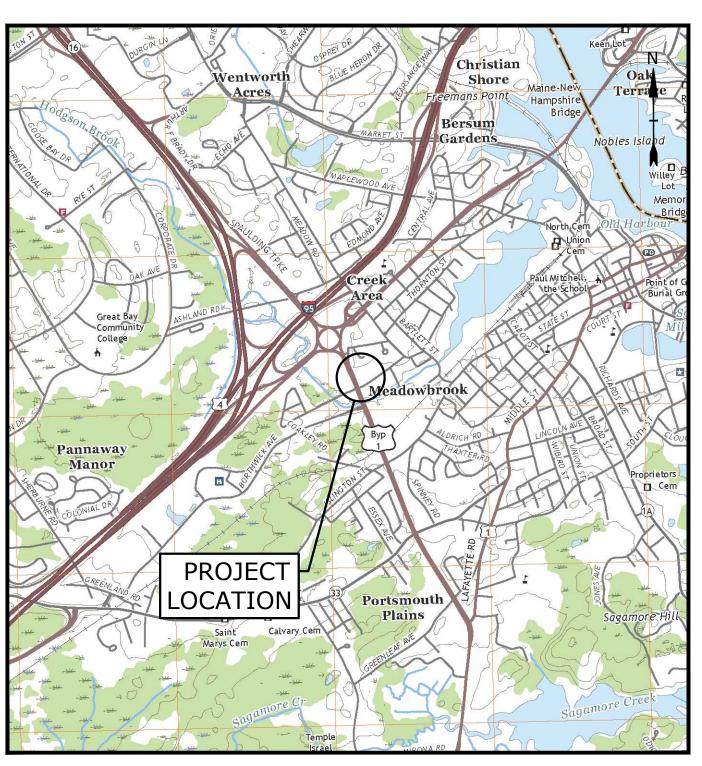
# PROPOSED MEDICAL OFFICE BUILDING

# 185 COTTAGE STREET PORTSMOUTH, NEW HAMPSHIRE PERMIT DESIGN DRAWINGS AUGUST 20, 2018

LAST REVISED: OCTOBER 9, 2018

LIST OF DRAWINGS			
SHEET NO.	EET NO. SHEET TITLE		
	COVER SHEET	10/09/2018	
1 OF 1	EXISTING CONDITIONS PLAN	08/14/2018	
C-101	DEMOLITION PLAN	10/09/2018	
C-102	SITE PLAN	10/09/2018	
C-103	GRADING, DRAINAGE & EROSION CONTROL PLAN	10/09/2018	
C-104	UTILITIES PLAN	10/09/2018	
C-105	LANDSCAPE PLAN	10/09/2018	
C-106	PHOTOMETRICS PLAN	10/09/2018	
C-501	EROSION CONTROL NOTES SHEET	10/09/2018	
C-502	DETAILS SHEET	10/09/2018	
C-503	DETAILS SHEET	10/09/2018	
C-504	DETAILS SHEET	10/09/2018	
C-505	DETAILS SHEET	10/09/2018	
C-506	DETAILS SHEET	10/09/2018	
A301	EXTERIOR ELEVATIONS	08/15/2018	

LIST OF PERMITS & APPROVALS			
LOCAL STATUS DATE			
SITE PLAN REVIEW PERMIT	PENDING		
ZONING BOARD OF ADJUSTMENT - VARIANCE FOR USE APPROVED 6/26/2018			



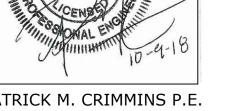
**LOCATION MAP** SCALE: 1" = 2,000'

THE CONTRACTOR SHALL NOT RELY ON SCALED DIMENSIONS AND SHALL CONTACT THE

OF THE CONTRACTOR, THEIR EMPLOYEES, AGENTS OR REPRESENTATIVES IN THE PERFORMANC OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING AND IMPLEMENTING SAFETY PROCEDURES AND SYSTEMS AS REQUIRED BY THE UNITED STATES OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION (OSHA), AND ANY STATE OR LOCAL SAFETY REGULATIONS.

. TIGHE & BOND. ASSUMES NO RESPONSIBILITY FOR ANY ISSUES LEGAL OR OTHERWISE RESULTING FROM CHANGES MADE TO THESE DRAWINGS WITHOUT WRITTEN AUTHORIZATION OF TIGHE & BOND.

## PREPARED BY:



DAR Real Estate, LLC 875 Greenland Road, Suite B-7 Portsmouth, NH 03801

## **SURVEY CONSULTANT:** Doucet Survey, Inc. 102 Kent Place

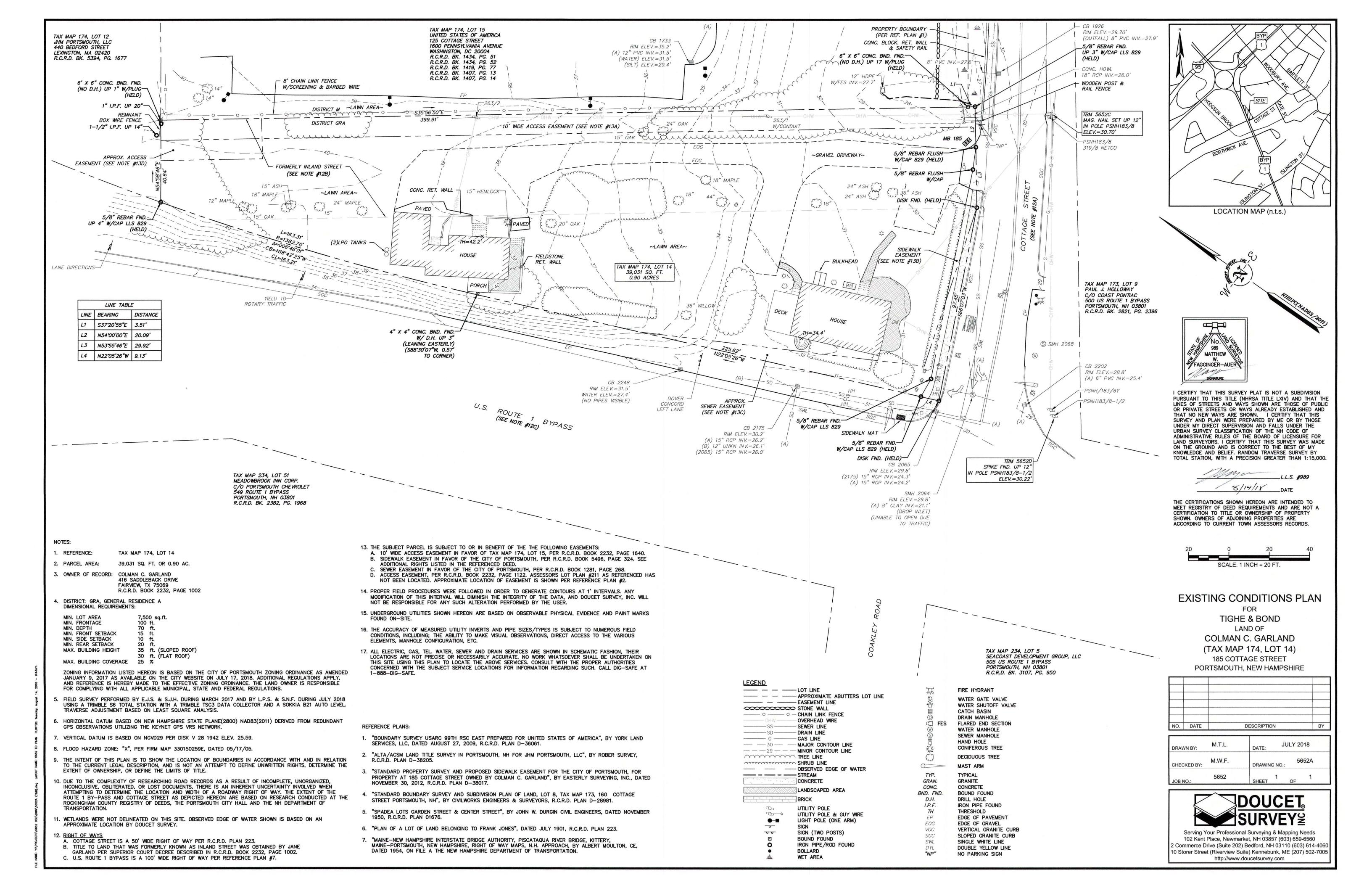
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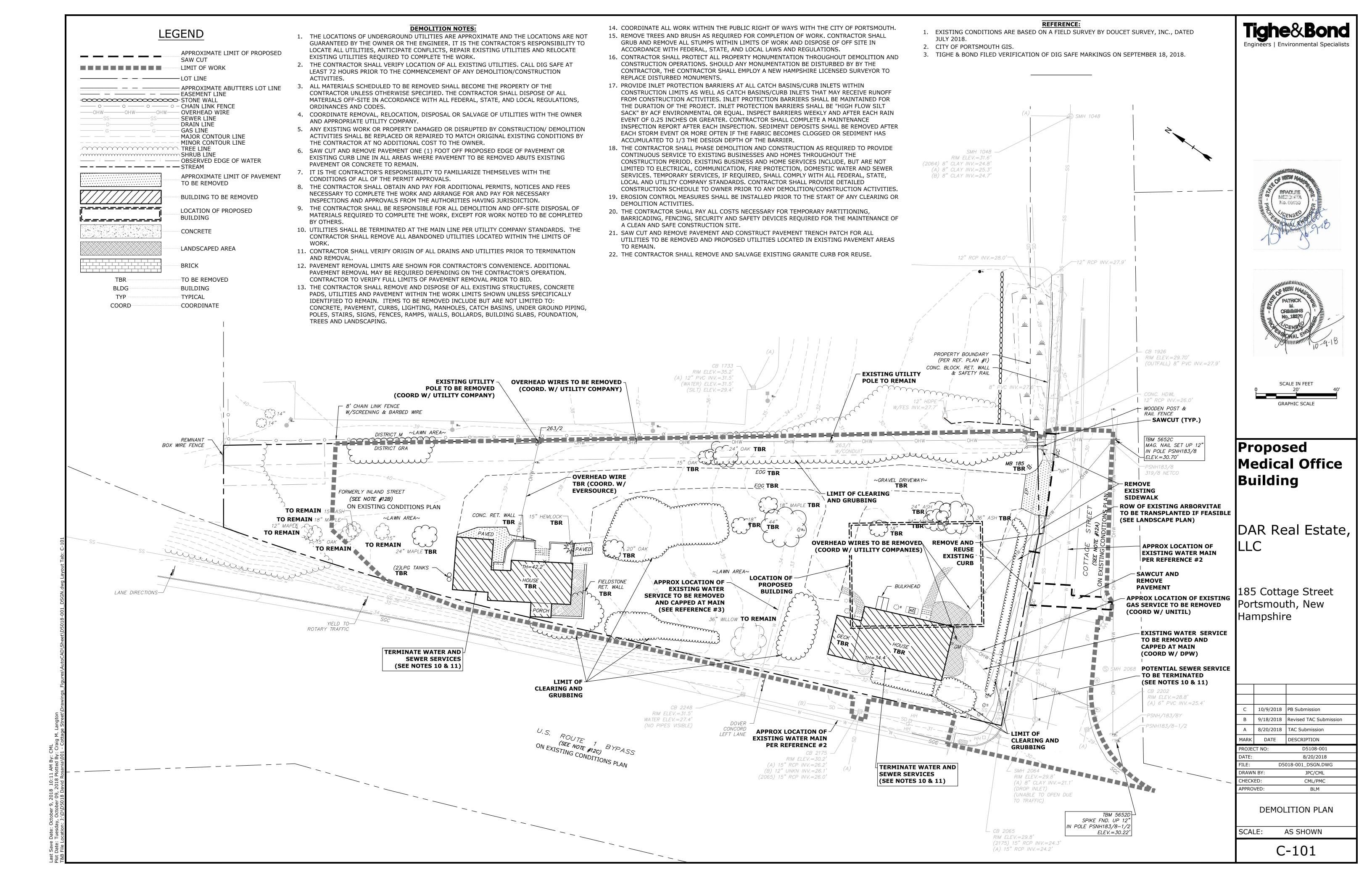
APPLICANT:

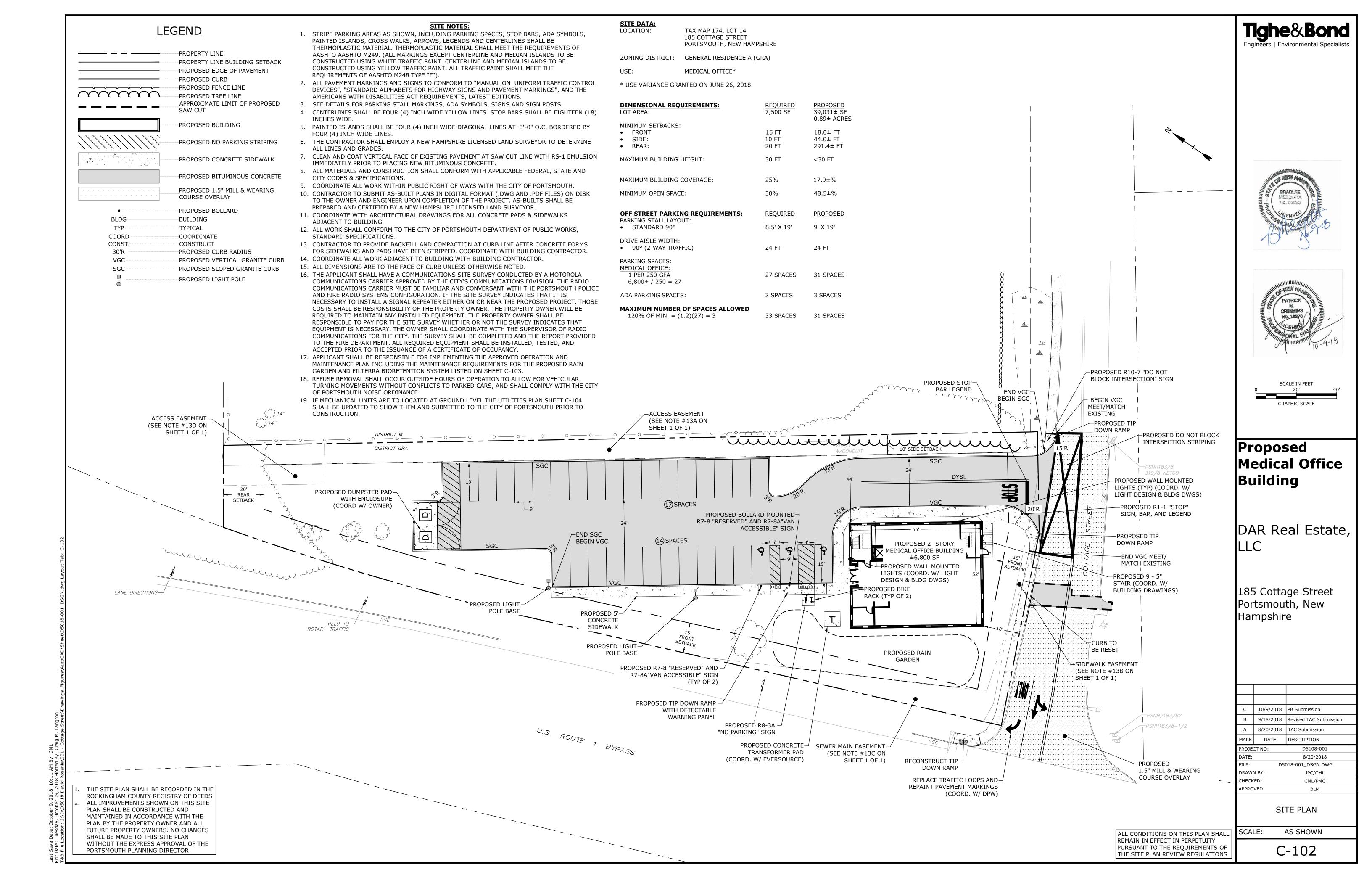
McHenry Architecture 4 Market Street Portsmouth, NH 03801

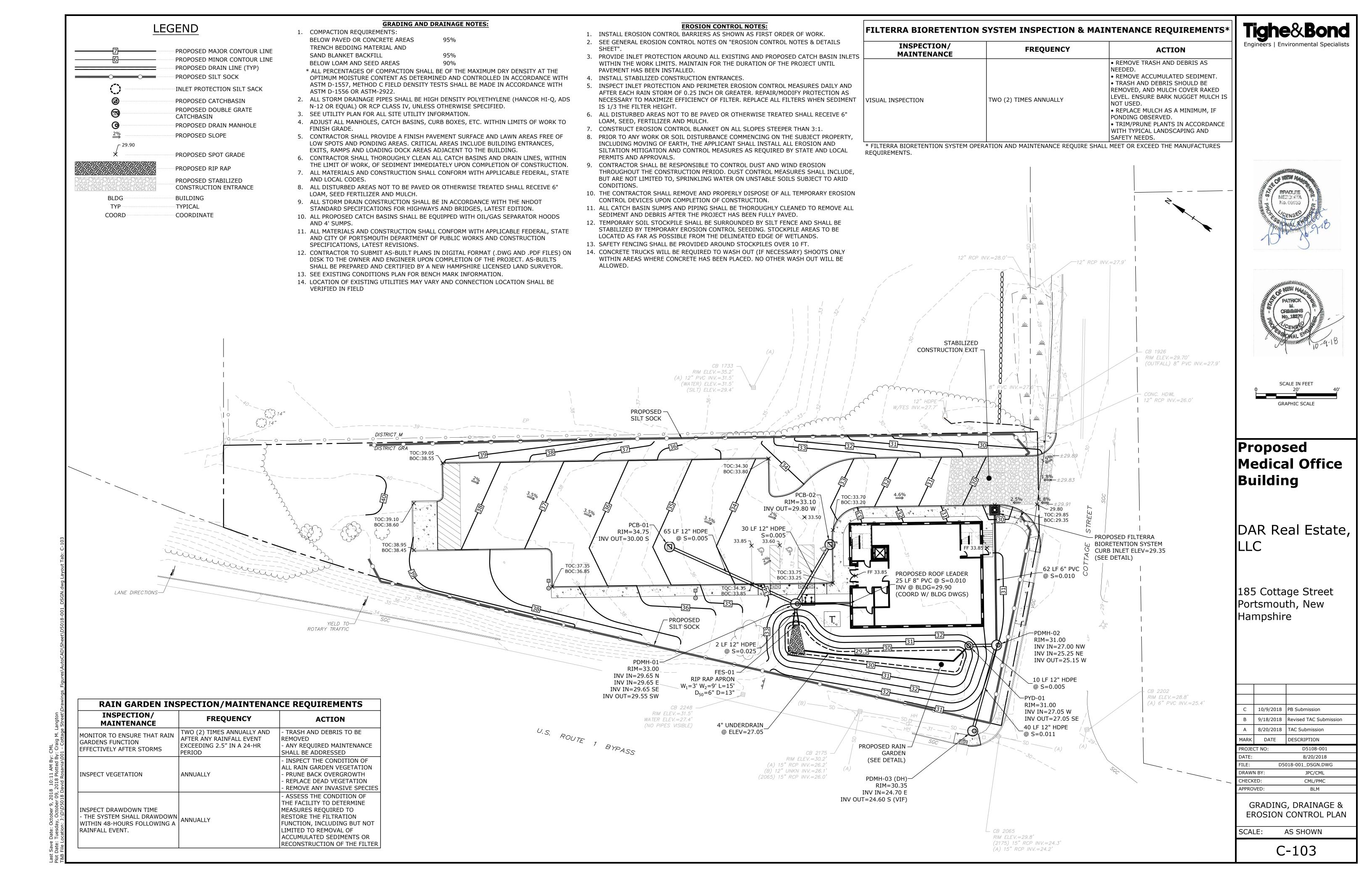


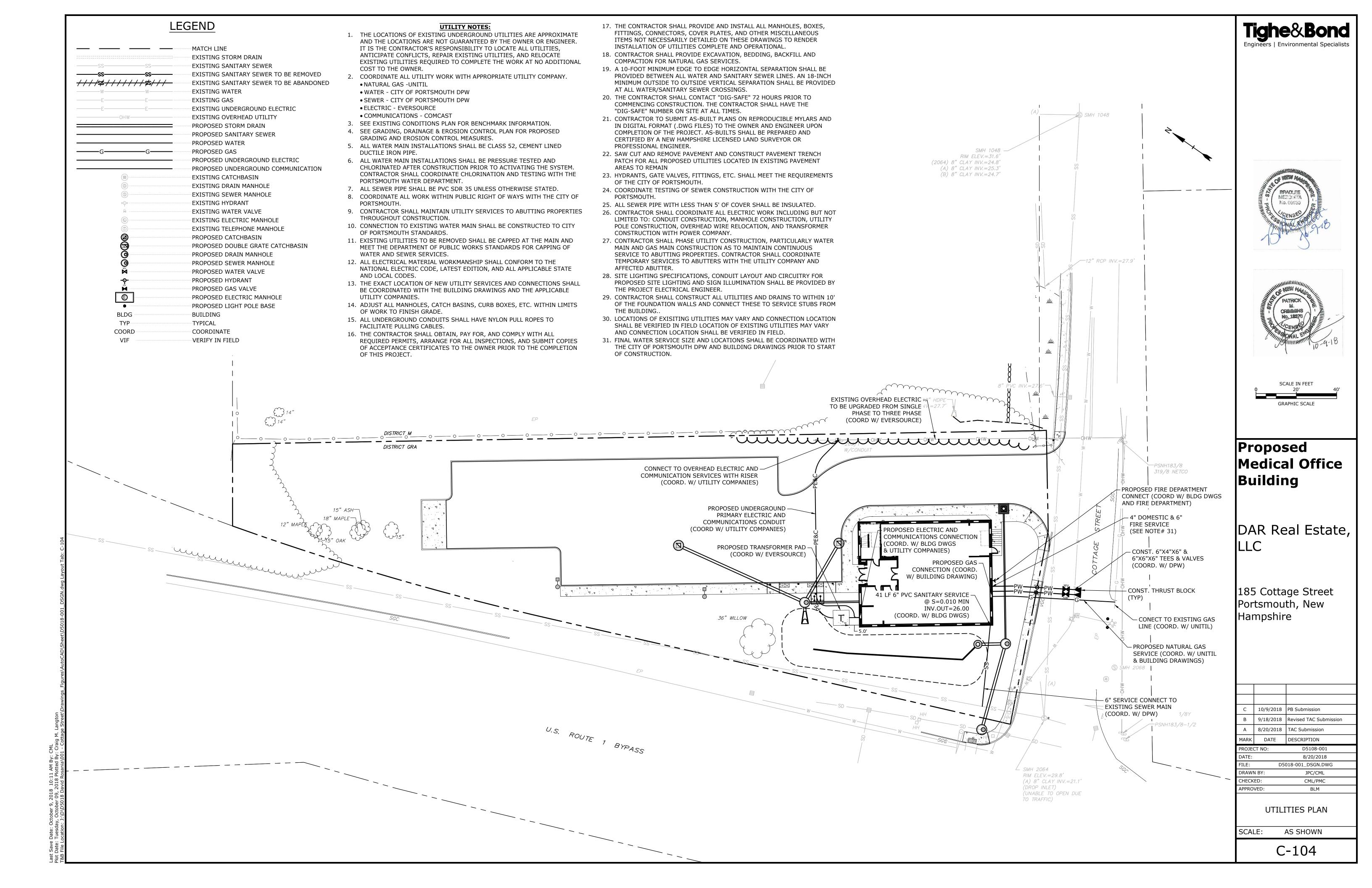












#### LANDSCAPE NOTES:

- THE CONTRACTOR SHALL FURNISH AND PLANT ALL PLANTS IN QUANTITIES AS SHOWN ON THIS PLAN. NO SUBSTITUTIONS WILL BE PERMITTED UNLESS APPROVED BY OWNER. ALL PLANTS SHALL BE NURSERY GROWN.
- ALL PLANTS SHALL BE NURSERY GROWN AND PLANTS AND WORKMANSHIP SHALL CONFORM TO THE AMERICAN ASSOCIATION OF NURSERYMEN STANDARDS, INCLUDING BUT NOT LIMITED TO SIZE, HEALTH, SHAPE, ETC., AND SHALL BE SUBJECT TO THE APPROVAL OF THE LANDSCAPE ARCHITECT PRIOR TO ARRIVAL
- PLANT STOCK SHALL BE GROWN WITHIN THE HARDINESS ZONES 4 THRU 7 ESTABLISHED BY THE PLANT HARDINESS ZONE MAP, MISCELLANEOUS PUBLICATIONS NO. 814, AGRICULTURAL RESEARCH SERVICE, UNITED STATES
- DEPARTMENT AGRICULTURE, LATEST REVISION. PLANT MATERIAL SHALL BARE THE SAME RELATIONSHIP TO FINISHED GRADE AS TO

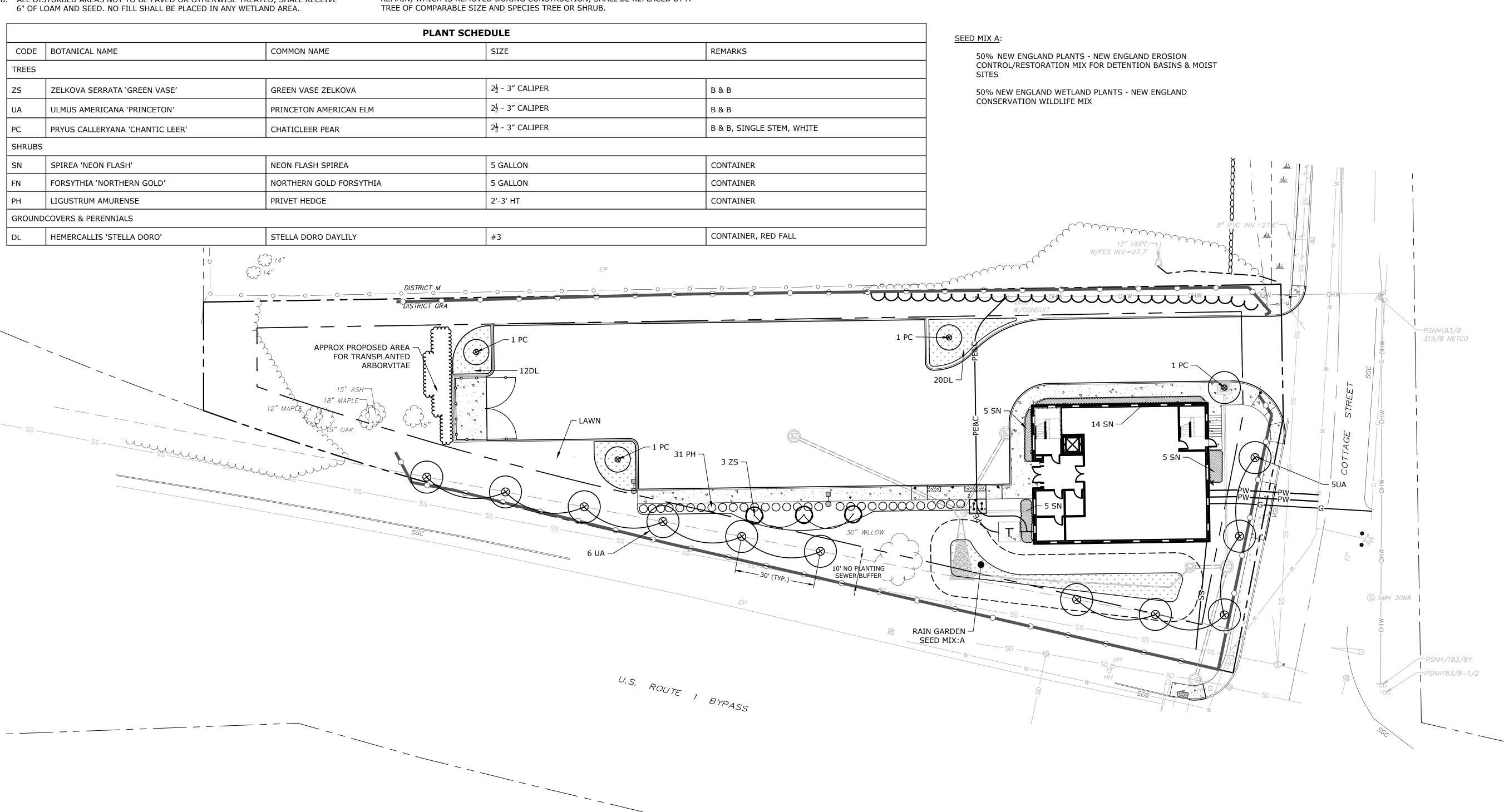
THE ORIGINAL PLANTING GRADE PRIOR TO DIGGING.

ON-SITE AND AFTER PLANTING.

- THE NUMBER OF EACH INDIVIDUAL PLANT TYPE AND SIZE PROVIDED IN THE PLANT LIST OR ON THE PLAN IS FOR THE CONTRACTOR'S CONVENIENCE ONLY. IF A DISCREPANCY EXISTS BETWEEN THE NUMBER OF PLANTS ON THE LABEL AND THE NUMBER OF SYMBOLS SHOWN ON THE DRAWINGS, THE GREATER NUMBER SHALL
- NO SUBSTITUTION OF PLANT MATERIALS WILL BE ALLOWED WITHOUT THE PRIOR WRITTEN APPROVAL OF THE OWNER'S REPRESENTATIVE.
- THE CONTRACTOR SHALL LOCATE, VERIFY AND MARK ALL EXISTING AND NEWLY INSTALLED UNDERGROUND UTILITIES PRIOR TO ANY LAWN WORK OR PLANTING. ANY CONFLICTS WHICH MIGHT OCCUR BETWEEN PLANTING AND UTILITIES SHALL IMMEDIATELY BE REPORTED TO THE OWNER SO THAT ALTERNATE PLANTING LOCATIONS CAN BE DETERMINED.
- ALL DISTURBED AREAS NOT TO BE PAVED OR OTHERWISE TREATED, SHALL RECEIVE

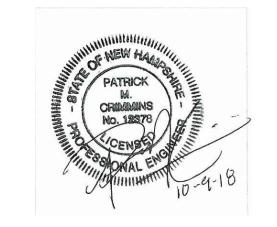
- 9. THREE INCHES (3") OF BARK MULCH IS TO BE USED AROUND THE TREE AND SHRUB PLANTING AS SPECIFIED IN THE DETAILS. WHERE BARK MULCH IS TO BE USED IN A CURBED ISLAND THE BARK MULCH SHALL MEET THE TOP INSIDE EDGE OF THE CURB. ALL OTHER AREAS SHALL RECEIVE 6" INCHES OF LOAM AND SEED.
- 10. LANDSCAPING SHALL BE LOCATED WITHIN 150 FT OF EXTERIOR HOSE ATTACHMENT OR SHALL BE PROVIDED WITH AN IRRIGATION SYSTEM.
- 11. SEE PLANTING DETAILS AND SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS. 12. TREE STAKES SHALL REMAIN IN PLACE FOR NO LESS THAN 6 MONTHS AND NO MORE
- 13. PLANTING SHALL BE COMPLETED FROM APRIL 15TH THROUGH OCTOBER 1ST. NO PLANTING DURING JULY AND AUGUST UNLESS SPECIAL PROVISIONS ARE MADE FOR
- 14. PARKING AREA PLANTED ISLANDS TO HAVE MINIMUM OF 1'-0" TOPSOIL PLACED TO WITHIN 3 INCHES OF THE TOP OF CURB ELEVATION. REMOVE ALL CONSTRUCTION DEBRIS BEFORE PLACING TOPSOIL.
- 15. TREES SHALL BE PRUNED IN ACCORDANCE WITH THE LATEST EDITION OF ANSI A300 'TREES, SHRUBS AND OTHER WOOD PLANT MAINTENANCE STANDARD
- 16. ALL PLANTS SHALL BE WATERED THOROUGHLY TWICE DURING THE FIRST 24 HOUR PERIOD AFTER PLANTING. ALL PLANTS SHALL BE WATERED WEEKLY, OR MORE OFTEN, IF NECESSARY DURING THE FIRST GROWING SEASON. LANDSCAPE CONTRACTOR SHALL COORDINATE WATERING SCHEDULE WITH OWNER DURING THE ONE (1) YEAR GUARANTEE PERIOD.
- 17. EXISTING TREES AND SHRUBS SHOWN ON THE PLAN ARE TO REMAIN UNDISTURBED. ALL EXISTING TREES AND SHRUBS SHOWN TO REMAIN ARE TO BE PROTECTED WITH A 4-FOOT SNOW FENCE PLACED AT THE DRIP LINE OF THE BRANCHES OR AT 8 FEET MINIMUM FROM THE TREE TRUNK. ANY EXISTING TREE OR SHRUB SHOWN TO REMAIN, WHICH IS REMOVED DURING CONSTRUCTION, SHALL BE REPLACED BY A

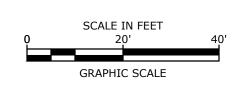
- 18. THE CONTRACTOR SHALL GUARANTEE ALL PLANTINGS TO BE IN GOOD HEALTHY, FLOURISHING AND ACCEPTABLE CONDITION FOR A PERIOD OF ONE (1) YEAR BEGINNING AT THE DATE OF ACCEPTANCE OF SUBSTANTIAL COMPLETION. ALL GRASSES, TREES AND SHRUBS THAT, IN THE OPINION OF THE LANDSCAPE ARCHITECT, SHOW LESS THAN 80% HEALTHY GROWTH AT THE END OF ONE YEAR PERIOD SHALL BE REPLACED BY THE CONTRACTOR.
- 19. UPON EXPIRATION OF THE CONTRACTOR'S ONE YEAR GUARANTEE PERIOD, THE OWNER SHALL BE RESPONSIBLE FOR LANDSCAPE MAINTENANCE INCLUDING WATERING DURING PERIODS OF DROUGHT
- 20. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING ALL PLANTING AND LAWNS AGAINST DAMAGE FROM ONGOING CONSTRUCTION. THIS PROTECTION SHALL BEGIN AT THE TIME THE PLANT IS INSTALLED AND CONTINUE UNTIL THE FORMAL ACCEPTANCE OF ALL THE PLANTINGS.
- 21. PRE-PURCHASE PLANT MATERIAL AND ARRANGE FOR DELIVERY TO MEET PROJECT SCHEDULE AS REQUIRED IT MAY BE NECESSARY TO PRE-DIG CERTAIN SPECIES WELL IN ADVANCE OF ACTUAL PLANTING DATES.
- 22. THE PROPERTY OWNER AND ALL FUTURE PROPERTY OWNERS SHALL BE RESPONSIBLE FOR THE MAINTENANCE, REPAIR AND REPLACEMENT OF ALL REQUIRED SCREENING AND LANDSCAPE MATERIALS.
- 23. ALL REQUIRED PLANT MATERIALS SHALL BE TENDED AND MAINTAINED IN A HEALTHY GROWING CONDITION, REPLACED WHEN NECESSARY, AND KEPT FREE OF REUSE AND DEBRIS. ALL REQUIRED FENCES AND WALLS SHALL BE MAINTAINED IN GOOD REPAIR.
- 24. THE PROPERTY OWNER SHALL BE RESPONSIBLE TO REMOVE AND REPLACE DEAD OR DISEASED PLANT MATERIALS IMMEDIATELY WITH THE SAME TYPE, SIZE AND QUANTITY OF PLANT MATERIALS AS ORIGINALLY INSTALLED, UNLESS ALTERNATIVE PLANTINGS ARE REQUESTED, JUSTIFIED AND APPROVED BY THE PLANNING BOARD OR PLANNING DIRECTOR.











