Tighe&Bond

C0960-006 April 29, 2020

Mr. Steve Miller, Chairman City of Portsmouth Conservation Commission 1 Junkins Avenue Portsmouth, New Hampshire 03801

Re: Conditional Use Permit Application Proposed Multi-Family Development, 105 Bartlett Street, Portsmouth, NH

Dear Steve:

On behalf of Iron Horse Properties, LLC, we are pleased to submit via the City of Portsmouth online permitting system the following information to support a request for a Conditional Use Permit for Wetland Buffer Impacts for the above referenced project:

- Site Plan Set last revised April 20, 2020;
- Wetland Delineation and Assessment of Functions and Values dated December, 2019;
- Drainage Analysis dated April 20, 2020;
- Site Constraints Plan dated February 26, 2020;
- Wetland Buffer Impact Exhibit dated April 29, 2020

The proposed project is located at 105 Bartlett Street on five (5) properties identified as Map 157 Lots 1 (1.42 acres) & 2 (2.34 acres), Map 164 Lots 1 (1.19 acres) & 4-2 (5.73 acres), and a private roadway lot (1.60 acres) identified on the City of Portsmouth Tax Maps. The proposed project consists of two (2) multi-family apartment buildings with basement level parking and one (1) mixed used building with first floor office and amenity space, and upper story apartments. The project will include a total of 174 dwelling units. The project will include associated site improvements that consist of a private road with cul-de-sac, parking, utilities, stormwater management and treatment, landscaping and lighting. The project will also include community space along the North Mill Pond. The land from North Mill Pond's mean high water (MHW) line to the 50ft wetland buffer will be designated as community space for the City's North Mill Pond Trail project. In addition, the project will construct a greenway park between the proposed buildings and North Mill Pond trail.

Proposed work within the 100-foot Tidal Buffer and subject to conditional use approval includes demolition and construction activities. The 100-foot tidal buffer within the development area is approximately 4.60 acres. The existing impervious area within the development area is 2.50 acres which is 54.3% of the tidal buffer area.

The work done by the proposed project within the 25-foot buffer to North Mill Pond is limited to the removal of existing impervious surfaces and the construction of three (3) stormwater outlets. An easement from the mean high-water line to the 50-foot buffer within the development lot will be granted to the City of Portsmouth by the developer to build the North Mill Pond Trail and Greenway. Work proposed under this application within the 50-foot buffer includes the removal of existing impervious surfaces and repaving of the existing private road. Work proposed between the 50-foot and 100-foot buffer includes the removal of existing impervious surfaces, repaving of the existing private road and Ricci Lumber parking lot, construction of a portion of the greenway park, reconstruction of a building within the footprint of the existing Great Rhythm building and a portion of two (2) new apartment buildings. Existing impacts to the 100-foot buffer will be reduced by 24,330 square feet through the removal and restoration of impervious surfaces (Table 1).

| Buffer Segment | Existing Impact (SF) | Final Impact (SF) | Net Impact (SF) |
|----------------|-------------------------|----------------------|--------------------|
| 0-25 feet | 12,788 | 6,691 | -6,097 |
| 25-50 feet | 30,479 | 18,391 | -12,088 |
| 50-100 feet | 66,812 | 60,667 | -6,145 |
| Total | 108,845 | 85,029 | -24,330 |

Table 1. 105 Bartlett Street, 100-Foot Tidal Buffer Impacts

Jurisdictional wetland areas, including 2,000+/- linear feet of tidal wetlands and buffers along the North Mill Pond, were identified by Leonard A. Lord, PhD, CSS, CWS, Senior Environmental Scientist at Tighe & Bond, Inc. on October 29 and December 2, 2019. The review was limited to the vicinity of the proposed development, extending from Bartlett Street to an area opposite Cornwall Street, which runs roughly perpendicular to the parcel.

The results of the tidal wetland and buffer review and the assessment of the wetlands functions and values in the vicinity of the proposed project are in the enclosed "Wetland Delineation and Assessment of Functions and Values" dated December 2019.

Section 10.1017.50 of the Zoning Ordinance includes the following criteria for approval:

(1) The land is reasonably suited to the use, activity or alteration.

The subject properties are within the CD4-W and CD4-L1 Zoning Districts (Character Districts). Section 10.5A41.10A of the Zoning Ordinance defines the CD4-L1 district as consisting "of medium density areas with a mix of medium to large houses. Upper floor uses are almost entirely residential" and the CD4-W district as consisting of "of a medium-to-high density area with a mix of building types and residential, retail, and other commercial uses". The proposed project is consistent with the descriptions of uses in these zoning districts. Additionally, the proposed project site consists of previously disturbed tidal buffer area which has historically been used as an industrial area.

(2) There is no alternative location outside the wetland buffer that is feasible and reasonable for the proposed use, activity or alteration.

The proposed project site has several constraints, as shown on the enclosed constraints plan, which limits the areas that are feasible for use. Section 10.5A42.40 of the Zoning Ordinance includes public view corridors extending from Dover Street and Cabot Street which breaks the development area into sections. An existing sewer easement runs through the site, along with a minimum 15' setback from the railroad. The placement of the proposed buildings and parking areas was done in a way to minimize the areas of impervious surface within the 100-foot tidal buffer. Of the 85,029 square feet of impervious area within the 100-foot buffer in the proposed condition, 82.5% of that area is within the existing roadway and Great Rhythm building footprint.

(3) There will be no adverse impact on the wetland functional values of the site or surrounding properties;

As previously stated, the impact to the Functions and values of the site and or surrounding properties will be negligible. The wetland area proposed to be impacted was is located within a man-made structure in a highly disturbed landscape and currently provides negligible wetland functions and values. (4) Alteration of the natural vegetative state or managed woodland will occur only to the extent necessary to achieve construction goals; and

The impact to the 100-foot buffer has been limited to the greatest extent possible. No unnecessary grading or clearing of vegetation will occur. The temporary disturbances of the wetland buffer for construction access and to construct the stormwater outlets will be restored following construction.

(5) The proposal is the alternative with the least adverse impact to areas and environments under the jurisdiction of this Section.

As previously noted, the proposed project offers the least possible impact to the wetland buffer. The jurisdictional wetlands impacted are located within a manmade structure in a highly disturbed landscape and currently provides negligible wetland functions and values. The placement of the proposed buildings and parking areas was done in a way to minimize the areas of impervious surface within the 100-foot tidal buffer. The proposed project will reduce the impervious area within the 100-foot tidal buffer by 24,330 square feet as shown in Table 1.

(6) Any area within the vegetated buffer strip will be returned to a natural state to the extent feasible.

The vegetated buffer strip will only be disturbed to construct the three (3) stormwater outlets from the stormwater collection and treatment systems. The existing property has no stormwater treatment measures. The proposed project will collect and treat the onsite existing and proposed impervious surfaces prior to discharging to North Mill Pond. Implementing these treatment measures will help improve the water quality in North Mill Pond. In order for this system to work, new discharge points need to be constructed. Areas temporarily disturbed for the construction of these outlets will be restored following construction. Additionally, the final design and construction of the outlets will need to be coordinated with the City of Portsmouth's design of the North Mill Pond trail.

Under separate cover, a Wetlands Conditional Use Permit application fee in the amount of \$1,000 has been mailed to the Planning Department by the applicant.

We respectfully request to be placed on the Conservation Committee meeting agenda for May 13, 2020. If you have any questions or need any additional information, please contact Patrick Crimmins by phone at (603) 433-8818 or by email at <u>pmcrimmins@tighebond.com</u>.

Sincerely, **TIGHE & BOND, INC.**

Patrick M. Crimmins, PE Senior Project Manager

Neil A. Hansen, PE Project Engineer

Enclosures Copy: Clipper Traders, LLC (via E-mail) Iron Horse Properties, LLC (via E-mail) Portsmouth Lumber & Hardware, LLC (via E-mail)

J:\C\C0960 Cathartes\C-0960-006 105 Bartlett Street\Report_Evaluation\Applications\City of Portsmouth\20200429 Wetlands Conditional Use\C0960-006 Wetlands CUP Letter.docx



Cathartes 105 Bartlett Street Project Portsmouth, NH

WETLAND DELINEATION AND ASSESSMENT OF FUNCTIONS AND VALUES

December 2019







| 1.0 | Intro | duction | 1 |
|-----|-------|--|---|
| 2.0 | Metho | ods | 1 |
| 3.0 | North | Mill Pond | 1 |
| | 3.1 | Ecological Integrity | 2 |
| | 3.2 | Wildlife, Finfish, and Shellfish Habitat | |
| | 3.3 | Recreational and Commercial Potential | |
| | 3.4 | Aesthetic Quality | 2 |
| | 3.5 | Educational Potential | 3 |
| | 3.6 | Noteworthiness | |
| 4.0 | North | Mill Pond Tidal Buffer | 3 |
| | 4.1 | Commercial Area Buffer | 3 |
| | 4.2 | Disturbed Forest Buffer | |
| | 4.3 | Shrub Thicket Buffer | 4 |
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| 5.0 | | vated Palustrine Forested Wetland | |
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1.0 Introduction

The purpose of this report is to characterize wetlands and buffers in the vicinity of a proposed multi-family development at 105 Bartlett Street in Portsmouth, NH. The site is long and narrow and is located between an active railroad and North Mill Pond. It includes commercial buildings with paved and gravel parking areas, abandoned railroad structures, disturbed forest, and a dense shrub thicket. The area is highly disturbed, being originally filled by the railroad in the late 1800s.

2.0 Methods

On October 29 and December 2, 2019, Tighe & Bond reviewed and assessed 2,000+/linear feet of tidal wetlands and buffers along the North Mill Pond. The review was limited to the vicinity of a proposed multi-family development, extending from Bartlett Street to an area opposite Cornwall Street, which runs roughly perpendicular to the parcel.

The wetland delineation review was based on criteria specified in the *Corps of Engineers Wetlands Delineation Manual, Technical Report Y-87-1* (January 1987), and the *Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Northcentral and Northeast Region* (January 2012). The Highest Observable Tide Line was reviewed based on the definition found in NH Department of Environmental Services Wetland Rules, Env-Wt 101.49/Env-Wt 602.23. Wetlands were classified based on *Classification of Wetlands and Deepwater Habitats of the United States* (Cowardin et al., 1979).

The Highest Observable Tide Line (HOTL) had been previously delineated by another consulting firm in 2017. This line was reviewed by exporting the 2017 surveyed line into ArcGIS to overlay on an aerial photographic base map. This base map was then uploaded to an iPad and paired with a Trimble R1 submeter GPS for in-field verification. Using the iPad and GPS as a guide, the line was then evaluated in the field. The HOTL was deemed accurate and the previous 2017 delineation was accepted by Tighe & Bond. A previously unidentified freshwater wetland was also found within a six-foot +/- deep abandoned railroad turntable. Tighe & Bond delineated this area with sequentially numbered flagging and located the wetland boundary using the GPS technology described above.

Functions and values were assessed in the vicinity of the proposed project. Assessment methodologies were adapted from the *Maine Citizens Guide to Evaluating, Restoring, and Managing Tidal Marshes* (Bryan et al., 1997) and *The Highway Methodology Workbook Supplement—Wetland Functions and Values: A Descriptive Approach*, NAEEP-360-1-30a, US Army Corps of Engineers, New England Division, September 1999.

Photographs of the wetlands and buffers are provided in Appendix A.

3.0 North Mill Pond

North Mill Pond is a 79+/- acre tidal pond at the outlet of Hodgson Brook. It receives tidal flows under Maplewood Avenue at the northeast end of the pond. The pond consists predominantly of exposed mudflats at low tide, and is classified as Estuarine, Intertidal, Unconsolidated Shore, Mud, Regularly Flooded (E2US3N). A narrow band of salt marsh reaching up to 35+/- feet wide was identified between the mudflats and upland (Photo 1). The marsh is dominated by smooth cordgrass (*Spartina alterniflora*), with species such as

saltmeadow cordgrass (*Spartina patens*), sea lavender (*Limonium carolinianum*), and seaside goldenrod (*Solidago sempervirens*) more dominant toward the upland edge. This marsh area was classified as Estuarine, Intertidal, Emergent, Persistent, Regularly Flooded (E2EM1N).

North Mill Pond provides several important wetland functions and values, though many have been degraded by development and human activity. The following functions and values were assessed for the wetland in the vicinity of the proposed project.

3.1 Ecological Integrity

Ecological Integrity relates to how much the wetland has retained its native biotic and abiotic features and how these may have been degraded by human influences.

The Ecological Integrity of North Mill Pond has been compromised due to the presence of a tidal restriction, development of the upland buffer, water quality degradation, and filling along the project site. Efforts have been made in recent years to improve water quality entering the pond, improve tidal flushing, and restore some of the salt marshes. The peripheral salt marsh appears to be healthy and is comprised of native species along the project area.

3.2 Wildlife, Finfish, and Shellfish Habitat

The Wildlife, Finfish, and Shellfish Habitat function is the suitability of the habitat to support wildlife.

North Mill Pond contains extensive mudflats and a healthy but narrow peripheral salt marsh that contribute to wildlife habitat value. However, this value has been compromised by all the factors affecting Ecological Integrity described above. The area is likely to support a variety of wildlife, including migratory birds, finfish, and shellfish. Wildlife Action Plan mapping (Appendix B) depicts several small areas of the highest ranked wildlife value habitat around the pond. These high value habitats include two salt marsh areas in the vicinity of the proposed project; one directly across from the project and another just to the northeast of it.

3.3 Recreational and Commercial Potential

Recreational and Commercial Potential is the suitability of the wetland to support activities such as hiking, boating, hunting, bird watching, and shellfish harvesting.

North Mill Pond has the potential for use by small boats during high tide, though access appears to be limited to a boat launch on Marsh Lane, north of Maplewood Avenue. Shellfish harvesting is not allowed within the mudflats. Bird watching is a potential activity but public access is limited. There is an informal trail that runs through the proposed project area between Bartlett Street and Maplewood Avenue across private property that could be used for bird watching, but public access is not currently guaranteed. There is no visitor center, formally maintained trails, or access for disabled persons that would make this a more valuable area for recreation.

3.4 Aesthetic Quality

Aesthetic Quality refers to the ability of the wetland to provide interesting views and natural vistas.

The areas surrounding North Mill Pond are highly developed commercial and residential areas. There are few public viewing areas, but in locations where the pond can be seen it generally offers wide vistas and aesthetically pleasing views.

3.5 Educational Potential

Educational Potential consists of the ability of the wetland to serve as an outdoor classroom.

There is no safe public access to North Mill Pond near the project site. In addition to being private property, the project site has dangerous construction debris and steep banks to the pond, further diminishing the educational potential of this wetland.

3.6 Noteworthiness

Noteworthiness includes important qualities of the wetland not identified in previous functions, such as historic sites or unique natural features.

