storage=198 cf Inflow=0.37 cfs 0.025 af fs 0.003 af Primary=0.35 cfs 0.018 af Outflow=0.36 cfs 0.021 af
Total Runoff Area = 2.263 a	c Runoff Volume = 0.435 af Average Runoff Depth = 2.30"

88.66% Pervious = 2.006 ac 11.34% Impervious = 0.257 ac

# Summary for Subcatchment 1.1S: (new Subcat)

Runoff = 0.52 cfs @ 12.09 hrs, Volume= 0.035 af, Depth> 3.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

^	rea (sf)	CN E	Description				
	1,758	98 F	Paved parki	ing, HSG B			
	2,373	61 >	75% Grass	s cover, Go	ood, HSG B		
	600	55 V	Voods, Goo	od, HSG B			
	4,731	74 V	74 Weighted Average				
	2,973	6	62.84% Per	vious Area			
	1,758	3	37.16% Imp	ervious Are	ea		
_							
Tc	Length	Slope	Velocity	Capacity	Description		
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)			
0.5	130	0.0750	4.11		Shallow Concentrated Flow,		
					Grassed Waterway Kv= 15.0 fps		
0.5	130	Total, I	ncreased t	o minimum	Tc = 6.0 min		
<u>(min)</u> 0.5	(feet) 130	(ft/ft) 0.0750	(ft/sec) 4.11	(cfs)	Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps		

# Summary for Subcatchment 1.2S: (new Subcat)

Runoff	=	1.58 cfs @	12.27 hrs,	Volume=	0.155 af, Depth> 1.98"
--------	---	------------	------------	---------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

Α	rea (sf)	CN [	Description		
	138	98 F	Roofs, HSG	βB	
	214	98 l	Jnconnecte	ed roofs, H	SG B
	486	98 F	Paved park	ing, HSG B	3
	3,131	61 >	>75% Ġras	s cover, Go	bod, HSG B
	4,903	48 E	Brush, Goo	d, HSG B	
	32,178	55 \	Noods, Go	od, HSG B	
	41,050	55 \	Veighted A	verage	
	40,212	ç	97.96% Pei	vious Area	l
	838	2	2.04% Impe	ervious Are	а
	214	2	25.54% Un	connected	
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
15.2	100	0.0400	0.11		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
2.7	100	0.0150	0.61		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.2	30	0.0200	2.12		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
18.1	230	Total			

## Summary for Subcatchment 2.1S: (new Subcat)

Runoff = 0.85 cfs @ 12.09 hrs, Volume= 0.058 af, Depth> 3.75"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

A	rea (sf)	CN E	Description		
	2,907	98 F	aved park	ing, HSG B	
	3,899	61 >	75% Gras	s cover, Go	ood, HSG B
	1,232	55 V	Voods, Go	od, HSG B	
	8,038	73 V	Veighted A	verage	
	5,131	6	3.83% Per	vious Area	
	2,907	3	6.17% Imp	pervious Are	ea
Тс	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
4.6	50	0.2000	0.18		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.2	35	0.0600	3.67		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
4.8	85	Tatal			Tc = 6.0 min

### Summary for Subcatchment 2.2S: (new Subcat)

Runoff = $1.08 \text{ cfs} @$	12.17 hrs, Volume=	0.089 af, Depth> 2.45"
-------------------------------	--------------------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

A	rea (sf)	CN A	Adj Desc	ription	
	626	98	Roof	s, HSG B	
	1,794	98	Unco	onnected ro	ofs, HSG B
	774	98	Pave	ed parking,	HSG B
	5,940	61	>75%	6 Grass co	ver, Good, HSG B
	4,945	48	Brus	h, Good, H	SG B
	4,853	55	Woo	ds, Good, I	HSG B
	18,932	62	60 Weig	hted Avera	age, UI Adjusted
	15,738			3% Perviou	
	3,194		16.8	7% Impervi	ous Area
	1,794		56.1	7% Unconr	nected
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
11.5	100	0.0800	0.15		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.4	90	0.0500	3.35		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
11.9	190	Total			

### Summary for Subcatchment 2.3S: (new Subcat)

Runoff = 0.37 cfs @ 12.09 hrs, Volume= 0.025 af, Depth> 3.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

	Area (sf)	CN [	Description		
	1,152	98 F	Paved park	ing, HSG B	3
	1,622	61 >	>75% Gras	s cover, Go	bod, HSG B
	880	55 \	Noods, Go	od, HSG B	
	3,654	71 \	Neighted A	verage	
	2,502	6	58.47% Pei	vious Area	
	1,152	3	31.53% Imp	pervious Ar	ea
Т	c Length	Slope	,	Capacity	Description
(mii	n) (feet)	(ft/ft)	(ft/sec)	(cfs)	
4	3 40	0.1500	0.16		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.	1 25	0.1800	6.36		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
4	4 65	Total	Increased t	o minimum	1 Tc = 6.0 min