## |Proposed **Medical Office** Building

DAR Real Estate,

185 Cottage Street Portsmouth, New Hampshire

C	10/9/2018	PB Submission	
В	9/18/2018	Revised TAC Submission	
Α	8/20/2018	TAC Submission	
MARK	DATE	DESCRIPTION	
PROJECT NO: D5108-001			
DATE:	: 8/20/2018		
FILE.	D5018-001 DSGN DWG		

CML/PMC

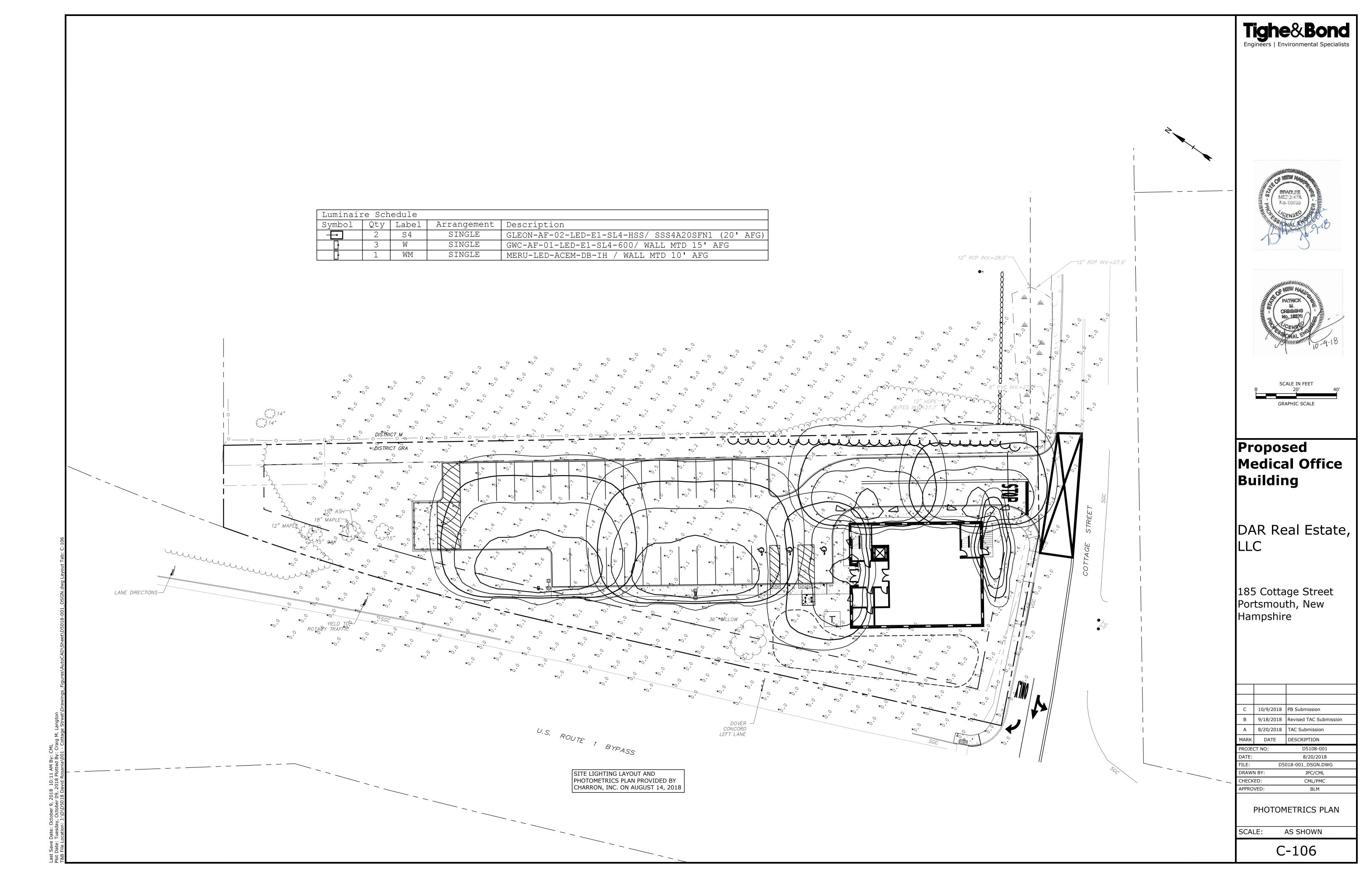
DRAWN BY: CHECKED:

SCALE:

C-105

LANDSCAPE PLAN

AS SHOWN



416 SADDLEBACK DRIVE FAIRVIEW, TX 75069

PROJECT NAME: COTTAGE STREET PROJECT ADDRESS: 185 COTTAGE STREET PORTSMOUTH, NH 03801 PROJECT MAP / LOT: 174 / LOT 14

PROJECT LATITUDE: 43°-04'-13.4"N PROJECT LONGITUDE: 70°-46'-42.7"W

## PROJECT DESCRIPTION

THE PROJECT CONSISTS OF THE DEMOLITION TWO (2) EXISTING RESIDENTIAL BUILDINGS, RE-GRADING OF SITE, CONSTRUCTION OF MEDICAL OFFICE BUILDING, PARKING LOT AND RELEVANT UTILITIES. THE WORK IS ANTICIPATED TO START IN FALL 2018, AND BE COMPLETED BY FALL 2019.

#### DISTURBED AREA THE TOTAL AREA TO BE DISTURBED IS APPROXIMATELY 0.91 ACRES.

#### SOIL CHARACTERISTICS

BASED ON THE NRCS WEB SOIL SURVEY FOR ROCKINGHAM COUNTY THE SOILS ON SITE CONSIST OF URBAN LAND-CANTON COMPLEX SOILS WHICH ARE WELL DRAINED SOILS WITH HYDROLOGIC SOIL GROUP RATINGS OF C.

#### NAME OF RECEIVING WATERS

THE STORMWATER RUNOFF FROM THE SITE WILL BE DISCHARGED VIA OVERLAND FLOW TO AN UNNAMED WETLAND AND ULTIMATELY FLOWS TO NORTH MILL POND.

### CONSTRUCTION SEQUENCE OF MAJOR ACTIVITIES:

### 1. CUT AND CLEAR TREES.

- 2. CONSTRUCT TEMPORARY AND PERMANENT SEDIMENT, EROSION AND DETENTION CONTROL FACILITIES. EROSION, SEDIMENT AND DETENTION MEASURES SHALL BE INSTALLED PRIOR TO ANY EARTH MOVING OPERATIONS THAT WILL INFLUENCE STORMWATER RUNOFF SUCH AS:
  - NEW CONSTRUCTION
  - DEVELOPMENT OF BORROW PIT AREAS
  - DISPOSAL OF SEDIMENT SPOIL, STUMP AND OTHER SOLID WASTE
  - FLOOD PLAIN EXCAVATION WORK
  - STREAM CHANNEL MODIFICATIONS
  - CONTROL OF DUST
  - CONSTRUCTION OF ACCESS AND HAUL ROAD
  - NEARNESS OF CONSTRUCTION SITE TO RECEIVING WATERS CONSTRUCTION DURING LATE WINTER AND EARLY SPRING
- 3. ALL PERMANENT DITCHES, SWALES, DETENTION, RETENTION AND SEDIMENTATION BASINS TO BE STABILIZED USING THE VEGETATIVE AND NON-STRUCTURAL BMPS PRIOR TO DIRECTING RUNOFF TO THEM.
- CLEAR AND DISPOSE OF DEBRIS.
- CONSTRUCT TEMPORARY CULVERTS AND DIVERSION CHANNELS AS REQUIRED.
- GRADE AND GRAVEL ROADWAYS AND PARKING AREAS ALL ROADS AND PARKING AREA **VEGETATION:** SHALL BE STABILIZED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE.
- BEGIN PERMANENT AND TEMPORARY SEEDING AND MULCHING. ALL CUT AND FILL SLOPES SHALL BE SEEDED AND MULCHED WITHIN 72 HOURS OF ACHIEVING FINISHED
- DAILY, OR AS REQUIRED, CONSTRUCT TEMPORARY BERMS, DRAINS, DITCHES, PERIMETER EROSION CONTROL MEASURES, SEDIMENT TRAPS, ETC., MULCH AND SEED
- 9. SEDIMENT TRAPS AND/OR BASINS SHALL BE USED AS NECESSARY TO CONTAIN RUNOFF UNTIL SOILS ARE STABILIZED.
- 10. FINISH PAVING ALL ROADWAYS AND PARKING LOTS.
- 11. INSPECT AND MAINTAIN ALL EROSION AND SEDIMENT CONTROL MEASURES. 12. COMPLETE PERMANENT SEEDING AND LANDSCAPING.
- 13. REMOVE TRAPPED SEDIMENTS FROM COLLECTOR DEVICES AS APPROPRIATE AND THEN REMOVE TEMPORARY EROSION CONTROL MEASURES.

DURING CONSTRUCTION" PREPARED BY THE NHDES

## **SPECIAL CONSTRUCTION NOTES:**

- 1. THE CONSTRUCTION SEQUENCE MUST LIMIT THE DURATION AND AREA OF DISTURBANCE
- 2. THE PROJECT IS TO BE MANAGED IN A MANNER THAT MEETS THE REQUIREMENTS AND INTENT OF RSA 430:53 AND CHAPTER AGR 3800 RELATIVE TO INVASIVE SPECIES. LOT DISTURBANCE, OTHER THAN THAT SHOWN ON THE APPROVED PLANS, SHALL NOT
- COMMENCE UNTIL AFTER THE ROADWAY HAS THE BASE COURSE TO DESIGN ELEVATION AND THE ASSOCIATED DRAINAGE IS COMPLETE AND STABLE. - THIS NOTE IS APPLICABLE TO SINGLE/DUPLEX FAMILY SUBDIVISIONS, WHEN LOT DEVELOPMENT IS NOT PART OF THE PERMIT.

## **EROSION CONTROL NOTES:**

- ALL EROSION CONTROL MEASURES AND PRACTICES SHALL CONFORM TO THE "NEW HAMPSHIRE STORMWATER MANUAL VOLUME 3: EROSION AND SEDIMENT CONTROLS
- PRIOR TO ANY WORK OR SOIL DISTURBANCE, CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR EROSION CONTROL MEASURES AS REQUIRED IN THE PROJECT MANUAL
- 3. CONTRACTOR SHALL INSTALL TEMPORARY EROSION CONTROL BARRIERS, INCLUDING HAY BALES, SILT FENCES, MULCH BERMS, SILT SACKS AND SILT SOCKS AS SHOWN IN THESE DRAWINGS AS THE FIRST ORDER OF WORK.
- 4. SILT SACK INLET PROTECTION SHALL BE INSTALLED IN ALL EXISTING AND PROPOSED CATCH BASIN INLETS WITHIN THE WORK LIMITS AND BE MAINTAINED FOR THE DURATION OF THE PROJECT.
- 5. PERIMETER CONTROLS INCLUDING SILT FENCES, MULCH BERM, SILT SOCK, AND/OR HAY BALE BARRIERS SHALL BE MAINTAINED FOR THE DURATION OF THE PROJECT UNTIL NON-PAVED AREAS HAVE BEEN STABILIZED.
- 6. THE CONTRACTOR SHALL REMOVE AND PROPERLY DISPOSE OF ALL TEMPORARY EROSION CONTROL DEVICES UPON COMPLETION OF CONSTRUCTION.
- 7. ALL DISTURBED AREAS NOT OTHERWISE BEING TREATED SHALL RECEIVE 6" LOAM, SEED AND FERTILIZER.
- 8. INSPECT ALL INLET PROTECTION AND PERIMETER CONTROLS WEEKLY AND AFTER EACH RAIN STORM OF 0.25 INCH OR GREATER. REPAIR/MODIFY PROTECTION AS NECESSARY TO MAXIMIZE EFFICIENCY OF FILTER. REPLACE ALL FILTERS WHEN SEDIMENT IS 1/3 THE FILTER HEIGHT.
- 9. CONSTRUCT EROSION CONTROL BLANKETS ON ALL SLOPES STEEPER THAN 3:1.

## STABILIZATION:

- OCCURRED:
- 1. AN AREA SHALL BE CONSIDERED STABLE WHEN ONE OF THE FOLLOWING HAS
  - A. BASE COURSE GRAVELS HAVE BEEN INSTALLED IN AREAS TO BE PAVED; B. A MINIMUM OF 85% VEGETATED GROWTH HAS BEEN ESTABLISHED;
  - C. A MINIMUM OF 3" OF NON-EROSIVE MATERIAL SUCH AS STONE OR RIPRAP HAS BEEN INSTALLED;
- EROSION CONTROL BLANKETS HAVE BEEN PROPERLY INSTALLED.;
- IN AREAS TO BE PAVED, "STABLE" MEANS THAT BASE COURSE GRAVELS MEETING THE REQUIREMENTS OF NHDOT STANDARD FOR ROAD AND BRIDGE CONSTRUCTION, 2016, ITEM 304.2 HAVE BEEN INSTALLED.
- WINTER STABILIZATION PRACTICES:
- A. ALL PROPOSED VEGETATED AREAS THAT DO NOT EXHIBIT A MINIMUM OF 85 PERCENT VEGETATIVE GROWTH BY OCTOBER 15, OR WHICH ARE DISTURBED AFTER OCTOBER 15, SHALL BE STABILIZED BY SEEDING AND INSTALLING EROSION CONTROL BLANKETS ON SLOPES GREATER THAN 3:1, AND SEEDING AND PLACING 3 TO 4 TONS OF MULCH PER ACRE, SECURED WITH ANCHORED NETTING, ELSEWHERE. THE INSTALLATION OF EROSION CONTROL BLANKETS OR MULCH AND NETTING SHALL NOT OCCUR OVER ACCUMULATED SNOW OR ON FROZEN GROUND AND SHALL BE COMPLETED IN ADVANCE OF THAW OR SPRING MELT EVENTS;
- B. ALL DITCHES OR SWALES WHICH DO NOT EXHIBIT A MINIMUM OF 85 PERCENT VEGETATIVE GROWTH BY OCTOBER 15, OR WHICH ARE DISTURBED AFTER OCTOBER 15, SHALL BE STABILIZED TEMPORARILY WITH STONE OR EROSION CONTROL BLANKETS APPROPRIATE FOR THE DESIGN FLOW CONDITIONS;
- C. AFTER OCTOBER 15, INCOMPLETE ROAD OR PARKING SURFACES, WHERE WORK HAS STOPPED FOR THE WINTER SEASON, SHALL BE PROTECTED WITH A MINIMUM OF 3 INCHES OF CRUSHED GRAVEL PER NHDOT ITEM 304.3, OR IF CONSTRUCTION IS TO CONTINUE THROUGH THE WINTER SEASON BE CLEARED OF ANY ACCUMULATED SNOW AFTER EACH STORM EVENT;

- 3. STABILIZATION SHALL BE INITIATED ON ALL LOAM STOCKPILES, AND DISTURBED AREAS, WHERE CONSTRUCTION ACTIVITY SHALL NOT OCCUR FOR MORE THAN TWENTY-ONE (21) CALENDAR DAYS BY THE FOURTEENTH (14TH) DAY AFTER CONSTRUCTION ACTIVITY HAS PERMANENTLY OR TEMPORARILY CEASED IN THAT AREA. STABILIZATION MEASURES TO BE USED INCLUDE:
- 3.1. TEMPORARY SEEDING;
- 3.2. MULCHING 4. ALL AREAS SHALL BE STABILIZED WITHIN 45 DAYS OF INITIAL DISTURBANCE.
- WHEN CONSTRUCTION ACTIVITY PERMANENTLY OR TEMPORARILY CEASES WITHIN 100 FEET OF NEARBY SURFACE WATERS OR DELINEATED WETLANDS, THE AREA SHALL BE STABILIZED WITHIN SEVEN (7) DAYS OR PRIOR TO A RAIN EVENT. ONCE CONSTRUCTION ACTIVITY CEASES PERMANENTLY IN AN THESE AREAS, SILT FENCES, MULCH BERMS, HAY BALE BARRIERS AND ANY EARTH/DIKES SHALL BE REMOVED ONCE PERMANENT MEASURES ARE ESTABLISHED.
- 6. DURING CONSTRUCTION, RUNOFF WILL BE DIVERTED AROUND THE SITE WITH EARTH DIKES, PIPING OR STABILIZED CHANNELS WHERE POSSIBLE. SHEET RUNOFF FROM THE SITE WILL BE FILTERED THROUGH SILT FENCES, MULCH BERMS, HAY BALE BARRIERS, OR SILT SOCKS. ALL STORM DRAIN BASIN INLETS SHALL BE PROVIDED WITH FLARED END SECTIONS AND TRASH RACKS. THE SITE SHALL BE STABILIZED FOR THE WINTER BY OCTOBER 15.

#### **DUST CONTROL:**

- 1. THE CONTRACTOR SHALL BE RESPONSIBLE TO CONTROL DUST THROUGHOUT THE
- DUST CONTROL METHODS SHALL INCLUDE, BUT BE NOT LIMITED TO SPRINKLING WATER ON EXPOSED AREAS, COVERING LOADED DUMP TRUCKS LEAVING THE SITE, AND
- DUST CONTROL MEASURES SHALL BE UTILIZED SO AS TO PREVENT THE MIGRATION OF DUST FROM THE SITE TO ABUTTING AREAS.
- 1. LOCATE STOCKPILES A MINIMUM OF 50 FEET AWAY FROM CATCH BASINS, SWALES, AND CULVERTS.
- 2. ALL STOCKPILES SHOULD BE SURROUNDED WITH TEMPORARY EROSION CONTROL MEASURES PRIOR TO THE ONSET OF PRECIPITATION.
- 3. PERIMETER BARRIERS SHOULD BE MAINTAINED AT ALL TIMES, AND ADJUSTED AS NEEDED TO ACCOMMODATE THE DELIVERY AND REMOVAL OF MATERIALS FROM THE STOCKPILE. THE INTEGRITY OF THE BARRIER SHOULD BE INSPECTED AT THE END OF EACH WORKING DAY.
- 4. PROTECT ALL STOCKPILES FROM STORMWATER RUN-OFF USING TEMPORARY EROSION CONTROL MEASURES SUCH AS BERMS, SILT SOCK, OR OTHER APPROVED PRACTICE TO PREVENT MIGRATION OF MATERIAL BEYOND THE IMMEDIATE CONFINES OF THE

#### **OFF SITE VEHICLE TRACKING:**

**STOCKPILES** 

THE CONTRACTOR SHALL CONSTRUCT STABILIZED CONSTRUCTION ENTRANCE(S) PRIOR TO ANY EXCAVATION ACTIVITIES.

## TEMPORARY GRASS COVER:

- A. SEEDBED PREPARATION:
  - APPLY FERTILIZER AT THE RATE OF 600 POUNDS PER ACRE OF 10-10-10. APPLY LIMESTONE (EQUIVALENT TO 50 PERCENT CALCIUM PLUS MAGNESIUM OXIDE) AT A RATE OF THREE (3) TONS PER ACRE;
- A. UTILIZE ANNUAL RYE GRASS AT A RATE OF 40 LBS/ACRE;
- B. WHERE THE SOIL HAS BEEN COMPACTED BY CONSTRUCTION OPERATIONS, LOOSEN SOIL TO A DEPTH OF TWO (2) INCHES BEFORE APPLYING FERTILIZER, LIME AND
- C. APPLY SEED UNIFORMLY BY HAND, CYCLONE SEEDER, OR HYDROSEEDER (SLURRY INCLUDING SEED AND FERTILIZER). HYDROSEEDINGS, WHICH INCLUDE MULCH, MAY BE LEFT ON SOIL SURFACE. SEEDING RATES MUST BE INCREASED 10% WHEN HYDROSEEDING;
- MAINTENANCE:
- A. TEMPORARY SEEDING SHALL BE PERIODICALLY INSPECTED. AT A MINIMUM, 95% OF THE SOIL SURFACE SHOULD BE COVERED BY VEGETATION. IF ANY EVIDENCE OF EROSION OR SEDIMENTATION IS APPARENT, REPAIRS SHALL BE MADE AND OTHER TEMPORARY MEASURES USED IN THE INTERIM (MULCH, FILTER BARRIERS, CHECK DAMS, ETC.).
- 4. VEGETATIVE PRACTICE:
- A. FOR PERMANENT MEASURES AND PLANTINGS: B. LIMESTONE SHALL BE THOROUGHLY INCORPORATED INTO THE LOAM LAYER AT A RATE OF THREE (3) TONS PER ACRE IN ORDER TO PROVIDE A PH VALUE OF 5.5 TO
- C. FERTILIZER SHALL BE SPREAD ON THE TOP LAYER OF LOAM AND WORKED INTO THE SURFACE. FERTILIZER APPLICATION RATE SHALL BE 800 POUNDS PER ACRE OF
- 10-20-20 FERTILIZER; D. SOIL CONDITIONERS AND FERTILIZER SHALL BE APPLIED AT THE RECOMMENDED RATES AND SHALL BE THOROUGHLY WORKED INTO THE LOAM. LOAM SHALL BE RAKED UNTIL THE SURFACE IS FINELY PULVERIZED, SMOOTH AND EVEN, AND THEN COMPACTED TO AN EVEN SURFACE CONFORMING TO THE REQUIRED LINES AND GRADES WITH APPROVED ROLLERS WEIGHING BETWEEN 4-1/2 POUNDS AND 5-1/2
- POUNDS PER INCH OF WIDTH; E. SEED SHALL BE SOWN AT THE RATE SHOWN BELOW. SOWING SHALL BE DONE ON A CALM, DRY DAY, PREFERABLY BY MACHINE, BUT IF BY HAND, ONLY BY EXPERIENCED WORKMEN. IMMEDIATELY BEFORE SEEDING, THE SOIL SHALL BE LIGHTLY RAKED. ONE HALF THE SEED SHALL BE SOWN IN ONE DIRECTION AND THE OTHER HALF AT RIGHT ANGLES TO THE ORIGINAL DIRECTION. IT SHALL BE LIGHTLY RAKED INTO THE SOIL TO A DEPTH NOT OVER 1/4 INCH AND ROLLED WITH A HAND ROLLER WEIGHING NOT OVER 100 POUNDS PER LINEAR FOOT OF WIDTH;
- F. HAY MULCH SHALL BE APPLIED IMMEDIATELY AFTER SEEDING AS INDICATED G. THE SURFACE SHALL BE WATERED AND KEPT MOIST WITH A FINE SPRAY AS REQUIRED, WITHOUT WASHING AWAY THE SOIL, UNTIL THE GRASS IS WELL ESTABLISHED. ANY AREAS WHICH ARE NOT SATISFACTORILY COVERED WITH

GRASS SHALL BE RESEEDED, AND ALL NOXIOUS WEEDS REMOVED;

- H. THE CONTRACTOR SHALL PROTECT AND MAINTAIN THE SEEDED AREAS UNTIL
- A GRASS SEED MIXTURE CONTAINING THE FOLLOWING SEED REQUIREMENTS SHALL BE APPLIED AT THE INDICATED RATE:
  - SEED MIX APPLICATION RATE CREEPING RED FESCUE 84 LBS/ACRE TALL FESCUE 84 LBS/ACRE
- IN NO CASE SHALL THE WEED CONTENT EXCEED ONE (1) PERCENT BY WEIGHT. ALL SEED SHALL COMPLY WITH STATE AND FEDERAL SEED LAWS. SEEDING SHALL BE DONE NO LATER THAN SEPTEMBER 15. IN NO CASE SHALL SEEDING TAKE PLACE OVER SNOW.

32 LBS/ACRE

- 5. DORMANT SEEDING (SEPTEMBER 15 TO FIRST SNOWFALL):
- A. FOLLOW PERMANENT MEASURES SLOPE, LIME, FERTILIZER AND GRADING REQUIREMENTS. APPLY SEED MIXTURE AT TWICE THE INDICATED RATE. APPLY MULCH AS INDICATED FOR PERMANENT MEASURES. **CONCRETE WASHOUT AREA:** 
  - 1. THE FOLLOWING ARE THE ONLY NON-STORMWATER DISCHARGES ALLOWED. ALL OTHER NON-STORMWATER DISCHARGES ARE PROHIBITED ON SITE:
  - A. THE CONCRETE DELIVERY TRUCKS SHALL, WHENEVER POSSIBLE, USE WASHOUT FACILITIES AT THEIR OWN PLANT OR DISPATCH FACILITY; B. IF IT IS NECESSARY, SITE CONTRACTOR SHALL DESIGNATE SPECIFIC WASHOUT
  - AREAS AND DESIGN FACILITIES TO HANDLE ANTICIPATED WASHOUT WATER; C. CONTRACTOR SHALL LOCATE WASHOUT AREAS AT LEAST 150 FEET AWAY FROM STORM DRAINS, SWALES AND SURFACE WATERS OR DELINEATED WETLANDS;
  - D. INSPECT WASHOUT FACILITIES DAILY TO DETECT LEAKS OR TEARS AND TO IDENTIFY WHEN MATERIALS NEED TO BE REMOVED.
  - FIRE-FIGHTING ACTIVITIES; 2. FIRE HYDRANT FLUSHING;

**ALLOWABLE NON-STORMWATER DISCHARGES:** 

REDTOP

- WATERS USED TO WASH VEHICLES WHERE DETERGENTS ARE NOT USED;
- WATER USED TO CONTROL DUST;
- POTABLE WATER INCLUDING UNCONTAMINATED WATER LINE FLUSHING;
  - ROUTINE EXTERNAL BUILDING WASH DOWN WHERE DETERGENTS ARE NOT USED;

  - PAVEMENT WASH WATERS WHERE DETERGENTS ARE NOT USED;
  - UNCONTAMINATED AIR CONDITIONING/COMPRESSOR CONDENSATION;
- 9. UNCONTAMINATED GROUND WATER OR SPRING WATER;
- 10. FOUNDATION OR FOOTING DRAINS WHICH ARE UNCONTAMINATED; 11. UNCONTAMINATED EXCAVATION DEWATERING;
- 12. LANDSCAPE IRRIGATION.

## **WASTE DISPOSAL:**

- A. ALL WASTE MATERIALS SHALL BE COLLECTED AND STORED IN SECURELY LIDDED RECEPTACLES. ALL TRASH AND CONSTRUCTION DEBRIS FROM THE SITE SHALL BE DEPOSITED IN A DUMPSTER;
- B. NO CONSTRUCTION WASTE MATERIALS SHALL BE BURIED ON SITE; ALL PERSONNEL SHALL BE INSTRUCTED REGARDING THE CORRECT PROCEDURE FOR
- WASTE DISPOSAL BY THE SUPERINTENDENT. HAZARDOUS WASTE:
- A. ALL HAZARDOUS WASTE MATERIALS SHALL BE DISPOSED OF IN THE MANNER SPECIFIED BY LOCAL OR STATE REGULATION OR BY THE MANUFACTURER;
- B. SITE PERSONNEL SHALL BE INSTRUCTED IN THESE PRACTICES BY THE
- SUPERINTENDENT. SANITARY WASTE:
- A. ALL SANITARY WASTE SHALL BE COLLECTED FROM THE PORTABLE UNITS A MINIMUM OF ONCE PER WEEK BY A LICENSED SANITARY WASTE MANAGEMENT CONTRACTOR.
- 1. CONTRACTOR SHALL BE FAMILIAR WITH SPILL PREVENTION MEASURES REQUIRED BY LOCAL, STATE AND FEDERAL AGENCIES. AT A MINIMUM, CONTRACTOR SHALL FOLLOW THE BEST MANAGEMENT SPILL PREVENTION PRACTICES OUTLINED BELOW
- 2. THE FOLLOWING ARE THE MATERIAL MANAGEMENT PRACTICES THAT SHALL BE USED TO REDUCE THE RISK OF SPILLS OR OTHER ACCIDENTAL EXPOSURE OF MATERIALS AND SUBSTANCES DURING CONSTRUCTION TO STORMWATER RUNOFF:

A. GOOD HOUSEKEEPING - THE FOLLOWING GOOD HOUSEKEEPING PRACTICE SHALL

- BE FOLLOWED ON SITE DURING CONSTRUCTION: ONLY SUFFICIENT AMOUNTS OF PRODUCTS TO DO THE JOB SHALL BE STORED
- ALL REGULATED MATERIALS STORED ON SITE SHALL BE STORED IN A NEAT, ORDERLY MANNER IN THEIR PROPER (ORIGINAL IF POSSIBLE) CONTAINERS
- AND, IF POSSIBLE, UNDER A ROOF OR OTHER ENCLOSURE, ON AN IMPERVIOUS MANUFACTURER'S RECOMMENDATIONS FOR PROPER USE AND DISPOSAL SHALL
- THE SITE SUPERINTENDENT SHALL INSPECT DAILY TO ENSURE PROPER USE
- AND DISPOSAL OF MATERIALS; • SUBSTANCES SHALL NOT BE MIXED WITH ONE ANOTHER UNLESS
- RECOMMENDED BY THE MANUFACTURER;
- WHENEVER POSSIBLE ALL OF A PRODUCT SHALL BE USED UP BEFORE DISPOSING OF THE CONTAINER.
- THE TRAINING OF ON-SITE EMPLOYEES AND THE ON-SITE POSTING OF RELEASE RESPONSE INFORMATION DESCRIBING WHAT TO DO IN THE EVENT OF A SPILL OF REGULATED SUBSTANCES.
- HAZARDOUS PRODUCTS THE FOLLOWING PRACTICES SHALL BE USED TO REDUCE THE RISKS ASSOCIATED WITH HAZARDOUS MATERIALS:
- B. PRODUCTS SHALL BE KEPT IN THEIR ORIGINAL CONTAINERS UNLESS THEY ARE NOT RESEALABLE; C. ORIGINAL LABELS AND MATERIAL SAFETY DATA SHALL BE RETAINED FOR
- IMPORTANT PRODUCT INFORMATION; D. SURPLUS PRODUCT THAT MUST BE DISPOSED OF SHALL BE DISCARDED ACCORDING
- TO THE MANUFACTURER'S RECOMMENDED METHODS OF DISPOSAL PRODUCT SPECIFIC PRACTICES - THE FOLLOWING PRODUCT SPECIFIC
- PRACTICES SHALL BE FOLLOWED ON SITE: PETROLEUM PRODUCTS:
- A. ALL ON SITE VEHICLES SHALL BE MONITORED FOR LEAKS AND RECEIVE REGULAR PREVENTIVE MAINTENANCE TO REDUCE LEAKAGE; B. PETROLEUM PRODUCTS SHALL BE STORED IN TIGHTLY SEALED CONTAINERS WHICH ARE CLEARLY LABELED. ANY ASPHALT BASED SUBSTANCES USED ON SITE SHALL BE
- APPLIED ACCORDING TO THE MANUFACTURER'S RECOMMENDATIONS. SECURE FUEL STORAGE AREAS AGAINST UNAUTHORIZED ENTRY;
- INSPECT FUEL STORAGE AREAS WEEKLY; E. WHEREVER POSSIBLE, KEEP REGULATED CONTAINERS THAT ARE STORED OUTSIDE MORE THAN 50 FEET FROM SURFACE WATER AND STORM DRAINS, 75 FEET FROM
- F. COVER REGULATED CONTAINERS IN OUTSIDE STORAGE AREAS; G. SECONDARY CONTAINMENT IS REQUIRED FOR CONTAINERS CONTAINING REGULATED SUBSTANCES STORED OUTSIDE, EXCEPT FOR ON PREMISE USE HEATING FUEL TANKS, OR ABOVEGROUND OR UNDERGROUND STORAGE TANKS
- OTHERWISE REGULATED.

PRIVATE WELLS, AND 400 FEET FROM PUBLIC WELLS;

- H. THE FUEL HANDLING REQUIREMENTS SHALL INCLUDE: EXCEPT WHEN IN USE, KEEP CONTAINERS CONTAINING REGULATED
  - SUBSTANCES CLOSED AND SEALED;
  - PLACE DRIP PANS UNDER SPIGOTS, VALVES, AND PUMPS; HAVE SPILL CONTROL AND CONTAINMENT EQUIPMENT READILY AVAILABLE IN ALL WORK AREAS;
- USE FUNNELS AND DRIP PANS WHEN TRANSFERRING REGULATED SUBSTANCES; PERFORM TRANSFERS OF REGULATED SUBSTANCES OVER AN IMPERVIOUS SURFACE.
- I. FUELING AND MAINTENANCE OF EXCAVATION, EARTHMOVING AND OTHER CONSTRUCTION RELATED EQUIPMENT SHALL COMPLY WITH THE REGULATIONS OF THE NEW HAMPSHIRE DEPARTMENT OF ENVIRONMENTAL SERVICES THESE REQUIREMENTS ARE SUMMARIZED IN WD-DWGB-22-6 BEST MANAGEMENT PRACTICES FOR FUELING AND MAINTENANCE OF EXCAVATION AND EARTHMOVING
- EQUIPMENT, OR ITS SUCCESSOR DOCUMENT. HTTPS://WWW.DES.NH.GOV/ORGANIZATION/COMMISSIONER/PIP/FACTSHEETS/DWGB/DOCUMENTS/DWGB-22-6.PDF
- 4. FERTILIZERS A. FERTILIZERS USED SHALL BE APPLIED ONLY IN THE MINIMUM AMOUNTS DIRECTED BY THE SPECIFICATIONS;
- B. ONCE APPLIED FERTILIZER SHALL BE WORKED INTO THE SOIL TO LIMIT EXPOSURE TO STORMWATER; C. STORAGE SHALL BE IN A COVERED SHED OR ENCLOSED TRAILERS. THE CONTENTS OF ANY PARTIALLY USED BAGS OF FERTILIZER SHALL BE TRANSFERRED TO A
- SEALABLE PLASTIC BIN TO AVOID SPILLS. PAINTS: A. ALL CONTAINERS SHALL BE TIGHTLY SEALED AND STORED WHEN NOT REQUIRED
- B. EXCESS PAINT SHALL NOT BE DISCHARGED TO THE STORM SEWER SYSTEM; C. EXCESS PAINT SHALL BE DISPOSED OF PROPERLY ACCORDING TO MANUFACTURER'S INSTRUCTIONS OR STATE AND LOCAL REGULATIONS.

SPILL CONTROL PRACTICES - IN ADDITION TO GOOD HOUSEKEEPING AND

MATERIAL MANAGEMENT PRACTICES DISCUSSED IN THE PREVIOUS SECTION,

THE FOLLOWING PRACTICES SHALL BE FOLLOWED FOR SPILL PREVENTION AND D. MANUFACTURER'S RECOMMENDED METHODS FOR SPILL CLEANUP SHALL BE CLEARLY POSTED AND SITE PERSONNEL SHALL BE MADE AWARE OF THE

PROCEDURES AND THE LOCATION OF THE INFORMATION AND CLEANUP SUPPLIES;

- E. MATERIALS AND EQUIPMENT NECESSARY FOR SPILL CLEANUP SHALL BE KEPT IN THE MATERIAL STORAGE AREA ON SITE. EQUIPMENT AND MATERIALS SHALL INCLUDE BUT NOT BE LIMITED TO BROOMS, DUSTPANS, MOPS, RAGS, GLOVES, GOGGLES, KITTY LITTER, SAND, SAWDUST AND PLASTIC OR METAL TRASH CONTAINERS SPECIFICALLY FOR THIS PURPOSE;
- F. ALL SPILLS SHALL BE CLEANED UP IMMEDIATELY AFTER DISCOVERY;

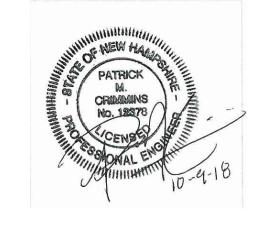
- G. THE SPILL AREA SHALL BE KEPT WELL VENTILATED AND PERSONNEL SHALL WEAR APPROPRIATE PROTECTIVE CLOTHING TO PREVENT INJURY FROM CONTACT WITH A HAZARDOUS SUBSTANCE;
- H. SPILLS OF TOXIC OR HAZARDOUS MATERIAL SHALL BE REPORTED TO THE APPROPRIATE LOCAL, STATE OR FEDERAL AGENCIES AS REQUIRED;
- THE SITE SUPERINTENDENT RESPONSIBLE FOR DAY-TO-DAY SITE OPERATIONS SHALL BE THE SPILL PREVENTION AND CLEANUP COORDINATOR.
- VEHICLE FUELING AND MAINTENANCE PRACTICE: CONTRACTOR SHALL MAKE AN EFFORT TO PERFORM EQUIPTMENT/VEHICAL FUELING
- AND MAINTENANCE AT AN OFF-SITE FACILITY;
- K. CONTRACTOR SHALL PROVIDE AN ON-SITE FUELING AND MAINTENANCE AREA THAT
- IS CLEAN AND DRY;
- L. IF POSSIBLE THE CONTRACTOR SHALL KEEP AREA COVERED;
- M. CONTRACTOR SHALL KEEP A SPILL KIT AT THE FUELING AND MAINTENANCE AREA; CONTRACTOR SHALL REGULARLY INSPECT VEHICLES FOR LEAKS AND DAMAGE; O. CONTRACTOR SHALL USE DRIP PANS, DRIP CLOTHS, OR ABSORBENT PADS WHEN
- REPLACING SPENT FLUID. **EROSION CONTROL OBSERVATIONS AND MAINTENANCE PRACTICES**
- 1. THIS PROJECT DOES NOT EXCEED ONE (1) ACRE OF DISTURBANCE AND THUS DOES NOT REQUIRE A SWPPP.THE FOLLOWING REPRESENTS THE GENERAL OBSERVATION AND

REPORTING PRACTICES THAT SHALL BE FOLLOWED AS PART OF THIS PROJECT

- A. OBSERVATIONS OF THE PROJECT FOR COMPLIANCE WITH THE SWPPP SHALL BE MADE BY THE CONTRACTOR AT LEAST ONCE A WEEK OR WITHIN 24 HOURS OF A
- STORM 0.25 INCHES OR GREATER; B. AN OBSERVATION REPORT SHALL BE MADE AFTER EACH OBSERVATION AND
- DISTRIBUTED TO THE ENGINEER, THE OWNER, AND THE CONTRACTOR; C. A REPRESENTATIVE OF THE SITE CONTRACTOR, SHALL BE RESPONSIBLE FOR
- MAINTENANCE AND REPAIR ACTIVITIES; D. IF A REPAIR IS NECESSARY, IT SHALL BE INITIATED WITHIN 24 HOURS OF REPORT.