This area of North Mill Pond is noteworthy as it contains a salt marsh in a developed setting, which adds to its importance aesthetically and as part of the character of the area. In addition, the adjacent uplands have been proposed as part of the North Mill Pond Greenways project, which was presented to stakeholders in January 2019. (https://www.cityofportsmouth.com/planportsmouth/north-mill-pond-trail-and-greenway).

The wetland itself is not known for having any important historical features in the vicinity of the project area, though there have been historic structures and activities along its banks. The project area has some significance as the site of an old railroad yard with a turntable and roundhouse.

4.0 North Mill Pond Tidal Buffer

The North Mill Pond 100-foot tidal buffer can be divided into three zones within the project area: 1) a commercial area, including the Ricci Supply and Ace Hardware complex, the Great Rhythm Brewery building, a former railroad machine shop, and all the paved and unpaved impervious surfaces associated with those buildings; 2) the disturbed forest directly northeast and northwest of Great Rhythm Brewery, including the area around the old railroad turntable and roundhouse remains; and 3) the shrub thicket extending along the narrow portion of the parcel to the northeast. These areas all include historic filling 2-16 feet deep associated with railroad activities. The fill includes coal, coal ash, and possible slag.

4.1 Commercial Area Buffer

The commercial area (Photos 1-3) is comprised almost completely of impervious surfaces. These include buildings, paved and compact gravel parking lots, and a narrow strip of vegetation 10-20 feet wide extending down a steep bank to the tidal wetland. The vegetation includes lawn and species associated with disturbed sites such as staghorn sumac (*Rhus typhina*), autumn olive (*Elaeagnus umbellata*), black cherry (*Prunus serotina*), and Asiatic bittersweet (*Celastrus orbiculatus*). This area has little to offer in the way of functions and values other than contributing to stabilization of steep eroding

banks along the wetland. Runoff from this area likely contributes to the degraded water quality in North Mill Pond.

4.2 Disturbed Forest Buffer

The disturbed forested area northeast and southwest of Great Rhythm Brewery (Photos 4-6) is dominated by Norway Maple (*Acer platanoides*), black cherry, and staghorn sumac. The area includes significant rubble and debris as well as the railroad turntable and roundhouse remains. This area provides some screening for wildlife using the North Mill Pond and provides cover and food for small mammals and birds. However, it is dominated by invasive vegetation, and is highly disturbed by human activity.

4.3 Shrub Thicket Buffer

The shrub thicket northeast of the commercial area (Photos 7-8) is dominated by autumn olive with lesser amounts of staghorn sumac and other shrub species. This area provides wildlife habitat for small mammals and birds as well as screening for wildlife using North Mill Pond. Though invasive, the autumn olive provides prolific fruits utilized by birds and other frugivores. Bedding, clothing, campfire remains, trash, and other evidence suggests that this densely vegetated area has been used as camp sites by homeless individuals.

4.4 Buffer Impacts and Mitigation

The proposed project will not include any work within the 25-foot buffer to North Mill Pond. In addition, a 50-foot easement from the mean high water will be granted to the City of Portsmouth by the developer to build the North Mill Pond Trail and Greenway, which will provide improvements to the buffer, including invasive species management and revegetation with native species. Installation of the trail and greenway would result in improved functions and values of the wetland and buffer including: Ecological Integrity, Recreation Potential, Aesthetic Quality, and possibly Educational Potential. Existing impacts to the 100-foot buffer will be reduced by 53,716 square feet from the trail and greenway improvements through the removal and restoration of impervious surfaces (Table 4.1).

Table 4.1

105 Bartlett Street Multi-Family Development Buffer Impact Reductions

| Buffer Segment | Existing Impact (SF) | Final Impact (SF) | Net Impact (SF) |
|----------------|-------------------------|----------------------|-----------------|
| 0-25 feet | 12,788 | 6,691 | -6,097 |
| 25-50 feet | 30,479 | 18,391 | -12,088 |
| 50-100 feet | 66,812 | 60,667 | -6,145 |
| Total | 108,845 | 85,029 | -24,330 |

5.0 Excavated Palustrine Forested Wetland

A small wetland was delineated by Tighe & Bond within the base of the six-foot +/- deep, concrete walled railroad roundtable (Photos 9-10) within the disturbed forested area. The soils in this wetland are poorly drained marine silts and clays. The vegetation is dominated by Norway maple and red osier dogwood (*Cornus sericea*). Nearby test pits identified approximately two to four feet of fill in the vicinity of the structure. Therefore, it is likely this wetland was at least partly excavated into native marine sediments during

construction of the turntable. It is unclear if this area was originally a wetland or if the wetland was created by the excavation. This wetland was classified as Palustrine, Forested, Deciduous, Saturated (PFO1B). The small size of the wetland and its location within a man-made structure in a highly disturbed landscape has resulted in this system providing negligible wetland functions and values.

6.0 Summary

Two wetlands were delineated and evaluated on the site:

North Mill Pond is a 79+/- acre tidal wetland with expansive mud flats (E2US3N) and a narrow fringe of salt marsh (E2EM1N). Wetland functions and values are primarily Wildlife, Finfish, and Shellfish Habitat, as well as Aesthetic Quality and Noteworthiness. It is noteworthy as an important aesthetic component of the area and as an important potential site for a greenways trail project. It also has compromised but improving Ecological Integrity and some Recreation Potential. Upland buffers to the wetland have been compromised by development and invasive species, but limited vegetation does provide some screening for wildlife in the wetland.

A small excavated forested wetland (PFO1B) was identified within the old railroad turntable, approximately six feet below existing grade within a concrete wall. The small size of the wetland and its location within a man-made structure in a highly disturbed landscape has resulted in this system providing negligible wetland functions and values.

J:\C\C0960 Cathartes\C-0960-006 105 Bartlett Street\Report_Evaluation\SSSM+Wetlands\Buffer\Wetland+Buffer Review Rev 2020-04-29.docx

Tighe&Bond

APPENDIX A



Job Number: C-0960006

Site: 105 Bartlett St., Portsmouth, NH

| Photograph No.: 1 | Date: 10/29/19 | Direction Taken: Northeast |
|-------------------|-----------------------|----------------------------|
|-------------------|-----------------------|----------------------------|

Description: Salt marsh fringe along North Mill Pond at low tide opposite a commercial area in southwest portion of the site.



Description: Paved buffer and eroding banks along North Mill Pond at low tide along the commercial area in southwest portion of the site.





Job Number: C-0960006

Site: 105 Bartlett St., Portsmouth, NH

| Photograph No.: 3 Date: 10/29/19 | Direction Taken: Northeast |
|---|----------------------------|
|---|----------------------------|

Description: Compact gravel drive and old railroad repair shop at the northern end of the commercial area with impervious surfaces.



| Photograph No.: 4 | Date: 10/29/19 | Direction Taken: Northeast |
|-------------------|----------------|----------------------------|
|-------------------|----------------|----------------------------|

Description: Buffer fill slope with rubble adjacent to a narrow salt marsh along the disturbed forest just northwest of the Great Rhythm Brewing Company.





Job Number: C-0960006

Site: 105 Bartlett St., Portsmouth, NH

| Photograph No.: 5 | Date: 10/29/19 | Direction Taken: Southwest |
|-------------------|----------------|----------------------------|
|-------------------|----------------|----------------------------|

Description: Lawn and disturbed forest buffer northwest of the Great Rhythm Brewing Company.



| Photograph No.: 6 | Date: 10/29/19 | Direction Taken: East |
|-------------------|----------------|-----------------------|
| | | |

Description: Remains of the railroad roundhouse and disturbed forest buffer northeast of the Great Rhythm Brewing Company.





Job Number: C-0960006

Site: 105 Bartlett St., Portsmouth, NH

| Photograph No.: 7 | Date: 10/29/19 | Direction Taken: Northeast |
|-------------------|----------------|----------------------------|
|-------------------|----------------|----------------------------|

Description: Shrub thicket and existing informal trail at the northeast end of the proposed project area.



| Photograph No.: 8 | Date: 10/29/19 | Direction Taken: Northeast |
|-------------------|----------------|----------------------------|
| | | |

Description: Evidence of use as camp sites by homeless individuals within the shrub thicket at the northeast end of the project area.





Job Number: C-0960006

Site: 105 Bartlett St., Portsmouth, NH

| Photograph No.: 9 | Date: 12/2/19 | Direction Taken: South |
|-------------------|---------------|------------------------|
|-------------------|---------------|------------------------|

Description: Wetland dominated by Norway maple and red osier dogwood within the old railroad turntable approximately six feet below grade.



| Photograph No.: 10 | Date: 12/2/19 | Direction Taken: n/a |
|--------------------|----------------------|----------------------|
|--------------------|----------------------|----------------------|

Description: Poorly drained marine silts and clays observed in the bottom of the old railroad turntable.



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APPENDIX B

2015 HIGHEST RANKED WILDLIFE HABITAT BY ECOLOGICAL CONDITION

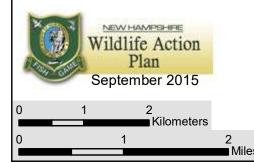
Highest Ranked Habitat in New Hampshire

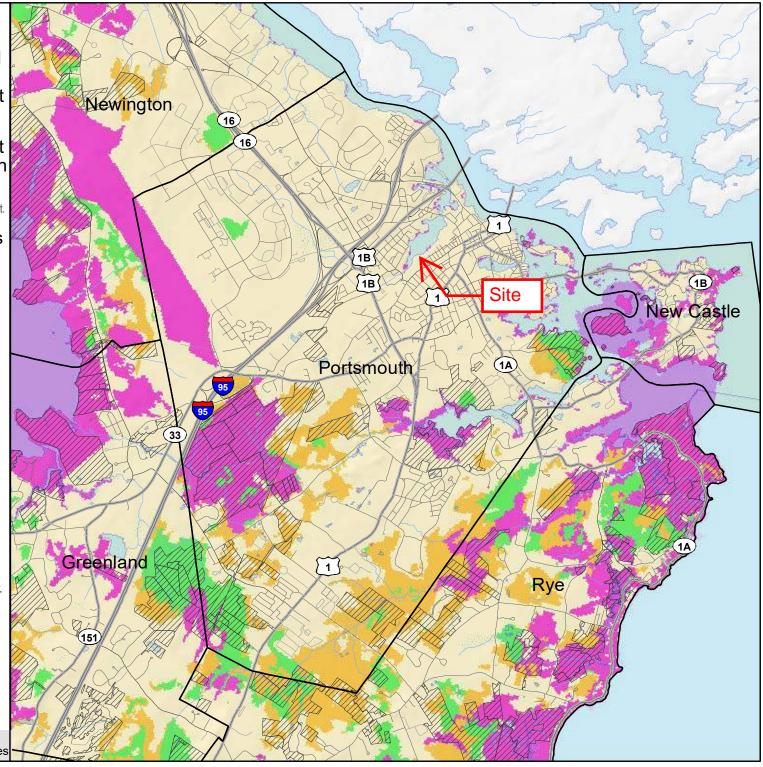
Highest Ranked Habitat in the Biological Region

Biological region = TNC ecoregional subsection for terrestrial habitats or Aquatic Resource Mitigation region for wetlands and floodplain forest.

Supporting Landscapes

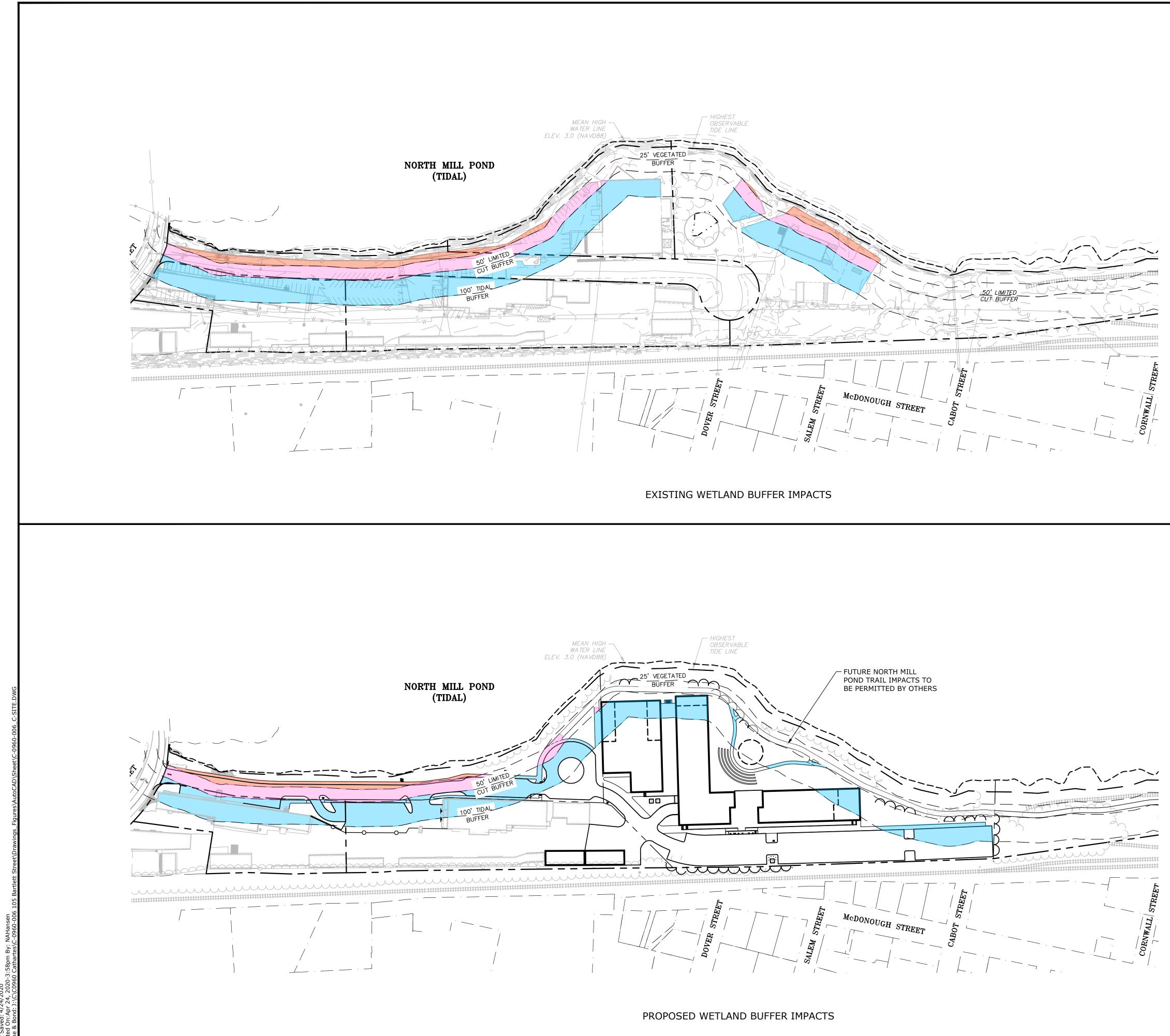
Base map data provided by NH GRANIT at UNH September 2019. Intended for planning use only.