## Summary for Subcatchment 2.4S: (new Subcat)

Runoff	=	0.88 cfs @	12.16 hrs,	Volume=	0.073 af, Depth> 1.72"
--------	---	------------	------------	---------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-yr storm Rainfall=7.10"

A	rea (sf)	CN A	Adj Desc	cription	
	317	98	Roof	s, HSG B	
	872	98	Unco	onnected ro	oofs, HSG B
	145	98	Pave	ed parking,	HSG B
	1,928	61	>75%	6 Grass co	ver, Good, HSG B
	17,752	48	Brus	h, Good, H	SG B
	1,157	55	Woo	ds, Good, I	HSG B
	22,171	53	52 Weig	hted Avera	age, UI Adjusted
	20,837		93.9	8% Perviou	is Area
	1,334		6.02	% Impervio	us Area
	872		65.3	7% Unconr	nected
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
9.3	80	0.0875	0.14		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
1.1	80	0.0625	1.25		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
10.4	160	Total			

### Summary for Reach 1R: (new Reach)

 Inflow Area =
 0.942 ac,
 2.04% Impervious, Inflow Depth >
 1.30" for 25-yr storm event

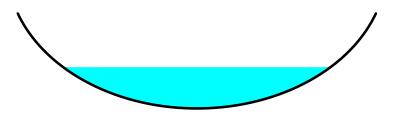
 Inflow =
 1.09 cfs @
 12.52 hrs, Volume=
 0.102 af

 Outflow =
 1.08 cfs @
 12.53 hrs, Volume=
 0.102 af, Atten= 1%, Lag= 1.1 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 5.67 fps, Min. Travel Time= 0.3 min Avg. Velocity = 2.96 fps, Avg. Travel Time= 0.6 min

Peak Storage= 19 cf @ 12.53 hrs Average Depth at Peak Storage= 0.22' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.31 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 100.0' Slope= 0.1020 '/' Inlet Invert= 50.20', Outlet Invert= 40.00'



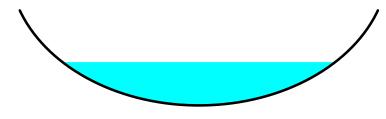
# Summary for Reach 2R: POA #2

Inflow Area	a =	1.051 ac,	5.67% Impervious,	Inflow Depth >	1.57"	for 25-yr storm event
Inflow	=	1.19 cfs @	12.52 hrs, Volume	= 0.137	af	-
Outflow	=	1.19 cfs @	12.52 hrs, Volume	= 0.137	af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 5.78 fps, Min. Travel Time= 0.0 min Avg. Velocity = 2.50 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.52 hrs Average Depth at Peak Storage= 0.23' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



#### Summary for Reach 3R: Roadside ditch

 Inflow Area =
 0.185 ac, 36.17% Impervious, Inflow Depth > 3.75" for 25-yr storm event

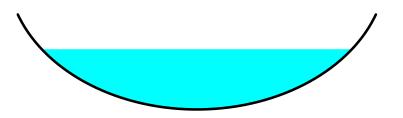
 Inflow =
 0.85 cfs @ 12.09 hrs, Volume=
 0.058 af

 Outflow =
 0.81 cfs @ 12.12 hrs, Volume=
 0.058 af, Atten= 5%, Lag= 1.8 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 2.52 fps, Min. Travel Time= 1.1 min Avg. Velocity = 0.95 fps, Avg. Travel Time= 2.8 min

Peak Storage= 54 cf @ 12.11 hrs Average Depth at Peak Storage= 0.32' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 2.21 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 160.0' Slope= 0.0125 '/' Inlet Invert= 42.00', Outlet Invert= 40.00'



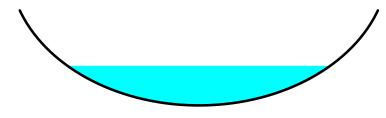
# Summary for Reach 4R: Swale

Inflow Area =	0.435 ac,	16.87% Impervious,	Inflow Depth > 1.	.59" for 25-yr storm event
Inflow =	0.80 cfs @	) 12.32 hrs, Volume	= 0.058 af	-
Outflow =	0.75 cfs @	) 12.34 hrs, Volume	= 0.058 af	, Atten= 6%, Lag= 1.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 4.34 fps, Min. Travel Time= 0.3 min Avg. Velocity = 2.03 fps, Avg. Travel Time= 0.7 min