## Proposed |Medical Office

DAR Real Estate

185 Cottage Street Portsmouth, New Hampshire

	10/9/2018	PB Submission	
3	9/18/2018	Revised TAC Submission	
4	8/20/2018	TAC Submission	
λRK	DATE	DESCRIPTION	
OJECT NO: D5108-001		D5108-001	
TE:		8/20/2018	
E:	D5	018-001_DTLS.DWG	
AWN BY:		JPC/CML	
ECKED:		CML/PMC	

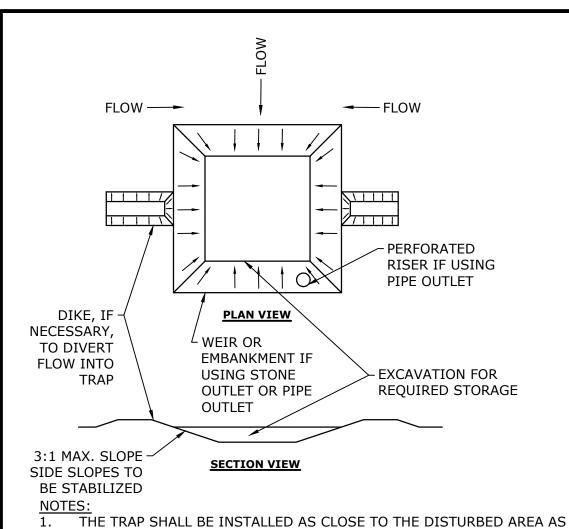
**EROSION CONTROL NOTES** SHEETS

AS SHOWN

APPROVED:

SCALE:

C-501

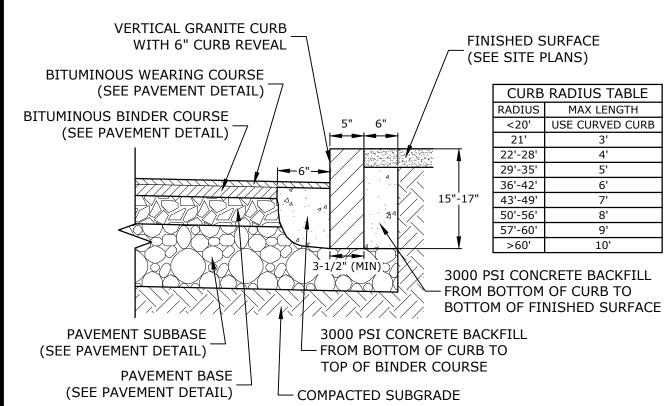


- POSSIBLE
- THE MAXIMUM CONTRIBUTING AREA TO A SINGLE TRAP SHALL BE LESS THAN 5 ACRES.
- THE MINIMUM VOLUME OF THE TRAP SHALL BE 3,600 CUBIC FEET OF
- STORAGE FOR EACH ACRE OF DRAINAGE AREA.
- TRAP SHALL DISCHARGE TO A STABILIZED AREA.

TRAP OUTLET SHALL BE MINIMUM OF ONE FOOT BELOW THE CREST OF THE

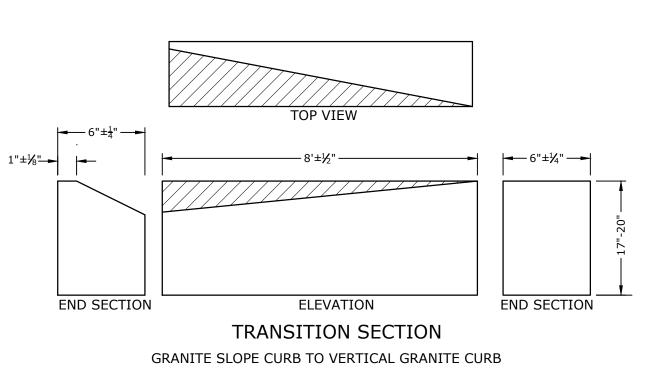
- TRAP SHALL BE CLEANED WHEN 50 PERCENT OF THE ORIGINAL VOLUME IS
- MATERIALS REMOVED FROM THE TRAP SHALL BE PROPERLY DISPOSED OF
- SEDIMENT TRAPS MUST BE USED AS NEEDED TO CONTAIN RUNOFF UNTIL SOILS ARE STABILIZED.

## SEDIMENT TRAP



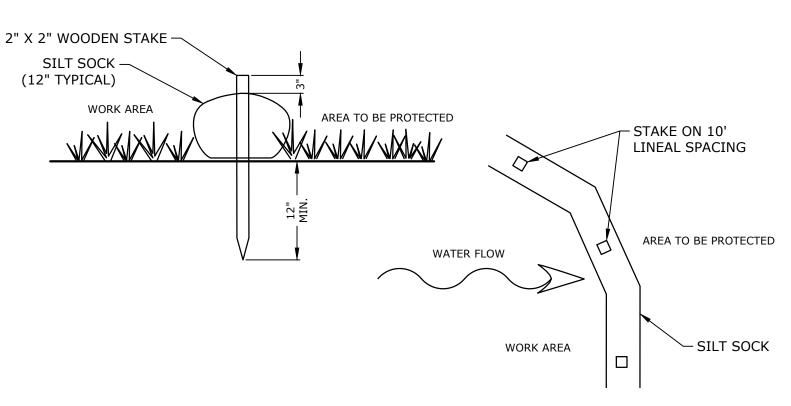
- 1. SEE SITE PLAN(S) FOR LIMITS OF VERTICAL GRANITE CURB (VGC).
- 2. ADJOINING STONES SHALL HAVE THE SAME OR APPROXIMATELY THE SAME LENGTH. 3. MINIMUM LENGTH OF STRAIGHT CURB STONES = 3'
- 4. MAXIMUM LENGTH OF STRAIGHT CURB STONES = 10'
- 5. MAXIMUM LENGTH OF STRAIGHT CURB STONES LAID ON CURVES (SEE TABLE).
- 6. ALL RADII 20 FEET AND SMALLER SHALL BE CONSTRUCTED USING CURVED SECTIONS
- 7. JOINTS BETWEEN STONES SHALL HAVE A MAXIMUM SPACING OF 1/2" AND SHALL BE MORTARED.

## VERTICAL GRANITE CURB NO SCALE

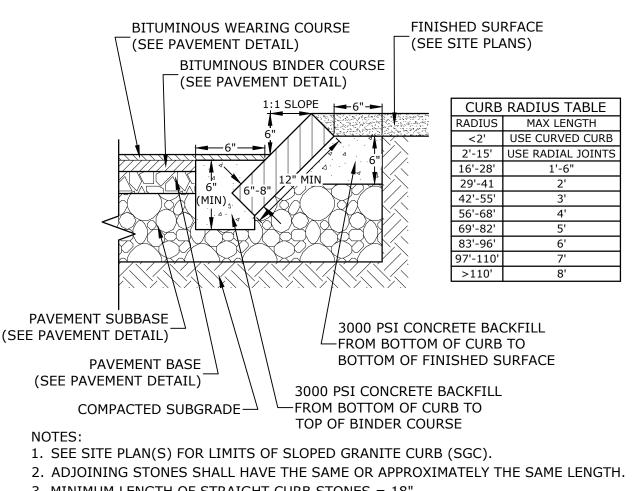


**CURB TRANSITION** 

NO SCALE



- 1. SILT SOCK SHALL BE SILT SOXX BY FILTREXX OR APPROVED EQUAL. 2. SILT SOCK SHALL BE FILLED WITH FILTERMEDIA BY FILTREXX OR APPROVED EQUAL.
- 3. WHERE TWO SILT SOCKS ARE JOINED, A MINIMUM OF 2 FEET OF OVERLAP SHALL BE MAINTAINED.
- 4. SILT SOCKS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS
- 5. CONTRACTOR TO INSTALL SILT SOCK IN J-HOOK OR SMILE CONFIGURATION TO LIMIT CONCENTRATION OF STORMWATER RUNOFF AT A SINGLE DISHCARGE POINT



3. MINIMUM LENGTH OF STRAIGHT CURB STONES = 18"

VERTICAL CURB (SEE DETAIL)

COMPRESSIVE STRENGTH OF 4000

P.S.I. AIR ENTRAINED WITH FIBER

TOOLED

1. SIDEWALK SHALL BE CONSTRUCTED TO CITY

**CONTROL JOINT A** 

OF PORTSMOUTH STANDARDS

4" CONCRETE WALK, 28 DAY

-1/8"x1" DEEP HAND

JOINT WITH 1/4"

REINFORCEMENT-

- 4. MAXIMUM LENGTH OF STRAIGHT CURB STONES = 8'
- 5. MAXIMUM LENGTH OF STRAIGHT CURB STONES LAID ON CURVES (SEE TABLE).
- 6. JOINTS BETWEEN STONES SHALL HAVE A MAXIMUM SPACING OF 1/2" AND SHALL BE MORTARED.

VARIES (SEE PLANS)

SMOOTH

TROWEL MED.

BROOM FINISH-

← 2% (MAX)

**SECTION** 

**EXPANSION JOINT B** 

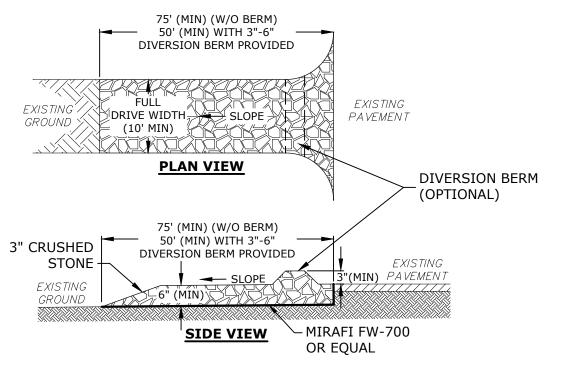
-1/4" RADIUS

-1/4" TO 1/2"

PREMOLDED

FILLER

## SLOPED GRANITE CURB NO SCALE



−6" LOAM

-COMPACTED **SUBGRADE** 

1/4" RADII-

9" COMPACTED

(NHDOT 304.3)

CRUSHED GRAVEL

AND SEED

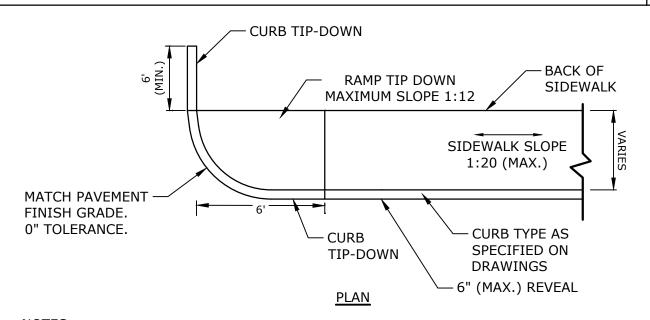
ISOLATION JOINT C

TYPICAL CONCRETE SIDEWALK WITH GRANITE CURB

NO SCALE

- THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OF SEDIMENT FROM THE SITE. WHEN WASHINGS IS REQUIRED, IT SHALL BE DONE SO RUNOFF DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE. ALL SEDIMENT SHALL BE PREVENTED FROM ENTERING STORM DRAINS, DITCHES, OR WATERWAYS.
- ONLY CONSTRUCTION TRAFFIC LEAVING THE SITE IS REQUIRED TO USE THE TEMPORARY
- CONTRACTOR MAY PROVIDE A SEPARATE, UNPROTECTED, ENTRANCE FOR TRAFFIC ENTERING THE SITE PROVIDED THAT IT IS SIGNED AS AN ENTRANCE ONLY.
- LOCATE CONSTRUCTION ENTRANCES AND EXITS TO LIMIT SEDIMENT LEAVING THE SITE AND TO
- PROVIDE FOR MAXIMUM UTILITY BY ALL CONSTRUCTION VEHICLES. ENTRANCES SHALL NOT BE LOCATED IN AREAS WITH STEEP GRADES OR AT CURVES IN PUBLIC
- THE ENTRANCE SHOULD BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR
- FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF WAY. a. WHEN THE CONTROL PAD BECOMES INEFFECTIVE, THE STONE SHOULD BE REMOVED ALONG
- WITH THE COLLECTED SOIL MATERIAL, REGRADED ON SITE, AND STABILIZED PRIOR TO RECONSTRUCTING THE EXIT.
- b. THE CONTRACTOR SHOULD SWEEP THE PAVEMENT AT EXITS WHENEVER SOIL MATERIALS ARE TRACKED ONTO THE ADJACENT PAVEMENT OR TRAVELED WAY.
- . WHEN WHEEL WASHING IS REQUIRED, IT SHOULD BE CONDUCTED ON AN AREA STABILIZED WITH AGGREGATE, WHICH DRAINS INTO AN APPROVED SEDIMENT-TRAPPING DEVICE.
- d. ALL SEDIMENT SHOULD BE PREVENTED FROM ENTERING STORM DRAINS, DITCHES, OR WATERWAYS TO THE GREATEST EXTENT PRACTICAL
- NATURAL DRAINAGE THAT CROSSES THE LOCATION OF THE STONE PAD SHOULD BE INTERCEPTED AND PIPED BENEATH THE PAD, AS NECESSARY, WITH SUITABLE OUTLET PROTECTION.

#### STABILIZED CONSTRUCTION EXIT NO SCALE



1. RAMPS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE AMERICANS WITH DISABILITIES ACT AND LOCAL AND STATE REQUIREMENTS

2. PROVIDE 6" COMPACTED CRUSHED GRAVEL BASE BENEATH RAMPS. 3. DETECTABLE WARNING STRIP SHALL BE ADA SOLUTIONS, INC. CAST IN PLACE RAMP. INSTALL PER MANUFACTURER'S RECOMMENDATIONS.

## CONCRETE SIDEWALK TIP-DOWN RAMP NO SCALE

<u>PLAN</u>

-1/8"x1" DEEP

RADII

**CONSTRUCTION JOINT** 

HAND TOOLED

JOINT WITH 1/4"

-#6 REBAR @ 12"

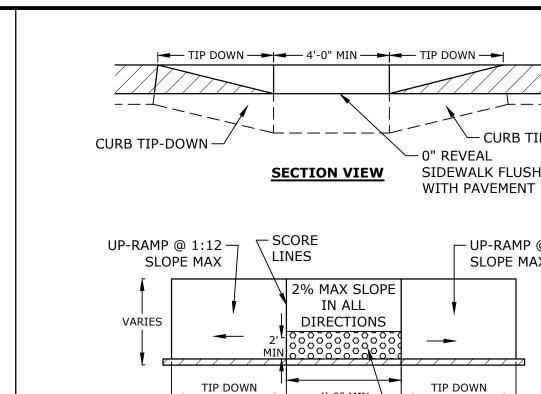
-1/4" TO 1/2"

EMECOLE 555

POLYUREA OR

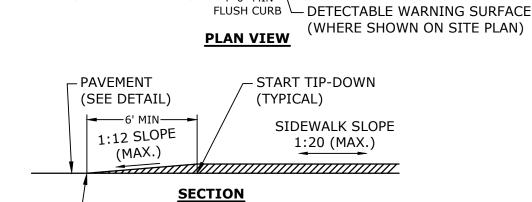
SILICA SAND AND

APPROVED EQUAL



L FLUSH CURB

0" REVEAL



PAVED ROADWAY -

(TYPICAL)

- 1. RAMPS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE AMERICANS WITH DISABILITIES ACT AND LOCAL AND STATE REQUIREMENTS.
- 2. A 9" COMPACTED CRUSHED GRAVEL BASE (NHDOT ITEM No. 304.3) SHALL BE PROVIDED BENEATH RAMPS.

CURB TIP-DOWN

─ UP-RAMP @ 1:12

SIDEWALK SLOPE

1:20 (MAX.)

— GUTTER LINE

START TIP-DOWN

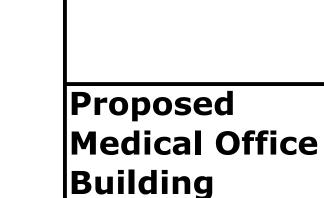
(TYPICAL)

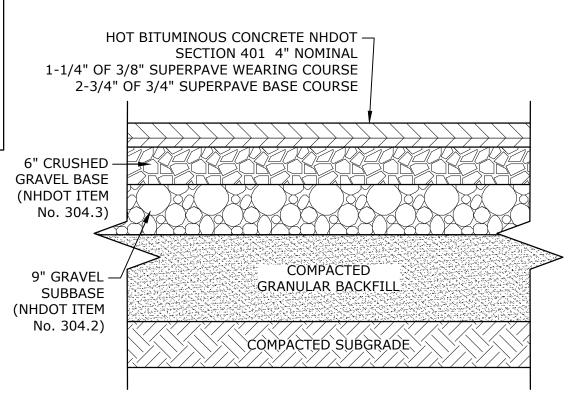
(6" REVEAL MAX.)

SLOPE MAX

3. DETECTABLE WARNING PANEL SHALL BE CAST IRON WITH BLACK COATING

## **CONCRETE SIDEWALK TIP-DOWN** RAMPS WITH DETECTABLE WARNING PANEL





- 1. SEE SITE PLAN FOR PAVEMENT WIDTH AND LOCATION.
- 2. SEE GRADING, DRAINAGE AND EROSION CONTROL PLAN FOR PAVEMENT SLOPE AND CROSS-SLOPE.
- 3. A TACK COAT SHALL BE PLACED ON TOP OF BINDER COURSE PAVEMENT PRIOR TO PLACING WEARING COURSE.
- 4. CONTRACTOR SHALL HAVE THE OPTION OF RECLAIMING THE EXISTING PAVEMENT AND REMOVING THE MATERIAL, THEN REUSING THE RECLAIMED MATERIAL AS A PAVEMENT SUBBASE.

NHDOT I	ΓΕΜ No. 304.2	NHDOT ITEM No. 304.3			
	GRAVEL)		(CRUSHED GRAVEL)		
SIEVE SIZE	% PASSING	SIEVE SIZE	% PASSING		
6"	6" 100		100		
#4	25-70	2"	95-100		
#200	#200 0-12		55-85		
		#4	27-52		
		#200	0-12		

TYPICAL STANDARD DUTY PAVEMENT SECTION

## DAR Real Estate,

185 Cottage Street Portsmouth, New Hampshire

С	10/9/2018	PB Submission	
В	9/18/2018	Revised TAC Submission	
Α	8/20/2018	TAC Submission	
MARK	DATE	DESCRIPTION	
PROJECT NO:		D5108-001	
DATE:		8/20/2018	

D5018-001\_DTLS.DWG DRAWN BY: JPC/CML CHECKED: CML/PMC APPROVED: BLM

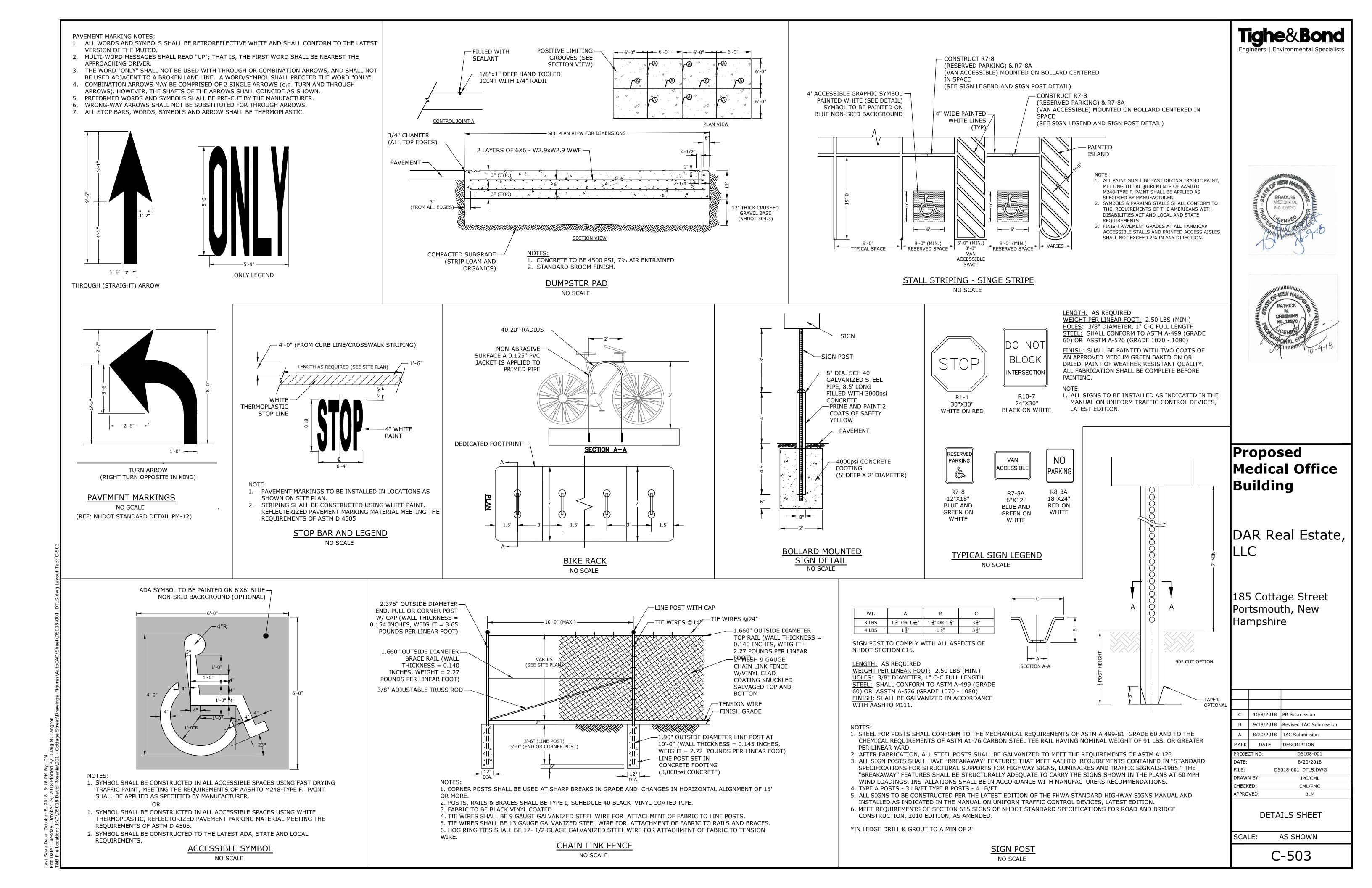
**DETAILS SHEET** 

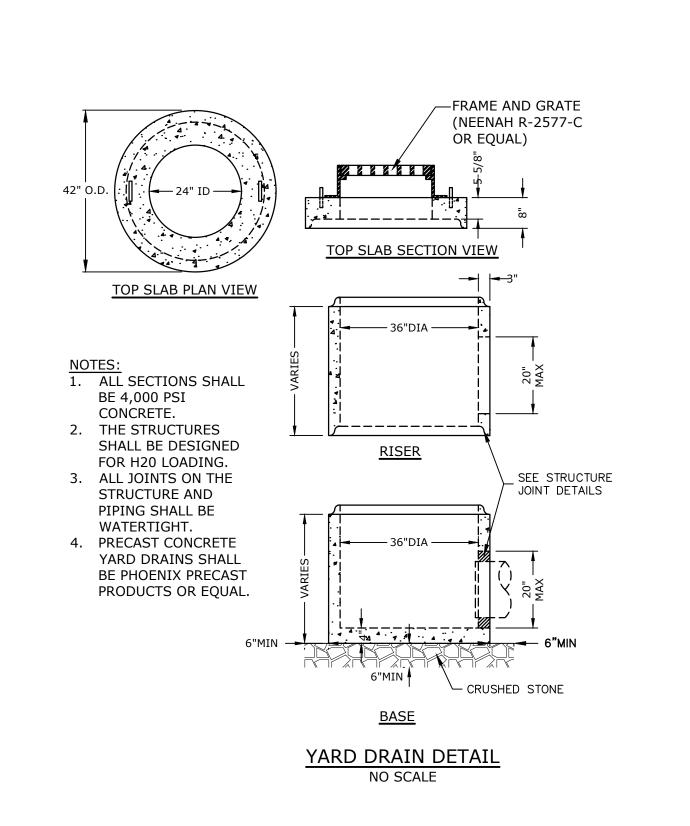
AS SHOWN SCALE:

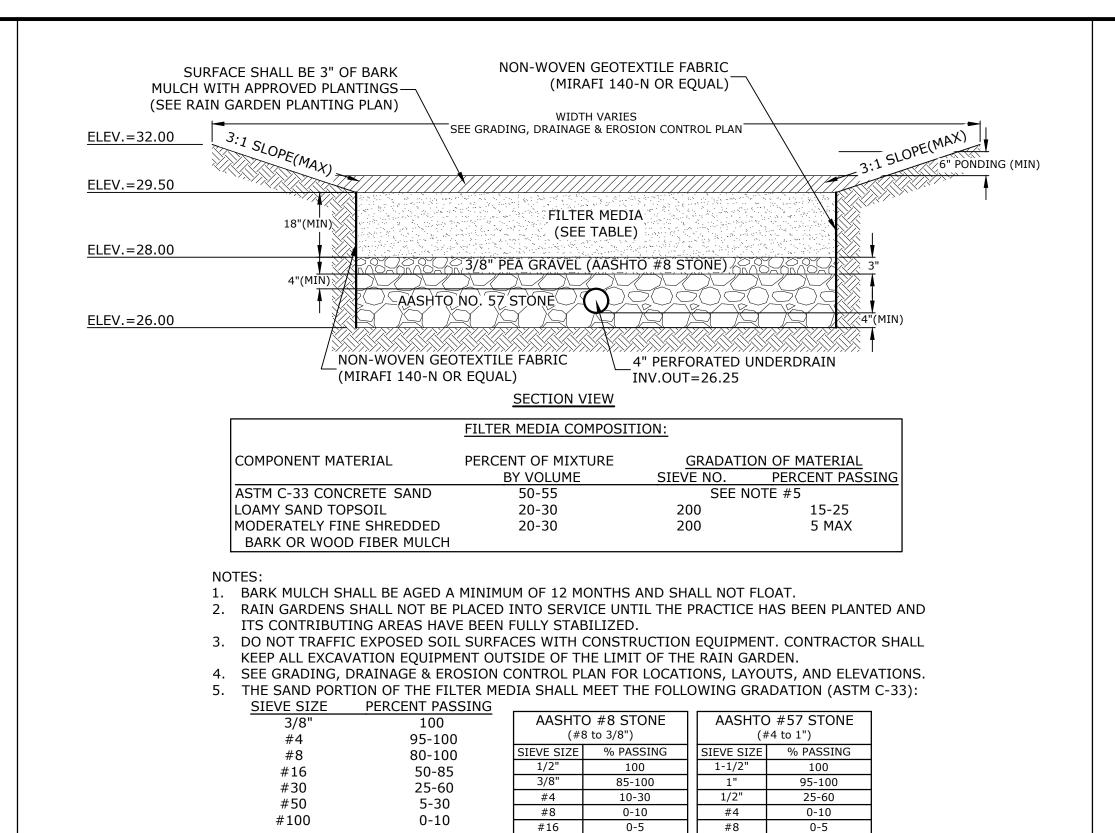
C-502

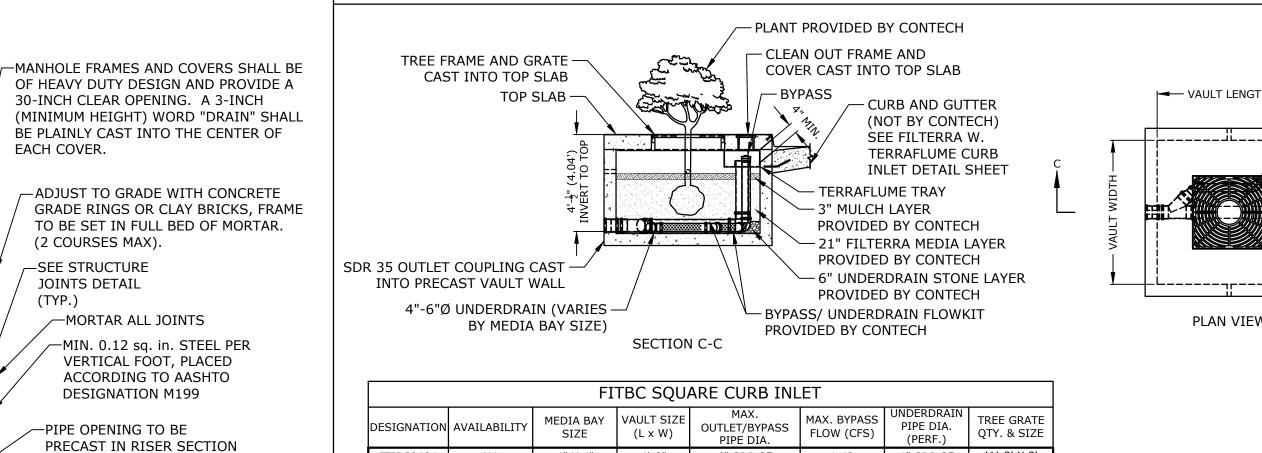


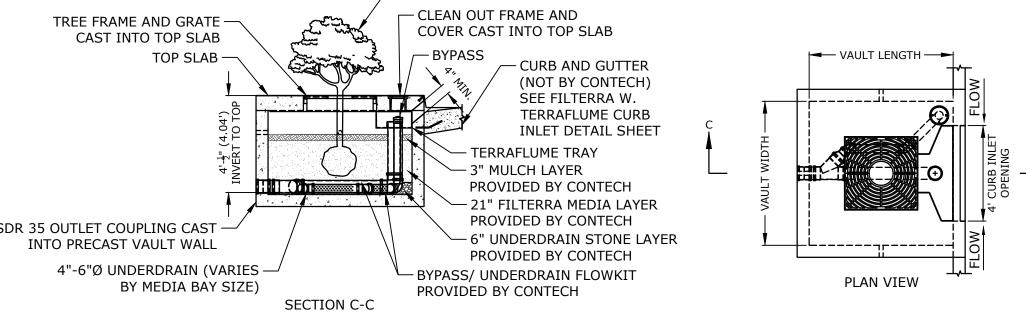






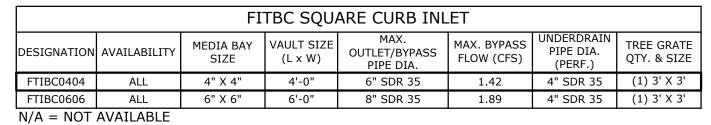




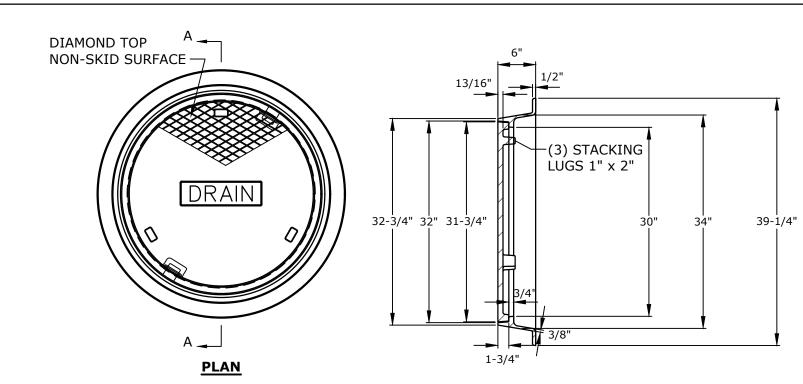


RAIN GARDEN

NO SCALE



FILTERRA BIORETENTION SYSTEM **NO SCALE** 

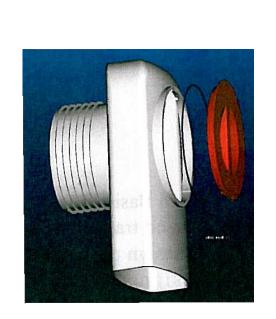


- ALL DIMENSIONS ARE NOMINAL.
- 2. FRAMES USING NARROWER DIMENSIONS FOR THICKNESS ARE ALLOWED PROVIDED:
- a. THE FRAMES MEET OR EXCEED THE SPECIFIED LOAD RATING.
- b. THE INTERIOR PERIMETER (SEAT AREA) DIMENSIONS OF THE FRAMES REMAIN THE SAME TO ALLOW CONTINUED USE OF EXISTING GRATES/COVERS AS THE EXISTING FRAMES ALLOW, WITHOUT SHIMS OR OTHER MODIFICATIONS OR ACCOMMODATIONS.

**SECTION A-A** 

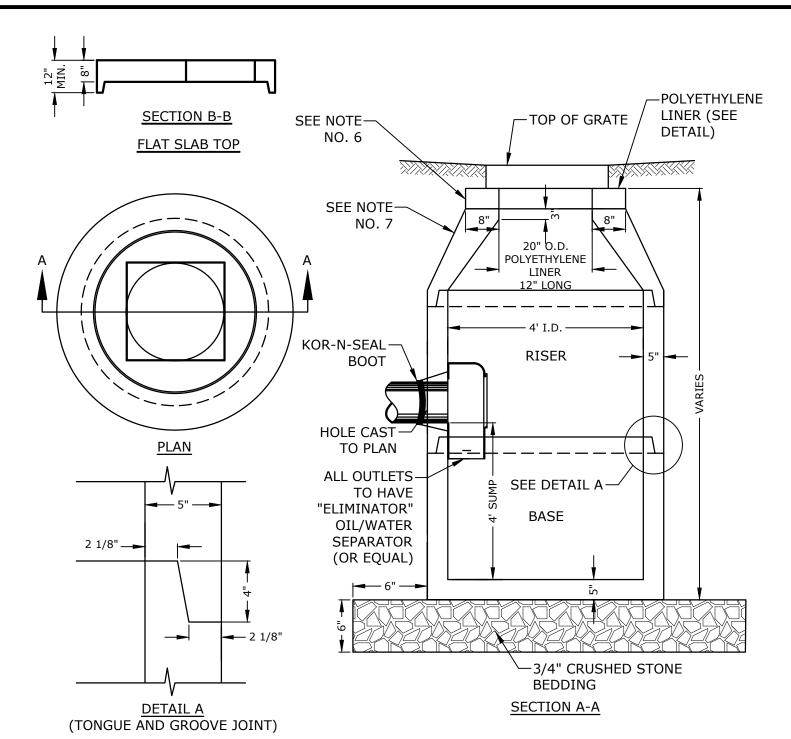
c. ALL OTHER PERTINENT REQUIREMENTS OF THE SPECIFICATIONS ARE MET 3. LABEL TYPE OF MANHOLE WITH 3" HIGH LETTERS IN HE CENTER OF THE COVER.