www.tighebond.com





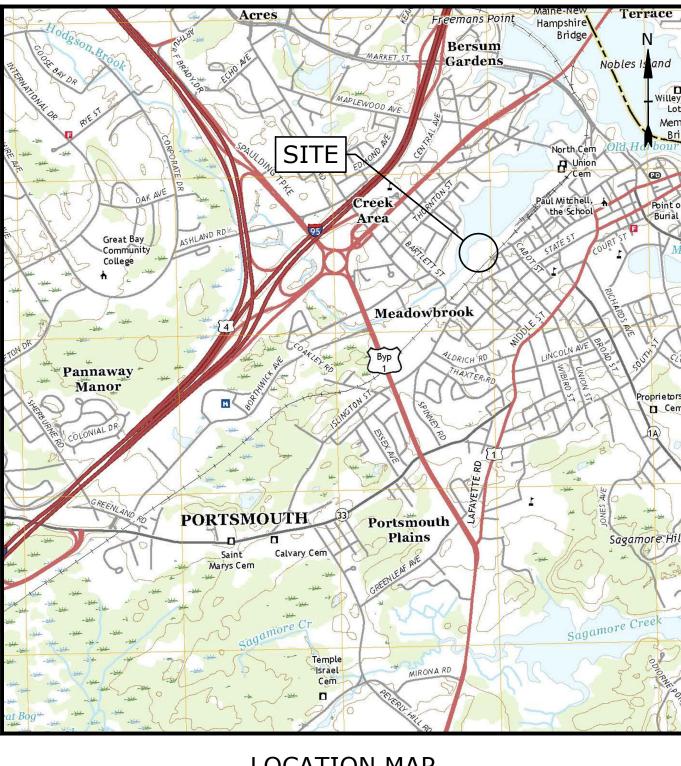
| | 2 | Tighe&Bond |
|-----------|--|---|
| | EGEND MPACTS WITHIN 25 FT VEGETATED BUFFER = 12,788 SF MPACTS WITHIN 25 FT TO 50 FT MITED CUT AREA = 30,479 ST MPACTS WITHIN 50 FT TO 100 FT CDAL BUFFER = 66,812 ST | |
| | | 0 100' 200' GRAPHIC SCALE |
| | | Proposed Multi-Family Development |
| <u>LE</u> | GEND IMPACTS WITHIN 25 FT VEGETATED BUFFER = 6,691 SF IMPACTS WITHIN 25 FT TO 50 FT | Iron Horse Properties, LLC |
| | LIMITED CUT AREA = 18,391 SF IMPACTS WITHIN 50 FT TO 100 FT TIDAL BUFFER = 60,667 SF | 105 Bartlett Street Portsmouth, New Hampshire |
| | | Image: Second system Image: Second system Image: Second |

PROPOSED MULTI-FAMILY DEVELOPMENT **105 BARTLETT STREET** PORTSMOUTH, NEW HAMPSHIRE JANUARY 2, 2020 LAST REVISED: APRIL 20, 2020

| LIST OF DRAWINGS | | |
|------------------|---|--------------|
| SHEET NO. | SHEET TITLE | LAST REVISED |
| | COVER SHEET | 04/20/2020 |
| C-101 | OVERALL EXISTING CONDITIONS AND DEMOLITION PLAN | 04/20/2020 |
| C-101.1 | EXISTING CONDITIONS AND DEMOLITION PLAN | 04/20/2020 |
| C-101.2 | EXISTING CONDITIONS AND DEMOLITION PLAN | 04/20/2020 |
| C-102 | OVERALL SITE PLAN | 04/20/2020 |
| C-102.1 | SITE PLAN | 04/20/2020 |
| C-102.2 | SITE PLAN | 04/20/2020 |
| C-102.3 | BASEMENT LEVEL SITE PLAN | 04/20/2020 |
| C-103.1 | GRADING, DRAINAGE, AND EROSION CONTROL PLAN | 04/20/2020 |
| C-103.2 | GRADING, DRAINAGE, AND EROSION CONTROL PLAN | 04/20/2020 |
| C-104.1 | UTILITIES PLAN | 04/20/2020 |
| C-104.2 | UTILITIES PLAN | 04/20/2020 |
| C-301.1 | EASEMENT PLAN | 04/20/2020 |
| C-301.2 | EASEMENT PLAN | 04/20/2020 |
| C-501 | EROSION CONTROL NOTES AND DETAILS SHEET | 04/20/2020 |
| C-502 | DETAILS SHEET | 04/20/2020 |
| C-503 | DETAILS SHEET | 04/20/2020 |
| C-504 | DETAILS SHEET | 04/20/2020 |
| C-505 | DETAILS SHEET | 04/20/2020 |
| C-506 | DETAILS SHEET | 04/20/2020 |
| L-1 | LANDSCAPE PLAN | 02/20/2020 |

| LIST OF PERMITS | | |
|---|---------|------|
| LOCAL | STATUS | DATE |
| SITE PLAN REVIEW PERMIT | PENDING | |
| LOT LINE REVISION PERMIT | PENDING | |
| CONDITIONAL USE PERMIT - SHARED PARKING | PENDING | |
| CONDITIONAL USE PERMIT - WETLAND BUFFER | PENDING | |
| STATE | | |
| NHDES - ALTERATION OF TERRAIN PERMIT | PENDING | |
| NHDES - WETLAND PERMIT | PENDING | |
| NHDES - SHORELAND PERMIT | PENDING | |
| NHDES - SEWER CONNECTION PERMIT | PENDING | |
| FEDERAL | | |
| EPA - NPDES CGP | PENDING | |

T&B PROJECT NO: C0960-006



LOCATION MAP SCALE: 1" = 2000'

PREPARED BY:

PORTSMOUTH, NEW HAMPSHIRE 03801 603-433-8818

OWNERS: TAX MAP 157, LOT 1 CLIPPER TRADERS, LLC **105 BARTLETT STREET** PORTSMOUTH, NEW HAMPSHIRE 03801

TAX MAP 164, LOT 4-2 IRON HORSE PROPERTIES, LLC **105 BARTLETT STREET** PORTSMOUTH, NH 03801

TAX MAP 157 LOT 2 TAX MAP 164, LOT 1 PORTSMOUTH HARDWARE & LUMBER, LLC 105 BARTLETT STREET PORTSMOUTH, NH 03801

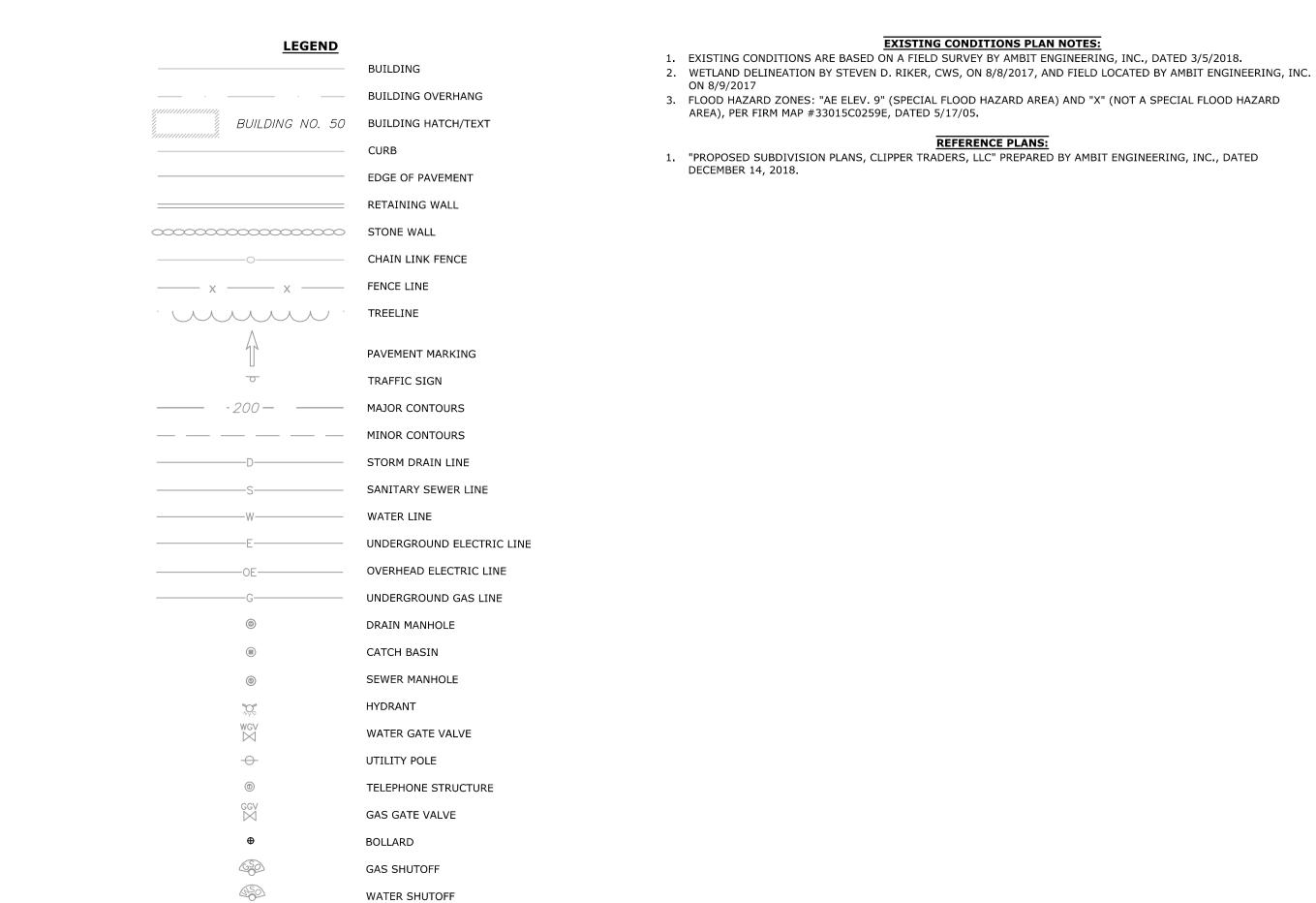
APPLICANT:

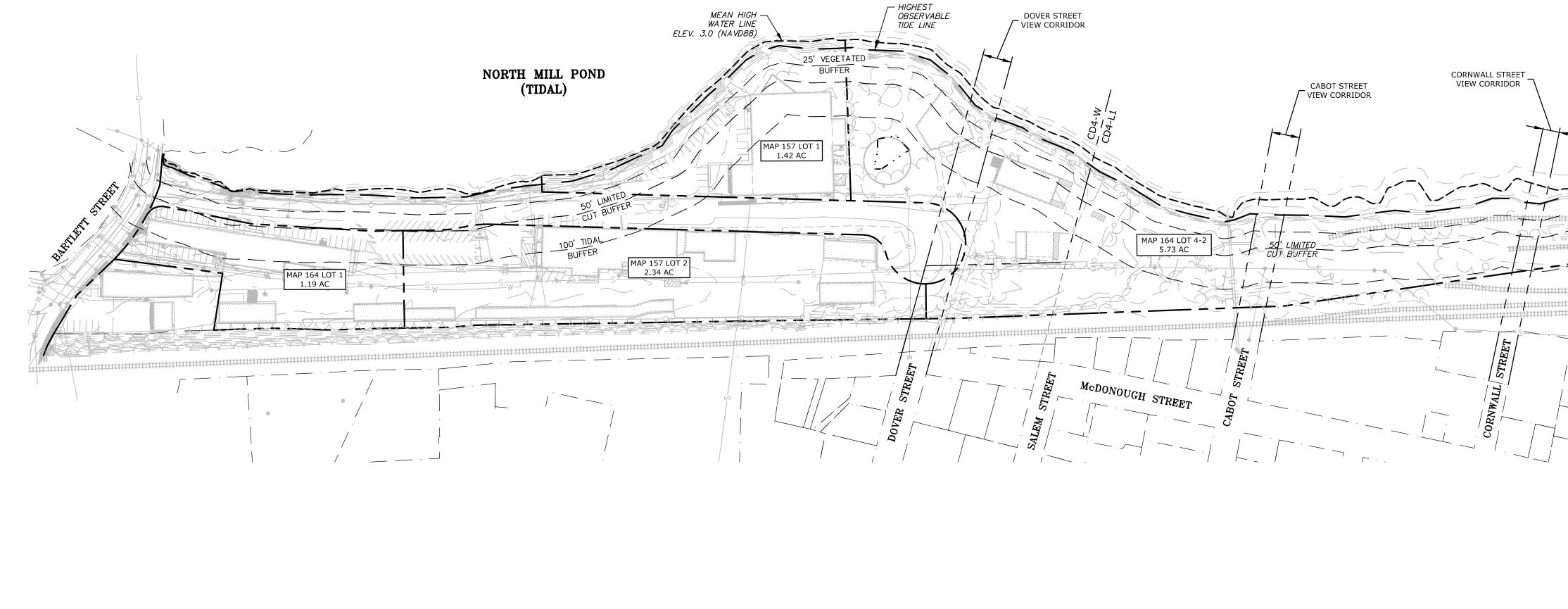
IRON HORSE PROPERTIES, LLC **105 BARTLETT STREET** PORTSMOUTH, NEW HAMPSHIRE 03801