Peak Storage= 16 cf @ 12.32 hrs Average Depth at Peak Storage= 0.21' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 4.93 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 85.0' Slope= 0.0624 '/' Inlet Invert= 47.30', Outlet Invert= 42.00'



# Summary for Reach 5R: POA #1

 Inflow Area =
 1.212 ac, 16.26% Impervious, Inflow Depth > 2.04" for 25-yr storm event

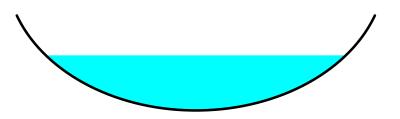
 Inflow =
 1.99 cfs @ 12.14 hrs, Volume=
 0.206 af

 Outflow =
 1.99 cfs @ 12.14 hrs, Volume=
 0.206 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 6.73 fps, Min. Travel Time= 0.0 min Avg. Velocity = 2.81 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.14 hrs Average Depth at Peak Storage= 0.29' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



# Summary for Pond 1P: G.U.S.F. #1

Inflow Area =	0.942 ac,	2.04% Impervious, Inflow D	epth > 1.98" for 25-yr storm event
Inflow =	1.58 cfs @	12.27 hrs, Volume=	0.155 af
Outflow =	1.11 cfs @	12.52 hrs, Volume=	0.114 af, Atten= 30%, Lag= 14.5 min
Discarded =	0.02 cfs @	12.52 hrs, Volume=	0.012 af
Primary =	1.09 cfs @	12.52 hrs, Volume=	0.102 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.43' @ 12.52 hrs Surf.Area= 1,392 sf Storage= 2,062 cf

Plug-Flow detention time= 105.2 min calculated for 0.114 af (74% of inflow) Center-of-Mass det. time= 40.6 min ( 870.3 - 829.8 )

Volume	Invert	Avail.	Storage	Storage Descrip	otion	
#1	46.17'	2	2,283 cf	Custom Stage	Data (Prismatic)Liste	d below
Elevation (feet)	Surf.A (sc	.rea ∖ q-ft)	/oids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet <u>)</u>	
46.17	(	615	0.0	0	0	
47.17	(	615	40.0	246	246	
48.67	(	615	20.0	185	431	
49.00	(	615 1	00.0	203	633	
50.00	1,	115 1	00.0	865	1,498	
50.60	1,	500 1	00.0	785	2,283	

4916 post

Type III 24-hr 25-yr storm Rainfall=7.10" Printed 10/5/2018

Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1 #2	Discarded Primary		0.600 in/hr Exfiltration over Surface area 4.0' long x 8.0' breadth Broad-Crested Rectangular Weir
π2	, mary	00.20	Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.66 2.64 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74

**Discarded OutFlow** Max=0.02 cfs @ 12.52 hrs HW=50.43' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

**Primary OutFlow** Max=1.07 cfs @ 12.52 hrs HW=50.43' (Free Discharge) **2=Broad-Crested Rectangular Weir** (Weir Controls 1.07 cfs @ 1.17 fps)

# Summary for Pond 3P: G.U.S.F. #2

Inflow Area =	0.435 ac, 16.87% Impervious, Inflow De	epth > 2.45" for 25-yr storm event
Inflow =	1.08 cfs @ 12.17 hrs, Volume=	0.089 af
Outflow =	0.81 cfs @ 12.32 hrs, Volume=	0.066 af, Atten= 25%, Lag= 9.0 min
Discarded =	0.01 cfs @ 12.32 hrs, Volume=	0.008 af
Primary =	0.80 cfs @ 12.32 hrs, Volume=	0.058 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 47.49' @ 12.32 hrs Surf.Area= 895 sf Storage= 1,129 cf

Plug-Flow detention time= 99.2 min calculated for 0.066 af (75% of inflow) Center-of-Mass det. time= 36.4 min (852.3 - 815.8)

Volume	Invert	. Ava	il.Stora	rage Storage Description				
#1	44.17'	1	1,351	51 cf Custom Stage Data (Prismatic)Listed below		:)Listed below		
Elevatio (fee 44.2	et)	urf.Area (sq-ft) 500	Voids (%) 0.0	) (cubic-feet)	Cum.Store (cubic-feet) 0			
44.6		500	40.0		100			
46.1	17	500	20.0	) 150	250			
46.5	50	500	100.0	) 165	415			
47.0	00	697	100.0	) 299	714			
47.7	75	1,000	100.0	) 636	1,351			
<u>Device</u> #1 #2	Routing Discarded Primary	44	.17' '.30'	Outlet Devices 0.600 in/hr Exfiltration over Surface area 4.0' long x 8.0' breadth Broad-Crested Rectangular Weir				
				Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74				