> DRAIN MANHOLE FRAME & COVER NO SCALE



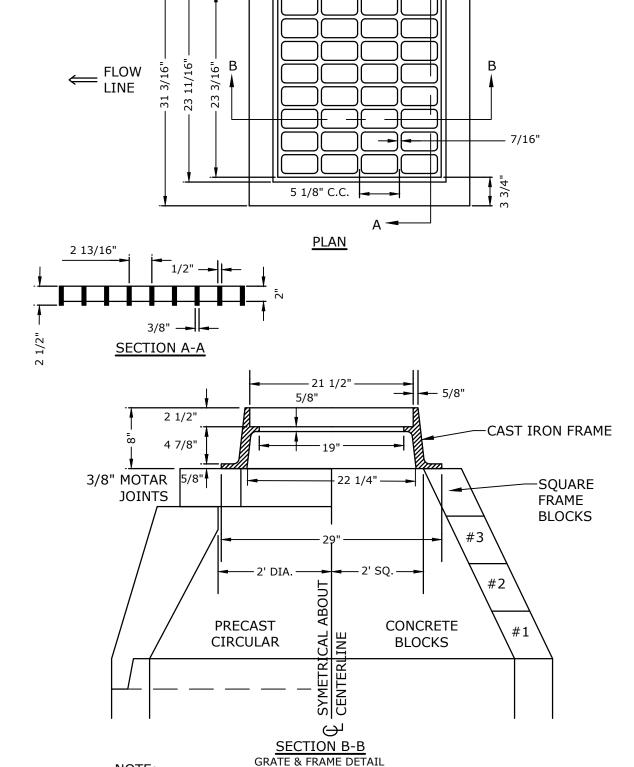
- 1. ALL CATCH BASIN OUTLETS TO HAVE "ELIMINATOR" OIL AND FLOATING DEBRIS TRAP MANUFACTURED BY KLEANSTREAM (NO EQUAL)
- 2. INSTALL DEBRIS TRAP TIGHT TO INSIDE OF STRUCTURE.
- 3. 1/4" HOLE SHALL BE DRILLED IN TOP OF DEBRIS TRAP

"ELMINATOR" OIL FLOATING DEBRIS TRAP NO SCALE



- ALL SECTIONS SHALL BE CONCRETE CLASS AA(4000 psi).
- CIRCUMFERENTIAL REINFORCEMENT SHALL BE 0.12 SQ.IN. PER LINEAR FT. IN ALL SECTIONS AND SHALL BE PLACED IN THE CENTER THIRD OF THE WALL.
- THE TONGUE AND GROOVE OF THE JOINT SHALL CONTAIN ONE LINE OF CIRCUMFERENTIAL
- REINFORCEMENT EQUAL TO 0.12 SQ. IN. PER LINEAR FT.
- RISERS OF 1', 2', 3' & 4' CAN BE USED TO REACH DESIRED DEPTH.
- THE STRUCTURES SHALL BE DESIGNED FOR H20 LOADING. FITTING FRAME TO GRADE MAY BE DONE WITH PREFABRICATED ADJUSTMENT RINGS OR CLAY BRICKS (2 COURSES MAX.)
- CONE SECTIONS MAY BE EITHER CONCENTRIC OR ECCENTRIC, OR FLAT SLAB TOPS MAY BE USED WHERE PIPE WOULD OTHERWISE ENTER INTO THE CONE SECTION OF THE STRUCTURE AND WHERE PERMITTED.
- PIPE ELEVATIONS SHOWN ON PLANS SHALL BE FIELD VERIFIED PRIOR TO PRECASTING.
- OUTSIDE EDGES OF PIPES SHALL PROJECT NO MORE THAN 3" BEYOND INSIDE WALL OF STRUCTURE. 10. PRECAST SECTIONS SHALL HAVE A TONGUE AND GROOVE JOINT 4" HIGH AT AN 11° ANGLE CENTERED IN THE WIDTH OF THE WALL AND SHALL BE ASSEMBLED USING AN APPROVED FLEXIBLE SEALANT IN JOINTS.
- 11. THE TONGUE AND GROOVE JOINT SHALL BE SEALED WITH ONE STRIP OF BUTYL RUBBER SEALANT. 12. "ELIMINATOR" OIL/WATER SEPARATOR SHALL BE INSTALLED TIGHT TO INSIDE OF CATCHBASIN.

### 4' DIAMETER CATCHBASIN NO SCALE



1. GRATE TO BE CAST IRON (NHDOT TYPE B ALTERNATE 1)

2. FRAME AND GRATE TO BE MANUFACTURED IN THE USA

**CATCH BASIN FRAME & GRATE** 

NO SCALE

A <del>-</del>

## Proposed **Medical Office** Building

DAR Real Estate,

185 Cottage Street Portsmouth, New Hampshire

С	10/9/2018	PB Submission	
В	9/18/2018	Revised TAC Submission	
Α	8/20/2018	TAC Submission	
MARK	DATE	DESCRIPTION	
PROJECT NO:		D5108-001	
DATE:		8/20/2018	
FILE: D50		018-001_DTLS.DWG	
DRAWN BY:		JPC/CML	
CHECKED:		CML/PMC	
APPROVED:		BLM	
	•		

**DETAILS SHEET** 

SCALE: AS SHOWN

C-504

5" MIN ECCENTRIC TOP HEIGHT OF RISER  $48" \pm 1"$  DIA. VARY FROM 1' TO 4' —1 - #3 BAR AROUND OPENING FOR PIPES 18" DIAMETER AND OVER, 1" COVER 5" MIN -INVERT OF STRUCTURE TO BE CONCRETE CLASS "B"  $-\frac{3}{4}$ " CRUSHED STONE BEDDING KOR-N-SEAL BOOT-5" MIN. OR EQUAL PROVIDE "V" OPENING FINISH-CONST. BRICK SHELF— SUBGRADE 6" TYP. ALL SECTIONS SHALL BE 4,000 PSI CONCRETE. AND SHALL BE PLACED IN THE CENTER THIRD OF THE WALL. THE TONGUE AND THE GROOVE OF THE JOINT SHALL CONTAIN ONE LINE OF CIRCUMFERENTIAL REINFORCEMENT EQUAL TO 0.12 SQUARE INCHES PER LINEAR FOOT. . THE STRUCTURES SHALL BE DESIGNED FOR H20 LOADING.

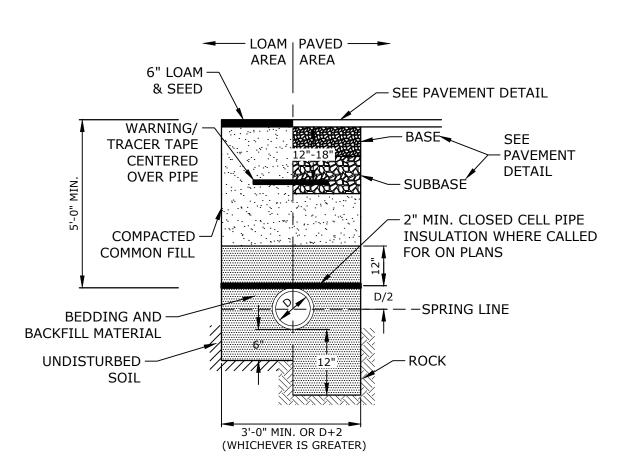
. CIRCUMFERENTIAL REINFORCEMENT SHALL BE 0.12 SQUARE INCHES PER LINEAR FOOT IN ALL SECTIONS

CONSTRUCT CRUSHED STONE BEDDING AND BACKFILL UNDER (6" MINIMUM THICKNESS)

THE TONGUE AND GROOVE JOINT SHALL BE SEALED WITH ONE STRIP OF BUTYL RUBBER SEALANT. PIPE ELEVATIONS SHOWN ON PLANS SHALL BE FIELD VERIFIED PRIOR TO PRECASTING. OUTSIDE EDGES OF PIPES SHALL PROJECT NO MORE THAN 3" BEYOND INSIDE WALL OF STRUCTURE. 9. PRECAST SECTIONS SHALL HAVE A TONGUE AND GROOVE JOINT 4" HIGH AT AN 11° ANGLE CENTERED IN

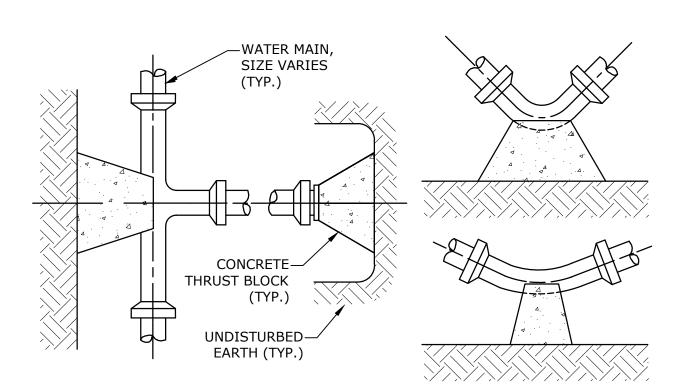
THE WIDTH OF THE WALL AND SHALL BE ASSEMBLED USING AN APPROVED FLEXIBLE SEALANT IN JOINTS 10. ALL STRUCTURES WITH MULTIPLE PIPES SHALL HAVE A MINIMUM OF 12" OF INSIDE SURFACE BETWEEN HOLES, NO MORE THAN 75% OF A HORIZNTAL CROSS SECTION SHALL BE HOLES, AND THERE SHALL BE NO HOLES CLOSER THAN 3" TO JOINTS.

4' DIAMTER DRAIN MANHOLE



- 1. SAND BEDDING AND BACKFILL FOR FULL WIDTH OF THE TRENCH FROM 6" BELOW PIPE IN EARTH AND 12" BELOW PIPE IN ROCK UP TO 12" ABOVE TOP OF PIPE.
- 2. ALL UTILITIES SHALL BE INSTALLED PER CITY OF PORTSMOUTH STANDARDS. COORDINATE ALL INSTALLATIONS THE CITY OF PORTSMOUTH

### WATER TRENCH NO SCALE



## POUR THRUST BLOCKS AGAINST

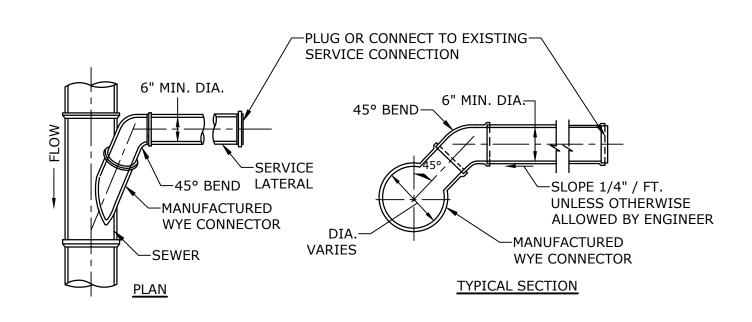
- UNDISTURBED MATERIAL, WHERE TRENCH WALL HAS BEEN DISTURBED, EXCAVATE LOOSE MATERIAL AND EXTEND THRUST BLOCK TO UNDISTURBED MATERIAL. NO JOINTS SHALL BE COVERED WITH CONCRETE.
- 2. ON BENDS AND TEES, EXTEND THRUST BLOCKS FULL LENGTH OF FITTING. 3. PLACE BOARD IN FRONT OF ALL PLUGS BEFORE POURING THRUST BLOCKS.
- 4. WHERE M.J. PIPE IS USED, M.J. PLUG WITH RETAINER GLAND MAY BE SUBSTITUTED FOR END BLOCKINGS. 5. INSTALLATION AND STANDARD DIMENSIONAL REQUIREMENTS SHALL

DEPARTMENT STANDARDS.

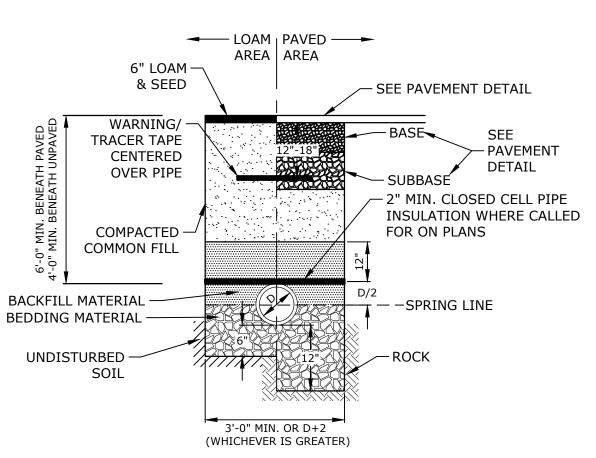
BE WITH CITY OF PORTSMOUTH WATER

SQUARE FEET OF CONCRETE THRUST BLOCKING BEARING ON UNDISTURBED MATERIAL PIPE SIZE REACTION 4" 6" 8" 10" 12" A 90° 0.89 2.19 3.82 11.14 17.24 B 180° 0.65 1.55 2.78 8.38 12.00 C 45° 0.48 1.19 2.12 6.02 9.32 D 22-1/2° 0.25 0.60 1.06 3.08 4.74 E 11-1/4° 0.13 0.30 0.54 1.54 2.38

## THRUST BLOCKING DETAIL NO SCALE



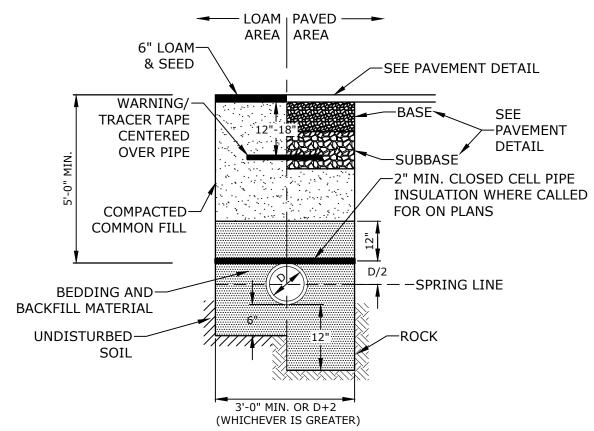
STANDARD SERVICE LATERAL CONNECTION NO SCALE



1. CRUSHED STONE BEDDING FOR FULL WIDTH OF THE TRENCH FROM 6" BELOW PIPE IN EARTH AND 12" BELOW PIPE IN ROCK UP

- 2. SAND BACKFILL FOR FULL WIDTH OF THE TRENCH FROM SPRING LINE UP TO 12" ABOVE TOP OF PIPE.
- 3. SEWER SHALL BE INSTALLED PER THE CITY OF PORTSMOUTH STANDARDS. COORDINATE ALL INSTALLATIONS WITH THE CITY OF PORTSMOUTH.

**SEWER TRENCH** 



- 1. SAND BEDDING AND BACKFILL FOR FULL WIDTH OF THE TRENCH FROM 6" BELOW PIPE IN EARTH AND 12" BELOW PIPE IN ROCK UP TO 12" ABOVE TOP OF PIPE.
- 2. ALL UTILITIES SHALL BE INSTALLED PER UNITIL'S STANDARDS. COORDINATE ALL INSTALLATIONS WITH UNITIL AND THE CITY OF PORTSMOUTH.

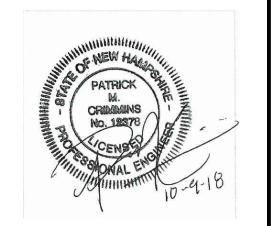
**GAS TRENCH** NO SCALE

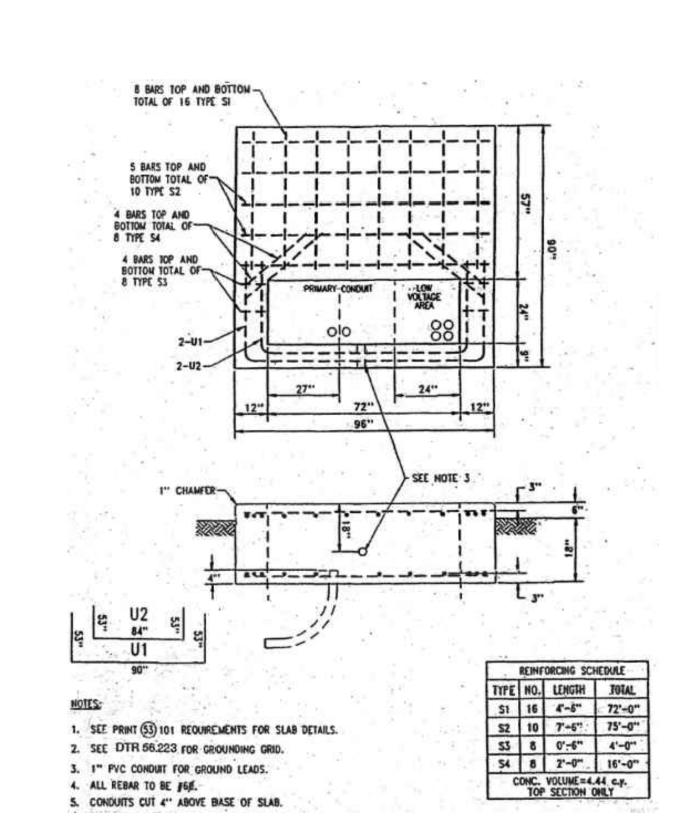
— LOAM | PAVED — ► AREA AREA 6" LOAM – & SEED SEE PAVEMENT DETAIL WARNING TRACER TAPE - PAVEMENT CENTERED DETAIL OVER PIPE - 2" MIN. CLOSED CELL PIPE INSULATION WHERE CALLED COMPACTED -FOR ON PLANS COMMON FILL BEDDING AND -**BACKFILL MATERIAL** UNDISTURBED 3'-0" MIN. OR D+2 (WHICHEVER IS GREATER)

- 1. CRUSHED STONE BEDDING AND BACKFILL FOR FULL WIDTH OF THE TRENCH FROM 6" BELOW PIPE IN EARTH AND 12" BELOW PIPE IN ROCK UP TO 6" ABOVE TOP OF PIPE.
- 2. COORDINATE ALL INSTALLATIONS WITHIN THE RIGHT OF WAY WITH THE CITY OF PORTSMOUTH

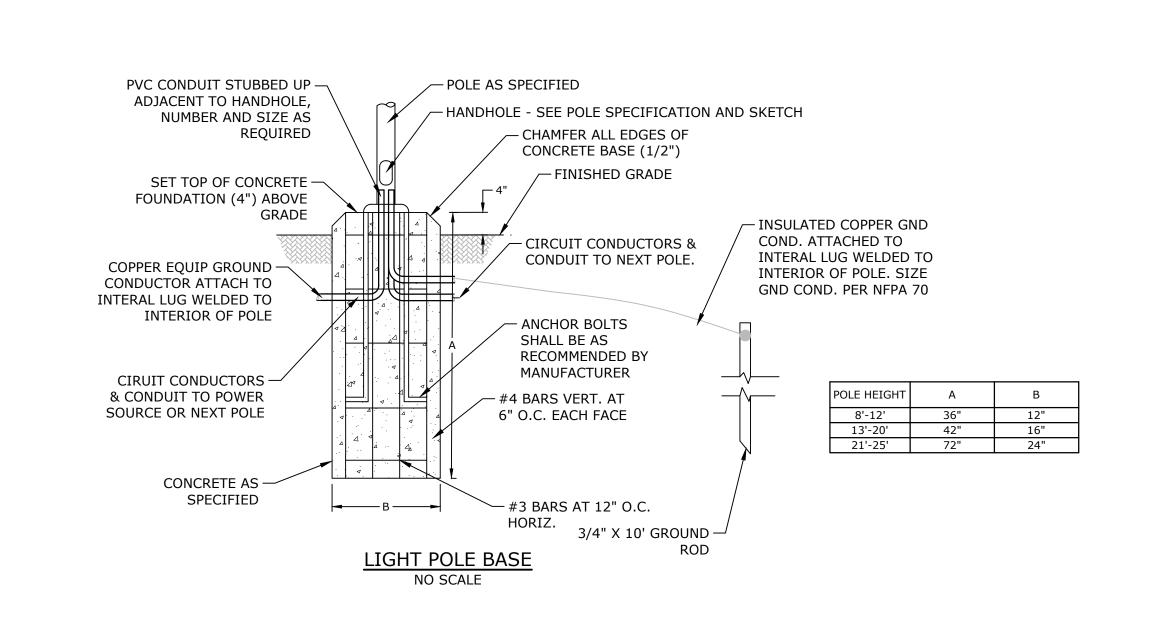
STORM DRAIN TRENCH NO SCALE







**EVERSOURCE TRANSFORMER SLAB DETAIL** NO SCALE



## Proposed **Medical Office** Building

DAR Real Estate,

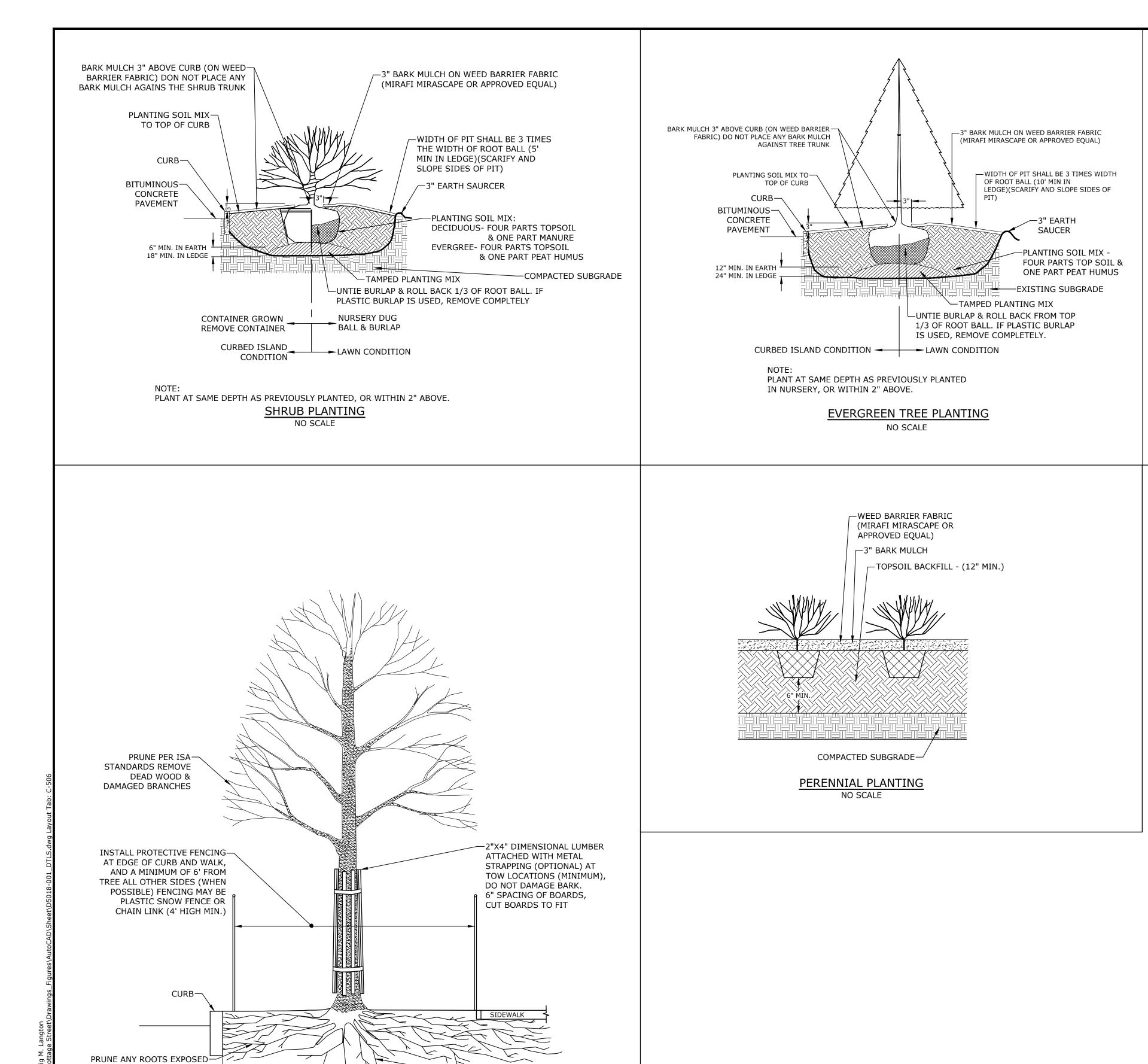
185 Cottage Street Portsmouth, New Hampshire

10/9/2018	PB Submission	
9/18/2018	Revised TAC Submission	
8/20/2018	TAC Submission	
DATE	DESCRIPTION	
PROJECT NO: D5108-001		
DATE: 8/20/2018		
FILE: D5018-001_DTLS.DWG		
DRAWN BY: JPC/CML		
CHECKED: CML/PMC		
VED:	BLM	
	9/18/2018 8/20/2018 DATE CT NO: D5 N BY: ED:	

**DETAILS SHEET** 

SCALE: AS SHOWN

C-505



-EXISTING ROOT SYSTEM

(APPROXIMATE)

AND/OR DAMAGED BY

LIMIT OF CONSTRUCTION IMPACT (VARIES-REFER TO PLANS)

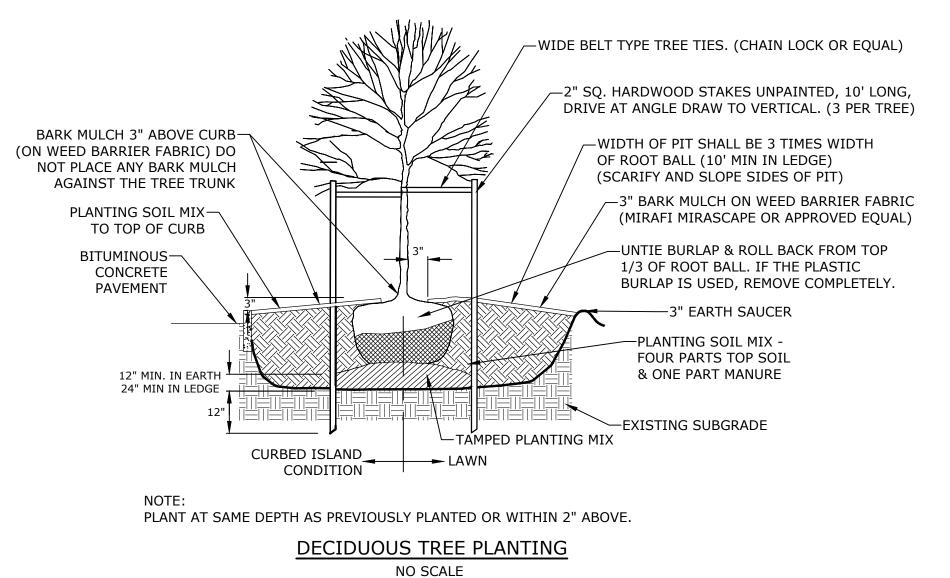
PROTECTED ROOT ZONE (DISTANCE VARIES) NO STORAGE OF EQUIPMENT

OR STOCKPILING OF MATERIALS

TREE PROTECTION FOR EXISTING TREE

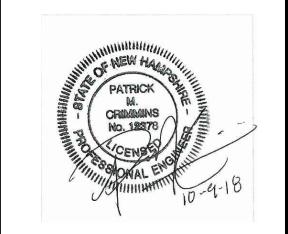
NO SCALE

EXCAVATION WITH A SHARP SAW









## Proposed Medical Office Building

DAR Real Estate, LLC

185 Cottage Street Portsmouth, New Hampshire

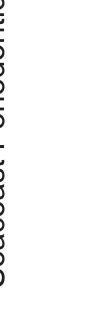
С	10/9/2018	PB Submission	
В	9/18/2018	Revised TAC Submission	
Α	8/20/2018	TAC Submission	
MARK	DATE	DESCRIPTION	
PROJECT NO: D5108-001			
DATE: 8/20/2018			
FILE: D5018-001_DTLS.DWG			
DRAWN BY: JPC/CML			
CHECKED: CML/PMC			

DETAILS SHEET

SCALE: AS SHOWN

APPROVED:

C-506



**McHENRY** ARCHITECTURE 4 Market Street Portsmouth, New Hampshire 603.430.0274

NOT FOR CONSTRUCTION REVIEW SET ONLY

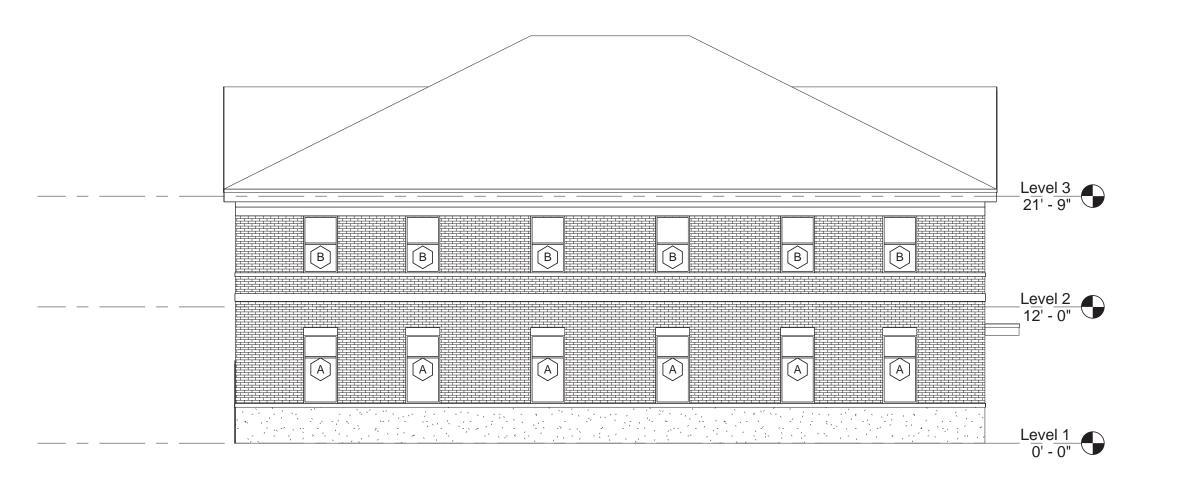
No.	Description	Date
	•	

Project Name: Seacoast Periodontics

Drawing Name: **EXTERIOR ELEVATIONS** 

18071 Project number: 8/15/2018 Checked by:

A301 1/8" = 1'-0"





LOUVERS

GRANITE HEADER

SLATELINE ROOF SHINGLES BY GAF

PAINTED COMPOSITE TRIM BANDS

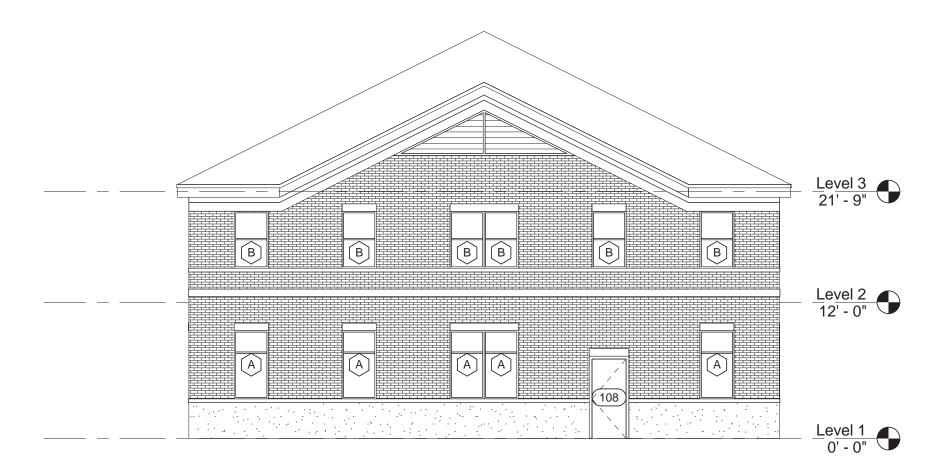
- ALUMINUM STOREFRONT

Level 2 12' - 0"

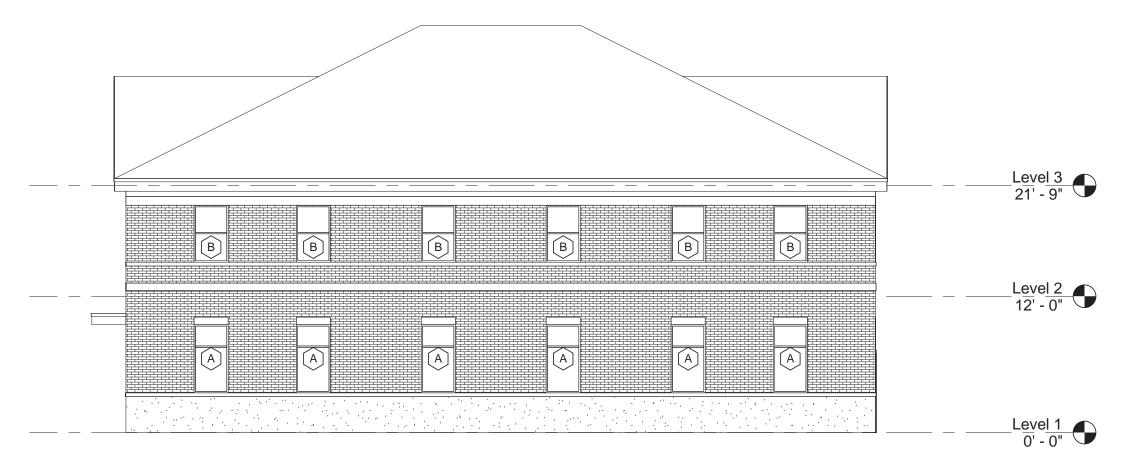
GRANITE BANDING

GRANITE SILL

GRANITE BASE







2 West 1/8" = 1'-0" | 1 - A101



D5018-001 September 18, 2018

Ms. Dexter Legg, Chairman City of Portsmouth Planning Board 1 Junkins Avenue Portsmouth, New Hampshire 03801

Waiver Request Re:

Proposed Medical Office Building, 185 Cottage Street

Dear Chairman Legg:

On behalf of DAR Real Estate, LLC, we are submitting this Waiver Request from Site Plan Review Regulation Section 8.1.2 for the Proposed Medical Office Building project located 185 Cottage Street. Section 8.1.2 indicates that all new and relocated wires conduits and cables shall be located underground.

There are two (2) existing residences located at 185 Cottage Street that are currently serviced by overhead wires. Service to the residence on the western side of the parcel is fed into the site from an existing utility pole located along Cottage Street across from the site's driveway. Overhead services are connected to the existing residence by two (2) utility poles located on the property. The proposed project proposes to keep the first utility pole located in the center of the site. This pole would be re-used bring a three (3) phase electric service and telecommunication service overhead into the site from Cottage Street. A riser would be constructed on this existing pole to drop the services underground to the proposed transformer and building.

We feel the waiver request is justified because using overhead wires to this pole would mimic the appearance of the existing condition to the first utility pole located in the center of the site. In addition, there are multiple utility crossings (gas, sewer, water and drainage) in Cottage Street which will pose a challenge for the construction of a new service underground in the street and may result in utility conflicts.

Sincerely,

TIGHE & BOND, INC.

Patrick M. Crimmins, P.E. Senior Project Manager

Cc: DAR Real Estate, LLC

McHenry Architecture Ricci Construction



D5018-001 October 9, 2018

Mr. Dexter Legg, Chairman City of Portsmouth Planning Board 1 Junkins Avenue Portsmouth, New Hampshire 03801

Re: Site Review Application
Proposed Medical Office Building, 185 Cottage Street

Dear Chairman Legg:

On behalf of DAR Real Estate, LLC, we are pleased to submit the following supplemental materials to support a Site Review Application for the above referenced project:

- Two (2) full size and ten (10) half size copies of the Site Plan Set last revised October 9, 2018
- Twelve (12) copies of the Site Review Application dated August 20, 2018
- Twelve (12) copies of the Owner Authorization Letter from Colman C. Garland
- Twelve (12) copies of the Waiver Request Letter dated September 18, 2018
- Twelve (12) copies of the TAC Stipulation Response dated October 9, 2018
- Twelve (12) copies of the Fire Truck Turning Exhibit last revised August 20, 2018
- Twelve (12) copies of the Trip Generation Analysis, dated May 29, 2018
- Twelve (12) copies of the Drainage Analysis Memorandum last revised September 18, 2018
- Twelve (12) copies of the Green Building Statement prepared by McHenry Architecture
- Twelve (12) copies of the Lighting Cut Sheets
- Twelve (12) copies of Will Serve Letters from Unitil & Eversource
- Twelve (12) copies of Access Easements referenced in Existing Conditions Note #13A & 13D
- One (1) CD containing digital copies of the above listed materials

The proposed project includes the demolition of two (2) existing residential structures and construction of a two (2) story,  $\pm 6,800$  SF medical office building with associated site improvements that consist of parking, stormwater management, utilities, lighting, and landscaping.

The proposed building layout shown on the Site Plans is based on a floor plan designed by McHenry Architecture, the project architect. Building elevations included in the Site Plan set and the Green Building Statement included in this package were prepared by McHenry Architecture.

On June 26, 2018 the Portsmouth Zoning Board of Adjustment (ZBA) granted a use variance for the site to allow medical (dental) offices where medical office us is not permitted in the General Residence A Zoning District. As part of the request for variance, a trip generation analysis was performed for this project and is enclosed with this package.

On October 2, 2018, the project received a recommendation for approval with stipulations from the Technical Advisory Committee (TAC). Enclosed is a TAC Stipulation Redponse which provides responses each of the stipulations.



We respectfully request to be placed on the Planning Board Agenda for October 18, 2018. Please contact me by phone at (603) 433-8818 or by email at <a href="mailto:pmcrimmins@tighebond.com">pmcrimmins@tighebond.com</a> if you have any questions or need any additional information.

Sincerely,

TIGHE & BOND, INC.