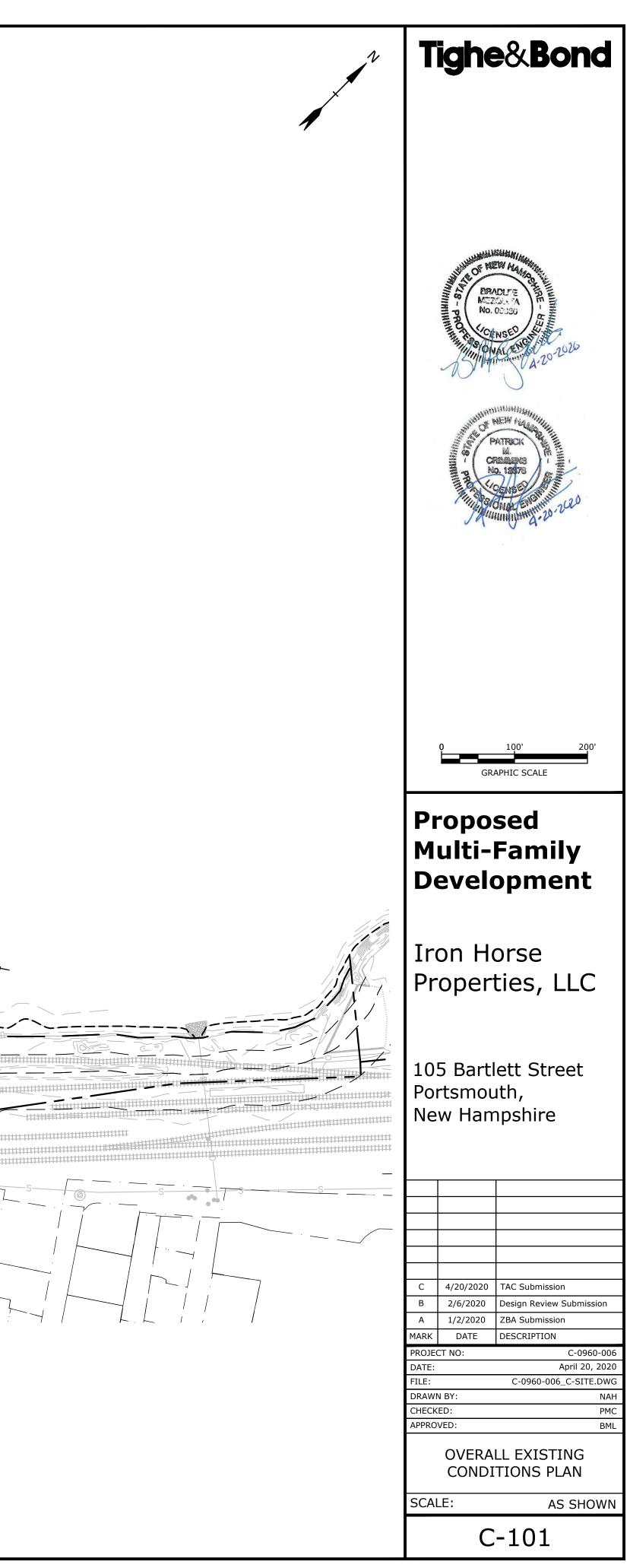


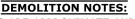


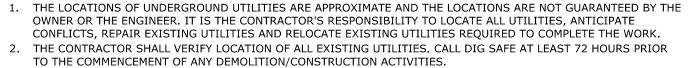
TAC REVIEW SUBMISSION COMPLETE SET 21 SHEETS











3. ALL MATERIALS SCHEDULED TO BE REMOVED SHALL BECOME THE PROPERTY OF THE CONTRACTOR UNLESS OTHERWISE SPECIFIED. THE CONTRACTOR SHALL DISPOSE OF ALL MATERIALS OFF-SITE IN ACCORDANCE WITH ALL FEDERAL, STATE, AND LOCAL REGULATIONS, ORDINANCES AND CODES.

4. COORDINATE REMOVAL, RELOCATION, DISPOSAL OR SALVAGE OF UTILITIES WITH THE OWNER AND APPROPRIATE UTILITY COMPANY. 5. ANY EXISTING WORK OR PROPERTY DAMAGED OR DISRUPTED BY CONSTRUCTION/ DEMOLITION ACTIVITIES SHALL BE

REPLACED OR REPAIRED TO MATCH ORIGINAL EXISTING CONDITIONS BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER. 6. SAW CUT AND REMOVE PAVEMENT ONE (1) FOOT OFF PROPOSED EDGE OF PAVEMENT OR EXISTING CURB LINE IN ALL

AREAS WHERE PAVEMENT TO BE REMOVED ABUTS EXISTING PAVEMENT OR CONCRETE TO REMAIN. 7. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FAMILIARIZE THEMSELVES WITH THE CONDITIONS OF ALL OF THE PERMIT APPROVALS.

8. THE CONTRACTOR SHALL OBTAIN AND PAY FOR ADDITIONAL PERMITS, NOTICES AND FEES NECESSARY TO COMPLETE THE WORK AND ARRANGE FOR AND PAY FOR NECESSARY INSPECTIONS AND APPROVALS FROM THE AUTHORITIES HAVING JURISDICTION.

9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL DEMOLITION AND OFF-SITE DISPOSAL OF MATERIALS REQUIRED TO COMPLETE THE WORK, EXCEPT FOR WORK NOTED TO BE COMPLETED BY OTHERS. 10. UTILITIES SHALL BE TERMINATED AT THE MAIN LINE PER UTILITY COMPANY STANDARDS. THE CONTRACTOR SHALL

REMOVE ALL ABANDONED UTILITIES LOCATED WITHIN THE LIMITS OF WORK. 11. CONTRACTOR SHALL VERIFY ORIGIN OF ALL DRAINS AND UTILITIES PRIOR TO REMOVAL/TERMINATION TO DETERMINE IF DRAINS OR UTILITY IS ACTIVE, AND SERVICES ANY ON OR OFF-SITE STRUCTURE TO REMAIN. THE CONTRACTOR SHALL NOTIFY ENGINEER IMMEDIATELY OF ANY SUCH UTILITY FOUND AND SHALL MAINTAIN THESE UTILITIES UNTIL

PERMANENT SOLUTION IS IN PLACE. 12. PAVEMENT REMOVAL LIMITS ARE SHOWN FOR CONTRACTOR'S CONVENIENCE. ADDITIONAL PAVEMENT REMOVAL MAY BE REQUIRED DEPENDING ON THE CONTRACTOR'S OPERATION. CONTRACTOR TO VERIFY FULL LIMITS OF PAVEMENT REMOVAL PRIOR TO BID.

13. ALL ITEMS WITHIN THE LIMIT OF WORK ARE TO REMAIN UNLESS SPECIFICALLY IDENTIFIED TO BE REMOVED OR OTHERWISE ALTERED BY THE CONTRACTOR. ITEMS TO BE REMOVED INCLUDE, BUT ARE NOT LIMITED TO: CONCRETE, PAVEMENT, MANHOLES, CATCH BASINS, UNDERGROUND PIPING & UTILITIES, POLES, STAIRS, STRUCTURES, FENCES, RAMPS, BUILDING FOUNDATIONS, TREES, AND LANDSCAPING. THE CONTRACTOR SHALL CONFIRM WITH THE ENGINEER IF THE TREATMENT OF CERTAIN ITEMS IS UNCLEAR.

14. COORDINATE ALL WORK WITHIN THE PUBLIC RIGHT OF WAYS WITH THE CITY OF PORTSMOUTH. 15. REMOVE TREES AND BRUSH AS REQUIRED FOR COMPLETION OF WORK. CONTRACTOR SHALL GRUB AND REMOVE ALL

STUMPS WITHIN LIMITS OF WORK AND DISPOSE OF OFF SITE IN ACCORDANCE WITH FEDERAL, STATE, AND LOCAL LAWS AND REGULATIONS. 16. CONTRACTOR SHALL PROTECT ALL PROPERTY MONUMENTATION THROUGHOUT DEMOLITION AND CONSTRUCTION

OPERATIONS. SHOULD ANY MONUMENTATION BE DISTURBED BY THE CONTRACTOR, THE CONTRACTOR SHALL EMPLOY A NEW HAMPSHIRE LICENSED SURVEYOR TO REPLACE DISTURBED MONUMENTS. 17. PROVIDE INLET PROTECTION BARRIERS AT ALL CATCH BASINS/CURB INLETS WITHIN CONSTRUCTION LIMITS AS WELL

AS CATCH BASINS/CURB INLETS THAT RECEIVE RUNOFF FROM CONSTRUCTION ACTIVITIES. INLET PROTECTION BARRIERS SHALL BE MAINTAINED FOR THE DURATION OF THE PROJECT. INLET PROTECTION BARRIERS SHALL BE "HIGH FLOW SILT SACK" BY ACF ENVIRONMENTAL OR EOUAL. INSPECT BARRIERS WEEKLY AND AFTER EACH RAIN EVENT OF 0.25 INCHES OR GREATER. CONTRACTOR SHALL COMPLETE A MAINTENANCE INSPECTION REPORT AFTER EACH INSPECTION. SEDIMENT DEPOSITS SHALL BE REMOVED AFTER EACH STORM EVENT OR MORE OFTEN IF THE FABRIC BECOMES CLOGGED OR SEDIMENT HAS ACCUMULATED TO 1/3 THE DESIGN DEPTH OF THE BARRIER.

18. THE CONTRACTOR SHALL PHASE DEMOLITION AND CONSTRUCTION AS REQUIRED TO PROVIDE CONTINUOUS SERVICE TO EXISTING BUSINESSES AND HOMES THROUGHOUT THE CONSTRUCTION PERIOD. EXISTING BUSINESS AND HOME SERVICES INCLUDE, BUT ARE NOT LIMITED TO ELECTRICAL, COMMUNICATION, FIRE PROTECTION, DOMESTIC WATER AND SEWER SERVICES. TEMPORARY SERVICES, IF REQUIRED, SHALL COMPLY WITH ALL FEDERAL, STATE, LOCAL AND UTILITY COMPANY STANDARDS. CONTRACTOR SHALL PROVIDE DETAILED CONSTRUCTION SCHEDULE TO OWNER PRIOR TO ANY DEMOLITION/CONSTRUCTION ACTIVITIES AND SHALL COORDINATE TEMPORARY SERVICES TO ABUTTERS WITH THE UTILITY COMPANY AND AFFECTED ABUTTER.

19. EROSION CONTROL MEASURES SHALL BE INSTALLED PRIOR TO THE START OF ANY CLEARING OR DEMOLITION ACTIVITIES.

20. THE CONTRACTOR SHALL PAY ALL COSTS NECESSARY FOR TEMPORARY PARTITIONING, BARRICADING, FENCING, SECURITY AND SAFETY DEVICES REQUIRED FOR THE MAINTENANCE OF A CLEAN AND SAFE CONSTRUCTION SITE.

21. SAW CUT AND REMOVE PAVEMENT AND CONSTRUCT PAVEMENT TRENCH PATCH FOR ALL UTILITIES TO BE REMOVED AND PROPOSED UTILITIES LOCATED IN EXISTING PAVEMENT AREAS TO REMAIN.

22. ALL CONDITIONS ON THIS PLAN SHALL REMAIN IN EFFECT IN PERPETUITY PURSUANT TO THE REQUIREMENTS OF THE SITE PLAN REVIEW REGULATIONS.

SIGN

IRON ROD w/MSC

ID CAP FOUND,

NGS PID OCO412

- V 28 1942 ----

LLS 844

UP 3"·

"STOP" -----

TAX MAP 163 LOT 1 M&B PROPERTIES LLC 54 BARTLETT STREET

PORTSMOUTH, NH 03801 RCRD BOOK 5794 PAGE 0996

TBM A SPIKE IN UPOLE EL. 10.79

<u>'----</u>

BEGIN SILT -

APPROXIMATE

SAWCUT LINE

GAS LINE

SOCK

(TYP)