Page 39

**Discarded OutFlow** Max=0.01 cfs @ 12.32 hrs HW=47.48' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

**Primary OutFlow** Max=0.75 cfs @ 12.32 hrs HW=47.48' (Free Discharge) **2=Broad-Crested Rectangular Weir** (Weir Controls 0.75 cfs @ 1.04 fps)

## Summary for Pond 4P: G.U.S.F. #3

Inflow Area =	0.084 ac, 31.53% Impervious, Inflow De	epth > 3.55" for 25-yr storm event
Inflow =	0.37 cfs @ 12.09 hrs, Volume=	0.025 af
Outflow =	0.36 cfs @ 12.11 hrs, Volume=	0.021 af, Atten= 3%, Lag= 1.0 min
Discarded =	0.00 cfs @ 12.11 hrs, Volume=	0.003 af
Primary =	0.35 cfs @ 12.11 hrs, Volume=	0.018 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 48.01' @ 12.11 hrs Surf.Area= 304 sf Storage= 198 cf

Plug-Flow detention time= 68.6 min calculated for 0.021 af (84% of inflow) Center-of-Mass det. time= 22.3 min (814.4 - 792.0)

Volume	Invert	t Ava	il.Storage	e Storage Descr	iption	
#1	45.17	1	372 c	f Custom Stage	e Data (Prismatic)	Listed below
Elevatio (fee 45.0 45.0 47.0 47.0 48.0 48.0	et) 17 57 17 50 00	urf.Area (sq-ft) 110 110 110 110 302 410	Voids (%) 0.0 40.0 20.0 100.0 100.0 100.0	Inc.Store (cubic-feet) 0 22 33 36 103	Inc.Store (cubic-feet)         Cum.Store (cubic-feet)           0         0           22         22           33         55           36         91           103         194	
48.5 <u>Device</u> #1 #2	50 Routing Discarded Primary	45	5.17' <b>0.</b> 0 '.90' <b>4.</b> 0 He 2.8 Co	<b>D' long x 8.0' bre</b> ead (feet) 0.20 0. 50 3.00 3.50 4.0 pef. (English) 2.43	40 0.60 0.80 1.0 0 4.50 5.00 5.50	ted Rectangular Weir 00 1.20 1.40 1.60 1.80 2.00 0 2.68 2.68 2.66 2.64 2.64

**Discarded OutFlow** Max=0.00 cfs @ 12.11 hrs HW=48.01' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.34 cfs @ 12.11 hrs HW=48.01' (Free Discharge) -2=Broad-Crested Rectangular Weir (Weir Controls 0.34 cfs @ 0.80 fps)

4916 post	Type III 24-hr 50-yr storm Rainfall=8.50"
Prepared by Altus Engineering, Inc.	Printed 10/5/2018
HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Sol	Plutions LLC Page 41

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1.1S: (new Subcat) Flow Length=130	Runoff Area=4,731 sf 37.16% Impervious Runoff Depth>5.04" )' Slope=0.0750 '/' Tc=6.0 min CN=74 Runoff=0.67 cfs 0.046 af
Subcatchment1.2S: (new Subcat)	Runoff Area=41,050 sf 2.04% Impervious Runoff Depth>2.86" Flow Length=230' Tc=18.1 min CN=55 Runoff=2.33 cfs 0.224 af
Subcatchment2.1S: (new Subcat)	Runoff Area=8,038 sf 36.17% Impervious Runoff Depth>4.92" Flow Length=85' Tc=6.0 min CN=73 Runoff=1.11 cfs 0.076 af
Subcatchment 2.2S: (new Subcat) Flow Length	Runoff Area=18,932 sf 16.87% Impervious Runoff Depth>3.42" =190' Tc=11.9 min UI Adjusted CN=60 Runoff=1.53 cfs 0.124 af
Subcatchment2.3S: (new Subcat)	Runoff Area=3,654 sf 31.53% Impervious Runoff Depth>4.69" Flow Length=65' Tc=6.0 min CN=71 Runoff=0.48 cfs 0.033 af
Subcatchment2.4S: (new Subcat) Flow Length	Runoff Area=22,171 sf   6.02% Impervious   Runoff Depth>2.54" =160'   Tc=10.4 min   UI Adjusted CN=52   Runoff=1.35 cfs   0.108 af
	Avg. Flow Depth=0.30' Max Vel=6.89 fps Inflow=2.12 cfs 0.170 af 100.0' S=0.1020 '/' Capacity=6.31 cfs Outflow=2.11 cfs 0.170 af
	Avg. Flow Depth=0.31' Max Vel=7.05 fps Inflow=2.35 cfs 0.216 af L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=2.35 cfs 0.216 af
	Avg. Flow Depth=0.36' Max Vel=2.72 fps Inflow=1.11 cfs 0.076 af 160.0' S=0.0125 '/' Capacity=2.21 cfs Outflow=1.06 cfs 0.076 af
	Avg. Flow Depth=0.29' Max Vel=5.21 fps Inflow=1.47 cfs 0.092 af .=85.0' S=0.0624 '/' Capacity=4.93 cfs Outflow=1.47 cfs 0.092 af
	Avg. Flow Depth=0.39' Max Vel=8.12 fps Inflow=3.78 cfs 0.301 af L=1.0' S=0.1000 '/' Capacity=6.24 cfs Outflow=3.78 cfs 0.301 af
Pond 1P: G.U.S.F. #1 Discarded=0.02 of	Peak Elev=50.55' Storage=2,223 cf Inflow=2.33 cfs 0.224 af fs 0.013 af Primary=2.12 cfs 0.170 af Outflow=2.14 cfs 0.183 af
Pond 3P: G.U.S.F. #2 Discarded=0.01 of	Peak Elev=47.58' Storage=1,207 cf Inflow=1.53 cfs 0.124 af cfs 0.009 af Primary=1.47 cfs 0.092 af Outflow=1.48 cfs 0.101 af
Pond 4P: G.U.S.F. #3 Discarded=0.00 of	Peak Elev=48.03' Storage=206 cf Inflow=0.48 cfs 0.033 af cfs 0.003 af Primary=0.46 cfs 0.026 af Outflow=0.47 cfs 0.029 af
Total Runoff Area = 2.263 a	ac Runoff Volume = 0.610 af Average Runoff Depth = 3.23"