Patrick M. Crimmins, P.E. Senior Project Manager

Cc: DAR Real Estate, LLC

McHenry Architecture Ricci Construction

### TAC Stipulation Response

## Proposed Medical Office Building - 185 Cottage Street

October 9, 2018

Prior to Planning Board review						
<u>#</u>	<u>Comment</u>	<u>Response</u>	Sheet #			
1	The 8 foot wide handicap access aisle shall have a NO PARKING sign to reinforce its intended use.	A "NO PARKING" has been added to the site plan and details.	C-102 & C-503			
2	All proposed mechanical units shall be shown on the utility plan.	The applicant intends to put mechanical units on the roof of the proposed building where feasible. If any mechanical units are required to be on the ground the utilities plan shall be updated to depict this. See site note #19.	C-102			
3	Consideration shall be given to adding additional street trees along Route 1 in the vicinity of the rain garden as long as adequate separation can be provided to the existing sewer line.	Two (2) additional elm trees have been added along Route 1.	C-105			
4	The Site Plan to be recorded shall include a reference to the required raingarden and infiltration	Site note #17, stating "Applicant shall be responsible for implementing the approved operation and maintenance plan including the maintenance requirements for the proposed rain garden and Filterra bioretention system listed on sheet C-103."	C-102			
5	Applicant shall look to relocate the existing arborvitae along the Cottage street frontage to another	The existing arborvitae along Cottage Street have been called out to be transplanted to the area behind the proposed dumpster pad.	C-101 & C-105			
6	Applicant shall provide a copy of the access easement to the abutting Doble Center property for review by the Planning Department.	Enclosed.	N/A			
	Prior to Planning Board review					
<u>#</u>	Comment	<u>Response</u>	Sheet #			
7	Stormwater system maintenance and enforcement oversight by City of Portsmouth shall be documented in a deed restriction.	The applicant agrees.	N/A			
8	Existing buildings shall be placarded for demolition as required by the demolition ordinance.	The applicant agrees.	N/A			

D5018-1 May 29, 2018



Mr. David Rosania P.O. Box 93

Rye Beach, New Hampshire 03871

Re: **Trip Generation Analysis** 

Proposed Medical Office Development - Cottage St., Portsmouth, NH

#### Dear David:

Tighe & Bond has performed a trip generation analysis for traffic related to the proposed 7,500 SF medical office development on a parcel of land located at the corner of Cottage Street and Route 1 Bypass that is identified as Map 174 Lot 14 on the City of Portsmouth Tax Maps. This trip generation analysis is provided to support a request for use variance as the parcel is currently zoned General Residence A.

This analysis was performed utilizing Institute of Transportation Engineers (ITE) Trip Generation Manual, latest edition. For purposes of analysis, we have compared the existing and proposed uses for the parcel. The parcel's existing use consists of three (3) residences. The proposed use for the parcel is 7,500 SF of medical office. The supporting trip generation calculations are enclosed with this letter.

	Existing <u>Residences</u>	Proposed <u>Medical Office</u>	Net Trips
Weekday AM Peak Hour			
Trips Entering	3	14	+11
Trips Exiting	9	4	- <u>5</u>
Total Vehicle Trips	12	18	+ 6
Weekday PM Peak Hour			
Trips Entering	3	8	+ 5
Trips Exiting	1	20	+19
Total Vehicle Trips	4	28	+24
Average Daily			
Trips Entering	21	46	+25
Trips Exiting	21	46	+ 25
Total Vehicle Trips	42	92	+50

As depicted above, the proposed 7,500 SF medical office development will result in approximately 1 additional vehicle trip every 10 minutes during the Weekday AM Peak Hour and approximately 1 additional vehicle every 2-1/2 minutes during the Weekday PM Peak Hour. It is anticipated these additional trips will have minimal impact to the surrounding roadway network during these times. Also, the total daily trips will be generated during weekday professional business hours.



#### Tighe&Bond

In October 2016, a proposed 24-hour fast food restaurant with drive thru previously requested and was denied several variances on this parcel including a use variance. Traffic impacts were cited as one of the primary concerns by the abutters and Zoning Board of Adjustment that ultimately led to the denial of the variance request. For purposes of comparison, we have also performed a trip generation analysis to compare the current proposed medical office to the previously denied fast food restaurant.

	Previously Dei Fast Food w/ Driv	-	
Weekday AM Peak	Hour		
Trips Entering	46	14	- 32
Trips Exiting	45	4	- 4 <u>1</u>
Total Vehicle	Trips 91	18	- 73
Weekday PM Peak	Hour		
Trips Entering	34	8	- 26
Trips Exiting	31	20	<u>- 11</u>
Total Vehicle	Trips 65	28	- 37
Average Daily			
Trips Entering	496	46	-450
Trips Exiting	496	46	-450
Total Vehicle	Trips 992	92	-900

As depicted above, the current proposed medical office will generate less trips than the previously denied 24-hour fast food restaurant with drive thru. In addition to the reduced trip generation, the proposed medical office will be less impactful as there will be no concerns with drive-thru queuing. Also, vehicle trips will typically be generated during weekday professional business hours for the medical office use where the proposed 24-hour fast food restaurant with drive thru would have been generating vehicle trips all throughout day and significant trips would have been generated on weekends as shown in the enclosed Saturday daily calculations.

Please feel free to contact me at 603.433.8818 or <a href="mailto:pmcrimmins@tighebond.com">pmcrimmins@tighebond.com</a> if you have any questions.

Very truly yours,

TIGHE & BOND, INC.

Patrick M. Crimmins, P.E.

Project Manager

Enclosures: ITE Trip Generation Spreadsheets (Land Use Codes 210, 720 & 934)

J:\D\D5018 David Rosania\Report\_Evaluation\Reports\Trip Generation\Trip Gen Letter.docx

## Institute of Transportation Engineers (ITE) Land Use Code (LUC) 210 - Single-Family Detached Housing

Average Vehicle Trips Ends vs: Dwelling Units Independent Variable (X): 3

#### AVERAGE WEEKDAY DAILY

#### WEEKDAY MORNING PEAK HOUR OF ADJACENT STREET TRAFFIC

```
T = 0.70 * (X) + 9.74 
 T = 0.70 * 3 + 9.74 
 T = 11.84 
 T = 12 vehicle trips 
 with 25% ( 3 vph) entering and 75% ( 9 vph) exiting.
```

#### WEEKDAY EVENING PEAK HOUR OF ADJACENT STREET TRAFFIC

## Institute of Transportation Engineers (ITE) Land Use Code (LUC) 720 - Medical-Dental Office Building

Average Vehicle Trips Ends vs: 1,000 Sq. Feet Gross Floor Area Independent Variable (X): 7.500

#### AVERAGE WEEKDAY DAILY

```
T = 40.89 * (X) -214.97

T = 40.89 * 7.500 - 214.97

T = 91.71

T = 92 vehicle trips

with 50% ( 46 vph) entering and 50% ( 46 vph) exiting.
```

#### WEEKDAY MORNING PEAK HOUR OF ADJACENT STREET TRAFFIC

```
T = 2.39 * (X) 
 T = 2.39 * 7.500 
 T = 17.93 
 T = 18 vehicle trips 
 with 79% ( 14 vph) entering and 21% ( 4 vph) exiting.
```

#### WEEKDAY EVENING PEAK HOUR OF ADJACENT STREET TRAFFIC

#### With Drive-Through Window

Average Vehicle Trips Ends vs: 1,000 Sq. Feet Gross Floor Area Independent Variable (X): 2.000

#### AVERAGE WEEKDAY DAILY

T = 496.12 \* (X)

T = 496.12 \* 2.000

T = 992.24

T = 992 vehicle trips

with 50% (496 vpd) entering and 50% (496 vpd) exiting.

#### WEEKDAY MORNING PEAK HOUR OF ADJACENT STREET TRAFFIC

T = 45.42 \* (X) T = 45.42 \* 2.000

T = 90.84

T = 91 vehicle trips

with 51% (46 vph) entering and 49% (45 vph) exiting.

#### WEEKDAY EVENING PEAK HOUR OF ADJACENT STREET TRAFFIC

T = 32.65 \* (X)

T = 32.65 \* 2.000

T = 65.30

T = 65 vehicle trips

with 52% ( 34 vph) entering and 48% ( 31 vph) exiting.

#### SATURDAY DAILY

T = 722.03 \* (X) T = 722.03 \* 2.000

T = 1444.06

T = 1,444 vehicle trips

with 50% (722 vpd) entering and 50% (722 vpd) exiting.

## CITY OF PORTSMOUTH NEW HAMPSHIRE

## SITE REVIEW APPLICATION

Building Permit Application Num	ber		Case Number	
			Fee	
Map: 174 Lot: 14 Zon	e: GRA Wetlands: Inland _	Coastal	Lot Area: 0.91	Acres
		rals (Indicate if Pending)		0/00/40
Conservation Commission	Condition	nal Use	Board of Adjustment	6/26/18
	Subdivisi			
Street Address 185 Cott	age Street			
Description of Project inclu	ding all use(s) This projec	t consists of the d	emolition of 2	existing buildings
and the construction	of an 7,000 SF, 2-stor	y medical office b	uilding with as	sociated site
improvements that in	clude parking, stormw	ater management	t, utilities, lighti	ng & landscaping
Building(s) Footprint3,	500 SF Gross FI	loor Area 7,000 SF	#of Stori	es <u>2</u>
# of Dwelling Units	Number of Parking	Spaces: Existing	Proposed 34	
		formation Below		
Property Owner's Name	Coleman Garland			
Street Address 416 Sado	lleback Drive	City/Town Fairview	State TX	Zip <u>75069</u>
(603) 427-0000				
Telephone #	Cell Phone #	Fax#		Email Address
		formation Below		
22	Name DAR Real Estate		7/11 (0/10 ± 2	
	enland Rd, Suite B-7	City/Town Portsmo	uth State NH	Zip03801
(603) 294-0110			drosania	@gmail.com
Telephone #	Cell Phone #	Fax #		Email Address
	Print Information Below (Include A	Additional Contact Information on N	Jext Page)	
Check One: Owner's Attorney	Applicant's Attorney □ Engineer X	Surveyor  Other  If	other, state relationship	
representative s manie_	ighe & Bond			
Street Address 177 Corp	orate Drive	City/Town Portsmo	outh State NH	<sub>Zip</sub> 03801
(603) 433-8818			pmcrimmins@	tighebond.com
Telephone #	Cell Phone #	Fax #		Email Address
I hereby apply for Site Review and City of Portsmouth in the develop	d acknowledge that I will comply with ment and construction of this project.	h all the ordinances and any s	stipulations of the Site F	Review Committee of the
Owner's Signature	Prin	nt Owner's Name		Date 1
Ull		avid Rosan	1 4	8/20/18
Applicant's/Developer's Signature	Prin	nt Applicant's/Developer's Na	ame	Date

			rmation Below			
Check One: Owner's Attorney	Applicant's Attorney □	Engineer $\square$	Surveyor	Other $\square$	If other, state relationsh	ip
Representative's Name_						
Street Address		(	City/Town		State	Zip
			•			
Telephone #	Cell Phone #	<del></del>		Fax #		Email Address
Telephone II		'				
		Print Info	rmation Below			
Check One: Owner's Attorney □	Applicant's Attorney □			Other	If other, state relationsh	ip
•		Ü	•			r ———
Representative's Name						
Street Address		(	City/Town		State	Zip
			•			•
Telephone #	Cell Phone #	<del></del>		Fax #		Email Address
		Print Info	rmation Below			
Check One: Owner's Attorney □	Applicant's Attorney □			Other	If other, state relationship	ip
•		Ü	•			1
Representative's Name						
Street Address		(	City/Town		State	Zip
			•			
Telephone #	Cell Phone #	<del>-</del>		Fax #		Email Address
Telephone "	Con i none "	'		I un n		Zinari riddioss
		Attac	hments	2		
		Attac		,		
The following materials n	nust he submitted to	n the Plant	ning Dengi	rtment a	olong with the comp	oleted Application
Form:	nust be submitted to	o the I lam	iing Depai	tillelit a	nong with the comp	neteu Application
☐ Site Plan Application C						
$\Box$ Ten (10) stamped and for				-size (22	" x 34") and six (6)	reduced (11" x 17")
☐ Digital copy of any plan	ns and/or exhibits (in	PDF form	at)			
<ul><li>□ Application Fee</li><li>□ Any required State or F</li></ul>	adaral Parmits					
Any required State of T	ederal Fermits					

September 4, 2018

City of Portsmouth Planning Board

#### **GREEN BUILDING STATEMENT**

Re: Proposed Medical Office Building at 185 Cottage Street, Portsmouth, NH

The building envelope of the proposed new medical office building at 185 Cottage Street is being designed to meet or exceed current State of NH adopted 2009 International Energy Code requirements. A U.S. Department of Energy "COMcheck" will be submitted with the building permit application.

- · Foundation system to be cast in place concrete with continuous rigid insulation installed to depths required by the energy code. Continuous insulation to be provided under the concrete slab on grade for 2 feet along the exterior wall.
- Exterior walls to have cavity filled with closed cell spray foam insulation and a continuous air barrier. Exterior skin of building to be masonry.
- Exterior Windows to have thermally broken aluminum framing with insulated, high-performance glazing to provide enhanced thermal performance and solar control.
- · Roofing system: Lighter colored architectural asphalt shingle roofing system over cavity filled with closed cell spray foam insulation.
- · HVAC systems to consist of high-efficiency, variable volume rooftop units with economizers and variable speed drives. High efficiency condensing boilers with variable frequency pumps for providing heat to hydronic variable air volume boxes at spaces. Digital controls with occupancy sensors and nighttime setbacks.
- Plumbing: All fixtures to be low flow. High efficiency gas fired condensing boiler for domestic hot water.
- Lighting: Exterior lighting to be LED cutoff fixtures for energy efficiency and to minimize light pollution. All interior lighting to be LED throughout using less than 1 watt / sf and perimeter daylight sensors. Occupancy sensors to be utilized as required by code.
- -Landscaping: local species that are drought tolerant to be incorporated into plantings list.

Sincerely,

Jeremiah Johnson

Senior Associate

## McGraw-Edison

The Galleon™ LED luminaire delivers exceptional performance in a highly scalable, low-profile design. Patented, high-efficiency AccuLED Optics™ system provides uniform and energy conscious illumination to walkways, parking lots, roadways, building areas and security lighting applications. IP66 rated and UL/cUL Listed for wet locations.

Catalog #	Туре
Project	
Comments	Date
Prepared by	

#### **SPECIFICATION FEATURES**

#### Construction

Extruded aluminum driver enclosure thermally isolated from Light Squares for optimal thermal performance. Heavy-wall, diecast aluminum end caps enclose housing and die-cast aluminum heat sinks. A unique, patent pending interlocking housing and heat sink provides scalability with superior structural rigidity. 3G vibration tested and rated. Optional tool-less hardware available for ease of entry into electrical chamber. Housing is IP66 rated.

#### Optics

Patented, high-efficiency injection-molded AccuLED Optics technology. Optics are precisely designed to shape the distribution maximizing efficiency and application spacing. AccuLED Optics create consistent distributions with the scalability to meet customized application requirements. Offered standard in 4000K (+/- 275K) CCT 70 CRI. Optional 3000K, 5000K and 6000K CCT.

#### **Electrical**

LED drivers are mounted to removable tray assembly for ease of maintenance. 120-277V 50/60Hz, 347V 60Hz or 480V 60Hz operation. 480V is compatible for use with 480V Wve systems only. Standard with 0-10V dimming. Shipped standard with Eaton proprietary circuit module designed to withstand 10kV of transient line surge. The Galleon LED luminaire is suitable for operation in -40°C to 40°C ambient environments. For applications with ambient temperatures exceeding 40°C, specify the HA (High Ambient) option. Light Squares are IP66 rated. Greater than 90% lumen maintenance expected at 60,000 hours. Available in standard 1A drive current and optional 600mA. 800mA and 1200mA drive currents (nominal).

#### Mounting

STANDARD ARM MOUNT: Extruded aluminum arm includes internal bolt guides allowing for easy positioning of fixture during mounting. When mounting two or more luminaires at 90° and 120° apart, the EA extended arm may be required. Refer to the

arm mounting requirement table. Round pole adapter included. For wall mounting, specify wall mount bracket option. QUICK MOUNT ARM: Adapter is bolted directly to the pole. Quick mount arm slide into place on the adapter and is secured via two screws, facilitating quick and easy installation. The versatile, patent pending, quick mount arm accommodates multiple drill patterns ranging from 1-1/2" to 4-7/8". Removal of the door on the quick mount arm enables wiring of the fixture without having to access the driver compartment. A knock-out enables round pole mounting.

#### Finish

Housing finished in super durable TGIC polyester powder coat paint, 2.5 mil nominal thickness for superior protection against fade and wear. Heat sink is powder coated black. Standard housing colors include black, bronze, grey, white, dark platinum and graphite metallic. RAL and custom color matches available.

#### Warrantv

Five-year warranty.

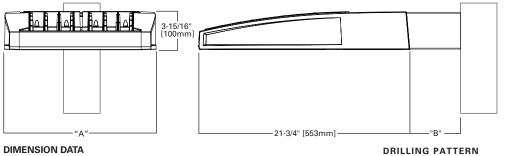


#### **GLEON GALLEON LED**

1-10 Light Squares Solid State LED

AREA/SITE LUMINAIRE

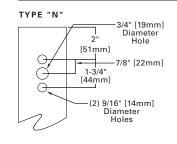
## **DIMENSIONS**



#### **DIMENSION DATA**

Number of Light Squares			Standard Ontional Arm		EPA with Arm <sup>2</sup> (Sq. Ft.)
1-4	15-1/2" (394mm)	7" 10" (178mm) (254mm)		33 (15.0 kgs.)	0.96
5-6	21-5/8" (549mm)	7" (178mm)	10" (254mm)	44 (20.0 kgs.)	1.00
7-8	27-5/8" (702mm)	7" (178mm)	13" (330mm)	54 (24.5 kgs.)	1.07
9-10	33-3/4" (857mm)	7" (178mm)	16" (406mm)	63 (28.6 kgs.)	1.12

NOTES: 1. Optional arm length to be used when mounting two fixtures at 90° on a single pole. 2. EPA calculated





#### CERTIFICATION DATA UL/cUL Wet Location Listed

ISO 9001 LM79 / LM80 Compliant 3G Vibration Rated DesignLights Consortium™ Qualified\*

#### **ENERGY DATA** Electronic LED Driver

>0.9 Power Factor

<20% Total Harmonic Distortion 120V-277V 50/60Hz 347V & 480V 60Hz

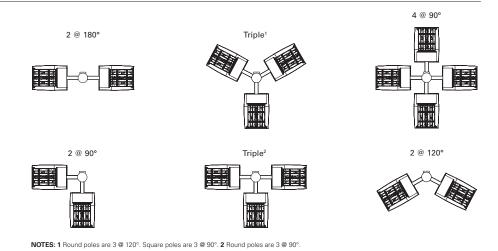
-40°C Min. Temperature 40°C Max. Temperature

50°C Max. Temperature (HA Option)



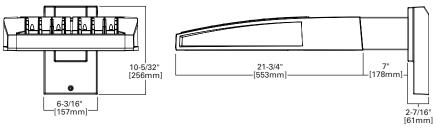
#### ARM MOUNTING REQUIREMENTS

	Г	Г
Configuration	90° Apart	120° Apart
GLEON-AF-01	7" Arm (Standard)	7" Arm (Standard)
GLEON-AF-02	7" Arm (Standard)	7" Arm (Standard)
GLEON-AF-03	7" Arm (Standard)	7" Arm (Standard)
GLEON-AF-04	7" Arm (Standard)	7" Arm (Standard)
GLEON-AF-05	10" Extended Arm (Required)	7" Arm (Standard)
GLEON-AF-06	10" Extended Arm (Required)	7" Arm (Standard)
GLEON-AF-07	13" Extended Arm (Required)	13" Extended Arm (Required)
GLEON-AF-08	13" Extended Arm (Required)	13" Extended Arm (Required)
GLEON-AF-09	16" Extended Arm (Required)	16" Extended Arm (Required)
GLEON-AF-10	16" Extended Arm (Required)	16" Extended Arm (Required)

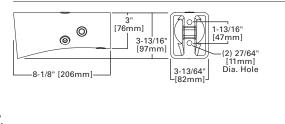


\_ 16" Extended Arm 16" Extended Arm

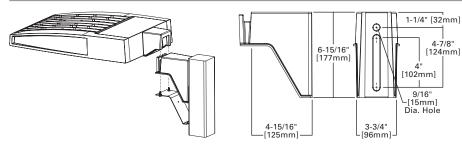
#### STANDARD WALL MOUNT

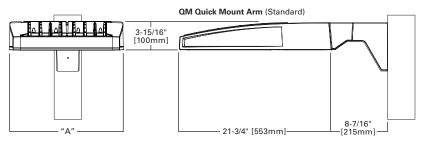


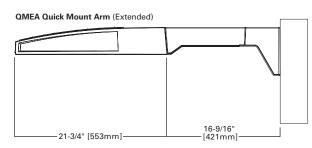
#### MAST ARM MOUNT



#### QUICK MOUNT ARM (INCLUDES FIXTURE ADAPTER)







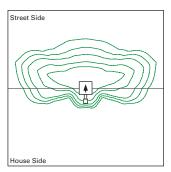
#### QUICK MOUNT ARM DATA

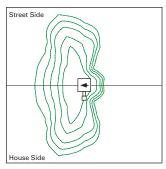
Number of Light Squares <sup>1, 2</sup>	"A" Width	Weight with QM Arm (lbs.)	Weight with QMEA Arm (lbs.)	<b>EPA</b> (Sq. Ft.)
1-4	15-1/2" (394mm)	35 (15.91 kgs.)	38 (17.27 kgs.)	
5-6 <sup>3</sup>	21-5/8" (549mm)	46 (20.91 kgs.)	49 (22.27 kgs.)	1.11
7-8	27-5/8" (702mm)	56 (25.45 kgs.)	59 (26.82 kgs.)	

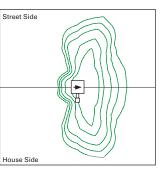
NOTES: 1 QM option available with 1-8 light square configurations. 2 QMEA option available with 1-6 light square configurations. 3 QMEA arm to be used when mounting two fixtures at 90° on a single pole.

(Type IV with Spill Control)

#### **OPTIC ORIENTATION**





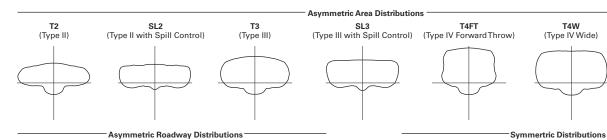


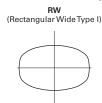
Standard

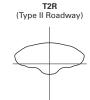
Optics Rotated Left @ 90° [L90]

Optics Rotated Right @ 90° [R90]

#### **OPTICAL DISTRIBUTIONS**

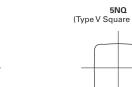


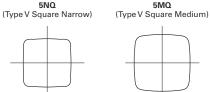


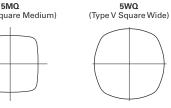




T3R





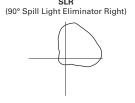


T4W

AFL (Automotive Frontline)

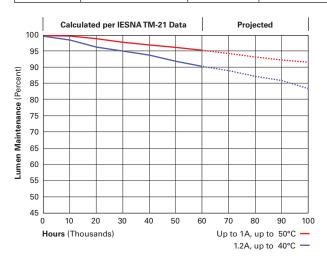


Specialized Distributions



#### **LUMEN MAINTENANCE**

Drive Current	Ambient Temperature	TM-21 Lumen Maintenance (60,000 Hours)	Projected L70 (Hours)
Up to 1A	Up to 50°C	> 95%	416,000
1.2A	Up to 40°C	> 90%	205,000



#### **LUMEN MULTIPLIER**

Lumen Multiplier
1.02
1.01
1.00
0.99
0.97

#### DESCRIPTION

The Galleon™ wall LED luminaire's appearance is complementary with the Galleon area and site luminaire bringing a modern architectural style to lighting applications. Flexible mounting options accommodate wall surfaces in both an upward and downward configuration. The Galleon family of LED products deliver exceptional performance with patented, high-efficiency AccuLED Optics™, providing uniform and energy conscious lighting for parking lots, building and security lighting applications.

Catalog #	Туре
Project	
Comments	Date
Prepared by	

McGraw-Edison

#### **SPECIFICATION FEATURES**

#### Construction

Driver enclosure thermally isolated from optics for optimal thermal performance. Heavy wall aluminum housing die-cast with integral external heat sinks to provide superior structural rigidity and an IP66 rated housing. Overall construction passes a 1.5G vibration test to ensure mechanical integrity. UPLIGHTING: Specify with the UPL option for inverted mount uplight housing with additional protections to maintain IP rating.

#### **Optics**

Choice of thirteen patented, highefficiency AccuLED Optics. The optics are precisely designed to shape the distribution maximizing efficiency and application spacing. AccuLED Optics create consistent distributions with the scalability to meet customized application requirements. Offered standard in 4000K (+/- 275K) CCT and minimum 70 CRI. Optional 3000K, 5000K and 6000K CCT. Greater than 90%

lumen maintenance expected at 60,000 hours. Available in standard 1A drive current and optional 1200mA, 800mA, and 600mA drive currents.

#### **Electrical**

LED drivers are mounted for ease of maintenance. 120-277V 50/60Hz, 347V or 480V 60Hz operation. 480V is compatible for use with 480V Wye systems only. Drivers are provided standard with 0-10V dimming. An optional Eaton proprietary surge protection module is available and designed to withstand 10kV of transient line surge. The Galleon Wall LED luminaire is suitable for operation in -30°C to 40°C ambient environments. For applications with ambient temperatures exceeding 40°C, specify the HA (High Ambient) option. Emergency egress options for -20°C ambient environments and occupancy sensor available.

#### Mounting

Gasketed and zinc plated rigid steel mounting attachment fits directly to 4" j-box or wall with the Galleon Wall "Hook-N-Lock" mechanism for guick installation. Secured with two captive corrosion resistant black oxide coated allen head set screws which are concealed but accessible from bottom of fixture.

#### Finish

Housing finished in super durable TGIC polyester powder coat paint, 2.5 mil nominal thickness for superior protection against fade and wear. Standard colors include black, bronze, grey, white, dark platinum and graphite metallic. RAL and custom color matches available. Consult the McGraw-Edison Architectural Colors brochure for the complete selection.

#### Warranty

Five-year warranty.

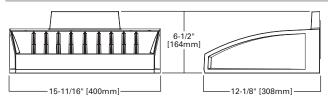


#### **GWC** GALLEON WALL LUMINAIRE

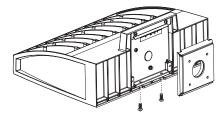
1-2 Light Squares Solid State LED

WALL MOUNT LUMINAIRE

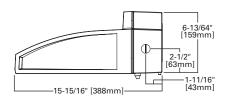
#### **DIMENSIONS**



#### HOOK-N-LOCK MOUNTING



#### BATTERY BACKUP AND THRU-BRANCH BACK BOX







#### **CERTIFICATION DATA**

UL/cUL Listed LM79 / LM80 Compliant IP66 Housing ISO 9001 DesignLights Consortium™ Qualified\*

#### ENERGY DATA

**Electronic LED Driver** 

>0.9 Power Factor <20% Total Harmonic Distortion 120-277V/50 & 60Hz, 347V/60Hz, 480V/60Hz -30°C Minimum Temperature 40°C Ambient Temperature Rating

SHIPPING DATA Approximate Net Weight:

27 lbs. (12.2 kgs.)

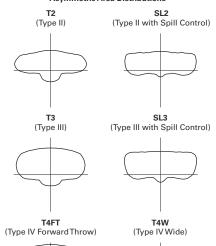


#### **POWER AND LUMENS**

Deble Current   Geoma   Geoma   Geoma   1.0A   1.2A   Geoma   3.00m   1.0A   1.2A	Number of Light Squares 1 2									
Nominal   Power   Natiss   S4		-	600mA	1		1 2A	600mA			1.24
Imput Current # 269V   W										
Input Current @ 208V   Q										
Input Current @ 240V   V		· · · · · · · · · · · · · · · · · · ·								
Input Current @ 427V (M)										
Imput Current @ 947V (mA)   0.11   0.15   0.17   0.20   0.19   0.24   0.32   0.39	-									
Input Current										
Optics										
March   Marc	<u> </u>	9 1007 ( y	0.00	0	0	0.10	0.10	0.10	0.21	0.00
Table	98.00	4000K/5000K Lumens	4 110	5 040	6 238	6 843	8 031	9 849	12 190	13 373
BUG Rating	To									,
Mook/Sook Lumens	12		· ·	,	,	,		,	· ·	,
TS         3000K Lumens         3,708         4,548         5,629         6,174         7,247         8,887         10,999         12,065           BUG Rating         B1-U0-GI         B1-U0-GI         B1-U0-G2         B1-U0-G2         B1-U0-G2         B2-U0-G2         B1-U0-G2         B1-U0-G2 <t< td=""><td></td><td><u> </u></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>		<u> </u>								
BUG Rating   B1-U0-G1   B1-U0-G2   B1-U0-G2   B1-U0-G2   B1-U0-G2   B2-U0-G2   B2-U0-G3   B2-U0-G	то		· ·	,			,	,		· ·
TAFT  A000K/5000K Lumens  A 214  A 5,167  A 6,395  A 7,016  B .233  A 10,097  A 12,497  A 13,709  TAFT  A000K/5000K Lumens  A 3,730  A 5,74  A 5,661  B -211  A 2,888  B ,938  A 11,062  B -10-G2  B	13				,	,		,		
T4FT         3000K Lumens         3,730         4,574         5,661         6,211         7,288         8,938         11,062         12,135           BUG Rating         B1-Uo-G1         B1-Uo-G2         B1-Uo-G3         B2-Uo-G3										
BUG Rating   B1-U0-G1   B1-U0-G2   B1-U0-G3   B2-U0-G3   B2-U0-G3   B2-U0-G3   B2-U0-G3   B1-U0-G2   B1-U0-G2   B1-U0-G2   B1-U0-G2   B1-U0-G2   B1-U0-G3   B2-U0-G3   B2-U0-G3   B2-U0-G3   B2-U0-G3   B1-U0-G2   B1-U0-G2   B1-U0-G2   B1-U0-G2   B1-U0-G3   B1-U0-G3   B2-U0-G3   B2-U0-G3   B1-U0-G3   B1-U0-G										
TAW         4000K/5000K Lumens         4,159         5,100         6,313         6,925         8,127         9,966         12,336         13,532           TAW         3000K Lumens         3,682         4,515         5,588         6,130         7,194         8,822         10,920         11,979           BUG Rating         B1-U-G1         B1-U-G2         B1-U-G2         B1-U-G2         B2-U-G2         B2-U-G2         B2-U-G2         B2-U-G3         B2-U-G3           SL2         4000K/5000K Lumens         4,102         5,032         6,227         6,831         8,018         9,832         12,170         13,350           SL2         3000K Lumens         3,631         4,454         5,512         6,047         7,098         8,703         10,773         11,817           BUG Rating         B1-U-G1         B1-U-G2	14F1			,	· ·	,	,	,		,
TAWN         3000K Lumens         3,682         4,515         5,588         6,130         7,194         8,822         10,920         11,979           BUG Rating         B1-UO-G1         B1-UO-G2         B1-UO-G2         B1-UO-G2         B1-UO-G2         B2-UO-G2         B2-UO-G2         B2-UO-G2         B2-UO-G3										
BUG Rating   B1-U0-G1   B1-U0-G2   B1-U0-G2   B1-U0-G2   B2-U0-G2   B2-U0-G2   B2-U0-G3   B2-U0-G3										,
Mount   Moun	T4W							·		
SL2         3000K Lumens         3,631         4,454         5,512         6,047         7,098         8,703         10,773         11,817           BUG Rating         B1-U0-G1         B1-U0-G2         B1-U0-G2         B1-U0-G2         B2-U0-G2         B2-U0-G3         <										
BUG Rating   B1-U0-G1   B1-U0-G2   B1-U0-G2   B1-U0-G2   B1-U0-G2   B2-U0-G3   B2-U0-G2   B2-U0-G3   B2-U0-G3   B2-U0-G3   B2-U0-G3   B2-U0-G3   B2-U0-G3   B2-U0-G3   B2-U0-G3   B2-U0-G				,			· ·	,		,
March   Marc	SL2		· ·	,		,		,		
SL3         3000K Lumens         3,707         4,547         5,628         6,173         7,246         8,886         10,998         12,064           BUG Rating         B1-U0-G1         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G3         B2-U0-G3         B1-U0-G2         B1-U0-G3         B2-U0-G3         B2-U0-G3         B2-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G3         <		BUG Rating	B1-U0-G1	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G2	B2-U0-G2	B2-U0-G3	B2-U0-G3
BUG Rating         B1-U0-G1         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G3         B2-U0-G3         B2-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G3         B2-U0-G3         B2-U0-G3         B2-U0-G3         B2-U0-G3         B2-U0-G3         B2-U0-G2         B1-U0-G2         B1-U0-G3         B2-U0-G3         B2-U0-G3         B2-U0-G2         B1-U0-G2         B1-U0-G3         B1-U0-G3         B2-U0-G3         B2-U0-G3         B2-U0-G1         B3-U0-G1         B3-U0-G2         B4-U0-G2         B4-U0-G2		4000K/5000K Lumens	4,188	5,137	6,358	6,974	8,186	10,038	12,424	13,628
SL4         4000K/5000K Lumens         3,980         4,880         6,040         6,626         7,776         9,537         11,803         12,949           3000K Lumens         3,523         4,320         5,347         5,865         6,883         8,442         10,448         11,462           BUG Rating         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G3         B1-U0-G3         B2-U0-G3           5NQ         3000K Lumens         4,321         5,298         6,558         7,193         8,443         10,353         12,814         14,057           5NQ         3000K Lumens         3,825         4,690         5,805         6,367         7,474         9,164         11,343         12,443           BUG Rating         B2-U0-G1         B2-U0-G1         B3-U0-G1         B3-U0-G2         B4-U0-G2         B4	SL3	3000K Lumens								
SL4         3000K Lumens         3,523         4,320         5,347         5,865         6,883         8,442         10,448         11,462           BUG Rating         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G3         B1-U0-G3         B2-U0-G3           5NQ         4000K/5000K Lumens         4,321         5,298         6,558         7,193         8,443         10,353         12,814         14,057           5NQ         3000K Lumens         3,825         4,690         5,805         6,367         7,474         9,164         11,343         12,443           BUG Rating         B2-U0-G1         B2-U0-G1         B3-U0-G1         B3-U0-G1         B3-U0-G1         B3-U0-G1         B3-U0-G2         B4-U0-G2         B4-		BUG Rating	B1-U0-G1	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G3	B2-U0-G3	B2-U0-G3
BUG Rating B1-U0-G2 B1-U0-G2 B1-U0-G2 B1-U0-G2 B1-U0-G2 B1-U0-G3 B1-U0-G3 B2-U0-G3 B1-U0-G3 B1-U0-G3 B2-U0-G3 B1-U0-G3 B1-U0-G3 B1-U0-G3 B1-U0-G3 B2-U0-G3 B1-U0-G3 B1-U0-G2 B1-U0-G2 B1-U0-G2 B1-U0-G2 B1-U0-G3 B		4000K/5000K Lumens	3,980	4,880	6,040	6,626	7,776	9,537	11,803	12,949
5NQ         4000K/5000K Lumens         4,321         5,298         6,558         7,193         8,443         10,353         12,814         14,057           5NQ         3000K Lumens         3,825         4,690         5,805         6,367         7,474         9,164         11,343         12,443           BUG Rating         B2-U0-G1         B2-U0-G1         B3-U0-G1         B3-U0-G2         B4-U0-G2	SL4	3000K Lumens	3,523	4,320	5,347	5,865	6,883	8,442	10,448	11,462
5NQ         3000K Lumens         3,825         4,690         5,805         6,367         7,474         9,164         11,343         12,443           BUG Rating         B2-U0-G1         B2-U0-G1         B2-U0-G1         B3-U0-G1         B3-U0-G1         B3-U0-G2         B3-U0-G2         B3-U0-G2         B3-U0-G2         B3-U0-G1         B3-U0-G1         B3-U0-G1         B3-U0-G2         B3-U0-G2         B3-U0-G2         B3-U0-G2         B3-U0-G2         B4-U0-G2         <		BUG Rating	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G3	B1-U0-G3	B2-U0-G3
BUG Rating         B2-U0-G1         B2-U0-G1         B2-U0-G1         B3-U0-G1         B3-U0-G1         B3-U0-G1         B3-U0-G2         B4-U0-G2		4000K/5000K Lumens	4,321	5,298	6,558	7,193	8,443	10,353	12,814	14,057
5MQ         4000K/5000K Lumens         4,400         5,396         6,678         7,326         8,598         10,544         13,050         14,315           5MQ         3000K Lumens         3,895         4,777         5,911         6,485         7,611         9,334         11,552         12,672           BUG Rating         B3-U0-G1         B3-U0-G1         B3-U0-G1         B3-U0-G1         B3-U0-G2         B4-U0-G2         B4-U0-G2         B4-U0-G2         B4-U0-G2           5WQ         4000K/5000K Lumens         4,412         5,410         6,695         7,345         8,621         10,572         13,085         14,354           5WQ         3000K Lumens         3,906         4,789         5,926         6,502         7,631         9,358         11,583         12,706           BUG Rating         B3-U0-G1         B3-U0-G2         B3-U0-G2         B4-U0-G2         B4-U0-G2         B4-U0-G2           SLL/SLR         4000K/5000K Lumens         3,681         4,515         5,588         6,129         7,193         8,821         10,917         11,976           SLL/SLR         BUG Rating         B1-U0-G1         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G2	5NQ	3000K Lumens	3,825	4,690	5,805	6,367	7,474	9,164	11,343	12,443
5MQ         3000K Lumens         3,895         4,777         5,911         6,485         7,611         9,334         11,552         12,672           BUG Rating         B3-U0-G1         B3-U0-G1         B3-U0-G1         B3-U0-G1         B3-U0-G2         B4-U0-G2         <		BUG Rating	B2-U0-G1	B2-U0-G1	B2-U0-G1	B3-U0-G1	B3-U0-G1	B3-U0-G1	B3-U0-G2	B3-U0-G2
BUG Rating B3-U0-G1 B3-U0-G1 B3-U0-G1 B3-U0-G1 B3-U0-G2 B4-U0-G2 B4-U0-G2 B4-U0-G2  4000K/5000K Lumens 4,412 5,410 6,695 7,345 8,621 10,572 13,085 14,354  5WQ 3000K Lumens 3,906 4,789 5,926 6,502 7,631 9,358 11,583 12,706  BUG Rating B3-U0-G1 B3-U0-G1 B3-U0-G2 B3-U0-G2 B3-U0-G2 B4-U0-G2 B4-U0-G2 B4-U0-G2  4000K/5000K Lumens 3,681 4,515 5,588 6,129 7,193 8,821 10,917 11,976  SLL/SLR 3000K Lumens 3,258 3,997 4,946 5,425 6,367 7,808 9,664 10,601  BUG Rating B1-U0-G1 B1-U0-G2 B1-U0-G2 B1-U0-G2 B1-U0-G2 B1-U0-G3 B2-U0-G3  BUG Rating B1-U0-G1 B1-U0-G2 B1-U0-G2 B1-U0-G2 B1-U0-G3 B1-U0-G3 B2-U0-G3  4000K/5000K Lumens 4,281 5,250 6,498 7,129 8,366 10,259 12,698 13,930  RW 3000K Lumens 3,790 4,647 5,752 6,311 7,406 9,081 11,240 12,331		4000K/5000K Lumens	4,400	5,396	6,678	7,326	8,598	10,544	13,050	14,315
5WQ         4000K/5000K Lumens         4,412         5,410         6,695         7,345         8,621         10,572         13,085         14,354           5WQ         3000K Lumens         3,906         4,789         5,926         6,502         7,631         9,358         11,583         12,706           BUG Rating         B3-U0-G1         B3-U0-G2         B3-U0-G2         B4-U0-G2         B4-U0-G2         B4-U0-G2           \$4000K/5000K Lumens         3,681         4,515         5,588         6,129         7,193         8,821         10,917         11,976           \$LL/SLR         3000K Lumens         3,258         3,997         4,946         5,425         6,367         7,808         9,664         10,601           BUG Rating         B1-U0-G1         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G3         B1-U0-G3         B2-U0-G3           \$4000K/5000K Lumens         4,281         5,250         6,498         7,129         8,366         10,259         12,698         13,930           \$8W         3000K Lumens         3,790         4,647         5,752         6,311         7,406         9,081         11,240         12,331	5MQ	3000K Lumens	3,895	4,777	5,911	6,485	7,611	9,334	11,552	12,672
5WQ         3000K Lumens         3,906         4,789         5,926         6,502         7,631         9,358         11,583         12,706           BUG Rating         B3-U0-G1         B3-U0-G2         B3-U0-G2         B3-U0-G2         B4-U0-G2         B4-U0-G3         <		BUG Rating	B3-U0-G1	B3-U0-G1	B3-U0-G1	B3-U0-G1	B3-U0-G2	B4-U0-G2	B4-U0-G2	B4-U0-G2
BUG Rating         B3-U0-G1         B3-U0-G1         B3-U0-G2         B3-U0-G2         B3-U0-G2         B4-U0-G2         B1-U0-G2         B1-U0-G3         B1-U0-G3         B1-U0-G3         B1-U0-G3         B1-U0-G3         B2-U0-G3           BW         4000K/5000K Lumens         4,281         5,250         6,498         7,129         8,366         10,259         12,698         13,930           RW         3000K Lumens         3,790         4,647         5,752         6,311         7,406         9,081         11,240         12,331		4000K/5000K Lumens	4,412	5,410	6,695	7,345	8,621	10,572	13,085	14,354
SLL/SLR         4000K/5000K Lumens         3,681         4,515         5,588         6,129         7,193         8,821         10,917         11,976           SLL/SLR         3000K Lumens         3,258         3,997         4,946         5,425         6,367         7,808         9,664         10,601           BUG Rating         B1-U0-G1         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G3         B1-U0-G3         B2-U0-G3           4000K/5000K Lumens         4,281         5,250         6,498         7,129         8,366         10,259         12,698         13,930           RW         3000K Lumens         3,790         4,647         5,752         6,311         7,406         9,081         11,240         12,331	5WQ	3000K Lumens	3,906	4,789	5,926	6,502	7,631	9,358	11,583	12,706
SLL/SLR         3000K Lumens         3,258         3,997         4,946         5,425         6,367         7,808         9,664         10,601           BUG Rating         B1-U0-G1         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G2         B1-U0-G3         B1-U0-G3         B2-U0-G3           4000K/5000K Lumens         4,281         5,250         6,498         7,129         8,366         10,259         12,698         13,930           RW         3000K Lumens         3,790         4,647         5,752         6,311         7,406         9,081         11,240         12,331		BUG Rating	B3-U0-G1	B3-U0-G1	B3-U0-G2	B3-U0-G2	B3-U0-G2	B4-U0-G2	B4-U0-G2	B4-U0-G2
BUG Rating B1-U0-G1 B1-U0-G2 B1-U0-G2 B1-U0-G2 B1-U0-G3 B1-U0-G3 B2-U0-G3 4000K/5000K Lumens 4,281 5,250 6,498 7,129 8,366 10,259 12,698 13,930 RW 3000K Lumens 3,790 4,647 5,752 6,311 7,406 9,081 11,240 12,331		4000K/5000K Lumens	3,681	4,515	5,588	6,129	7,193	8,821	10,917	11,976
RW         4000K/5000K Lumens         4,281         5,250         6,498         7,129         8,366         10,259         12,698         13,930           RW         3000K Lumens         3,790         4,647         5,752         6,311         7,406         9,081         11,240         12,331	SLL/SLR	3000K Lumens	3,258	3,997	4,946	5,425	6,367	7,808	9,664	10,601
RW 3000K Lumens 3,790 4,647 5,752 6,311 7,406 9,081 11,240 12,331		BUG Rating	B1-U0-G1	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G3	B1-U0-G3	B2-U0-G3
		4000K/5000K Lumens	4,281	5,250	6,498	7,129	8,366	10,259	12,698	13,930
BUG Rating B2-U0-G1 B2-U0-G1 B3-U0-G1 B3-U0-G1 B3-U0-G1 B3-U0-G1 B3-U0-G2 B3-U0-G2	RW	3000K Lumens	3,790	4,647	5,752	6,311	7,406	9,081	11,240	12,331
		BUG Rating	B2-U0-G1	B2-U0-G1	B3-U0-G1	B3-U0-G1	B3-U0-G1	B3-U0-G1	B3-U0-G2	B3-U0-G2