PSNH 176/6

RETAINING WALL

^{8/3} TAX MAP 164 LOT 2 PORTSMOUTH HARDWARE &

LUMBER, LLC

105 BARTLETT STREET

PORTSMOUTH. NH 03801

RCRD BOOK 5808 PAGE 1379

- CONCRETE BLOCK

RETAINING WALL

- GRANITE BLOCK

- DRILL HOLE FOUND IN PARAPET-CENTERLINE

ABUTMENT/WINGWALL

OF LOCATION STA 2969+04.87

1 STORY

BRICK/CINDER BLOCK

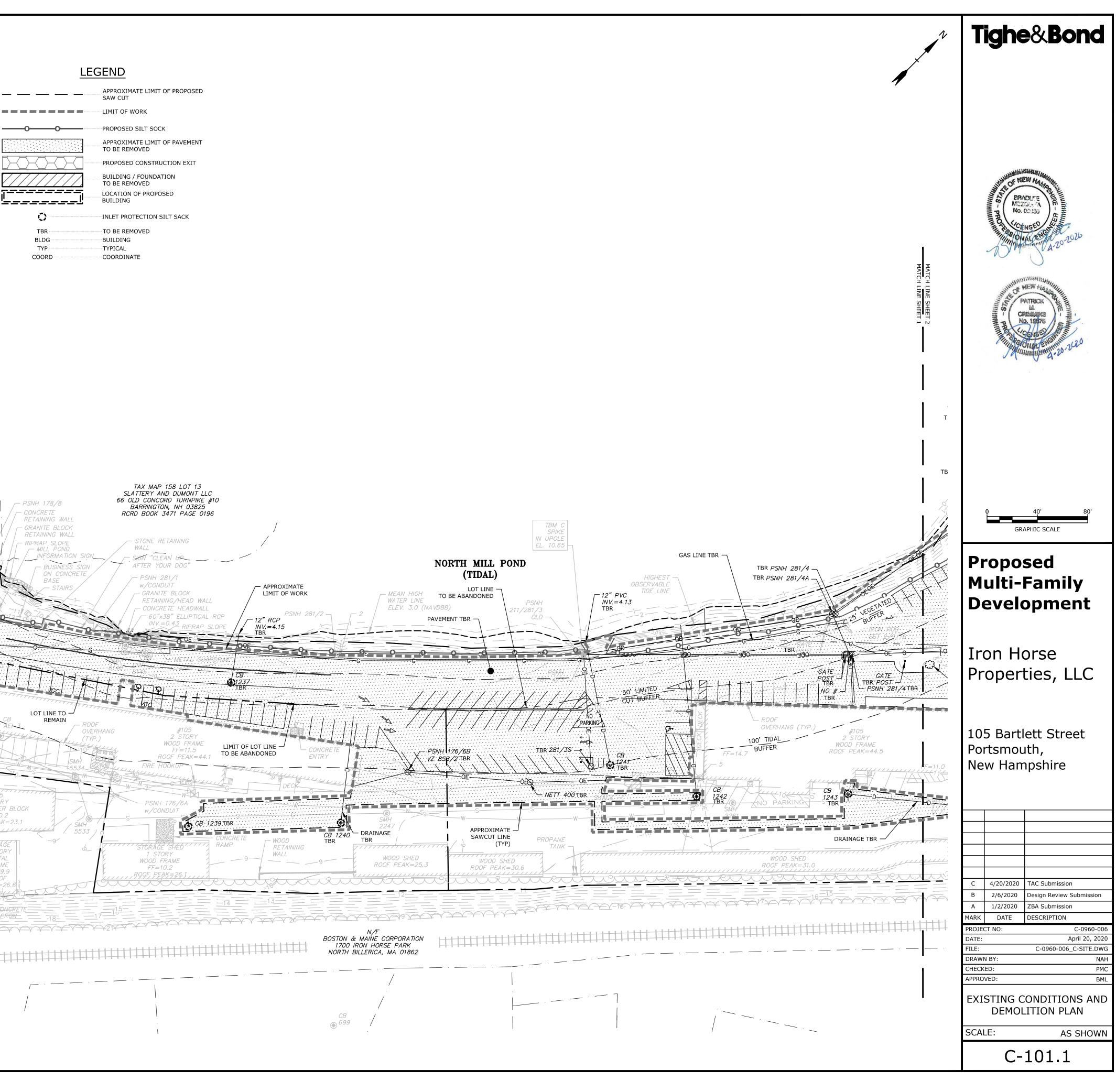
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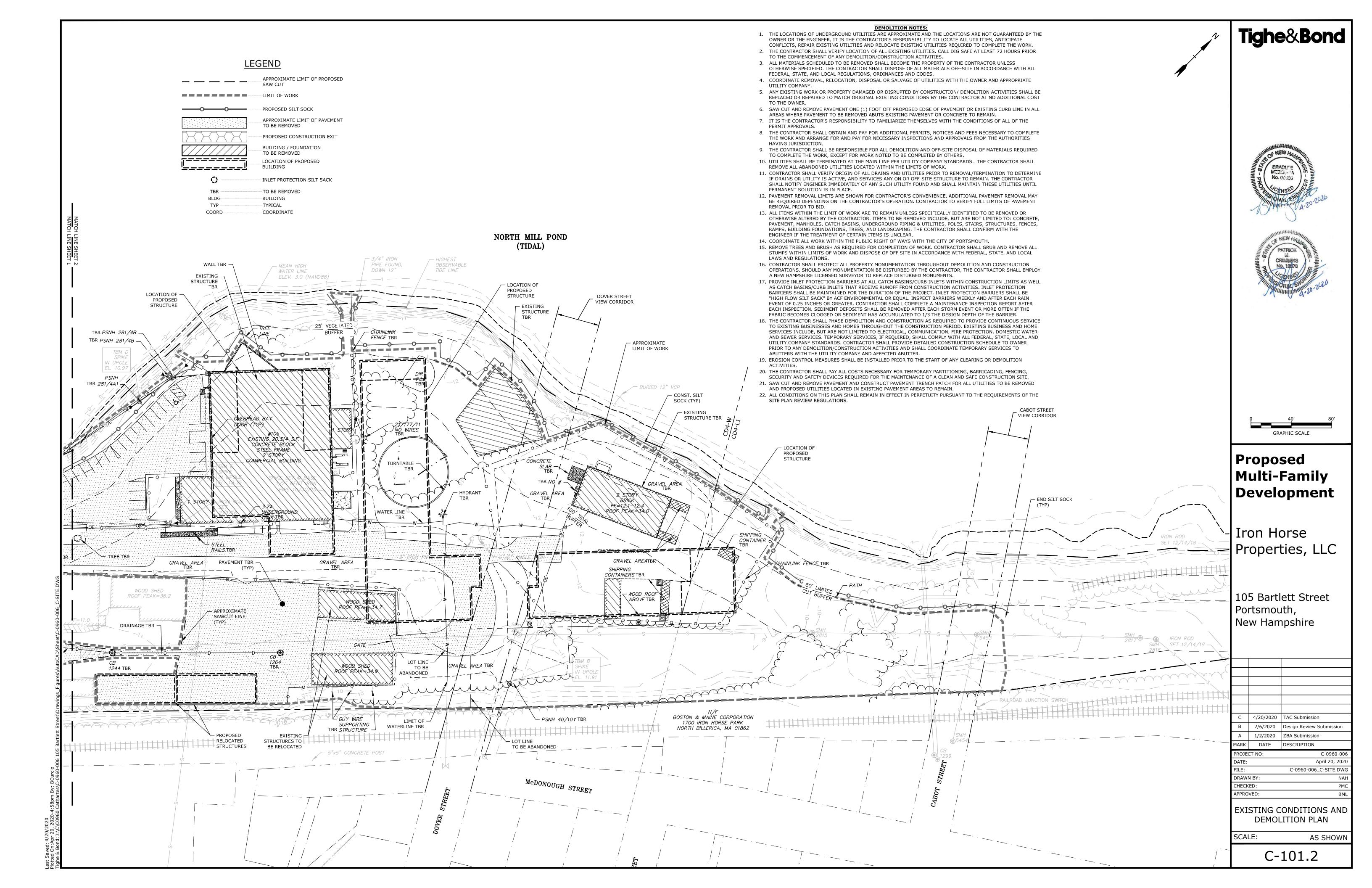
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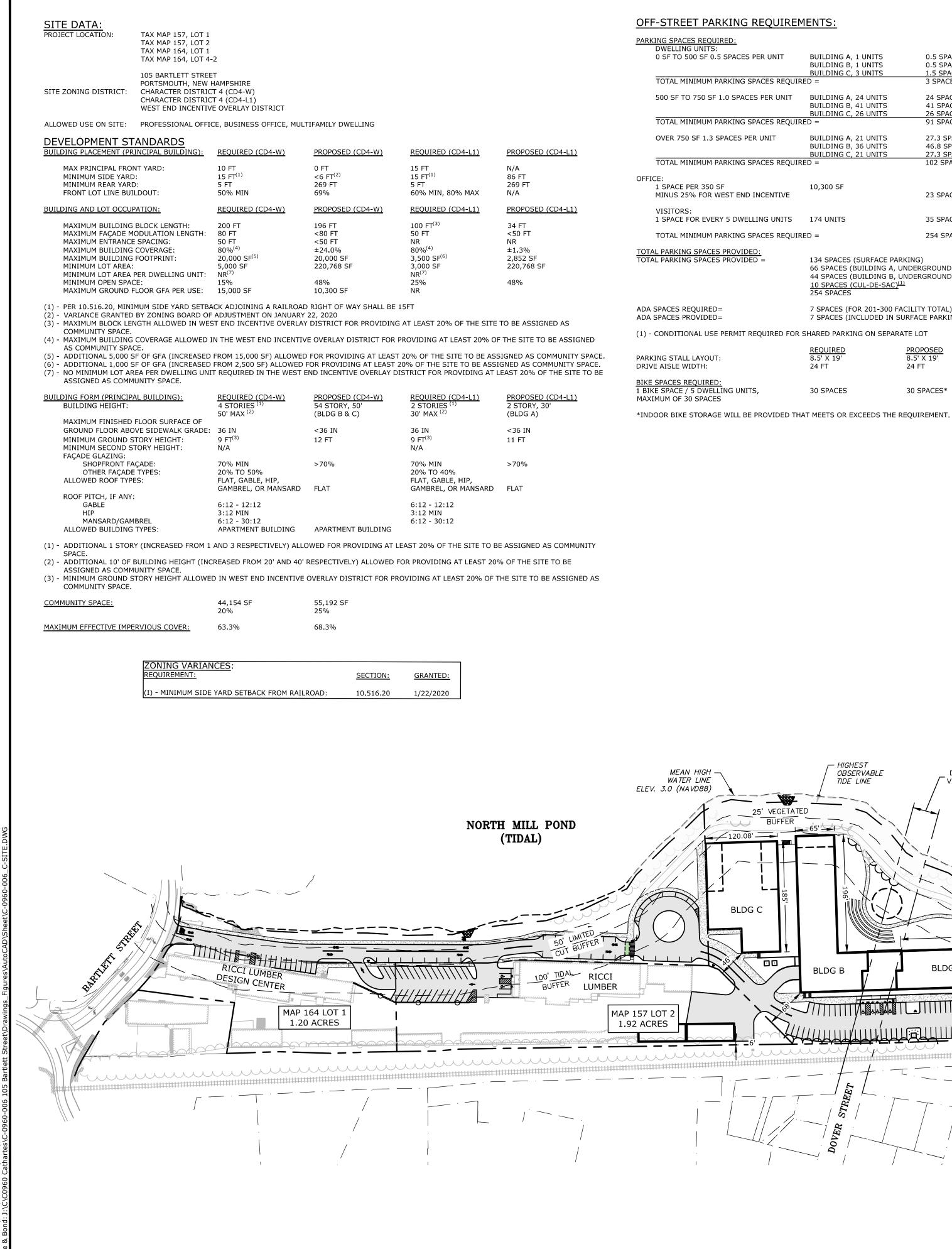
Ö TBR BLDG TYP COORD

BUILDING / FOUNDATION TO BE REMOVED

BUILDING TYPICAL







| REET | PARKING | REQUIREMENTS: |
|------|---------|----------------------|
| | | |

| REET I ARAMING REQUIRED | | |
|--|--|---|
| SPACES REQUIRED: ELLING UNITS: | | |
| TO 500 SF 0.5 SPACES PER UNIT | BUILDING A, 1 UNITS BUILDING B, 1 UNITS BUILDING C, 3 UNITS | 0.5 SPACES 0.5 SPACES 1.5 SPACES |
| AL MINIMUM PARKING SPACES REQUIRE | D = | 3 SPACES |
| SF TO 750 SF 1.0 SPACES PER UNIT | BUILDING A, 24 UNITS BUILDING B, 41 UNITS BUILDING C, 26 UNITS | 24 SPACES 41 SPACES 26 SPACES |
| AL MINIMUM PARKING SPACES REQUIRE | :D = | 91 SPACES |
| R 750 SF 1.3 SPACES PER UNIT | BUILDING A, 21 UNITS BUILDING B, 36 UNITS BUILDING C, 21 UNITS | 27.3 SPACES 46.8 SPACES 27.3 SPACES |
| AL MINIMUM PARKING SPACES REQUIRE | D = | 102 SPACES |
| ACE PER 350 SF JS 25% FOR WEST END INCENTIVE | 10,300 SF | 23 SPACES |
| TORS: ACE FOR EVERY 5 DWELLING UNITS | 174 UNITS | 35 SPACES |
| AL MINIMUM PARKING SPACES REQUIRE | :D = | 254 SPACES |
| RKING SPACES PROVIDED: 134 SPACES (SURFACE PARKING) 66 SPACES (BUILDING A, UNDERGROUND) 44 SPACES (BUILDING B, UNDERGROUND) 10 SPACES (CUL-DE-SAC) ⁽¹⁾ 254 SPACES | | NDERGROUND) NDERGROUND) |
| ES REQUIRED= ES PROVIDED= | 7 SPACES (FOR 201-300 FA 7 SPACES (INCLUDED IN SU | CILITY TOTAL) IRFACE PARKING COUNT OF 134) |
| DITIONAL USE PERMIT REQUIRED FOR S | HARED PARKING ON SEPARA | TE LOT |
| STALL LAYOUT: SLE WIDTH: | <u>REQUIRED</u> 8.5' X 19' 24 FT | PROPOSED 8.5' X 19' 24 FT |
| ES REQUIRED: | | |

30 SPACES

30 SPACES*

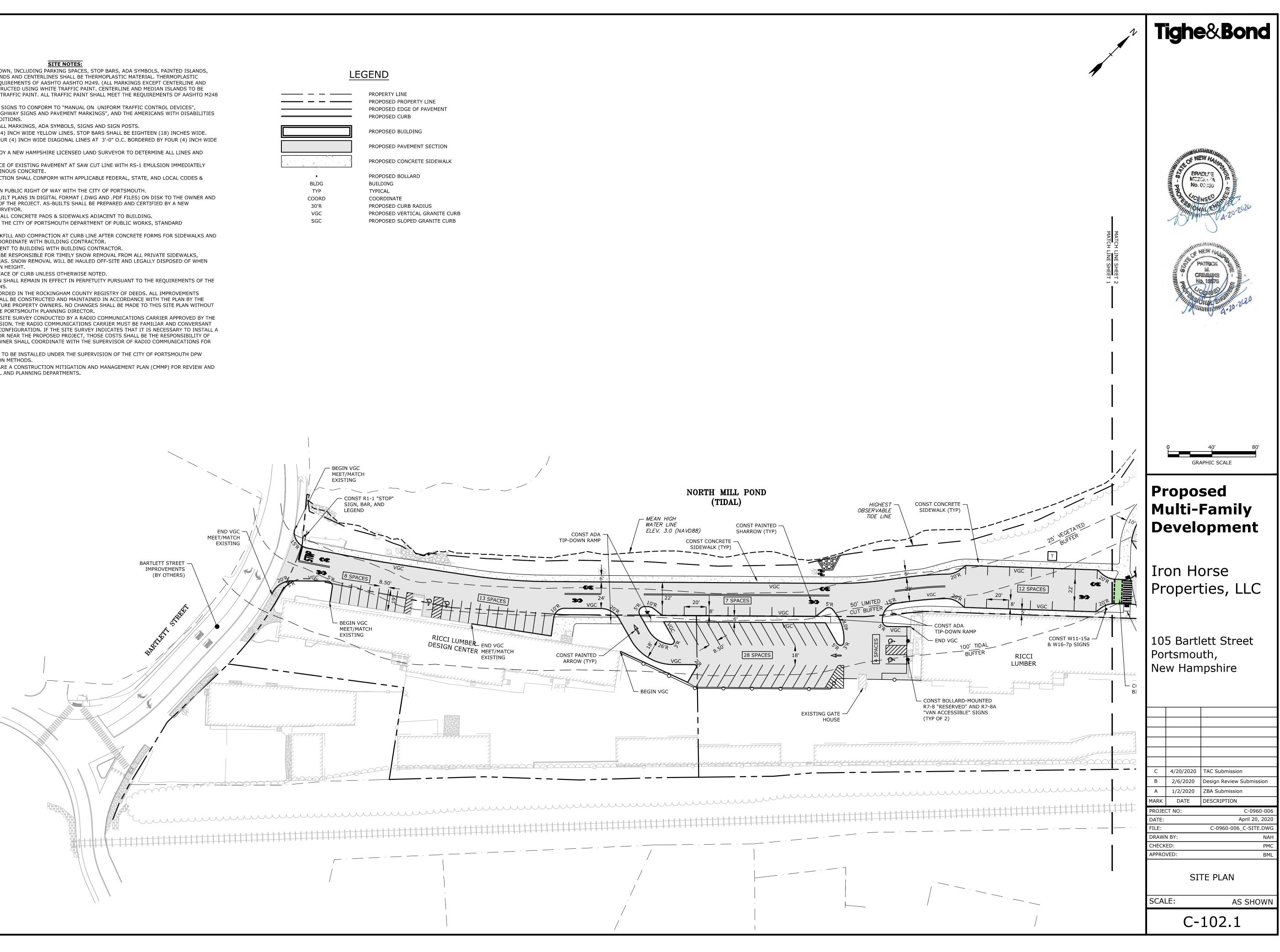
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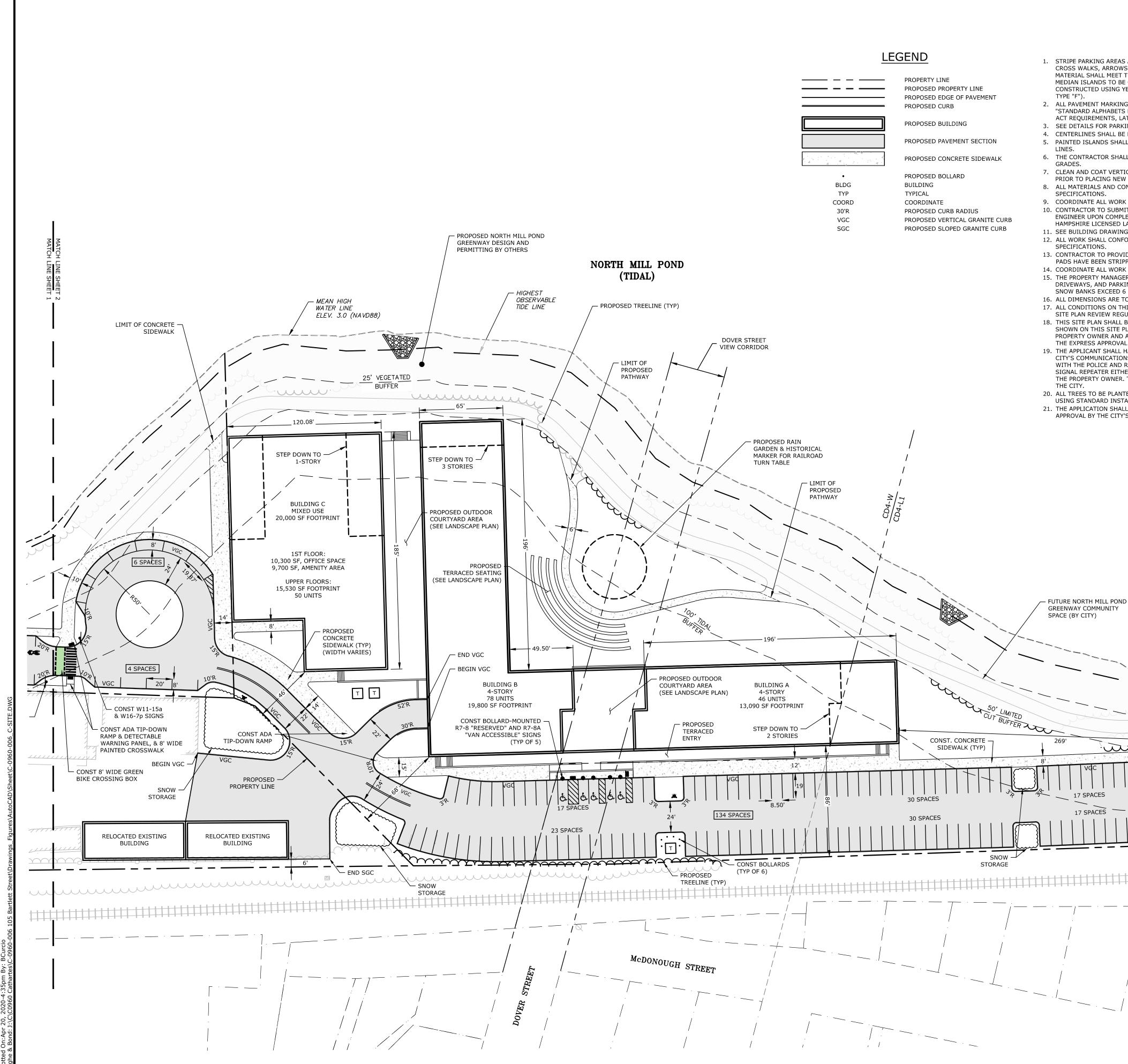
| | PROPI |
|-------|-------|
| | PROP |
| • | PROP |
| BLDG | BUILD |
| ТҮР | TYPIC |
| COORD | COOR |
| 30'R | PROP |
| VGC | PROP |
| SGC | PROP |
| | |