88.66% Pervious = 2.006 ac 11.34% Impervious = 0.257 ac

# Summary for Subcatchment 1.1S: (new Subcat)

Runoff = 0.67 cfs @ 12.09 hrs, Volume= 0.046 af, Depth> 5.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

^	rea (sf)	CN E	Description					
	1,758	98 F	Paved parking, HSG B					
	2,373	61 >	75% Grass	s cover, Go	ood, HSG B			
	600	55 V	Voods, Goo	od, HSG B				
	4,731	74 V	Veighted A	verage				
	2,973	6	62.84% Pervious Area					
	1,758	3	37.16% Impervious Area					
_								
Tc	Length	Slope	Velocity	Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
0.5	130	0.0750	4.11		Shallow Concentrated Flow,			
					Grassed Waterway Kv= 15.0 fps			
0.5	130	Total, I	ncreased t	o minimum	Tc = 6.0 min			
<u>(min)</u> 0.5	(feet) 130	(ft/ft) 0.0750	(ft/sec) 4.11	(cfs)	Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps			

### Summary for Subcatchment 1.2S: (new Subcat)

Runoff	=	2.33 cfs @	12.27 hrs,	Volume=	0.224 af, D	Depth> 2.86"
--------	---	------------	------------	---------	-------------	--------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

Α	rea (sf)	CN [	Description				
	138	98 F	Roofs, HSG	βB			
	214	98 l	Unconnected roofs, HSG B				
	486	98 F	Paved park	ing, HSG B	3		
	3,131	61 >	>75% Ġras	s cover, Go	bod, HSG B		
	4,903	48 E	Brush, Goo	d, HSG B			
	32,178	55 \	Noods, Go	od, HSG B			
	41,050	55 \	Veighted A	verage			
	40,212	ç	97.96% Pei	vious Area	l		
	838	2	2.04% Impe	ervious Are	а		
	214	2	25.54% Un	connected			
Тс	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
15.2	100	0.0400	0.11		Sheet Flow,		
					Woods: Light underbrush n= 0.400 P2= 3.69"		
2.7	100	0.0150	0.61		Shallow Concentrated Flow,		
					Woodland Kv= 5.0 fps		
0.2	30	0.0200	2.12		Shallow Concentrated Flow,		
					Grassed Waterway Kv= 15.0 fps		
18.1	230	Total					

## Summary for Subcatchment 2.1S: (new Subcat)

Runoff = 1.11 cfs @ 12.09 hrs, Volume= 0.076 af, Depth> 4.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

A	rea (sf)	CN E	Description		
	2,907	98 F	aved park	ing, HSG B	
	3,899	61 >	75% Gras	s cover, Go	ood, HSG B
	1,232	55 V	Voods, Go	od, HSG B	
	8,038	73 V	Veighted A	verage	
	5,131	6	3.83% Per	vious Area	
	2,907	3	6.17% Imp	pervious Are	ea
Тс	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
4.6	50	0.2000	0.18		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.2	35	0.0600	3.67		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
4.8	85	Tatal			Tc = 6.0 min