 $<sup>^{\</sup>star}$  Nominal lumen data for 70 CRI. BUG rating for 4000K/5000K. Refer to IES files for 3000K BUG ratings.

#### - Asymmetric Area Distributions



SL4 (Type IV with Spill Control)



Symmertric	Distributions
5ΝΩ (Type V Square Narrow)	<b>5MQ</b> (Type V Square Medium)



5WQ (Type V Square Wide)



Specialized Distributions SLL RW

Rectangular Wide Type I)	(90° Spill Light Eliminator Left)

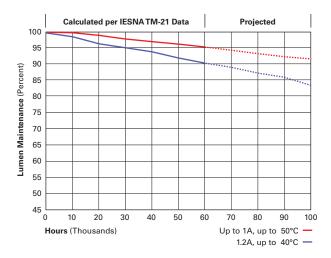
SLR (90° Spill Light Eliminator Right)



## Eaton 1121 Highway 74 South Peachtree City, GA 30269 P: 770-486-4800 www.eaton.com/lighting

#### **LUMEN MAINTENANCE**

Drive Current	Ambient Temperature	TM-21 Lumen Maintenance (60,000 Hours)	Projected L70 (Hours)
Up to 1A	Up to 50°C	> 95%	> 416,000
1.2A	Up to 40°C	> 90%	> 205,000



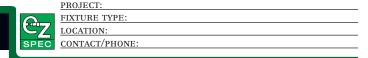
#### LUMEN MULTIPLIER

Ambient Temperature	Lumen Multiplier
0°C	1.02
10°C	1.01
25°C	1.00
40°C	0.99
50°C	0.97



#### **MERU Series**

LED GENERAL & EMERGENCY LIGHTING



#### PRODUCT DESCRIPTION

The MERU Series is an architectural, low-profile outdoor light, offering "normally On" AC and emergency lighting with powerful LED illumination. The housing is fully sealed and gasketed, and has an IP65 rating. Designed for wall mounting with universal K/O pattern in back-plate for easy installation to most standard size junction boxes. Includes a single ½" NPT conduit entry in the top, center of the housing. Illumination provided by 8 high power LEDs which achieve 1,600 lumens in AC and 600 lumens in emergency. LED color at 4000K.

#### **PRODUCT SPECIFICATIONS**

#### CONSTRUCTION

Die cast aluminum housing with superior heat sink • Scratch resistant Polyester powder coat finish • UV resistant polycarbonate lens • Snap-fit housing and mounting plate are held together by four stainless steel clips • Universal mounting pattern molded into the back plate • 1/2" threaded top access for surface conduit installation • Silicone rubber seal with hollow center, shape adaptive design protects the electrical components • Junction box neoprene seal is attached to the back plate for a weather proof installation • Dark Bronze or White textured finish.

#### **ELECTRICAL**

Dual voltage 120/277VAC 60Hz input • Solid state charging and switching • Battery low voltage disconnect • AC power indicator and test switch at the bottom of the unit • Standard with Self Diagnostics to monitor proper operation.

Supplied with eight (8) LG SMD 4000K LED'S • L70 > 72,000hours • 17 Watts total (32 Watts with IH option) • 1600 Lumens in AC mode, 600 Lumens in Emergency mode • Full cut-off optics for Dark Sky compliance

#### **BATTERY**

Maintenance-free, long-life rechargeable NiCad battery will operate fixture for a minimum of 90 minutes in the event of a power outage • 24 hour recharge after 90 minute discharge.

#### CODE COMPLIANCE

UL924 • Listed for wet location applications (0°C-50°C) • Optional "IH" cold weather package for (-40°C-50°C) • IP65 Rated • NFPA 101 Life Safety Code compliant • NEC and OSHA compliant • DLC Listed • RoHS Compliant

#### WARRANTY

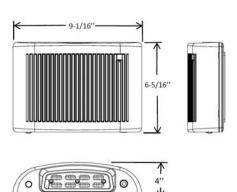
5-year warranty. Product specifications subject to change without notice.

#### **INSTALLATION**

#### MOUNTING

Suitable for indoor or outdoor wall mounting on junction box, or with surface conduit using the supplied 1/2" threaded top access • Mounting plate has molded universal mounting pattern for simple mounting over junction box.





#### ACEM Model (NiCad Battery Backup)

Integral photocell: Unit operates as a dusk to dawn luminaire and in the event of a power failure as an emergency light. **Remote Switched**: The integral photocell can be defeated to allow remote switching for normal operation. In the event of a power

failure unit operates as an emergency light.











ORDERING INFORMATION			
model	operation mode	housing color	options
MERU-LED	ACEM = General & Emergency Lighting	DB = Dark Bronze	Self-Diagnostics & Photocell (Included Standard)
	AC = General Lighting	WH = White	IH = Internal Heater
		BK = Black	PIR = Passive Infra-Red Motion Sensor
Ordering Example:	MERU-ACEM-DB	NK = Nickel	

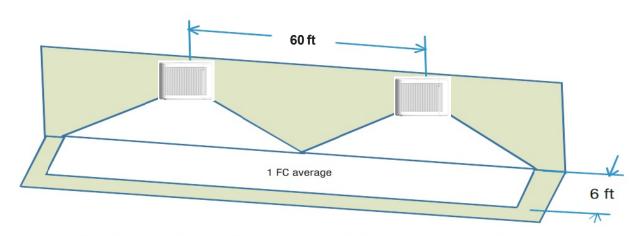
# 5/1

## **MERU Series**

PROJECT:
FIXTURE TYPE:
LOCATION:
CONTACT/PHONE:

#### LED GENERAL & EMERGENCY LIGHTING

#### **PHOTOMETRICS**



**Note:** Meets Life Safety Code standard minimum illuminance of 0.1 FC and average illuminance of 1.0 FC. Illustration shown is a guideline for corridor center-to-center with 9 ft mounting height and Minimum 80-50-20 reflectance values.

Mounting Height	Center to center distance
7.2ft	45ft
9ft	60ft
10ft	65ft

#### **SELF DIAGNOSTICS**

#### **Included Self Diagnostic**



Full self-test, self-diagnostic system is standard in every unit, performs a monthly, test as well as continuously monitoring all functions to ensure reliability, a manual test may be initiated at any time



## **Steel Poles**



**SSS** SQUARE STRAIGHT STEEL

Catalog #	Туре
Project	
Comments	Date
Prepared by	

#### **FEATURES**

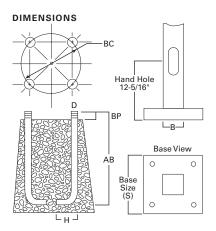
- ASTM Grade steel base plate with ASTM A366 base cover
- Hand hole assembly 3" x 5" on 5" and 6" pole; and 2" x 4" on 4" pole
- 10'-39' mounting heights
- Drilled or tenon (specify)

#### ORDERING INFORMATION

SAMPLE NUMBER: SSS5A20SFM1XG

Product Family	Shaft Size (Inches) <sup>1</sup>	Wall Thickness (Inches)	Mounting Height (Feet)	Base Type	Finish	Mounting Type	Number and Location of Arms	Arm Lengths (Feet)	Options (Add as Suffix)
SSS=Square Straight Steel	4=4" 5=5" 6=6"	A=0.120" M=0.188" X=0.250"	10=10' 15=15' 20=20' 25=25' 30=30' 35=35' 39=39'	S=Square Steel Base	F=Dark Bronze G=Galvanized Steel J=Summit White K=Carbon Bronze L=Dark Platinum P=Primer Powder Coat R=Hartford Green S=Silver T=Graphite Metallic V=Grey W=White X=Custom Color Y=Black	2=2-3/8" O.D.Tenon (4" Long) 3=3-1/2" O.D.Tenon (5" Long) 4=4" O.D.Tenon (6" Long) 5=3" O.D.Tenon (6" Long) 6=2-3/8" O.D.Tenon (6" Long) 7=4" O.D.Tenon (10" Long) A=Type A Drilling C=Type C Drilling E=Type E Drilling F=Type F Drilling G=Type J Drilling J=Type J Drilling M=Type J Drilling M=Type M Drilling R=Type R Drilling R=Type R Drilling	1=Single 2=2 at 180° 3=Triple <sup>2</sup> 4=4 at 90° 5=2 at 90° X=None	X=None	A=1/2"Tapped Hub (Specify location desired) B=3/4"Tapped Hub (Specify location desired) C=Convenience Outlet <sup>3</sup> E=GFCI Convenience Outlet <sup>3</sup> G=Ground Lug H=Additional Hand Hole <sup>4</sup> L=Drilled for Bumper Glitter V=Vibration Dampener

NOTES: 1. All shaft sizes nominal. 2. Square poles are 3 at 90°, round poles are 3 at 120°. 3. Outlet is located 4' above base and on same side of pole as hand hole, unless specified otherwise. Receptacle not included, provision only. 4. Additional hand hole is located 12" below pole top and 90° from standard hand hole location, unless otherwise specified.



WARNING: Customer is responsible for engineering analysis to confirm pole and fixture compatibility for all applications. Refer to pole white paper WP513001EN for additional support information. Before installing, make sure proper anchor bolts and templates are obtained. The use of unauthorized accessories such as banners, signs, cameras or pennants for which the pole warranty and may result in pole failure causing serious injury or property damage. Information regarding total loading capacity can be supplied upon request. The pole warranty is void unless poles are used and installed as a complete pole and luminaire combination. This warranty specifically excludes failure as the result of a third party act or omission, misuse, unanticipated uses, fatigue failure or similar phenomena resulting from induced vibration, harmonic oscillation or resonance associated with movement of air currents around the product.

Specifications and dimensions subject to change without notice. Consult your lighting representative at Eaton or visit www.eaton.com/lighting for available options, accessories and ordering information.



#### Effective Projected Area (At Pole Top)

Mounting Height (Feet)	Catalog Number <sup>1, 2</sup>	Wall Thickness (Inches)	Base Square <sup>3</sup> (Inches)	Bolt Circle Diameter (Inches)	Anchor Bolt Projection <sup>3</sup> (Inches)	Shaft Size <sup>3</sup> (Inches)	Anchor Bolt Diameter x Length x Hook (Inches)	Net Weight (Pounds)	Maximum Effective Projected Area (Square Feet) <sup>4</sup>		ed Area	Max. Fixture Load - Includes Bracket (Pounds)	
МН			s	ВС	ВР	В	D x AB x H		80 mph	90 mph	100 mph	110 mph	
10	SSS4A10S	0.120	10-1/2	11	4-1/2	4	3/4 x 25 x 3	85	30.0	22.0	17.0	13.0	100
15	SSS4A15S	0.120	10-1/2	11	4-1/2	4	3/4 x 25 x 3	118	15.0	11.5	8.7	6.5	100
20	SSS4A20S	0.120	10-1/2	11	4-1/2	4	3/4 x 25 x 3	150	8.7	5.9	3.9	2.5	150
20	SSS5A20S	0.120	10-1/2	11	4-1/2	5	3/4 x 25 x 3	183	15.4	11.1	7.9	5.5	150
25	SSS4A25S	0.120	10-1/2	11	4-1/2	4	3/4 x 25 x 3	181	3.7	1.7	0.3		200
25	SSS5A25S	0.120	10-1/2	11	5	5	3/4 x 25 x 3	222	9.3	6.0	3.5	1.6	200
25	SSS6A25S	0.120	12-1/2	12-1/2	5	6	1 x 36 x 4	284	9.9	6.1	3.5	1.2	200
30	SSS5A30S	0.120	10-1/2	11	4-1/2	5	3/4 x 25 x 3	260	4.7	2.1			200
30	SSS5M30S	0.188	10-1/2	11	4-1/2	5	3/4 x 25 x 3	392	10.4	6.4	3.5	1.5	200
30	SSS6A30S	0.120	12-1/2	12-1/2	5	6	1 x 36 x 4	330	4.3	1.4			200
30	SSS6M30S	0.188	12-1/2	12-1/2	5	6	1 x 36 x 4	489	19.0	13.0	8.7	5.6	200
35	SSS5M35S	0.188	10-1/2	11	4-1/2	5	3/4 x 25 x 3	453	5.8	2.8			200
35	SSS6M35S	0.188	12-1/2	12-1/2	5	6	1 x 36 x 4	564	12.8	7.2	3.7	1.0	200
35	SSS6X35S	0.250	12-1/2	12-1/2	5	6	1 x 36 x 4	738	16.5	11.0	6.8	3.5	200
39	SSS6M39S	0.188	12-1/2	12-1/2	5	6	1 x 36 x 4	618	7.3	3.0			300
39	SSS6X39S	0.250	12-1/2	12-1/2	5	6	1 x 36 x 4	816	13.0	7.0	3.7	0.8	300

#### Effective Projected Area (Two Feet Above Pole Top)

Effective Projected Area (Two Feet Above Pole Top)													
Mounting Height (Feet)	Catalog Number <sup>1, 2</sup>	Wall Thickness (Inches)	Base Square <sup>3</sup> (Inches)	Bolt Circle Diameter (Inches)	Anchor Bolt Projection <sup>3</sup> (Inches)	Shaft Size <sup>3</sup> (Inches)	Anchor Bolt Diameter x Length x Hook (Inches)	Net Weight (Pounds)	Maximum Effective Projected Area (Square Feet) <sup>4</sup>		Max. Fixture Load - Includes Bracket (Pounds)		
МН			s	ВС	ВР	В	D x AB x H		80 mph	90 mph	100 mph	110 mph	
10	SSS4A10S	0.120	10-1/2	11	4-1/2	4	3/4 x 25 x 3	85	23.0	17.5	14.0	11.0	100
15	SSS4A15S	0.120	10-1/2	11	4-1/2	4	3/4 x 25 x 3	118	13.4	10.0	7.5	5.7	100
20	SSS4A20S	0.120	10-1/2	11	4-1/2	4	3/4 x 25 x 3	150	7.6	5.2	3.4	2.1	150
20	SSS5A20S	0.120	10-1/2	11	4-1/2	5	3/4 x 25 x 3	183	13.8	9.9	7.1	4.9	150
25	SSS4A25S	0.120	10-1/2	11	4-1/2	4	3/4 x 25 x 3	181	3.4	1.6	0.3		200
25	SSS5A25S	0.120	10-1/2	11	5	5	3/4 x 25 x 3	222	8.5	5.5	3.2	1.5	200
25	SSS6A25S	0.120	12-1/2	12-1/2	5	6	1 x 36 x 4	284	9.1	5.6	3.0	1.2	200
30	SSS5A30S	0.120	10-1/2	11	4-1/2	5	3/4 x 25 x 3	260	1.8				200
30	SSS5M30S	0.188	10-1/2	11	4-1/2	5	3/4 x 25 x 3	392	9.6	5.9	1.9	0.2	200
30	SSS6A30S	0.120	12-1/2	12-1/2	5	6	1 x 36 x 4	330	4.1	1.3			200
30	SSS6M30S	0.188	12-1/2	12-1/2	5	6	1 x 36 x 4	489	18.5	12.5	8.4	5.3	200
35	SSS5M35S	0.188	10-1/2	11	4-1/2	5	3/4 x 25 x 3	453	5.5	2.4			200
35	SSS6M35S	0.188	12-1/2	12-1/2	5	6	1 x 36 x 4	564	11.8	7.0	3.5	1.0	200
35	SSS6X35S	0.250	12-1/2	12-1/2	5	6	1 x 36 x 4	738	16.0	10.5	6.4	3.4	200
39	SSS6M39S	0.188	12-1/2	12-1/2	5	6	1 x 36 x 4	618	7.0	2.4			300
39	SSS6X39S	0.250	12-1/2	12-1/2	5	6	1 x 36 x 4	816	12.0	6.7	3.0	0.5	300

#### NOTES:

1. Catalog number includes pole with hardware kit. Anchor bolts not included. Before installing, make sure proper anchor bolts and templates are obtained.

Specifications and dimensions subject to change without notice.

- 2. Tenon size or machining for rectangular arms must be specified. Hand hole position relative to drill location.

  3. Shaft size, base square, anchor bolts and projections may vary slightly. All dimensions nominal.

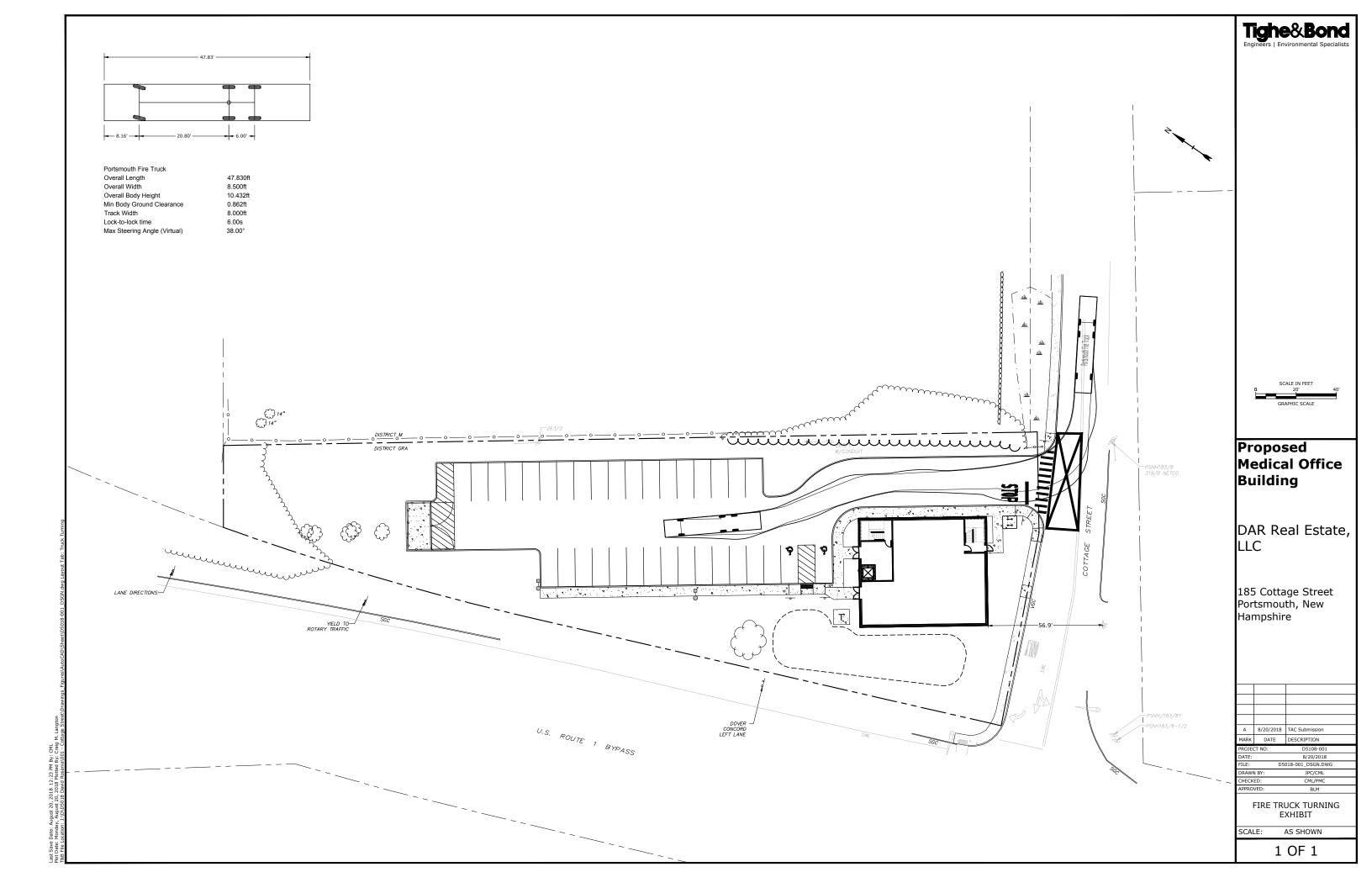
  4. EPAs based on shaft properties with wind normal to flat. EPAs calculated using base wind velocity as indicated plus 30% gust factor.



#### **AUTHORIZATION**

The undersigned, Colman C. Garland, owner of the property located at 185 Cottage Street, Portsmouth, New Hampshire and identified as Portsmouth Tax Map 19, Lot 6 (the "Property"), hereby authorize DAR Real Estate, LLC ("DAR") and its advisors Tighe & Bond and Hoefle, Phoenix, Gormley and Roberts, P.A., to file documents and appear before the Portsmouth Zoning Board of Adjustment, Planning Board and/or Technical Advisory Committee in all matters relating to applications by DAR to permit the construction of a medical office building on the Property.

Colman C. Garland, Owner





9/10/18

DAR Real Estate, LLC 185 Cottage Street Portsmouth, NH 03801

RE: Natural gas service to 185 Cottage Street, Portsmouth, NH

Unitil's natural gas division has reviewed the requested site for natural gas service.

Unitil hereby confirms natural gas is available from Cottage Street to supply the proposed lot development.

Please contact me with any questions at 603-294-5144.

Sincerely,

David Beaulieu

**Business Development Executive** 

Unitil

325 West Road

Portsmouth, NH 03801



September 12, 2018

Craig Langton Tighe & Bond 177 Corporate Drive Portsmouth, NH 03801

Dear Mr. Langton:

1700 Lafayette Road Portsmouth, NH 03801

Michael J Busby 603-436-7708 x555-5678 michael.busby@eversource.com

I am responding to your request to confirm the availability of electric service for the proposed 185 Cottage Street Portsmouth NH, 03801 project being constructed for/by DAR Real Estate, LLC.

The proposed project consists of a 2-story medical office building (±6,800 SF), and associated site improvements. The proposed development will be constructed along Cottage Street and US Route 1 Bypass.

The developer will be responsible for the installation of all underground and overhead facilities and infrastructure improvements required to service the new building. The proposed building service is to be fed from existing utility pole on site (263/1) as depicted on Utilities Plan Sheet C-104. Three phase overhead conductors will need to be extended from Cottage street to the proposed riser pole (263/1). The developer will work with Eversource to obtain all necessary tree trimming, easements, and licenses for the proposed underground and overhead facilities listed above.

This letter serves as confirmation that Eversource has sufficient capacity in the area to provide service to this proposed development. The cost of extending service to the aforementioned location and any associated infrastructure improvements necessary to provide service will be borne by the developer unless otherwise agreed upon.

The attached drawing titled "Utilities Plan" dated 8/20/2018, shows transformer location and proposed underground conduit to service your proposed project.

Eversource approves the locations shown; assuming the final installed locations meet all clearances, physical protection, and access requirements as outlined in Eversource's "Information & Requirements For Electric Supply" (https://www.eversource.com/content/docs/default-source/pdfs/requirements-for-electric-service-connections.pdf?sfvrsn=2).

If you require additional information or I can be of further assistance please do not hesitate to contact me at our Portsmouth Office, 603-436-7708 Ext. 555-5678

Respectfully.

Michael J. Busby, PE

NH Eastern Regional Engineering and Design Manager, Eversource

cc: (via e-mail)

Michael Lee, Eastern Region Operations Manager, Eversource Mary Jo Hanson, Field Supervisor, Electric Design, Eversource Technical Memorandum Tighe&Bond

## **Drainage Analysis**

To: City of Portsmouth Technical Advisory Committee (TAC)

FROM: Patrick M. Crimmins, P.E., Tighe & Bond

Craig M. Langton, P.E., Tighe & Bond

CC: DAR Real Estate, LLC

**DATE:** August 20, 2018 **Revised:** September 18, 2018

## 1.0 Project Description

The existing Site is comprised of a 0.90-acre lot which currently includes two residential structures. The proposed redevelopment includes the demolition of the two existing residential structures and construction of a two (2) story medical office building and associated site improvements. These site improvements include a stormwater management system which includes a proposed rain garden and an underground detention/infiltration system. The topography of the site generally slopes northwest to southwest from the rear of the site towards Cottage Street. Based on the NRCS Web Soil Survey for Rockingham County, the site consists of Urban Land-canton Complex. Stormwater runoff analyzed within this study has been modeled assuming hydrologic soils group C and an infiltration rate of 0.30 inches/hour.

For the purposes of this analysis, runoff generated by the site has been analyzed at a single distinct point of analysis, existing catch basin CB 2065. CB 2065 is part of the existing closed drainage system within Cottage Street and US Route 1 Bypass which ultimately drains into Hodgson Brook.

## 2.0 Drainage Analysis

#### 2.1 Calculation Methods

The parcels on-site watersheds were analyzed under this section. The design storms analyzed in this study are the 2-year, 10-year, 25-year, and 50-year 24-hour duration storm as per NHDES AoT Regulations (Env-Wq 1500). The stormwater modeling system, HydroCAD 10.0 was utilized to predict the peak runoff rates from these storm events. A Type III storm pattern was used in the model.

The time of concentration was computed using the TR-55 Method, which provides a means of determining the time for an entire watershed to contribute runoff to a specific location via sheet flows, shallow concentrated flow and channel flow. Runoff curve numbers were calculated by estimating the coverage areas and then summing the curve number for the coverage area as a percent of the entire watershed.

#### References

- 1. HydroCAD Stormwater Modeling System, by HydroCAD Software Solutions LLC, Chocorua, New Hampshire.
- New Hampshire Stormwater Management Manual, Volume 2, Post-Construction Best Management Practices Selection and Design, December 2008.
- 3. "Extreme Precipitation in New York & New England." Extreme Precipitation in New York & New England by Northeast Regional Climate Center (NRCC), 26 June 2012.

**Tighe**&**Bond** 

## 2.2 Pre-Development Conditions

Analyzing the pre-development condition was accomplished by modeling the entire property as one (1) watershed which all ultimately drains to the point of analysis, CB 2065.

The point of analysis and the contributing watershed area is described below:

#### Point of Analysis One (CB 2065)

Runoff from the site travels northwest to southwest from the rear of the site towards Cottage Street, where is enters the closed drainage system and ultimately flows through CB 2065. A visual representation of the designed stormwater runoff characteristics for the overall proposed pre-development watershed can be found on the attached plan entitled "Pre-Development Watershed Plan", Sheet C-801.

## 2.3 Post-Development Conditions

Analyzing the post-development drainage condition is characterized by six (6) watershed areas. Ultimately all of the post-development stormwater runoff is directed to the point of analysis, CB 2065.

The routing of the six (6) sub watershed areas ultimately through the point of analysis, CB 2065, is described below:

Each of the points of analysis and their contributing watershed areas are described below:

#### Point of Analysis One (CB 2065)

Modeling the post development drainage required dividing the overall post development watershed into six (6) distinct sub areas, to demonstrate the modeled treatment and detention capabilities of the proposed rain garden and Filterra Bioretention System. All new impervious areas in the post-development (i.e. paved drives, concrete sidewalks and clean roof runoff) are directed through either the rain garden or Filterra Bioretention System. This provides water quality treatment as well as detention/retention to reduce peak flow rates as compared to the pre-development drainage analysis. A visual representation of the designed stormwater runoff characteristics for the overall proposed post-development watershed can be found on the attached plan entitled "Post-Development Watershed Plan", Sheet C-802.

## 2.4 Peak Rate Comparisons

Table 2.4.1 summarizes and compares the pre- and post-development peak runoff rates for the 1-year, 2-year, 10-year, 25-year, and 50-year storm events at each discharge point.

Table 2.4.1 - Comparison of Pre- and Post-Development flows (cfs)							
2-Year 10-Year 25-Year 50-Year Storm Storm Storm Storm							
Pre-Development Watershed							
PA-1 (CB 2065)	1.69	3.50	5.00	6.44			
Post-Development Watershed							
PA-1 (CB 2065)	1.43	2.33	3.01	3.65			

As depicted in Table 2.4.1, the post-development peak runoff rates are less than the predevelopment rates. TECHNICAL MEMORANDUM Tighe&Bond

## 2.5 Groundwater Recharge

The City of Portsmouth Site Review Regulations indicates efforts shall be made to infiltrate runoff throughout the site. The project will result in an increase of approximately 0.37 acres of impervious area. Based on the NRCS Web Soil Survey, the runoff analyzed within this study has been modeled assuming hydrologic soil group (HSG) C.

Infiltration will be provided on-site by the proposed rain garden. The designed infiltration rate of the system has been modeled at a conservative rate of 0.30 inches per hour (in./hr.) which is based on the NRCS Web Soil Survey, the minimal infiltration rate for the site's soils.

The proposed project is required to recharge 133 CF of runoff to groundwater based on the NH Stormwater Manual criteria. As shown in the enclosed groundwater recharge volume calculations, the proposed rain garden will provide 2,681 CF of storage below the overflow grate.

#### 2.6 Stormwater Treatment

The stormwater management system has been designed to provide stormwater treatment as required by the City of Portsmouth Site Review.

The stormwater management system includes a proposed rain garden and Filterra Bioretention System for the treatment of stormwater runoff. Prior to the entering the proposed rain garden, runoff will be pre-treated with off-line catch basins equipped with oil separator hoods and deep sumps.

## 2.7 Summary

The proposed project will result in a reduction in post-development peak runoff rates from the pre-development condition. The impervious area resulting from the proposed project will be treated by the proposed rain garden and underground detention/infiltration system. The project's proposed rain garden and underground detention/infiltration system will provide groundwater recharge and stormwater treatment that meet the criteria of the NH Stormwater Manual.

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## 3.0 Long Term Operation & Maintenance Plan

It is the intent of this Operation and Maintenance Plan to identify the areas of this site that need special attention and consideration, as well as implementing a plan to assure routine maintenance. By identifying the areas of concern as well as implementing a frequent and routine maintenance schedule the site will maintain a high quality stormwater runoff.

## 3.1 Contact/Responsible Party

DAR Real Estate, LLC 875 Greenland Road, Suite B-7 Portsmouth NH, 03801 603.294.0110

(Note: The contact information for the Contact/Responsible Party shall be kept current. If ownership changes, the Operation and Maintenance Plan must be transferred to the new party.)