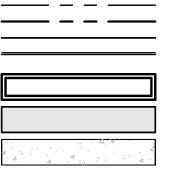
– HIGHEST OBSERVABLE DOVER STREET TIDE LINE VIEW CORRIDOR 25' VEGETATED BUFFER CORNWALL STREET - 65' -MAP 157 LOT 1 VIEW CORRIDOR CABOT STREET ∠120.08['] 5.07 ACRES VIEW CORRIDOR BLDG C ТТ BLDG A BLDG B 269'-·un McDONOUGH STREET Ō

| | Tighe&Bond |
|---|---|
| D PERTY LINE MODE DECORPTY LINE | HINNING OF NEEW HAMPING |
| | Development |
| | Iron Horse Properties, LLC 105 Bartlett Street Portsmouth, New Hampshire |
| | Image: Second system Image: Second system Image: Second |
| | SCALE: AS SHOWN |

| 1. | STRIPE PARKING AREAS AS SHOWN, INCLUDING PARKING SPACES, STOP BARS, ADA SYMBOLS, PAINTED ISLANDS, CROSS WALKS, ARROWS, LEGENDS AND CENTERLINES SHALL BE THERMOPLASTIC MATERIAL. THERMOPLASTIC MATERIAL SHALL MEET THE REQUIREMENTS OF AASHTO AASHTO M249. (ALL MARKINGS EXCEPT CENTERLINE AND MEDIAN ISLANDS TO BE CONSTRUCTED USING WHITE TRAFFIC PAINT. CENTERLINE AND MEDIAN ISLANDS TO BE CONSTRUCTED USING YELLOW TRAFFIC PAINT. ALL TRAFFIC PAINT SHALL MEET THE REQUIREMENTS OF AASHTO M248 |
|-----|--|
| 2. | TYPE "F"). ALL PAVEMENT MARKINGS AND SIGNS TO CONFORM TO "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES", "STANDARD ALPHABETS FOR HIGHWAY SIGNS AND PAVEMENT MARKINGS", AND THE AMERICANS WITH DISABILITIES ACT REQUIREMENTS, LATEST EDITIONS. |
| 3. | SEE DETAILS FOR PARKING STALL MARKINGS, ADA SYMBOLS, SIGNS AND SIGN POSTS. |
| | CENTERLINES SHALL BE FOUR (4) INCH WIDE YELLOW LINES. STOP BARS SHALL BE EIGHTEEN (18) INCHES WIDE. |
| 5. | PAINTED ISLANDS SHALL BE FOUR (4) INCH WIDE DIAGONAL LINES AT 3'-0" O.C. BORDERED BY FOUR (4) INCH WIDE LINES. |
| | THE CONTRACTOR SHALL EMPLOY A NEW HAMPSHIRE LICENSED LAND SURVEYOR TO DETERMINE ALL LINES AND GRADES. |
| | CLEAN AND COAT VERTICAL FACE OF EXISTING PAVEMENT AT SAW CUT LINE WITH RS-1 EMULSION IMMEDIATELY PRIOR TO PLACING NEW BITUMINOUS CONCRETE. ALL MATERIALS AND CONSTRUCTION SHALL CONFORM WITH APPLICABLE FEDERAL, STATE, AND LOCAL CODES & |
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| | SEE BUILDING DRAWINGS FOR ALL CONCRETE PADS & SIDEWALKS ADJACENT TO BUILDING. ALL WORK SHALL CONFORM TO THE CITY OF PORTSMOUTH DEPARTMENT OF PUBLIC WORKS, STANDARD SPECIFICATIONS. |
| 13. | CONTRACTOR TO PROVIDE BACKFILL AND COMPACTION AT CURB LINE AFTER CONCRETE FORMS FOR SIDEWALKS AND PADS HAVE BEEN STRIPPED. COORDINATE WITH BUILDING CONTRACTOR. |
| | COORDINATE ALL WORK ADJACENT TO BUILDING WITH BUILDING CONTRACTOR. THE PROPERTY MANAGER WILL BE RESPONSIBLE FOR TIMELY SNOW REMOVAL FROM ALL PRIVATE SIDEWALKS, |
| 15. | DRIVEWAYS, AND PARKING AREAS. SNOW REMOVAL WILL BE HAULED OFF-SITE AND LEGALLY DISPOSED OF WHEN SNOW BANKS EXCEED 6 FEET IN HEIGHT. |
| | ALL DIMENSIONS ARE TO THE FACE OF CURB UNLESS OTHERWISE NOTED. |
| | ALL CONDITIONS ON THIS PLAN SHALL REMAIN IN EFFECT IN PERPETUITY PURSUANT TO THE REQUIREMENTS OF THE SITE PLAN REVIEW REGULATIONS. |
| 18. | THIS SITE PLAN SHALL BE RECORDED IN THE ROCKINGHAM COUNTY REGISTRY OF DEEDS. ALL IMPROVEMENTS SHOWN ON THIS SITE PLAN SHALL BE CONSTRUCTED AND MAINTAINED IN ACCORDANCE WITH THE PLAN BY THE PROPERTY OWNER AND ALL FUTURE PROPERTY OWNERS. NO CHANGES SHALL BE MADE TO THIS SITE PLAN WITHOUT |
| 10 | THE EXPRESS APPROVAL OF THE PORTSMOUTH PLANNING DIRECTOR. THE APPLICANT SHALL HAVE A SITE SURVEY CONDUCTED BY A RADIO COMMUNICATIONS CARRIER APPROVED BY THE |
| 19. | CITY'S COMMUNICATIONS DIVISION. THE RADIO COMMUNICATIONS CARRIER MUST BE FAMILIAR AND CONVERSANT WITH THE POLICE AND RADIO CONFIGURATION. IF THE SITE SURVEY INDICATES THAT IT IS NECESSARY TO INSTALL A SIGNAL REPEATER EITHER ON OR NEAR THE PROPOSED PROJECT, THOSE COSTS SHALL BE THE RESPONSIBILITY OF THE PROPERTY OWNER. THE OWNER SHALL COORDINATE WITH THE SUPERVISOR OF RADIO COMMUNICATIONS FOR |
| 20. | THE CITY. ALL TREES TO BE PLANTED ARE TO BE INSTALLED UNDER THE SUPERVISION OF THE CITY OF PORTSMOUTH DPW |
| 21. | USING STANDARD INSTALLATION METHODS. THE APPLICATION SHALL PREPARE A CONSTRUCTION MITIGATION AND MANAGEMENT PLAN (CMMP) FOR REVIEW AND |
| | APPROVAL BY THE CITY'S LEGAL AND PLANNING DEPARTMENTS. |
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| | |
| | END VGC MEET/MATCH EXISTING |
| | |
| | BARTLETT STREET |
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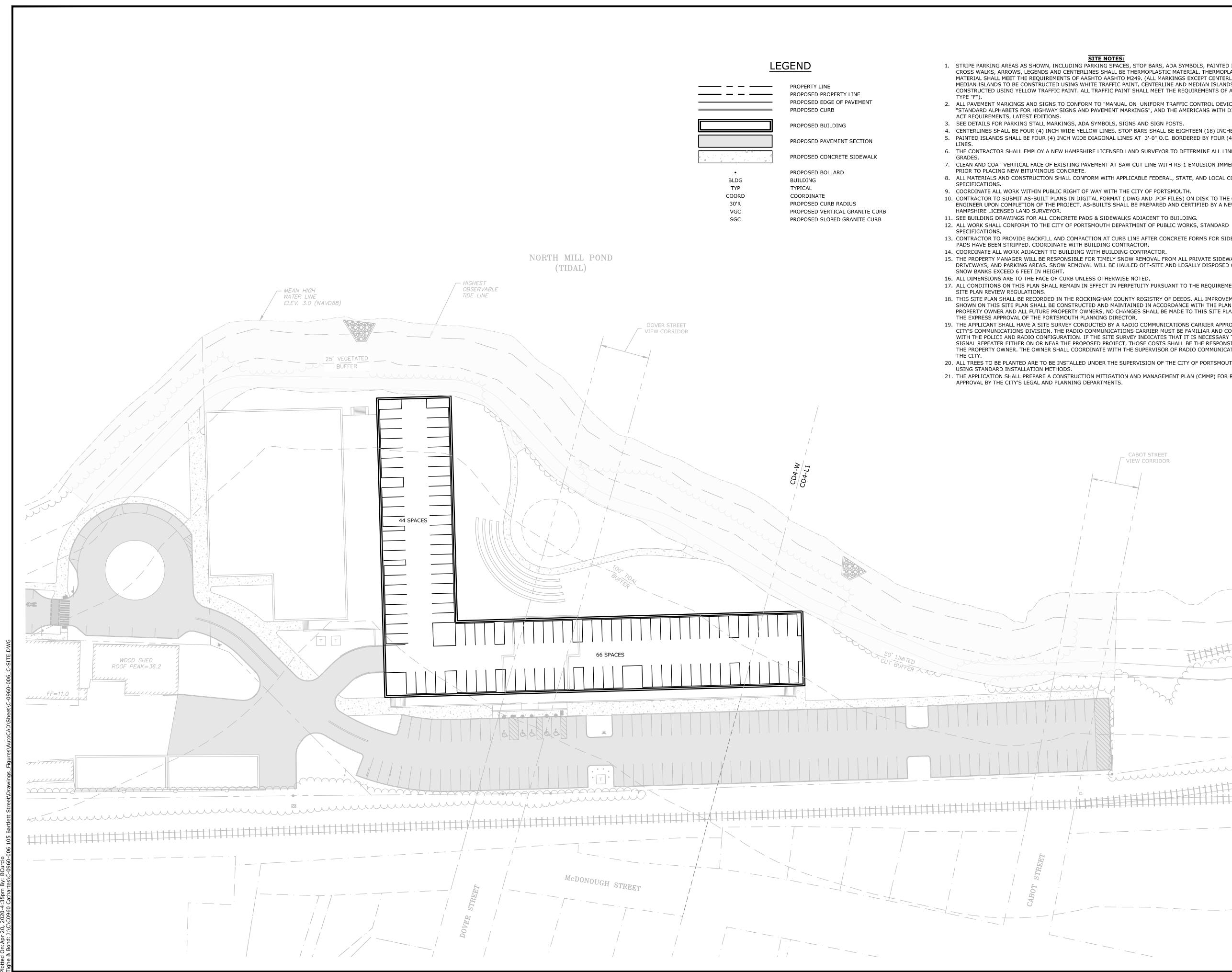


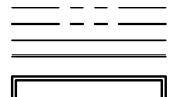
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Tighe&Bond 1. STRIPE PARKING AREAS AS SHOWN, INCLUDING PARKING SPACES, STOP BARS, ADA SYMBOLS, PAINTED ISLANDS, CONSTRUCTED USING YELLOW TRAFFIC PAINT. ALL TRAFFIC PAINT SHALL MEET THE REQUIREMENTS OF AASHTO M248 "STANDARD ALPHABETS FOR HIGHWAY SIGNS AND PAVEMENT MARKINGS", AND THE AMERICANS WITH DISABILITIES 4. CENTERLINES SHALL BE FOUR (4) INCH WIDE YELLOW LINES. STOP BARS SHALL BE EIGHTEEN (18) INCHES WIDE. PROPERTY OWNER AND ALL FUTURE PROPERTY OWNERS. NO CHANGES SHALL BE MADE TO THIS SITE PLAN WITHOUT WITH THE POLICE AND RADIO CONFIGURATION. IF THE SITE SURVEY INDICATES THAT IT IS NECESSARY TO INSTALL A CABOT STREET VIEW CORRIDOR GRAPHIC SCALE Proposed Multi-Family Development Iron Horse Properties, LLC 105 Bartlett Street Portsmouth, manufit · END VGC New Hampshire BEGIN SGC PROPOSED TREELINE (TYP) - SNOW STORAGE - PROPOSED PROPERTY LINE _ _ C 4/20/2020 TAC Submission В 2/6/2020 Design Review Submission А 1/2/2020 ZBA Submission MARK DATE DESCRIPTION PROJECT NO: C-0960-00 April 20, 202 DATE: FILE: C-0960-006_C-SITE.DW0 DRAWN BY: NAI CHECKED: PM APPROVED: SITE PLAN SCALE: AS SHOWN