# Summary for Subcatchment 2.2S: (new Subcat)

Runoff = 1.53 cfs @ 12.17 hrs, Volume= 0.124 af, Depth> 3.42"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

A	rea (sf)	CN A	Adj Desc	ription			
	626	98	Roof	s, HSG B			
	1,794	98	Unco	onnected ro	oofs, HSG B		
	774	98	Pave	ed parking,	HSG B		
	5,940	61			ver, Good, HSG B		
	4,945	48	Brus	h, Good, H	SG B		
	4,853	55	Woo	ds, Good, H	HSG B		
	18,932	62	60 Weig	hted Avera	age, UI Adjusted		
	15,738		83.13% Pervious Area				
	3,194		16.8	7% Impervi	ous Area		
	1,794		56.1	7% Unconr	nected		
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
11.5	100	0.0800	0.15		Sheet Flow,		
					Woods: Light underbrush n= 0.400 P2= 3.69"		
0.4	90	0.0500	3.35		Shallow Concentrated Flow,		
					Grassed Waterway Kv= 15.0 fps		
11.9	190	Total					

## Summary for Subcatchment 2.3S: (new Subcat)

Runoff = 0.48 cfs @ 12.09 hrs, Volume= 0.033 af, Depth> 4.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

	Area (sf)	CN [	Description		
	1,152	98 F	Paved park	ing, HSG B	3
	1,622	61 >	>75% Gras	s cover, Go	bod, HSG B
	880	55 \	Noods, Go	od, HSG B	
	3,654	71 \	Neighted A	verage	
	2,502	6	58.47% Pei	vious Area	
	1,152	3	31.53% Imp	pervious Ar	ea
Т	c Length	Slope	,	Capacity	Description
(mii	n) (feet)	(ft/ft)	(ft/sec)	(cfs)	
4	3 40	0.1500	0.16		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
0.	1 25	0.1800	6.36		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
4	4 65	Total	Increased t	o minimum	1 Tc = 6.0 min

## Summary for Subcatchment 2.4S: (new Subcat)

Runoff	=	1.35 cfs @	12.16 hrs,	Volume=	0.108 af, Depth> 2.54"
--------	---	------------	------------	---------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr storm Rainfall=8.50"

A	rea (sf)	CN /	Adj Desc	ription	
	317	98	Roof	s, HSG B	
	872	98	Unco	onnected ro	oofs, HSG B
	145	98	Pave	ed parking,	HSG B
	1,928	61	>75%	6 Grass co	ver, Good, HSG B
	17,752	48		h, Good, H	
	1,157	55	Woo	ds, Good, I	HSG B
	22,171	53	52 Weig	hted Avera	age, UI Adjusted
	20,837		93.9	is Area	
	1,334		6.029	% Impervio	us Area
	872		65.3	7% Unconr	nected
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
9.3	80	0.0875	0.14		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.69"
1.1	80	0.0625	1.25		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
10.4	160	Total			

## Summary for Reach 1R: (new Reach)

 Inflow Area =
 0.942 ac,
 2.04% Impervious, Inflow Depth >
 2.17" for 50-yr storm event

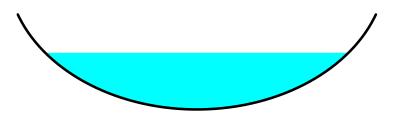
 Inflow =
 2.12 cfs @
 12.37 hrs, Volume=
 0.170 af

 Outflow =
 2.11 cfs @
 12.37 hrs, Volume=
 0.170 af, Atten= 0%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 6.89 fps, Min. Travel Time= 0.2 min Avg. Velocity = 3.34 fps, Avg. Travel Time= 0.5 min

Peak Storage= 31 cf @ 12.37 hrs Average Depth at Peak Storage= 0.30' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.31 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 100.0' Slope= 0.1020 '/' Inlet Invert= 50.20', Outlet Invert= 40.00'



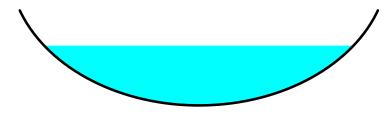
# Summary for Reach 2R: POA #2

Inflow Area	a =	1.051 ac,	5.67% Impervious,	Inflow Depth >	2.46"	for 50-yr storm event
Inflow	=	2.35 cfs @	12.37 hrs, Volume	= 0.216	af	-
Outflow	=	2.35 cfs @	12.37 hrs, Volume	= 0.216	af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 7.05 fps, Min. Travel Time= 0.0 min Avg. Velocity = 2.70 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.37 hrs Average Depth at Peak Storage= 0.31' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