#### 3.2 Maintenance Items

Maintenance of the following items shall be recorded:

- Litter/Debris Removal
- Landscaping
- Catch basin Cleaning
- Pavement Sweeping
- Rain Garden Maintenance
- Rip Rap Maintenance
- Filterra Bioretention System

The following maintenance items and schedule represent the minimum action required. Periodic site inspections shall be conducted, and all measures must be maintained in effective operating condition. The following items shall be observed during site inspection and maintenance:

- Inspect vegetated areas, particularly slopes and embankments for areas of erosion.
   Replant and restore as necessary
- Inspect catch basins for sediment buildup
- Inspect site for trash and debris

## 3.3 Overall Site Operation & Maintenance Schedule

Overall Site Operation and	d Maintenance Schedule		
Maintenance I tem	Frequency of Maintenance	Operation	
Litter/Debris Removal	Weekly	Management Company	
Pavement Sweeping  • Sweep impervious areas to remove sand and litter.	Annually	Parking Lot Sweeper	
Catch Basin (CB) Cleaning  CB to be cleaned of solids and oils.	Annually	Vacuum Truck	
Landscaping  • Landscaped islands to be maintained and mulched.	Maintained as required and mulched each Spring	Management Company	
Rain Garden  - Trash and debris to be removed.  - Any required maintenance shall be addressed.	Two (2) times annually and after any rainfall event exceeding 2.5" in a 24-hr period	Management Company	
Rip Rap Aprons  - Trash and debris to be removed.  - Any required maintenance shall be addressed.	Annually	Management Company	
Filterra Bioretention System *  Remove debris from inlet and outlet structures.  Trash and debris to be removed from mulched area  Assessment of plant condition.	Periodically (At least two (2) times annually)	Management Company	

<sup>\*</sup> Filterra Bioretention System Operation and Maintenance require shall meet or exceed the manufactures requirements.

Rain Garde	Rain Garden Inspection/Maintenance Requirements						
Inspection/ Maintenance	Frequency	Action					
Monitor to ensure that Rain Gardens function effectively after storms	Two (2) times annually and after any rainfall event exceeding 2.5" in a 24-hr period	- Trash and debris to be removed - Any required maintenance shall be addressed					
Inspect Vegetation	Annually	<ul> <li>Inspect the condition of all Rain</li> <li>Garden vegetation</li> <li>Prune back overgrowth</li> <li>Replace dead vegetation</li> <li>Remove any invasive species</li> </ul>					
Inspect Drawdown Time - The system shall drawdown within 48- hours following a rainfall event.	Annually	- Assess the condition of the facility to determine measures required to restore the filtration function, including but not limited to removal of accumulated sediments or reconstruction of the filter.					

Rip Rap Inspection/Maintenance Requirements						
Inspection/ Frequency Action						
Maintenance						
Visual Inspection	Annually	- Visually inspect for damage and deterioration				
		- Repair damages immediately				

Filterra Bioretention System Inspection & Maintenance Requirements*						
Inspection/ Maintenance	Frequency	Action				
Visual Inspection	Two (2) times annually	<ul> <li>Remove trash and debris as needed.</li> <li>Remove accumulated sediment.</li> <li>Trash and debris should be removed, and mulch cover raked level. Ensure bark nugget mulch is not used.</li> <li>Replace mulch as a minimum, if ponding observed.</li> <li>Trim/prune plants in accordance with typical landscaping and safety needs.</li> </ul>				

<sup>\*</sup> Filterra Bioretention System Operation and Maintenance require shall meet or exceed the manufactures requirements.

## 3.4 Disposal Requirements

Disposal of debris, trash, sediment and other waste material should be done at suitable disposal/recycling sites and in compliance with all applicable local, state and federal waste regulations.

## 3.5 Snow & Ice Management for Standard Asphalt and Walkways

Snow storage areas shall be located such that no direct untreated discharges are possible to receiving waters from the storage site (snow storage areas have been shown on the Site Plan). Salt storage areas shall be covered or located such that no direct untreated discharges are possible to receiving waters from the storage site. Salt and sand shall be used to the minimum extent practical (refer to the New Hampshire Stormwater Management Manual, Volume 2, for de-icing application rate guidelines). Snow and ice removal shall be performed by a contractor certified by the "Green SnoPro" program, or approved equivalent, following best management practices for the application of deicing materials.

Typical Deicing Log Fo	orm	
Truck Station:		
Date:		
Air Temperature Pavement Temp. Relative Humidity	<u>Dew Point</u>	<u>Sky</u>
Reason for applying:		
Route:		
Chemical:		
Application Time:		
Application Amount:		
Observation (first day):		
Observation (after event):		
Observation (before next application):		
Name:		

TECHNICAL MEMORANDUM

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## 3.6 Annual Updates & Log Requirements

The Owner and/or Contact/Responsible Party shall review this Operation and Maintenance Plan once per year for its effectiveness and adjust the plan and deed as necessary.

A log of all preventative and corrective measures for the stormwater system shall be kept onsite and be made available upon request by any public entity with administrative, health environmental or safety authority over the site. The flowing report logs and check lists are typical for overall site operation and maintenance, rain gardens and the Filterra Bioretention System.

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	Typical Overall Site Operation and Maintenance Report Log					
Project Name:						
Observation Item	Date of Inspection	Observer	Maintenance Needed?	Comments	Date of Cleaning/ Repair	Performed By
			□Yes □No			
			□Yes □No			
			□Yes □No			
			□Yes □No			
			□Yes □No			
			□Yes □No			
			□Yes □No			
			□Yes □No			

J:\D\D5018 David Rosania\001 - Cottage Street\Report\_Evaluation\Applications\City of Portsmouth\20180918\_TAC Responce\D5018-001\_Drainage Analysis\_Revised-1.docx

## **Maintenance Checklist**

Drainage System Failure	Problem	Conditions to Check	Condition that Should Exist	Actions
Inlet	Excessive sediment or trash accumulation.	Accumulated sediments or trash impair free flow of water into Filterra.	Inlet should be free of obstructions allowing free distributed flow of water into Filterra.	Sediments and/or trash should be removed.
Mulch Cover	Trash and floatable debris accumulation.	Excessive trash and/or debris accumulation.	Minimal trash or other debris on mulch cover.	Trash and debris should be removed and mulch cover raked level. Ensure bark nugget mulch is not used.
Mulch Cover	"Ponding" of water on mulch cover.	"Ponding" in unit could be indicative of clogging due to excessive fine sediment accumulation or spill of petroleum oils.	Stormwater should drain freely and evenly through mulch cover.	Recommend contact manufacturer and replace mulch as a minimum.
Vegetation	Plants not growing or in poor condition.	Soil/mulch too wet, evidence of spill. Incorrect plant selection. Pest infestation. Vandalism to plants.	Plants should be healthy and pest free.	Contact manufacturer for advice.
Vegetation	Plant growth excessive.	Plants should be appropriate to the species and location of Filterra.		Trim/prune plants in accordance with typical landscaping and safety needs.
Structure	Structure has visible cracks.	Cracks wider than 1/2 inch or evidence of soil particles entering the structure through the cracks.		Vault should be repaired.
Maintenance is ideally	y to be performed twice an	nually.		

## Filterra Inspection & Maintenance Log Filterra System Size/Model: Location: \_\_\_\_\_\_

	,						
Date	Mulch & Debris Removed	Depth of Mulch Added	Mulch Brand	Height of Vegetation Above Grate	Vegetation Species	Issues with System	Comments
1/1/17	5 – 5 gal Buckets	3″	Lowe's Premium Brown Mulch	4'	Galaxy Magnolia	- Standing water in downstream structure	- Removed blockage in downstream structure

## Regular Inspection and Maintenance Guidance for Bioretention Systems / Tree Filters

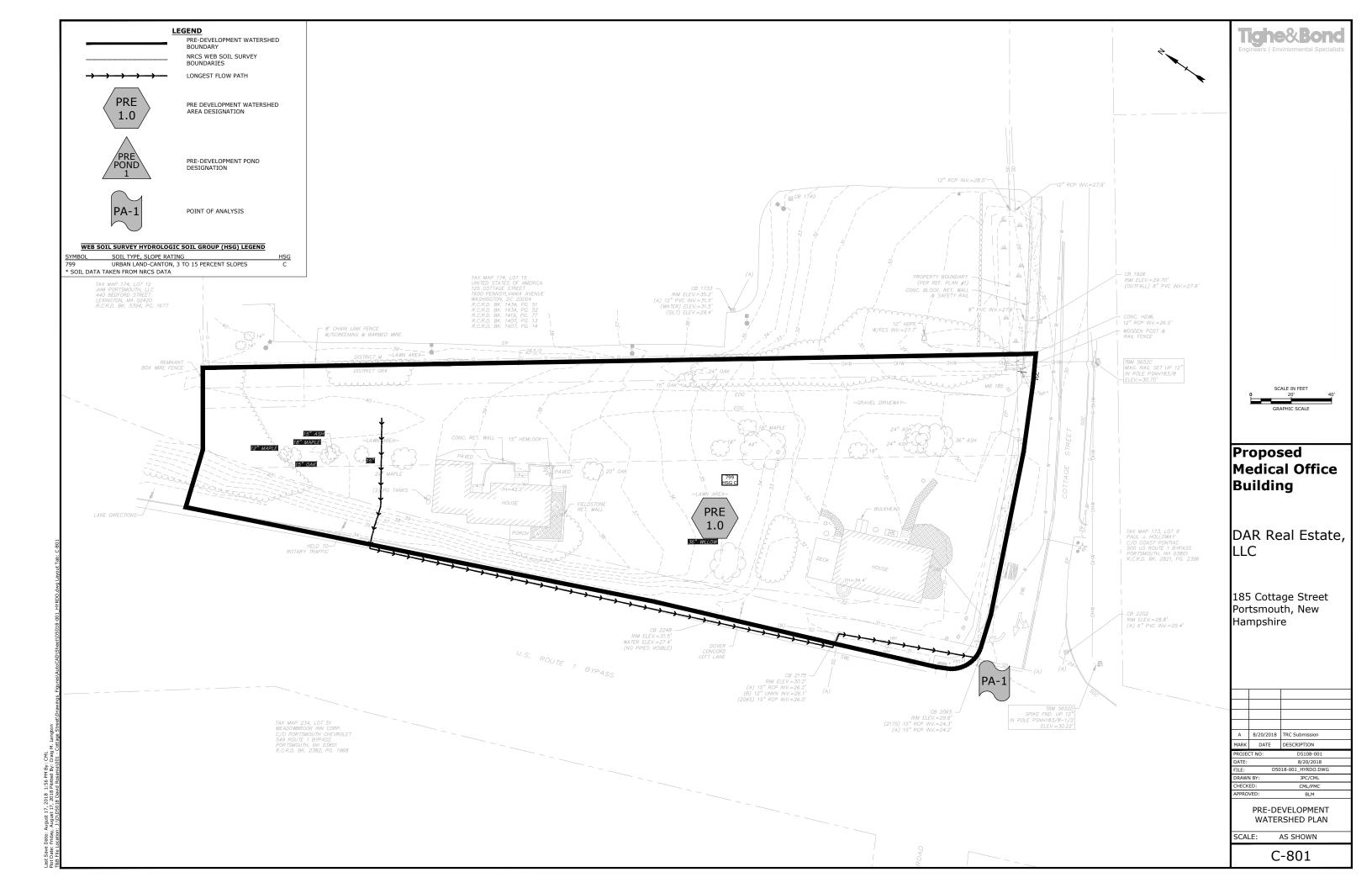
Maintenance of bioretention systems and tree filters can typically be performed as part of standard landscaping. Regular inspection and maintenance is critical to the effective operation of bioretention systems and tree filters to insure they remain clear of leaves and debris and free draining. This page provides guidance on maintenance activities that are typically required for these systems, along with the suggested frequency for each activity. Individual systems may have more, or less frequent maintenance needs depending on a variety of factors including but not limited to: the occurrence of large storm events, overly wet or dry periods, regional hydrologic conditions, and the upstream land use.

#### **ACTIVITIES**

The most common maintenance activity is the removal of sediment and organic debris from the system and bypass structures. Visual inspections are routine for system maintenance. This includes looking for standing water, accumulated leaves, holes in the soil media, signs of plant distress, and debris and sediment accumulation in the system. Vegetation coverage is integral to the performance of the system, including infiltration rate and nutrient uptake. Vegetation care is important to system productivity and health.

ACTIVITY	FREQUENCY			
CLOGGING AND SYSTEM PERFORMANCE				
A record should be kept of the time to drain for the system completely after a storm event. The system should drain completely within 72 hours.  Check to insure the filter surface remains well draining after storm events.	After every major storm in the first few months, then annually			
<b>Remedy</b> : If filter bed is clogged, draining poorly, or standing water covers more than 50% of the surface 48 hours after a precipitation event, then remove top few inches of discolored material. Till, or rake remaining material as needed.	at minimum.			
Check inlets and outlets for leaves and debris.				
<b>Remedy</b> : Rake in and around the system to clear it of debris. Also, clear the inlet and overflow if obstructed.				
Check for animal burrows and short-circuiting in the system.	Quarterly initially, annually as a minimum thereafter.			
<b>Remedy:</b> Soil erosion from short circuiting or animal boroughs should be repaired when they occur. The holes should be filled and lightly compacted				
Inspect inlets and outlets to ensure good condition and no evidence of deterioration. Check to see if high-flow bypass is functioning.  Remedy: Repair or replace any damaged structural parts, inlets, outlets, sidewalls.				
VEGETATION				
Check for robust vegetation coverage throughout the system and dead or dying plants.  Remedy: Vegetation should cover > 75% of the system and should be cared for	Annually or as needed			
as needed.				

CHECKLIST FOR INSPECTION OF BIORETENTION SYSTEM / TREE FILTERS				
Location:				
Inspector:				
Date:				
Time:				
Site Conditions:				
Days Since Last Rain Event:				
Inspection Items		ory (S) or actory (U)	Comments/Corrective Action	
1. Initial Inspection After Planting and Mulching				
Plants are stable, roots not exposed	S	U		
Surface is at design level, no evidence of preferential flow/shoving	S	U		
Inlet and outlet/bypass are functional	S	U	7	
2. Debris Cleanup (1 time/year minimum, Spring/Fall)				
Litter, leaves, and dead vegetation removed from the system	S	U		
Prune/mow vegetation	S	U		
3. Standing Water (1 time/year and/or after large storm even	ents)			
No evidence of standing water after 24-48 hours since rainfall	S	U		
4. Vegetation Condition and Coverage				
Vegetation condition good with good coverage (typically > 75%)	S	U		
5. Other Issues				
Note any additional issues not previously covered.	S	U		
Corrective Action Needed			Due Date	
1.				
2.				
3.				
Inspector Signature			Date	













## **Area Listing (all nodes)**

Area	CN	Description
(acres)		(subcatchment-numbers)
0.632	74	>75% Grass cover, Good, HSG C (PRE 1.0)
0.145	96	Gravel surface, HSG C (PRE 1.0)
0.016	98	Paved parking, HSG C (PRE 1.0)
0.096	98	Unconnected roofs, HSG C (PRE 1.0)
0.172	70	Woods, Good, HSG C (PRE 1.0)
1.061	79	TOTAL AREA

Type III 24-hr 2-year Rainfall=3.21"

Prepared by Tighe & Bond

HydroCAD® 10.00-20 s/n 03436 © 2017 HydroCAD Software Solutions LLC

Printed 8/16/2018 Page 3

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PRE 1.0:

Runoff Area=46,210 sf 10.61% Impervious Runoff Depth>1.18" Flow Length=406' Tc=3.2 min UI Adjusted CN=78 Runoff=1.69 cfs 0.104 af

Link PA1:

Inflow=1.69 cfs 0.104 af Primary=1.69 cfs 0.104 af

Total Runoff Area = 1.061 ac Runoff Volume = 0.104 af Average Runoff Depth = 1.18" 89.39% Pervious = 0.948 ac 10.61% Impervious = 0.113 ac

Type III 24-hr 10-year Rainfall=4.87"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PRE 1.0: Runoff Area=46,210 sf 10.61% Impervious Runoff Depth>2.42"

Flow Length=406' Tc=3.2 min UI Adjusted CN=78 Runoff=3.50 cfs 0.214 af

**Link PA1:**Inflow=3.50 cfs 0.214 af
Primary=3.50 cfs 0.214 af

Total Runoff Area = 1.061 ac Runoff Volume = 0.214 af Average Runoff Depth = 2.42" 89.39% Pervious = 0.948 ac 10.61% Impervious = 0.113 ac

Type III 24-hr 25-year Rainfall=6.17"

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Page 5

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PRE 1.0:

Runoff Area=46,210 sf 10.61% Impervious Runoff Depth>3.49"

Flow Length=406' Tc=3.2 min UI Adjusted CN=78 Runoff=5.00 cfs 0.309 af

Link PA1:

Inflow=5.00 cfs 0.309 af Primary=5.00 cfs 0.309 af

Total Runoff Area = 1.061 ac Runoff Volume = 0.309 af Average Runoff Depth = 3.49" 89.39% Pervious = 0.948 ac 10.61% Impervious = 0.113 ac

Page 6

## **Summary for Subcatchment PRE 1.0:**

Runoff = 5.00 cfs @ 12.05 hrs, Volume= 0.309 af, Depth> 3.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-year Rainfall=6.17"

_	Α	rea (sf)	CN	Adj Desc	j Description		
		6,317	96	Grav	Gravel surface, HSG C		
		7,478	70	Woo	Woods, Good, HSG C		
		4,198	98	Unconnected roofs, HSG C			
		27,511	74	>75% Grass cover, Good, HSG C			
706 98				Pave	Paved parking, HSG C		
		46,210	79	78 Weig	Weighted Average, UI Adjusted		
41,306 89.39% Pervious Area							
4,904 10.61% Impervious Area							
	4,198 85.60% Unconnected						
	_		0.1		<b>.</b>		
	Tc	Length	Slope		Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	1.1	10	0.0500	0.16		Sheet Flow,	
						Grass: Short n= 0.150 P2= 3.21"	
	0.3	86	0.0756	4.12		Shallow Concentrated Flow,	
	4 7	0.44	0.0407	0.00		Grassed Waterway Kv= 15.0 fps	
	1.7	241	0.0137	2.38		Shallow Concentrated Flow,	
	0.4	00	0.0040	0.04	40.00	Paved Kv= 20.3 fps	
	0.1	69	0.0246	8.94	10.98	·	
						15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'	
-						n= 0.012 Concrete pipe, finished	
	3.2	406	Total				

Type III 24-hr 25-year Rainfall=6.17"

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Page 7

## **Summary for Link PA1:**

Inflow Area = 1.061 ac, 10.61% Impervious, Inflow Depth > 3.49" for 25-year event

Inflow = 5.00 cfs @ 12.05 hrs, Volume= 0.309 af

Primary = 5.00 cfs @ 12.05 hrs, Volume= 0.309 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Type III 24-hr 50-year Rainfall=7.39"

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Page 8

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

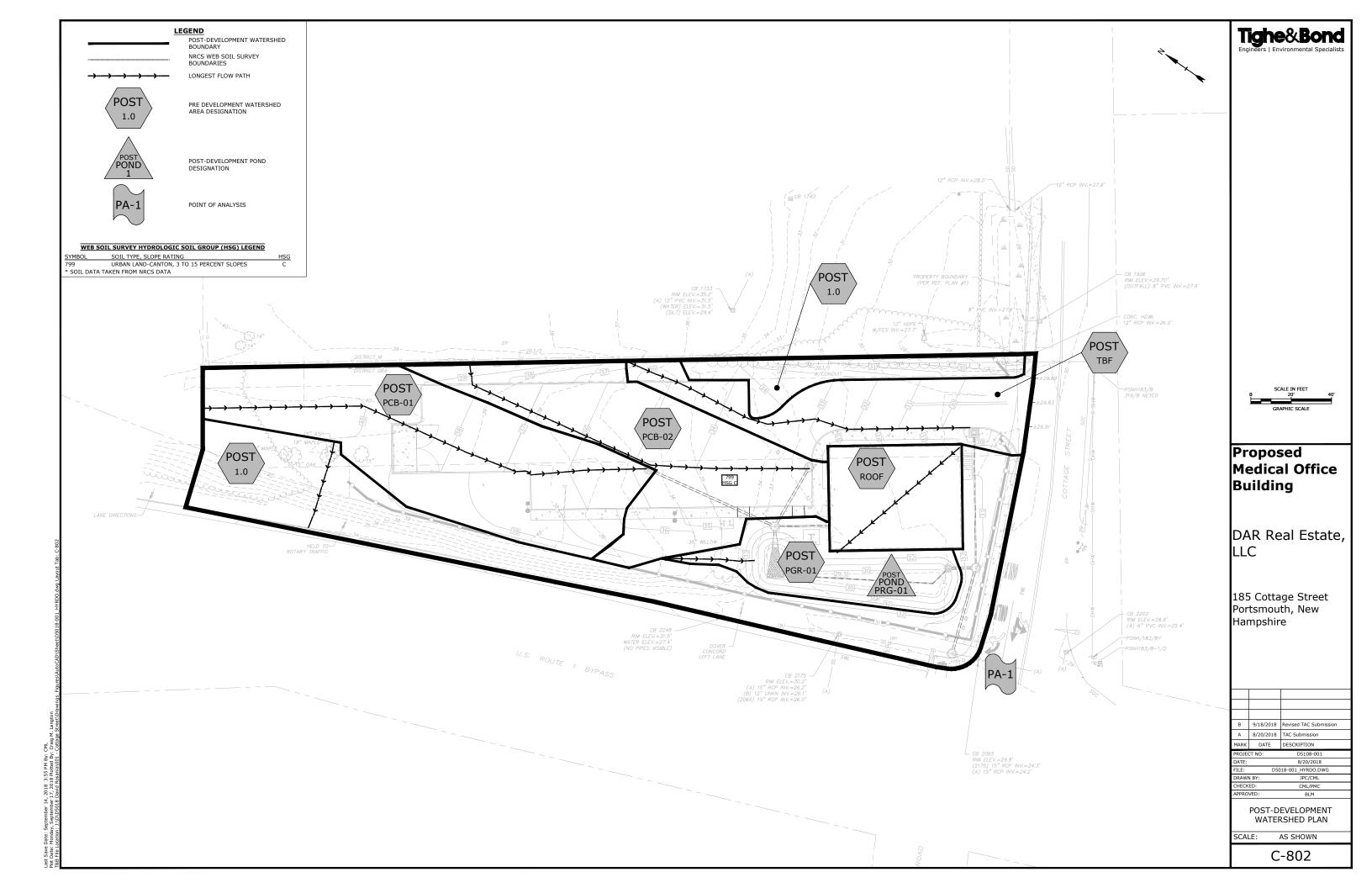
Subcatchment PRE 1.0:

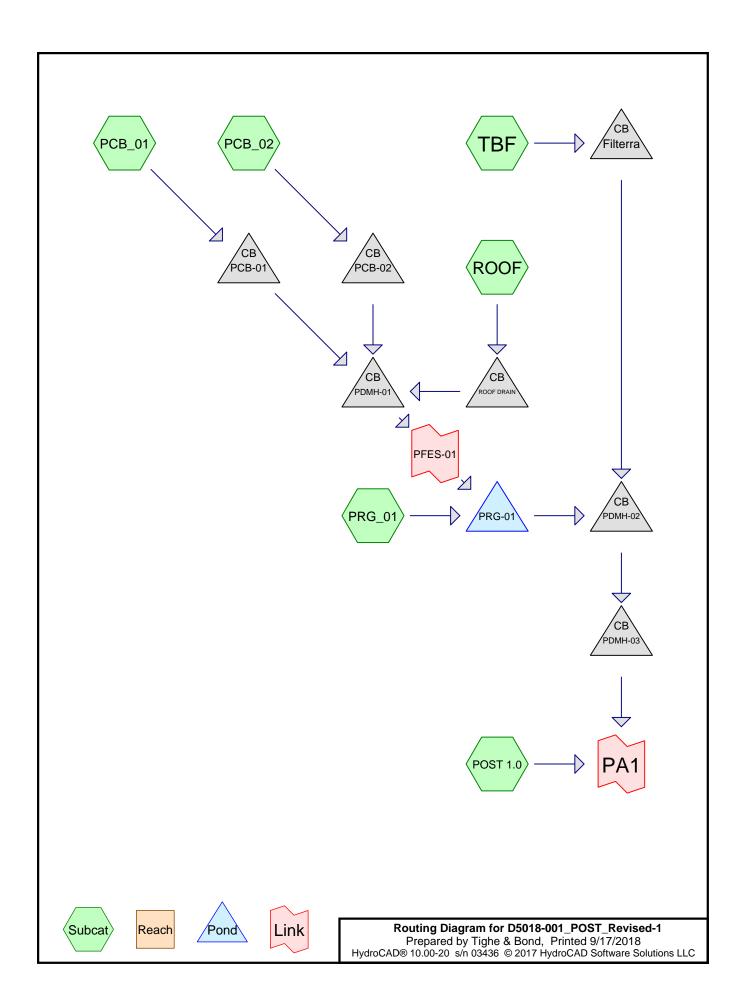
Runoff Area=46,210 sf 10.61% Impervious Runoff Depth>4.54" Flow Length=406' Tc=3.2 min UI Adjusted CN=78 Runoff=6.44 cfs 0.401 af

Link PA1:

Inflow=6.44 cfs 0.401 af Primary=6.44 cfs 0.401 af

Total Runoff Area = 1.061 ac Runoff Volume = 0.401 af Average Runoff Depth = 4.54" 89.39% Pervious = 0.948 ac 10.61% Impervious = 0.113 ac





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## **Area Listing (all nodes)**

Are	a CN	Description
(acres	s)	(subcatchment-numbers)
0.53	74	>75% Grass cover, Good, HSG C (PCB_01, PCB_02, POST 1.0, PRG_01, TBF)
0.39	98	Paved parking, HSG C (PCB_01, PCB_02, POST 1.0, TBF)
0.07	9 98	Unconnected roofs, HSG C (ROOF)
0.04	6 70	Woods, Good, HSG C (PCB_01, POST 1.0)
1.06	85	TOTAL AREA

Type III 24-hr 2-year Rainfall=3.21"

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Page 3

Time span=0.00-30.00 hrs, dt=0.05 hrs, 601 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PCB\_01: Runoff Area=10,751 sf 40.55% Impervious Runoff Depth=1.69"

Flow Length=207' Tc=6.5 min CN=84 Runoff=0.47 cfs 0.035 af

Subcatchment PCB\_02: Runoff Area=8,781 sf 74.18% Impervious Runoff Depth=2.36"

Flow Length=186' Tc=3.0 min CN=92 Runoff=0.59 cfs 0.040 af

Subcatchment POST 1.0: Runoff Area=14,592 sf 12.31% Impervious Runoff Depth=1.22"

Flow Length=406' Tc=3.2 min CN=77 Runoff=0.51 cfs 0.034 af

Subcatchment PRG\_01: Runoff Area=3,483 sf 0.00% Impervious Runoff Depth=1.04"

Flow Length=46' Slope=0.1086 '/' Tc=0.3 min CN=74 Runoff=0.11 cfs 0.007 af

**Subcatchment ROOF:** Runoff Area=3,429 sf 100.00% Impervious Runoff Depth=2.98"

Flow Length=80' Slope=0.0050 '/' Tc=1.8 min CN=98 Runoff=0.27 cfs 0.020 af

Subcatchment TBF: Runoff Area=5,168 sf 90.75% Impervious Runoff Depth=2.76"

Flow Length=179' Tc=1.7 min CN=96 Runoff=0.39 cfs 0.027 af

Pond Filterra: Peak Elev=26.29' Inflow=0.39 cfs 0.027 af

6.0" Round Culvert n=0.013 L=62.0' S=0.0097 '/' Outflow=0.39 cfs 0.027 af

Pond PCB-01: Peak Elev=30.49' Inflow=0.47 cfs 0.035 af

12.0" Round Culvert n=0.013 L=74.0' S=0.0047 '/' Outflow=0.47 cfs 0.035 af

**Pond PCB-02:** Peak Elev=30.35' Inflow=0.59 cfs 0.040 af

12.0" Round Culvert n=0.013 L=26.0' S=0.0058 '/' Outflow=0.59 cfs 0.040 af

Pond PDMH-01: Peak Elev=30.25' Inflow=1.26 cfs 0.094 af

12.0" Round Culvert n=0.013 L=4.0' S=0.0125 '/' Outflow=1.26 cfs 0.094 af

**Pond PDMH-02:** Peak Elev=25.65' Inflow=0.93 cfs 0.116 af

12.0" Round Culvert n=0.013 L=40.0' S=0.0113 '/' Outflow=0.93 cfs 0.116 af

Pond PDMH-03: Peak Elev=25.06' Inflow=0.93 cfs 0.116 af

15.0" Round Culvert n=0.012 L=10.0' S=0.0300 '/' Outflow=0.93 cfs 0.116 af

**Pond PRG-01:** Peak Elev=29.58' Storage=795 cf Inflow=1.35 cfs 0.101 af

Discarded=0.02 cfs 0.013 af Primary=0.65 cfs 0.088 af Outflow=0.66 cfs 0.101 af

Pond ROOF DRAIN: Peak Elev=30.31' Inflow=0.27 cfs 0.020 af

8.0" Round Culvert n=0.013 L=22.0' S=0.0114 '/' Outflow=0.27 cfs 0.020 af

Link PA1: Inflow=1.43 cfs 0.150 af

Primary=1.43 cfs 0.150 af

Link PFES-01: Inflow=1.26 cfs 0.094 af

Primary=1.26 cfs 0.094 af

Type III 24-hr 2-year Rainfall=3.21"

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Page 4

Total Runoff Area = 1.061 ac Runoff Volume = 0.162 af Average Runoff Depth = 1.84" 55.01% Pervious = 0.583 ac 44.99% Impervious = 0.477 ac

Type III 24-hr 10-year Rainfall=4.87"

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Page 5

Time span=0.00-30.00 hrs, dt=0.05 hrs, 601 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PCB\_01: Runoff Area=10,751 sf 40.55% Impervious Runoff Depth=3.15"

Flow Length=207' Tc=6.5 min CN=84 Runoff=0.88 cfs 0.065 af

Subcatchment PCB\_02: Runoff Area=8,781 sf 74.18% Impervious Runoff Depth=3.96"

Flow Length=186' Tc=3.0 min CN=92 Runoff=0.96 cfs 0.067 af

Subcatchment POST 1.0: Runoff Area=14,592 sf 12.31% Impervious Runoff Depth=2.51"

Flow Length=406' Tc=3.2 min CN=77 Runoff=1.07 cfs 0.070 af

Subcatchment PRG\_01: Runoff Area=3,483 sf 0.00% Impervious Runoff Depth=2.26"

Flow Length=46' Slope=0.1086 '/' Tc=0.3 min CN=74 Runoff=0.24 cfs 0.015 af

**Subcatchment ROOF:** Runoff Area=3,429 sf 100.00% Impervious Runoff Depth=4.63"

Flow Length=80' Slope=0.0050 '/' Tc=1.8 min CN=98 Runoff=0.41 cfs 0.030 af

**Subcatchment TBF:** Runoff Area=5,168 sf 90.75% Impervious Runoff Depth=4.40"

Flow Length=179' Tc=1.7 min CN=96 Runoff=0.60 cfs 0.044 af

Pond Filterra: Peak Elev=26.65' Inflow=0.60 cfs 0.044 af

6.0" Round Culvert n=0.013 L=62.0' S=0.0097 '/' Outflow=0.60 cfs 0.044 af

**Pond PCB-01:** Peak Elev=30.74' Inflow=0.88 cfs 0.065 af

12.0" Round Culvert n=0.013 L=74.0' S=0.0047 '/' Outflow=0.88 cfs 0.065 af

Pond PCB-02: Peak Elev=30.61' Inflow=0.96 cfs 0.067 af

12.0" Round Culvert  $\,$  n=0.013 L=26.0' S=0.0058 '/' Outflow=0.96 cfs 0.067 af

Pond PDMH-01: Peak Elev=30.53' Inflow=2.13 cfs 0.162 af

12.0" Round Culvert n=0.013 L=4.0' S=0.0125 '/' Outflow=2.13 cfs 0.162 af

Pond PDMH-02: Peak Elev=25.75' Inflow=1.27 cfs 0.206 af

12.0" Round Culvert n=0.013 L=40.0' S=0.0113 '/' Outflow=1.27 cfs 0.206 af

Pond PDMH-03: Peak Elev=25.14' Inflow=1.27 cfs 0.206 af

15.0" Round Culvert n=0.012 L=10.0' S=0.0300 '/' Outflow=1.27 cfs 0.206 af

Pond PRG-01: Peak Elev=30.39' Storage=1,729 cf Inflow=2.33 cfs 0.177 af

Discarded=0.02 cfs 0.015 af Primary=0.75 cfs 0.162 af Outflow=0.77 cfs 0.177 af

Pond ROOF DRAIN: Peak Elev=30.56' Inflow=0.41 cfs 0.030 af

8.0" Round Culvert n=0.013 L=22.0' S=0.0114 '/' Outflow=0.41 cfs 0.030 af

Link PA1: Inflow=2.33 cfs 0.276 af

Primary=2.33 cfs 0.276 af

Link PFES-01: Inflow=2.13 cfs 0.162 af

Primary=2.13 cfs 0.162 af

Type III 24-hr 10-year Rainfall=4.87" Printed 9/17/2018

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Page 6

Total Runoff Area = 1.061 ac Runoff Volume = 0.291 af Average Runoff Depth = 3.29" 55.01% Pervious = 0.583 ac 44.99% Impervious = 0.477 ac

Type III 24-hr 25-year Rainfall=6.17"

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Page 7

Time span=0.00-30.00 hrs, dt=0.05 hrs, 601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Runoff Area=10,751 sf 40.55% Impervious Runoff Depth=4.36" Subcatchment PCB\_01:

Flow Length=207' Tc=6.5 min CN=84 Runoff=1.20 cfs 0.090 af

Runoff Area=8,781 sf 74.18% Impervious Runoff Depth=5.24" Subcatchment PCB 02:

Flow Length=186' Tc=3.0 min CN=92 Runoff=1.25 cfs 0.088 af

Subcatchment POST 1.0: Runoff Area=14,592 sf 12.31% Impervious Runoff Depth=3.63"

Flow Length=406' Tc=3.2 min CN=77 Runoff=1.54 cfs 0.101 af

Runoff Area=3,483 sf 0.00% Impervious Runoff Depth=3.33" Subcatchment PRG\_01:

Flow Length=46' Slope=0.1086 '/' Tc=0.3 min CN=74 Runoff=0.36 cfs 0.022 af

Runoff Area=3,429 sf 100.00% Impervious Runoff Depth=5.93" Subcatchment ROOF:

Flow Length=80' Slope=0.0050 '/' Tc=1.8 min CN=98 Runoff=0.52 cfs 0.039 af

Runoff Area=5,168 sf 90.75% Impervious Runoff Depth=5.70" Subcatchment TBF: Flow Length=179' Tc=1.7 min CN=96 Runoff=0.77 cfs 0.056 af

Peak Elev=27.28' Inflow=0.77 cfs 0.056 af **Pond Filterra:** 

6.0" Round Culvert n=0.013 L=62.0' S=0.0097 '/' Outflow=0.77 cfs 0.056 af

Peak Elev=30.96' Inflow=1.20 cfs 0.090 af Pond PCB-01:

12.0" Round Culvert n=0.013 L=74.0' S=0.0047 '/' Outflow=1.20 cfs 0.090 af

Peak Elev=30.83' Inflow=1.25 cfs 0.088 af Pond PCB-02:

12.0" Round Culvert n=0.013 L=26.0' S=0.0058 '/' Outflow=1.25 cfs 0.088 af

Peak Elev=30.76' Inflow=2.82 cfs 0.216 af Pond PDMH-01:

12.0" Round Culvert n=0.013 L=4.0' S=0.0125 '/' Outflow=2.82 cfs 0.216 af

Peak Elev=25.81' Inflow=1.49 cfs 0.279 af Pond PDMH-02:

12.0" Round Culvert n=0.013 L=40.0' S=0.0113 '/' Outflow=1.49 cfs 0.279 af

Peak Elev=25.19' Inflow=1.49 cfs 0.279 af Pond PDMH-03:

15.0" Round Culvert n=0.012 L=10.0' S=0.0300 '/' Outflow=1.49 cfs 0.279 af

Pond PRG-01: Peak Elev=30.97' Storage=2,639 cf Inflow=3.10 cfs 0.239 af

Discarded=0.02 cfs 0.016 af Primary=0.81 cfs 0.223 af Outflow=0.84 cfs 0.239 af

Peak Elev=30.79' Inflow=0.52 cfs 0.039 af **Pond ROOF DRAIN:** 

8.0" Round Culvert n=0.013 L=22.0' S=0.0114 '/' Outflow=0.52 cfs 0.039 af

Inflow=3.01 cfs 0.380 af Link PA1:

Primary=3.01 cfs 0.380 af

Link PFES-01: Inflow=2.82 cfs 0.216 af

Primary=2.82 cfs 0.216 af

Type III 24-hr 25-year Rainfall=6.17"

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Page 8

Total Runoff Area = 1.061 ac Runoff Volume = 0.396 af Average Runoff Depth = 4.48" 55.01% Pervious = 0.583 ac 44.99% Impervious = 0.477 ac

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Page 9

#### **Summary for Subcatchment PCB\_01:**

Runoff = 1.20 cfs @ 12.10 hrs, Volume= 0.090 af, Depth= 4.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs Type III 24-hr 25-year Rainfall=6.17"

A	rea (sf)	CN E	Description		
	609	70 V	Voods, Go	od, HSG C	
	5,782	74 >	75% Gras	s cover, Go	ood, HSG C
	4,360	98 F	Paved park	ing, HSG C	
	10,751	84 V	Veighted A	verage	
	6,391	5	9.45% Per	vious Area	
	4,360	4	0.55% lmp	ervious Are	ea
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
3.4	10	0.0200	0.05		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.21"
2.6	76	0.0050	0.49		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.5	121	0.0350	3.80		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
6.5	207	Total			

#### **Summary for Subcatchment PCB\_02:**

Runoff = 1.25 cfs @ 12.05 hrs, Volume= 0.088 af, Depth= 5.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs Type III 24-hr 25-year Rainfall=6.17"

	Α	rea (sf)	CN D	escription			
		2,267				ood, HSG C	
		6,514	98 F	aved park	ing, HSG C		
		8,781	92 V	Veighted A	verage		
		2,267	2	5.82% Per	vious Area		
		6,514	7	4.18% lmp	ervious Ar	ea	
	Tc	Length	Slope	Velocity	Capacity	Description	
(	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	2.2	10	0.0080	0.07		Sheet Flow,	
						Grass: Short n= 0.150 P2= 3.21"	
	8.0	176	0.0350	3.80		Shallow Concentrated Flow,	
						Paved Kv= 20.3 fps	
	3.0	186	Total				_

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Page 10

### **Summary for Subcatchment POST 1.0:**

Runoff = 1.54 cfs @ 12.05 hrs, Volume= 0.101 af, Depth= 3.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs Type III 24-hr 25-year Rainfall=6.17"

	Α	rea (sf)	CN I	Description		
_		1,407	70 \	Noods, Go	od, HSG C	
		11,389	74 :	>75% Gras	s cover, Go	ood, HSG C
_		1,796	98 I	Paved park	ing, HSG C	
		14,592	77 \	Neighted A	verage	
		12,796	3	37.69% Pei	vious Area	
		1,796	•	12.31% lmp	ervious Ar	ea
	т.	l a .a a.tla	Olana.	Malaa!te.	0	Description
	Tc	Length	Slope		Capacity	Description
-	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	1.1	10	0.0500	0.16		Sheet Flow,
	0.0	00	0.0750	4.40		Grass: Short n= 0.150 P2= 3.21"
	0.3	86	0.0756	4.12		Shallow Concentrated Flow,
	1.7	241	0.0137	2.38		Grassed Waterway Kv= 15.0 fps Shallow Concentrated Flow,
	1.7	2 <del>4</del> I	0.0137	2.30		Paved Kv= 20.3 fps
	0.1	69	0.0246	8.94	10.98	Pipe Channel, RCP_Round 15"
	0.1	00	0.0210	0.04	10.00	15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
						n= 0.012 Concrete pipe, finished
-	3.2	406	Total			11,

#### 406 Total

## **Summary for Subcatchment PRG\_01:**

Runoff = 0.36 cfs @ 12.01 hrs, Volume= 0.022 af, Depth= 3.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs Type III 24-hr 25-year Rainfall=6.17"

	Area (sf)	CN	Description					
	3,483	74	>75% Gras	s cover, Go	ood, HSG C			
	3,483		100.00% Pe	ervious Are	a			
T (mir	c Length	Slope (ft/ft	,	Capacity (cfs)	Description			
0.	, ,	0.1086	, ,	(0.0)	Sheet Flow, Smooth surfaces	n= 0.011	P2= 3.21"	

#### **Summary for Subcatchment ROOF:**

Runoff = 0.52 cfs @ 12.03 hrs, Volume= 0.039 af, Depth= 5.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs Type III 24-hr 25-year Rainfall=6.17"

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Page 11

A	rea (sf)	CN [	Description					
	3,429	98 l	<b>Jnconnecte</b>	ed roofs, HS	SG C			
	3,429 3,429			npervious Anconnected				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
1.8	80	0.0050	0.76		Sheet Flow, Smooth surfaces	n= 0.011	P2= 3.21"	

#### **Summary for Subcatchment TBF:**

Runoff = 0.77 cfs @ 12.03 hrs, Volume= 0.056 af, Depth= 5.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs Type III 24-hr 25-year Rainfall=6.17"

	Area (sf)	CN [	Description			
	478	74 >	75% Gras	s cover, Go	ood, HSG C	
	4,690	98 F	Paved park	ing, HSG C		
	5,168	96 V	Veighted A	verage		
	478	g	.25% Perv	ious Area		
	4,690	S	90.75% Impervious Area			
To	3	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
1.0	10	0.0570	0.16		Sheet Flow,	
					Grass: Short n= 0.150 P2= 3.21"	
0.7	169	0.3700	4.26		Shallow Concentrated Flow,	
					Short Grass Pasture Kv= 7.0 fps	
1.7	179	Total				

#### **Summary for Pond Filterra:**

Inflow Area = 0.119 ac, 90.75% Impervious, Inflow Depth = 5.70" for 25-year event

Inflow = 0.77 cfs @ 12.03 hrs, Volume= 0.056 af

Outflow = 0.77 cfs @ 12.03 hrs, Volume= 0.056 af, Atten= 0%, Lag= 0.0 min

Primary = 0.77 cfs @ 12.03 hrs, Volume= 0.056 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs

Peak Elev= 27.28' @ 12.03 hrs

Flood Elev= 29.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	25.85'	6.0" Round Culvert
	-		L= 62.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 25.85' / 25.25' S= 0.0097 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Type III 24-hr 25-year Rainfall=6.17"

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Page 12

Primary OutFlow Max=0.73 cfs @ 12.03 hrs HW=27.17' TW=25.80' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.73 cfs @ 3.71 fps)

#### **Summary for Pond PCB-01:**

Inflow Area = 0.247 ac, 40.55% Impervious, Inflow Depth = 4.36" for 25-year event

Inflow = 1.20 cfs @ 12.10 hrs, Volume= 0.090 af

Outflow = 1.20 cfs @ 12.10 hrs, Volume= 0.090 af, Atten= 0%, Lag= 0.0 min

Primary = 1.20 cfs @ 12.10 hrs, Volume= 0.090 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs

Peak Elev= 30.96' @ 12.10 hrs

Flood Elev= 35.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	30.00'	<b>12.0" Round Culvert</b> L= 74.0' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 30.00' / 29.65' S= 0.0047 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.41 cfs @ 12.10 hrs HW=30.95' TW=30.65' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.41 cfs @ 2.37 fps)

#### **Summary for Pond PCB-02:**

Inflow Area = 0.202 ac, 74.18% Impervious, Inflow Depth = 5.24" for 25-year event

Inflow = 1.25 cfs @ 12.05 hrs, Volume= 0.088 af

Outflow = 1.25 cfs @ 12.05 hrs, Volume= 0.088 af, Atten= 0%, Lag= 0.0 min

Primary = 1.25 cfs @ 12.05 hrs, Volume= 0.088 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs

Peak Elev= 30.83' @ 12.10 hrs

Flood Elev= 33.10'

Device	Routing	Invert	Outlet Devices	
#1	Primary	29.80'	12.0" Round Culvert	
	_		L= 26.0' CMP, square edge headwall, Ke= 0.500	
			Inlet / Outlet Invert= 29.80' / 29.65' S= 0.0058 '/' Cc= 0.900	
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf	

Primary OutFlow Max=0.00 cfs @ 12.05 hrs HW=30.71' TW=30.74' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

#### **Summary for Pond PDMH-01:**

Inflow Area =	0.527 ac, 62.29% Impervious, I	nflow Depth = 4.93" for 25-year event	
Inflow =	2.82 cfs @ 12.06 hrs, Volume=	0.216 af	

Outflow = 2.82 cfs @ 12.06 hrs, Volume= 0.216 af, Atten= 0%, Lag= 0.0 min

Primary = 2.82 cfs @ 12.06 hrs, Volume= 0.216 af

Type III 24-hr 25-year Rainfall=6.17"

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Page 13

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs

Peak Elev= 30.76' @ 12.06 hrs

Flood Elev= 33.00'

Device	Routing	Invert	Outlet Devices	
#1	Primary	29.55'	12.0" Round Culvert	
	-		L= 4.0' CMP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 29.55' / 29.50' S= 0.0125 '/' Cc= 0.900	
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf	

Primary OutFlow Max=2.76 cfs @ 12.06 hrs HW=30.74' TW=0.00' (Dynamic Tailwater) 1=Culvert (Barrel Controls 2.76 cfs @ 3.74 fps)

#### **Summary for Pond PDMH-02:**

Inflow Area = 0.726 ac, 60.08% Impervious, Inflow Depth = 4.61" for 25-year event

Inflow = 1.49 cfs @ 12.03 hrs, Volume= 0.279 af

Outflow = 1.49 cfs @ 12.03 hrs, Volume= 0.279 af, Atten= 0%, Lag= 0.0 min

Primary = 1.49 cfs @ 12.03 hrs, Volume= 0.279 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs

Peak Elev= 25.81' @ 12.03 hrs

Flood Elev= 31.00'

Device	Routing	Invert	Outlet Devices		
#1	Primary	25.15'	12.0" Round Culvert		
			40.0' CMP, end-section conforming to fill, Ke= 0.500		
			Inlet / Outlet Invert= 25.15' / 24.70' S= 0.0113 '/' Cc= 0.900		
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf		

Primary OutFlow Max=1.46 cfs @ 12.03 hrs HW=25.81' TW=25.18' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.46 cfs @ 3.78 fps)

## **Summary for Pond PDMH-03:**

Inflow Area = 0.726 ac, 60.08% Impervious, Inflow Depth = 4.61" for 25-year event

Inflow = 1.49 cfs @ 12.03 hrs, Volume= 0.279 af

Outflow = 1.49 cfs @ 12.03 hrs, Volume= 0.279 af, Atten= 0%, Lag= 0.0 min

Primary = 1.49 cfs @ 12.03 hrs, Volume= 0.279 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs

Peak Elev= 25.19' @ 12.03 hrs

Flood Elev= 31.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	24.60'	15.0" Round Culvert
			L= 10.0' CMP, end-section conforming to fill, Ke= 0.500
			Inlet / Outlet Invert= 24.60' / 24.30' S= 0.0300 '/' Cc= 0.900
			n= 0.012 Concrete pipe, finished, Flow Area= 1.23 sf

Primary

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Page 14

Primary OutFlow Max=1.46 cfs @ 12.03 hrs HW=25.18' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.46 cfs @ 2.60 fps)

#### **Summary for Pond PRG-01:**

0.223 af

Inflow Area =	0.607 ac, 54.09% Impervious, Inflow D	epth = 4.72" for 25-year event
Inflow =	3.10 cfs @ 12.05 hrs, Volume=	0.239 af
Outflow =	0.84 cfs @ 12.42 hrs, Volume=	0.239 af, Atten= 73%, Lag= 22.0 min
Discarded =	0.02 cfs @ 12.42 hrs, Volume=	0.016 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs Peak Elev= 30.97' @ 12.42 hrs Surf.Area= 3,478 sf Storage= 2,639 cf Flood Elev= 32.00' Surf.Area= 4,202 sf Storage= 4,848 cf

0.81 cfs @ 12.42 hrs, Volume=

Plug-Flow detention time= 36.0 min calculated for 0.238 af (100% of inflow)

Center-of-Mass det. time= 36.5 min (820.3 - 783.8)

Volume	Invert	Avail.Storage	Storage Description
#1	29.50'	4,125 cf	Ponding Area (Prismatic)Listed below (Recalc)
#2	28.00'	383 cf	Filter Media (Prismatic)Listed below (Recalc)
			1,277 cf Overall x 30.0% Voids
#3	26.75'	340 cf	Reservoir Course (Prismatic)Listed below (Recalc)
			851 cf Overall x 40.0% Voids

4,848 cf T	otal Ava	ilable :	Storage
------------	----------	----------	---------

		4,848 cf Total Av	ailable Storage
Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
29.50	850	0	0
30.00	1,152	501	501
31.00	1,798	1,475	1,976
32.00	2,500	2,149	4,125
Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
28.00	851	0	0
29.50	851	1,277	1,277
		,	,
Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
26.75	851	0	0
27.75	851	851	851
_			

Device	Routing	Invert	Outlet Devices
#1	Primary	27.05'	12.0" Round Culvert
	•		L= 26.0' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 27.05' / 26.10' S= 0.0365 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	27.05'	<b>4.0" Vert. Underdrain</b> C= 0.600
#3	Device 1	31.00'	<b>15.1" Horiz. Neenah R-2577-C</b> C= 0.600

Type III 24-hr 25-year Rainfall=6.17"

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Page 15

Limited to weir flow at low heads

#4 Discarded 26.75' 0.300 in/hr Exfiltration over Surface area

**Discarded OutFlow** Max=0.02 cfs @ 12.42 hrs HW=30.96' (Free Discharge) **4=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.81 cfs @ 12.42 hrs HW=30.96' TW=25.66' (Dynamic Tailwater)

**1=Culvert** (Passes 0.81 cfs of 6.99 cfs potential flow)

**2=Underdrain** (Orifice Controls 0.81 cfs @ 9.32 fps)

**-3=Neenah R-2577-C** (Controls 0.00 cfs)

#### **Summary for Pond ROOF DRAIN:**

Inflow Area = 0.079 ac,100.00% Impervious, Inflow Depth = 5.93" for 25-year event

Inflow = 0.52 cfs @ 12.03 hrs, Volume= 0.039 af

Outflow = 0.52 cfs @ 12.03 hrs, Volume= 0.039 af, Atten= 0%, Lag= 0.0 min

Primary = 0.52 cfs @ 12.03 hrs, Volume= 0.039 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs

Peak Elev= 30.79' @ 12.10 hrs

Flood Elev= 33.85'

Device Routing Invert Outlet Devices

#1 Primary

29.90'

8.0" Round Culvert

L= 22.0' CMP, end-section conforming to fill, Ke= 0.500

Inlet / Outlet Invert= 29.90' / 29.65' S= 0.0114 '/' Cc= 0.900

n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf

Primary OutFlow Max=0.00 cfs @ 12.03 hrs HW=30.58' TW=30.67' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

#### **Summary for Link PA1:**

Inflow Area = 1.061 ac, 44.99% Impervious, Inflow Depth = 4.30" for 25-year event

Inflow = 3.01 cfs @ 12.05 hrs, Volume= 0.380 af

Primary = 3.01 cfs @ 12.05 hrs, Volume= 0.380 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs

#### **Summary for Link PFES-01:**

Inflow Area = 0.527 ac, 62.29% Impervious, Inflow Depth = 4.93" for 25-year event

Inflow = 2.82 cfs @ 12.06 hrs, Volume= 0.216 af

Primary = 2.82 cfs @ 12.06 hrs, Volume= 0.216 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs

Type III 24-hr 50-year Rainfall=7.39"

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Page 16

Time span=0.00-30.00 hrs, dt=0.05 hrs, 601 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PCB\_01: Runoff Area=10,751 sf 40.55% Impervious Runoff Depth=5.51"

Flow Length=207' Tc=6.5 min CN=84 Runoff=1.50 cfs 0.113 af

Subcatchment PCB\_02: Runoff Area=8,781 sf 74.18% Impervious Runoff Depth=6.44"

Flow Length=186' Tc=3.0 min CN=92 Runoff=1.52 cfs 0.108 af

Subcatchment POST 1.0: Runoff Area=14,592 sf 12.31% Impervious Runoff Depth=4.72"

Flow Length=406' Tc=3.2 min CN=77 Runoff=1.99 cfs 0.132 af

Subcatchment PRG\_01: Runoff Area=3,483 sf 0.00% Impervious Runoff Depth=4.38"

Flow Length=46' Slope=0.1086 '/' Tc=0.3 min CN=74 Runoff=0.47 cfs 0.029 af

Subcatchment ROOF: Runoff Area=3,429 sf 100.00% Impervious Runoff Depth=7.15"

Flow Length=80' Slope=0.0050 '/' Tc=1.8 min CN=98 Runoff=0.62 cfs 0.047 af

Subcatchment TBF: Runoff Area=5,168 sf 90.75% Impervious Runoff Depth=6.91"

Flow Length=179' Tc=1.7 min CN=96 Runoff=0.92 cfs 0.068 af

Pond Filterra: Peak Elev=27.99' Inflow=0.92 cfs 0.068 af

6.0" Round Culvert n=0.013 L=62.0' S=0.0097 '/' Outflow=0.92 cfs 0.068 af

Pond PCB-01: Peak Elev=31.20' Inflow=1.50 cfs 0.113 af

12.0" Round Culvert n=0.013 L=74.0' S=0.0047 '/' Outflow=1.50 cfs 0.113 af

Pond PCB-02: Peak Elev=31.08' Inflow=1.52 cfs 0.108 af

12.0" Round Culvert n=0.013 L=26.0' S=0.0058 '/' Outflow=1.52 cfs 0.108 af

Pond PDMH-01: Peak Elev=30.99' Inflow=3.46 cfs 0.268 af

12.0" Round Culvert n=0.013 L=4.0' S=0.0125 '/' Outflow=3.46 cfs 0.268 af

**Pond PDMH-02:** Peak Elev=25.97' Inflow=2.05 cfs 0.349 af

12.0" Round Culvert  $\,$  n=0.013 L=40.0' S=0.0113 '/' Outflow=2.05 cfs  $\,$  0.349 af

Pond PDMH-03: Peak Elev=25.31' Inflow=2.05 cfs 0.349 af

15.0" Round Culvert n=0.012 L=10.0' S=0.0300 '/' Outflow=2.05 cfs 0.349 af

Pond PRG-01: Peak Elev=31.17' Storage=3.011 cf Inflow=3.83 cfs 0.298 af

Discarded=0.03 cfs 0.017 af Primary=1.73 cfs 0.281 af Outflow=1.75 cfs 0.298 af

Pond ROOF DRAIN: Peak Elev=31.04' Inflow=0.62 cfs 0.047 af

8.0" Round Culvert n=0.013 L=22.0' S=0.0114 '/' Outflow=0.62 cfs 0.047 af

Link PA1: Inflow=3.65 cfs 0.481 af

Primary=3.65 cfs 0.481 af

Link PFES-01: Inflow=3.46 cfs 0.268 af

Primary=3.46 cfs 0.268 af

Type III 24-hr 50-year Rainfall=7.39"

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Total Runoff Area = 1.061 ac Runoff Volume = 0.498 af Average Runoff Depth = 5.63" 55.01% Pervious = 0.583 ac 44.99% Impervious = 0.477 ac



## Engineers | Environmental Specialists

Project: Proposed Medical Office Location: 185 Cottage Street

T&B #: D5018-001

Calculations By: CML Checked By: PMC

Date: 8/20/2018 Last Revised: 9/18/2018

#### **APRON DESIGN**

Terms: FES 1

length of apron (ft.) L<sub>a</sub>

discharge from pipe (cfs) Q (25 YR STORM EVENT)

pipe dia. or channel width (ft.) Do tailwater depth (ft.)  $T_w$  width of apron (at outlet)(ft) W1 width of apron (downstream)(ft) W2 median stone diameter (ft.)  $d_{50}$ 

Equations Used:	
Length of Apron ( $L_a$ ) when Tw < .5*Do $L_a$ =	<u>1.8(Q)</u> + 7Do Do^(3/2)
when Tw >= .5*Do L <sub>a</sub> =	<u>3(Q)</u> + 7Do Do^(3/2)
Width of Apron (W1)	,
W1=	3Do
Width of Apron (W2) when Tw < .5*Do W2=	3Do + La
when Tw >= .5*Do W2=	3Do + 0.4La
Median Diameter d <sub>50</sub> =	0.02 * Q^(1.3) (Tw * Do)
Input:	
Q (cfs)	
Do (ft.)	
T <sub>w</sub> (ft.)	1.36 ft
Output:	
Width of Apron (W1)	3 ft.
Width of Apron (W2)	
Length of Apron (L <sub>a</sub> )	
- 0 - 1 - 1 a/	

0.50 ft.

1.13 ft.

Median Diameter

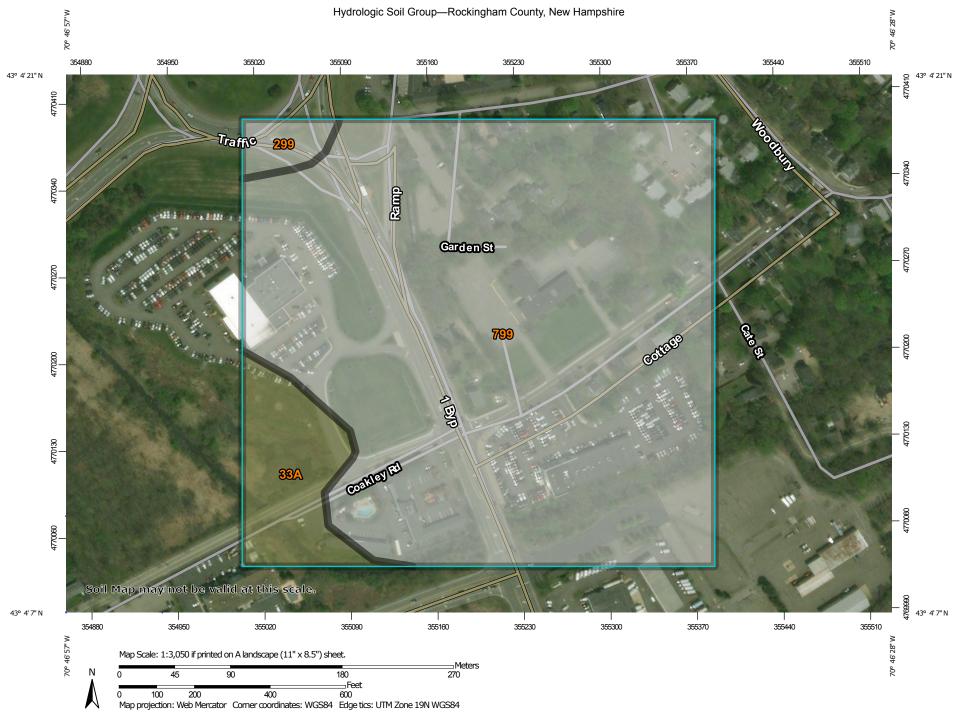
Riprap min. depth



## **Groundwater Recharge Volume (GRV) Calculation**

	ac	Area of HSG A soil that was replaced by impervious cover	0.40"
	ac	Area of HSG B soil that was replaced by impervious cover	0.25"
0.37	ac	Area of HSG C soil that was replaced by impervious cover	0.10"
	ac	Area of HSG D soil or impervious cover that was replaced by impervious cover	0.0"
0.10 inches		Rd = weighted groundwater recharge depth	
0.0366 ac-in		GRV = AI * Rd	
133	cf	GRV conversion (ac-in x 43,560 sf/ac x 1ft/12")	

Provide calculations below showing that the project meets the groundwater recharge requirements (Env-Wq 1507.04): Proposed Rain Garden #1 Volume GRVp = 2,681 CFThe groundwater recharge volume provided is greater than the required groundwater recharge volume required



#### MAP LEGEND MAP INFORMATION The soil surveys that comprise your AOI were mapped at Area of Interest (AOI) С 1:24.000. Area of Interest (AOI) C/D Soils Warning: Soil Map may not be valid at this scale. D **Soil Rating Polygons** Enlargement of maps beyond the scale of mapping can cause Not rated or not available Α misunderstanding of the detail of mapping and accuracy of soil **Water Features** line placement. The maps do not show the small areas of A/D contrasting soils that could have been shown at a more detailed Streams and Canals В Transportation B/D Rails ---Please rely on the bar scale on each map sheet for map measurements. Interstate Highways C/D Source of Map: Natural Resources Conservation Service **US Routes** Web Soil Survey URL: D Major Roads Coordinate System: Web Mercator (EPSG:3857) Not rated or not available -Local Roads Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Soil Rating Lines Background distance and area. A projection that preserves area, such as the Aerial Photography Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. B/D Soil Survey Area: Rockingham County, New Hampshire Survey Area Data: Version 19, Sep 11, 2017 C/D Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. D Date(s) aerial images were photographed: Dec 31, 2009—Jun Not rated or not available 26. 2016 **Soil Rating Points** The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background A/D imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. B/D

## **Hydrologic Soil Group**

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
33A	Scitico silt loam, 0 to 5 percent slopes	C/D	3.0	8.8%
299	Udorthents, smoothed		0.7	2.1%
799	Urban land-Canton complex, 3 to 15 percent slopes		30.5	89.1%
Totals for Area of Intere	est	34.2	100.0%	

## **Description**

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

# **Rating Options**

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified

Tie-break Rule: Higher

# **Extreme Precipitation Tables**

## **Northeast Regional Climate Center**

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Smoothing Yes

State New Hampshire

Location

**Longitude** 70.779 degrees West **Latitude** 43.071 degrees North

**Elevation** 0 feet

**Date/Time** Thu, 26 Jul 2018 14:23:32 -0400

## **Extreme Precipitation Estimates**

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.26	0.40	0.50	0.65	0.81	1.04	1yr	0.70	0.98	1.21	1.56	2.03	2.66	2.92	1yr	2.35	2.81	3.22	3.94	4.55	1yr
2yr	0.32	0.50	0.62	0.81	1.02	1.30	2yr	0.88	1.18	1.52	1.94	2.49	3.21	3.57	2yr	2.84	3.43	3.94	4.68	5.33	2yr
5yr	0.37	0.58	0.73	0.97	1.25	1.60	5yr	1.08	1.46	1.88	2.43	3.14	4.07	4.58	5yr	3.60	4.40	5.04	5.93	6.70	5yr
10yr	0.41	0.65	0.82	1.11	1.45	1.89	10yr	1.25	1.72	2.23	2.89	3.75	4.87	5.53	10yr	4.31	5.32	6.08	7.11	7.98	10yr
25yr	0.48	0.76	0.96	1.33	1.77	2.33	25yr	1.53	2.14	2.77	3.62	4.74	6.17	7.10	25yr	5.46	6.83	7.80	9.02	10.05	25yr
50yr	0.53	0.86	1.10	1.53	2.06	2.75	50yr	1.78	2.52	3.28	4.32	5.66	7.39	8.58	50yr	6.54	8.25	9.42	10.81	11.98	50yr
100yr	0.59	0.96	1.24	1.76	2.41	3.24	100yr	2.08	2.97	3.89	5.15	6.76	8.86	10.38	100yr	7.84	9.98	11.37	12.96	14.28	100yr
200yr	0.67	1.10	1.42	2.04	2.81	3.82	200yr	2.43	3.50	4.60	6.11	8.07	10.61	12.55	200yr	9.39	12.07	13.74	15.55	17.04	200yr
500yr	0.79	1.31	1.70	2.47	3.46	4.74	500yr	2.98	4.36	5.74	7.68	10.21	13.49	16.15	500yr	11.94	15.53	17.65	19.78	21.52	500yr

## **Lower Confidence Limits**

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.23	0.36	0.44	0.59	0.73	0.89	1yr	0.63	0.87	0.92	1.32	1.67	2.22	2.51	1yr	1.97	2.41	2.86	3.16	3.88	1yr
2yr	0.31	0.49	0.60	0.81	1.00	1.19	2yr	0.86	1.16	1.37	1.82	2.34	3.06	3.45	2yr	2.70	3.32	3.82	4.55	5.08	2yr
5yr	0.35	0.54	0.67	0.92	1.17	1.40	5yr	1.01	1.37	1.61	2.12	2.74	3.79	4.20	5yr	3.36	4.04	4.72	5.54	6.25	5yr
10yr	0.39	0.59	0.73	1.03	1.33	1.60	10yr	1.14	1.56	1.81	2.39	3.06	4.38	4.87	10yr	3.87	4.69	5.45	6.42	7.21	10yr
25yr	0.44	0.67	0.83	1.19	1.56	1.90	25yr	1.35	1.86	2.10	2.76	3.54	4.70	5.91	25yr	4.16	5.69	6.67	7.81	8.70	25yr
50yr	0.48	0.73	0.91	1.31	1.77	2.17	50yr	1.52	2.12	2.35	3.08	3.94	5.31	6.83	50yr	4.70	6.57	7.76	9.07	10.04	50yr
100yr	0.54	0.81	1.02	1.47	2.01	2.47	100yr	1.74	2.42	2.63	3.43	4.37	5.96	7.89	100yr	5.27	7.59	9.02	10.54	11.59	100yr
200yr	0.59	0.89	1.13	1.64	2.28	2.82	200yr	1.97	2.75	2.94	3.80	4.82	6.67	9.12	200yr	5.90	8.77	10.49	12.27	13.41	200yr
500yr	0.69	1.02	1.32	1.91	2.72	3.37	500yr	2.35	3.29	3.41	4.34	5.49	7.75	11.03	500yr	6.86	10.61	12.81	15.02	16.23	500yr

## **Upper Confidence Limits**

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.28	0.44	0.54	0.72	0.89	1.08	1yr	0.77	1.06	1.26	1.74	2.21	2.99	3.15	1yr	2.65	3.03	3.58	4.38	5.05	1yr
2yr	0.34	0.52	0.64	0.86	1.06	1.27	2yr	0.92	1.24	1.48	1.96	2.51	3.43	3.70	2yr	3.03	3.56	4.08	4.83	5.64	2yr
5yr	0.40	0.62	0.76	1.05	1.33	1.62	5yr	1.15	1.58	1.88	2.53	3.25	4.34	4.95	5yr	3.84	4.76	5.37	6.36	7.14	5yr
10yr	0.47	0.72	0.89	1.24	1.61	1.97	10yr	1.39	1.93	2.28	3.10	3.94	5.34	6.19	10yr	4.72	5.95	6.79	7.82	8.74	10yr
25yr	0.57	0.87	1.09	1.55	2.04	2.56	25yr	1.76	2.50	2.95	4.06	5.13	7.81	8.31	25yr	6.91	7.99	9.10	10.31	11.39	25yr
50yr	0.67	1.02	1.27	1.82	2.45	3.12	50yr	2.11	3.05	3.59	4.99	6.29	9.78	10.41	50yr	8.66	10.01	11.37	12.69	13.93	50yr
100yr	0.78	1.19	1.49	2.15	2.94	3.79	100yr	2.54	3.71	4.36	6.14	7.72	12.25	13.04	100yr	10.84	12.54	14.20	15.65	17.05	100yr
200yr	0.92	1.38	1.75	2.53	3.53	4.63	200yr	3.05	4.52	5.32	7.55	9.47	15.38	16.35	200yr	13.61	15.72	17.75	19.28	20.87	200yr
500yr	1.14	1.69	2.18	3.16	4.50	6.00	500yr	3.88	5.87	6.90	9.98	12.44	20.79	22.06	500yr	18.40	21.21	23.87	25.41	27.28	500yr



#### EASEMENT

INDENTURE, made the day of , 1975, between JANE C. GARLAND, Grantor and the UNITED STATES OF AMERICA, Grantee.

WHEREAS, the Grantor has petitioned the Rockingham County Superior Court to vest in her title to the following described premises:

A certain tract of land situated in Portsmouth, County of Rockingham and State of New Hampshire, and bounded and described as follows:

That entire portion of Inland Street, Portsmouth, New Hampshire as shown on Plan of Land of Frank Jones, said plan being recorded in the Rockingham County Registry of Deeds as Plan No. 00223, which lies adjacent to and is contiguous with the eastern boundary of the premises of your Petitioner, said premises being shown on City of Portsmouth Assessor's Lot Plan 211, and being designated therein as Lot No. 37; and

WHEREAS, the Grantor has petitioned said Court to order and declare that said portion of Inland Street as hereinabove described, be declared free and clear of all claims by any person, firm or corporation whatever, and free of any servitude of travel, public or private; and

WHEREAS, the Grantee is seized in fee simple of another parcel of land near the Grantor's land, said Grantee's land being known as the United States Armory; and

WHEREAS, the Grantor has agreed, in consideration of the Grantee's promise not to oppose said petition as hereinabove referred to, to grant to the Grantee an easement or right-of-way over that portion of Inland Street as hereinabove described;

#### WITNESSETH:

THAT, in pursuance of said agreement, the Grantor hereby grants to the Grantee, its heirs and assigns;

FULL and FREE RIGHT and LIBERTY for the Grantee, its heirs and assigns, at all times hereafter, with or without vehicles, for all purposes connected with the use and enjoyment of the said land of the Grantee, to pass and repass along that portion of Inland Street as hereinabove described, a distance of ten (10) feet in width, said ten (10) feet being measured from the Easterly boundary line of said Inland Street as shown on said Lot Plan No. 94, for the purpose of going from the said Inland Street to the said land of the Grantee, or vice versa. And the right of the Grantee to use said described property for any other legal use without restriction of any kind.

TO HAVE AND TO HOLD the easement or right-of-way hereby granted unto the Grantee, its heirs and assigns as appurtenant to the said land of the grantee.

TAYLOR AND GRAY ATTORNEYS AT LAW 400 MIDDLE STREET PORTEMOUTH,

MANAGEMENT COOK

IN WITNESS WHEREOF, the Grantor has hereunto set her hand and seal the day and year first above-written.

WITNESS:

Stress Jan Bligliste

Desfrite Jane C. Garland

STATE OF NEW HAMPSHIRE ROCKINGHAM, ss

On this the day of , 1973, personally appeared the above-named, JANE C. GARLAND and acknowledged the foregoing instrument to be her voluntary act and deed, before me,

Justice of the Peace

TAYLOR AND GRAY ATTORNEYS AT LAW 400 MINDOLE STREET PORTEMOUTH. CONSIDERATION LESS THAN \$100.00, no STAMPS REQUIRED

#### **EASEMENT**

INDENTURE, made the day of the between JANE C. GARLAND, Grantor and the heirs and assigns of the said heirs of the late DANIEL A. YOKEN, said DANIEL A. YOKEN now being deceased, Grantee.

WHEREAS, the Grantor has petitioned the Rockingham County Superior Court to vest in her title to the following described premises:

A certain tract of land situated in Portsmouth, County of Rockingham, and State of New Hampshire, and bounded and described as follows:

That entire portion of Inland Street, Portsmouth, New Hampshire as shown on Plan of Land of Frank Jones, said plan being recorded in the Rockingham County Registry of Deeds as Plan No. 00223, which lies adjacent to and is contiguous with the eastern boundary of the premises of your Petitioner, said premises being shown on City of Portsmouth assessor's Lot Plan 211, and being designated therein as Lot No. 37; and

WHEREAS, the Grantor has petitioned said Court to order and declare that said portion of Inland Street as hereinabove described, be declared free and clear of all claims by anyperson, firm or corporation whatever, and free of any servitude of travel, public or private; and

WHEREAS, the Grantee is seized in fee simple of another parcel of land near the Grantor's land, said Grantee's land being shown on City of Portsmouth assessor's Lot Plan 94, designated as Lot No. 48; and

WHEREAS, the Grantor has agreed, in consideration of the Grantee's promise not to oppose said petition as hereinabove referred to, to grant to the Grantee an easement or right-of-way over that portion of Inland Street as hereinabove described;

#### WITNESSETH:

THAT, in pursuance of said agreement, the Grantor hereby grants to the Grantee, his heirs and assigns, at all times hereafter, with or without vehicles, for all purposes connected with the use and enjoyment of the said land of the Grantee, to pass and repass along that portion of Inland Street as hereinabove described, a distance of twenty (20) feet in width, said twenty (20) feet being measured from the Easterly boundary line of said Inland Street as shown on said Lot Plan No. 94, for the purpose of going from the said Inland Street to the said land of the Grantee, or vice versa.

TO HAVE AND TO HOLD the easement or right-of-way hereby granted unto the Grantee, his heirs and assigns, as appurtenant to the said land of the Grantee.

TAYLOR AND GRAY
ATTORREYS AT LAW
486 IMDDLE STREET
PORYBRIGHTH,

IN WITNESS WHEREOF, the Grantor has hereunto set her hand and seal the day and year first above-written.

WITNESS:

Aven Gen Blistite Jane C. Garland

STATE OF NEW HAMPSHIRE ROCKINGHAM, 88

On this the day of , 1975, personally appeared the above-named JANE C. GAMMAND and acknowledged the foregoing instrument to be her voluntary act and deed, before me,

Justice of the Prace

TAYLOR AND GRAY ATTORNEYS AT LAW 488 MIDDLE STREET PORTSHOUTH.