C-102.2

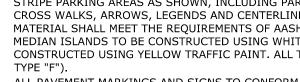












Tighe&Bond SITE NOTES: 1. STRIPE PARKING AREAS AS SHOWN, INCLUDING PARKING SPACES, STOP BARS, ADA SYMBOLS, PAINTED ISLANDS, CROSS WALKS, ARROWS, LEGENDS AND CENTERLINES SHALL BE THERMOPLASTIC MATERIAL. THERMOPLASTIC MATERIAL SHALL MEET THE REQUIREMENTS OF AASHTO AASHTO M249. (ALL MARKINGS EXCEPT CENTERLINE AND MEDIAN ISLANDS TO BE CONSTRUCTED USING WHITE TRAFFIC PAINT. CENTERLINE AND MEDIAN ISLANDS TO BE CONSTRUCTED USING YELLOW TRAFFIC PAINT. ALL TRAFFIC PAINT SHALL MEET THE REQUIREMENTS OF AASHTO M248 2. ALL PAVEMENT MARKINGS AND SIGNS TO CONFORM TO "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES", "STANDARD ALPHABETS FOR HIGHWAY SIGNS AND PAVEMENT MARKINGS", AND THE AMERICANS WITH DISABILITIES 4. CENTERLINES SHALL BE FOUR (4) INCH WIDE YELLOW LINES. STOP BARS SHALL BE EIGHTEEN (18) INCHES WIDE. 5. PAINTED ISLANDS SHALL BE FOUR (4) INCH WIDE DIAGONAL LINES AT 3'-0" O.C. BORDERED BY FOUR (4) INCH WIDE 6. THE CONTRACTOR SHALL EMPLOY A NEW HAMPSHIRE LICENSED LAND SURVEYOR TO DETERMINE ALL LINES AND 7. CLEAN AND COAT VERTICAL FACE OF EXISTING PAVEMENT AT SAW CUT LINE WITH RS-1 EMULSION IMMEDIATELY 8. ALL MATERIALS AND CONSTRUCTION SHALL CONFORM WITH APPLICABLE FEDERAL, STATE, AND LOCAL CODES & 10. CONTRACTOR TO SUBMIT AS-BUILT PLANS IN DIGITAL FORMAT (.DWG AND .PDF FILES) ON DISK TO THE OWNER AND ENGINEER UPON COMPLETION OF THE PROJECT. AS-BUILTS SHALL BE PREPARED AND CERTIFIED BY A NEW 13. CONTRACTOR TO PROVIDE BACKFILL AND COMPACTION AT CURB LINE AFTER CONCRETE FORMS FOR SIDEWALKS AND 15. THE PROPERTY MANAGER WILL BE RESPONSIBLE FOR TIMELY SNOW REMOVAL FROM ALL PRIVATE SIDEWALKS, DRIVEWAYS, AND PARKING AREAS. SNOW REMOVAL WILL BE HAULED OFF-SITE AND LEGALLY DISPOSED OF WHEN 17. ALL CONDITIONS ON THIS PLAN SHALL REMAIN IN EFFECT IN PERPETUITY PURSUANT TO THE REQUIREMENTS OF THE 18. THIS SITE PLAN SHALL BE RECORDED IN THE ROCKINGHAM COUNTY REGISTRY OF DEEDS. ALL IMPROVEMENTS SHOWN ON THIS SITE PLAN SHALL BE CONSTRUCTED AND MAINTAINED IN ACCORDANCE WITH THE PLAN BY THE PROPERTY OWNER AND ALL FUTURE PROPERTY OWNERS. NO CHANGES SHALL BE MADE TO THIS SITE PLAN WITHOUT 19. THE APPLICANT SHALL HAVE A SITE SURVEY CONDUCTED BY A RADIO COMMUNICATIONS CARRIER APPROVED BY THE CITY'S COMMUNICATIONS DIVISION. THE RADIO COMMUNICATIONS CARRIER MUST BE FAMILIAR AND CONVERSANT WITH THE POLICE AND RADIO CONFIGURATION. IF THE SITE SURVEY INDICATES THAT IT IS NECESSARY TO INSTALL A SIGNAL REPEATER EITHER ON OR NEAR THE PROPOSED PROJECT, THOSE COSTS SHALL BE THE RESPONSIBILITY OF THE PROPERTY OWNER. THE OWNER SHALL COORDINATE WITH THE SUPERVISOR OF RADIO COMMUNICATIONS FOR 20. ALL TREES TO BE PLANTED ARE TO BE INSTALLED UNDER THE SUPERVISION OF THE CITY OF PORTSMOUTH DPW 21. THE APPLICATION SHALL PREPARE A CONSTRUCTION MITIGATION AND MANAGEMENT PLAN (CMMP) FOR REVIEW AND CABOT STREET VIEW CORRIDOR GRAPHIC SCALE Proposed Multi-Family Development Iron Horse Properties, LLC 105 Bartlett Street Portsmouth, New Hampshire Jun C 4/20/2020 TAC Submission В 2/6/2020 Design Review Submissio А 1/2/2020 ZBA Submissior MARK DATE DESCRIPTION PROJECT NO: C-0960-00 April 20, 202 DATE: C-0960-006_C-SITE.DWG FILE: DRAWN BY: NAF CHECKED: PM APPROVED: BASEMENT LEVEL SITE PLAN SCALE: AS SHOWN C-102.3

GRADING AND DRAINAGE NOTES:

| 1. | COMPACTION REQUIREMENTS: | | |
|----|-------------------------------|--|--|
| | BELOW PAVED OR CONCRETE AREAS | | |

- TRENCH BEDDING MATERIAL AND SAND BLANKET BACKFILL
- BELOW LOAM AND SEED AREAS
- * ALL PERCENTAGES OF COMPACTION SHALL BE OF THE MAXIMUM DRY DENSITY AT THE OPTIMUM MOISTURE CONTENT AS DETERMINED AND CONTROLLED IN ACCORDANCE WITH ASTM D-1557, METHOD C FIELD DENSITY TESTS SHALL BE MADE IN ACCORDANCE WITH ASTM D-1556 OR ASTM-2922.
- 2. ALL STORM DRAINAGE PIPES SHALL BE HIGH DENSITY POLYETHYLENE (HANCOR HI-Q, ADS N-12 OR EQUAL), UNLESS OTHERWISE SPECIFIED.
- 3. SEE UTILITY PLAN FOR ALL SITE UTILITY INFORMATION. 4. ADJUST ALL MANHOLES, CATCH BASINS, CURB BOXES, ETC. WITHIN LIMITS OF WORK TO FINISH GRADE. 5. CONTRACTOR SHALL PROVIDE A FINISH PAVEMENT SURFACE AND LAWN AREAS FREE OF LOW SPOTS AND PONDING

95%

95%

90%

- AREAS. CRITICAL AREAS INCLUDE BUILDING ENTRANCES, EXITS, RAMPS AND LOADING DOCK AREAS ADJACENT TO THE BUILDING. 6. CONTRACTOR SHALL THOROUGHLY CLEAN ALL CATCH BASINS AND DRAIN LINES, WITHIN THE LIMIT OF WORK, OF
- SEDIMENT IMMEDIATELY UPON COMPLETION OF CONSTRUCTION. 7. ALL MATERIALS AND CONSTRUCTION SHALL CONFORM WITH APPLICABLE FEDERAL, STATE AND LOCAL CODES.
- 8. ALL DISTURBED AREAS NOT TO BE PAVED OR OTHERWISE TREATED SHALL RECEIVE 6" LOAM, SEED FERTILIZER AND MULCH.
- 9. ALL STORM DRAIN CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE NHOOT STANDARD SPECIFICATIONS FOR HIGHWAYS AND BRIDGES, LATEST EDITION.
- 10. ALL PROPOSED CATCH BASINS SHALL BE EQUIPPED WITH OIL/GAS SEPARATOR HOODS AND 4' SUMPS.
- 11. ALL WORK SHALL CONFORM TO THE CITY OF PORTSMOUTH DEPARTMENT OF PUBLIC WORKS, STANDARD SPECIFICATIONS AND WITH THE STATE OF NEW HAMPSHIRE DEPARTMENT OF TRANSPORTATION, "STANDARD SPECIFICATIONS OF ROAD AND BRIDGE CONSTRUCTION", CURRENT EDITION.
- 12. CONTRACTOR TO SUBMIT AS-BUILT PLANS IN DIGITAL FORMAT (.DWG AND .PDF FILES) ON DISK TO THE OWNER AND ENGINEER UPON COMPLETION OF THE PROJECT. AS-BUILTS SHALL BE PREPARED AND CERTIFIED BY A NEW HAMPSHIRE LICENSED LAND SURVEYOR.
- 13. SEE EXISTING CONDITIONS PLAN FOR BENCH MARK INFORMATION.

EROSION CONTROL NOTES

- 1. INSTALL EROSION CONTROL BARRIERS AS SHOWN AS FIRST ORDER OF WORK. 2. SEE GENERAL EROSION CONTROL NOTES ON "EROSION CONTROL NOTES & DETAILS SHEET"
- 3. PROVIDE INLET PROTECTION AROUND ALL EXISTING AND PROPOSED CATCH BASIN INLETS WITHIN THE WORK LIMITS AS WELL AS CATCH BASINS/CURB INLETS THAT RECEIVE RUNOFF FROM CONSTRUCTION ACTIVITIES. MAINTAIN FOR THE DURATION OF THE PROJECT.
- 4. INSTALL STABILIZED CONSTRUCTION EXIT(S).
- 5. INSPECT INLET PROTECTION AND PERIMETER EROSION CONTROL MEASURES DAILY AND AFTER EACH RAIN STORM OF 0.25 INCH OR GREATER. REPAIR/MODIFY PROTECTION AS NECESSARY TO MAXIMIZE EFFICIENCY OF FILTER. REPLACE ALL FILTERS WHEN SEDIMENT IS 1/3 THE FILTER HEIGHT. 6. ALL DISTURBED AREAS NOT TO BE PAVED OR OTHERWISE TREATED SHALL RECEIVE 6" LOAM, SEED, FERTILIZER AND
- MULCH.
- 7. CONSTRUCT EROSION CONTROL BLANKET ON ALL SLOPES STEEPER THAN 3:1.
- 8. PRIOR TO ANY WORK OR SOIL DISTURBANCE COMMENCING ON THE SUBJECT PROPERTY, INCLUDING MOVING OF EARTH, THE APPLICANT SHALL INSTALL ALL EROSION AND SILTATION MITIGATION AND CONTROL MEASURES AS REQUIRED BY STATE AND LOCAL PERMITS AND APPROVALS.
- 9. CONTRACTOR SHALL BE RESPONSIBLE TO CONTROL DUST AND WIND EROSION THROUGHOUT THE CONSTRUCTION PERIOD. DUST CONTROL MEASURES SHALL INCLUDE, BUT ARE NOT LIMITED TO, SPRINKLING WATER ON UNSTABLE SOILS SUBJECT TO ARID CONDITIONS.
- 10. THE CONTRACTOR SHALL REMOVE AND PROPERLY DISPOSE OF ALL TEMPORARY EROSION CONTROL DEVICES UPON COMPLETION OF CONSTRUCTION.
- 11. ALL CATCH BASIN SUMPS AND PIPING SHALL BE THOROUGHLY CLEANED TO REMOVE ALL SEDIMENT AND DEBRIS AFTER THE PROJECT HAS BEEN FULLY PAVED.
- 12. TEMPORARY SOIL STOCKPILE SHALL BE SURROUNDED WITH PERIMETER CONTROLS AND SHALL BE STABILIZED BY TEMPORARY EROSION CONTROL SEEDING. STOCKPILE AREAS TO BE LOCATED AS FAR AS POSSIBLE FROM THE DELINEATED EDGE OF WETLANDS.
- 13. SAFETY FENCING SHALL BE PROVIDED AROUND STOCKPILES OVER 10 FT.
- 14. CONCRETE TRUCKS WILL BE REQUIRED TO WASH OUT (IF NECESSARY) SHOOTS ONLY WITHIN AREAS WHERE
- CONCRETE HAS BEEN PLACED. NO OTHER WASH OUT WILL BE ALLOWED. 15. ALL CONDITIONS ON THIS PLAN SHALL REMAIN IN EFFECT IN PERPETUITY PURSUANT TO THE REQUIREMENTS OF THE SITE PLAN REVIEW REGULATIONS.