## Summary for Reach 3R: Roadside ditch

 Inflow Area =
 0.185 ac, 36.17% Impervious, Inflow Depth > 4.92" for 50-yr storm event

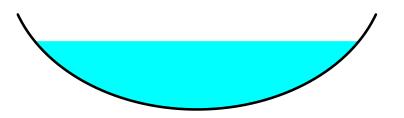
 Inflow =
 1.11 cfs @ 12.09 hrs, Volume=
 0.076 af

 Outflow =
 1.06 cfs @ 12.12 hrs, Volume=
 0.076 af, Atten= 5%, Lag= 1.6 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 2.72 fps, Min. Travel Time= 1.0 min Avg. Velocity = 1.01 fps, Avg. Travel Time= 2.7 min

Peak Storage= 65 cf @ 12.10 hrs Average Depth at Peak Storage= 0.36' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 2.21 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 160.0' Slope= 0.0125 '/' Inlet Invert= 42.00', Outlet Invert= 40.00'



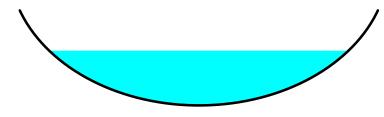
# Summary for Reach 4R: Swale

Inflow Area =	0.435 ac, 16.87% Impervious,	Inflow Depth > 2.55" for 50-yr storm event
Inflow =	1.47 cfs @ 12.22 hrs, Volume	= 0.092 af
Outflow =	1.47 cfs @ 12.22 hrs, Volume	= 0.092 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 5.21 fps, Min. Travel Time= 0.3 min Avg. Velocity = 2.28 fps, Avg. Travel Time= 0.6 min

Peak Storage= 25 cf @ 12.22 hrs Average Depth at Peak Storage= 0.29' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 4.93 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 85.0' Slope= 0.0624 '/' Inlet Invert= 47.30', Outlet Invert= 42.00'



# Summary for Reach 5R: POA #1

 Inflow Area =
 1.212 ac, 16.26% Impervious, Inflow Depth > 2.98" for 50-yr storm event

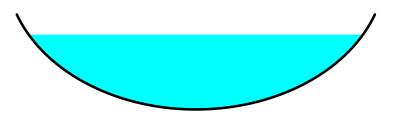
 Inflow =
 3.78 cfs @ 12.20 hrs, Volume=
 0.301 af

 Outflow =
 3.78 cfs @ 12.20 hrs, Volume=
 0.301 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 8.12 fps, Min. Travel Time= 0.0 min Avg. Velocity = 3.01 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 12.20 hrs Average Depth at Peak Storage= 0.39' Bank-Full Depth= 0.50' Flow Area= 0.7 sf, Capacity= 6.24 cfs

2.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 1.0' Slope= 0.1000 '/' Inlet Invert= 40.00', Outlet Invert= 39.90'



# Summary for Pond 1P: G.U.S.F. #1

Inflow Area =	0.942 ac,	2.04% Impervious, Inflow De	epth > 2.86" for 50-yr storm event
Inflow =	2.33 cfs @	12.27 hrs, Volume=	0.224 af
Outflow =	2.14 cfs @	12.37 hrs, Volume=	0.183 af, Atten= 8%, Lag= 6.1 min
Discarded =	0.02 cfs @	12.37 hrs, Volume=	0.013 af
Primary =	2.12 cfs @	12.37 hrs, Volume=	0.170 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 50.55' @ 12.37 hrs Surf.Area= 1,470 sf Storage= 2,223 cf

Plug-Flow detention time= 76.5 min calculated for 0.182 af (81% of inflow) Center-of-Mass det. time= 27.1 min (848.7 - 821.6)

Volume	Invert	Ava	il.Storage	Storage Descrip	tion	
#1	46.17'		2,283 cf	Custom Stage	Data (Prismatic)Listed	below
Elevation (feet)	Surf./ (s	Area q-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
46.17		615	0.0	0	0	
47.17		615	40.0	246	246	
48.67		615	20.0	185	431	
49.00		615	100.0	203	633	
50.00	1,	,115	100.0	865	1,498	
50.60	1,	,500	100.0	785	2,283	

4916 post

Type III 24-hr 50-yr storm Rainfall=8.50" Printed 10/5/2018

Prepared by Altus Engineering, Inc. HydroCAD® 10.00-22 s/n 01222 © 2018 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1 #2	Discarded Primary		0.600 in/hr Exfiltration over Surface area 4.0' long x 8.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74

**Discarded OutFlow** Max=0.02 cfs @ 12.37 hrs HW=50.55' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

**Primary OutFlow** Max=2.07 cfs @ 12.37 hrs HW=50.55' (Free Discharge) **2=Broad-Crested Rectangular Weir** (Weir Controls 2.07 cfs @ 1.48 fps)

# Summary for Pond 3P: G.U.S.F. #2

Inflow Area =	0.435 ac, 16.87% Impervious, Inflow De	epth > 3.42" for 50-yr storm event
Inflow =	1.53 cfs @ 12.17 hrs, Volume=	0.124 af
Outflow =	1.48 cfs @ 12.22 hrs, Volume=	0.101 af, Atten= 3%, Lag= 3.1 min
Discarded =	0.01 cfs @ 12.22 hrs, Volume=	0.009 af
Primary =	1.47 cfs @ 12.22 hrs, Volume=	0.092 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 47.58' @ 12.22 hrs Surf.Area= 932 sf Storage= 1,207 cf

Plug-Flow detention time= 75.8 min calculated for 0.101 af (82% of inflow) Center-of-Mass det. time= 25.8 min ( 834.2 - 808.4 )

Volume	Invert	t Ava	il.Storage	Storage Descri	ption	
#1	44.17'	ı	1,351 cf	Custom Stage	Data (Prismatic	)Listed below
Elevatio	et) 17	urf.Area (sq-ft) 500	Voids (%) 0.0	Inc.Store (cubic-feet) 0	Cum.Store (cubic-feet) 0	
44.6 46.5 46.5 47.0	17 50 00	500 500 500 697	40.0 20.0 100.0 100.0	100 150 165 299	100 250 415 714	
47.7 Device	Routing	1,000 In	100.0 vert Out	636 let Devices	1,351	
#1 #2	Discarded Primary		.17' <b>0.6</b> '.30' <b>4.0</b> ' Hea 2.5 Coe	<b>00 in/hr Exfiltrati</b> <b>long x 8.0' brea</b> ad (feet) 0.20 0.4 0 3.00 3.50 4.00 ef. (English) 2.43	40 0.60 0.80 1.0 0 4.50 5.00 5.50	ted Rectangular Weir 00 1.20 1.40 1.60 1.80 2.00 0 2.68 2.68 2.66 2.64 2.64

Page 48

**Discarded OutFlow** Max=0.01 cfs @ 12.22 hrs HW=47.57' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

**Primary OutFlow** Max=1.40 cfs @ 12.22 hrs HW=47.57' (Free Discharge) **2=Broad-Crested Rectangular Weir** (Weir Controls 1.40 cfs @ 1.29 fps)

## Summary for Pond 4P: G.U.S.F. #3

Inflow Area =	0.084 ac, 31.53% Impervious, Inflow De	epth > 4.69" for 50-yr storm event
Inflow =	0.48 cfs @ 12.09 hrs, Volume=	0.033 af
Outflow =	0.47 cfs @ 12.11 hrs, Volume=	0.029 af, Atten= 3%, Lag= 1.1 min
Discarded =	0.00 cfs @ 12.11 hrs, Volume=	0.003 af
Primary =	0.46 cfs @ 12.11 hrs, Volume=	0.026 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 48.03' @ 12.11 hrs Surf.Area= 309 sf Storage= 206 cf

Plug-Flow detention time= 57.1 min calculated for 0.029 af (88% of inflow) Center-of-Mass det. time= 19.2 min ( 804.9 - 785.6 )

Volume	Invert	Ava	il.Storag	ge Storage Descr	iption	
#1	45.17'		372	2 cf Custom Stage Data (Prismatic)Listed below		
Elevatio (fee 45.7 45.6 47.7 47.5 48.0 48.5	et) 17 57 17 50 00	urf.Area (sq-ft) 110 110 110 110 302 410	Voids (%) 0.0 40.0 20.0 100.0 100.0 100.0	(cubic-feet) 0 22 33 36 103	Cum.Store (cubic-feet) 0 22 55 91 194 372	
#1 Discarded 45.17' 0.6 #2 Primary 47.90' 4.0 He 2.5 Co		Dutlet Devices 0.600 in/hr Exfiltrat 0.0' long x 8.0' bre Head (feet) 0.20 0. 2.50 3.00 3.50 4.0	tion over Surface adth Broad-Crest 40 0.60 0.80 1.0 0 4.50 5.00 5.50 3 2.54 2.70 2.69	<b>ted Rectangular Weir</b> 00 1.20 1.40 1.60 1.80 2.00 0 2.68 2.68 2.66 2.64 2.64		

**Discarded OutFlow** Max=0.00 cfs @ 12.11 hrs HW=48.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.45 cfs @ 12.11 hrs HW=48.03' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 0.45 cfs @ 0.87 fps)