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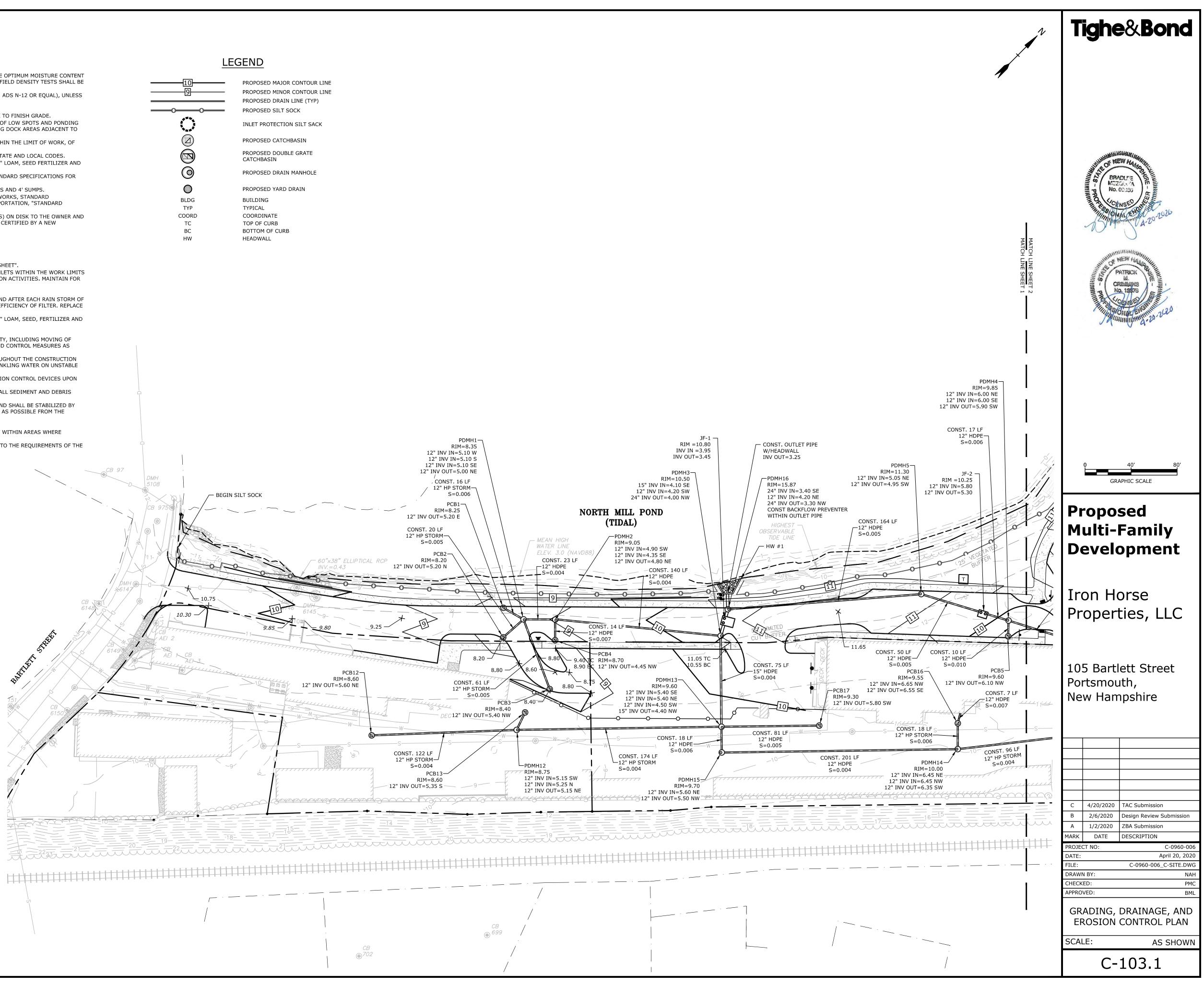
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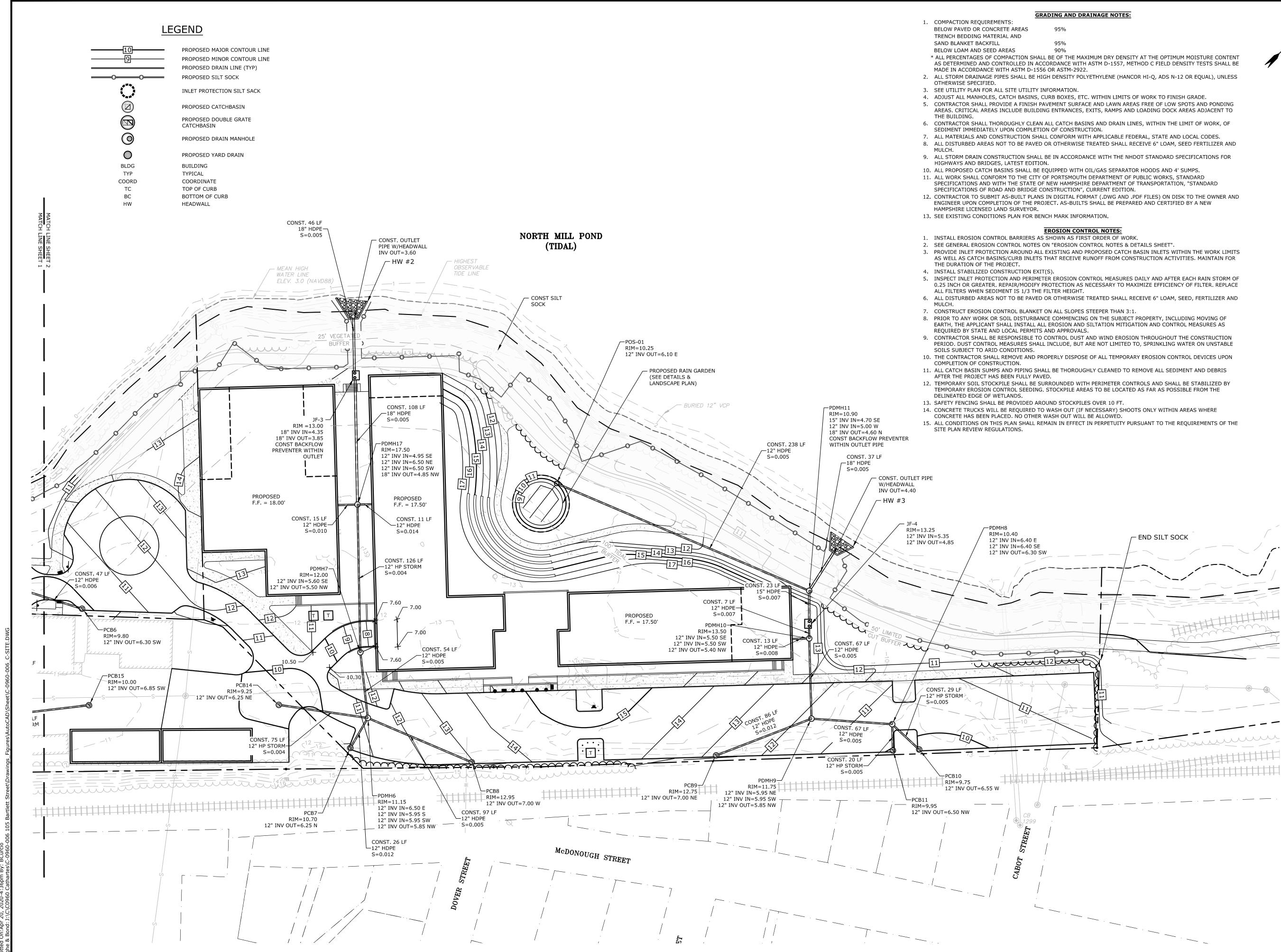
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F. TTYSTER

- PROPOSED MAJOR CONTOUR LINE PROPOSED MINOR CONTOUR LINE PROPOSED DRAIN LINE (TYP) PROPOSED SILT SOCK





Tighe&Bond >/ATTRE GRAPHIC SCALE Proposed Multi-Family Development Iron Horse Properties, LLC 105 Bartlett Street Portsmouth, New Hampshire C 4/20/2020 TAC Submission В 2/6/2020 Design Review Submissio Α 1/2/2020 ZBA Submission MARK DATE DESCRIPTION PROJECT NO: C-0960-00 April 20, 202 DATE: FILE: C-0960-006_C-SITE.DW0 DRAWN BY: NAI CHECKED: APPROVED:

GRADING, DRAINAGE, AND EROSION CONTROL PLAN

C-103.2

SCALE: AS SHOWN

UTILITY NOTES:

- 1. THE LOCATIONS OF EXISTING UNDERGROUND UTILITIES ARE APPROXIMATE AND THE LOCATIONS ARE NOT GUARANTEED BY THE OWNER OR ENGINEER. IT IS THE CONTRACTOR'S RESPONSIBILITY TO LOCATE ALL UTILITIES, ANTICIPATE CONFLICTS, REPAIR EXISTING UTILITIES, AND RELOCATE EXISTING UTILITIES REQUIRED TO COMPLETE THE WORK AT NO ADDITIONAL COST TO THE OWNER.
- 2. COORDINATE ALL UTILITY WORK WITH APPROPRIATE UTILITY COMPANY. • NATURAL GAS - UNITIL
- WATER/SEWER CITY OF PORTSMOUTH
- ELECTRIC EVERSOURCE • COMMUNICATIONS - CONSOLIDATED COMMUNICATIONS & COMCAST
- 3. SEE EXISTING CONDITIONS PLAN FOR BENCHMARK INFORMATION.
- 4. SEE GRADING, DRAINAGE & EROSION CONTROL PLAN FOR PROPOSED GRADING AND EROSION CONTROL MEASURES. 5. THE APPLICANT SHALL COORDINATE WITH THE CITY'S CONSULTANT TO COMPLETE A WATER CAPACITY ANALYSIS USING THE CITY'S CAPACITY MODELING AND SHALL MODIFY THE WATER SERVICE DESIGN AS REQUIRED. THE PRIVATE WATER LINE THAT CURRENTLY FEEDS THE DEVELOPMENT LOT SHALL BE EITHER REPLACED OR ABANDONED DEPENDING ON THE OUTCOME OF THE STUDY. ALL MODIFICATIONS SHALL BE REVIEWED AND APPROVED BY THE DPW AND THE FIRE DEPARTMENT.
- 6. ALL WATER MAIN INSTALLATIONS SHALL BE CLASS 52, CEMENT LINED DUCTILE IRON PIPE.
- 7. ALL WATER MAIN INSTALLATIONS SHALL BE PRESSURE TESTED AND CHLORINATED AFTER CONSTRUCTION PRIOR TO ACTIVATING THE SYSTEM. CONTRACTOR SHALL COORDINATE CHLORINATION AND TESTING WITH THE CITY OF PORTSMOUTH WATER DEPARTMENT.
- 8. ALL SEWER PIPE SHALL BE PVC SDR 35 UNLESS OTHERWISE STATED.
- 9. COORDINATE ALL WORK WITHIN PUBLIC RIGHT OF WAYS WITH THE CITY OF PORTSMOUTH.
- 10. CONTRACTOR SHALL MAINTAIN UTILITY SERVICES TO ABUTTING PROPERTIES THROUGHOUT CONSTRUCTION. 11. CONNECTION TO EXISTING WATER MAIN SHALL BE CONSTRUCTED TO CITY OF PORTSMOUTH STANDARDS.
- 12. EXISTING UTILITIES TO BE REMOVED SHALL BE CAPPED AT THE MAIN AND MEET THE DEPARTMENT OF PUBLIC WORKS
- STANDARDS FOR CAPPING OF WATER AND SEWER SERVICES. 13. ALL ELECTRICAL MATERIAL WORKMANSHIP SHALL CONFORM TO THE NATIONAL ELECTRIC CODE, LATEST EDITION, AND ALL APPLICABLE STATE AND LOCAL CODES.
- 14. THE EXACT LOCATION OF NEW UTILITY SERVICES AND CONNECTIONS SHALL BE COORDINATED WITH THE BUILDING
- DRAWINGS AND THE APPLICABLE UTILITY COMPANIES. 15. ADJUST ALL MANHOLES, CATCH BASINS, CURB BOXES, ETC. WITHIN LIMITS OF WORK TO FINISH GRADE.
- 16. ALL UNDERGROUND CONDUITS SHALL HAVE NYLON PULL ROPES TO FACILITATE PULLING CABLES.
- 17. THE CONTRACTOR SHALL OBTAIN, PAY FOR, AND COMPLY WITH ALL REOUIRED PERMITS, ARRANGE FOR ALL INSPECTIONS, AND SUBMIT COPIES OF ACCEPTANCE CERTIFICATES TO THE OWNER PRIOR TO THE COMPLETION OF THIS PROJECT.
- 18. THE CONTRACTOR SHALL PROVIDE AND INSTALL ALL MANHOLES, BOXES, FITTINGS, CONNECTORS, COVER PLATES, AND OTHER MISCELLANEOUS ITEMS NOT NECESSARILY DETAILED ON THESE DRAWINGS TO RENDER INSTALLATION OF UTILITIES COMPLETE AND OPERATIONAL.
- 19. CONTRACTOR SHALL PROVIDE EXCAVATION, BEDDING, BACKFILL AND COMPACTION FOR NATURAL GAS SERVICES. 20. A 10-FOOT MINIMUM EDGE TO EDGE HORIZONTAL SEPARATION SHALL BE PROVIDED BETWEEN ALL WATER AND SANITARY SEWER LINES. AN 18-INCH MINIMUM OUTSIDE TO OUTSIDE VERTICAL SEPARATION SHALL BE PROVIDED AT ALL WATER/SANITARY SEWER CROSSINGS.
- 21. THE CONTRACTOR SHALL CONTACT "DIG-SAFE" 72 HOURS PRIOR TO COMMENCING CONSTRUCTION. THE CONTRACTOR SHALL HAVE THE "DIG-SAFE" NUMBER ON SITE AT ALL TIMES.
- 22. CONTRACTOR TO SUBMIT AS-BUILT PLANS ON REPRODUCIBLE MYLARS AND IN DIGITAL FORMAT (.DWG FILES) TO THE OWNER AND ENGINEER UPON COMPLETION OF THE PROJECT. AS-BUILTS SHALL BE PREPARED AND CERTIFIED BY A NEW HAMPSHIRE LICENSED LAND SURVEYOR OR PROFESSIONAL ENGINEER. 23. SAW CUT AND REMOVE PAVEMENT AND CONSTRUCT PAVEMENT TRENCH PATCH FOR ALL PROPOSED UTILITIES
- LOCATED IN EXISTING PAVEMENT AREAS TO REMAIN 24. HYDRANTS, GATE VALVES, FITTINGS, ETC. SHALL MEET THE REQUIREMENTS OF THE CITY OF PORTSMOUTH.
- 25. COORDINATE TESTING OF SEWER CONSTRUCTION WITH THE CITY OF PORTSMOUTH. 26. ALL SEWER PIPE WITH LESS THAN 6' OF COVER IN PAVED AREAS OR LESS THAT 4' OF COVER IN UNPAVED AREAS
- SHALL BE INSULATED.
- 27. CONTRACTOR SHALL COORDINATE ALL ELECTRIC WORK INCLUDING BUT NOT LIMITED TO: CONDUIT CONSTRUCTION, MANHOLE CONSTRUCTION, UTILITY POLE CONSTRUCTION, OVERHEAD WIRE RELOCATION, AND TRANSFORMER CONSTRUCTION WITH POWER COMPANY.
- 28. CONTRACTOR SHALL PHASE UTILITY CONSTRUCTION, PARTICULARLY WATER MAIN AND GAS MAIN CONSTRUCTION AS TO MAINTAIN CONTINUOUS SERVICE TO ABUTTING PROPERTIES. CONTRACTOR SHALL COORDINATE TEMPORARY SERVICES TO ABUTTERS WITH THE UTILITY COMPANY AND AFFECTED ABUTTER.
- 29. SITE LIGHTING SPECIFICATIONS, CONDUIT LAYOUT AND CIRCUITRY FOR PROPOSED SITE LIGHTING AND SIGN ILLUMINATION SHALL BE PROVIDED BY THE PROJECT ELECTRICAL ENGINEER.
- 30. CONTRACTOR SHALL CONSTRUCT ALL UTILITIES AND DRAINS TO WITHIN 10' OF THE FOUNDATION WALLS AND CONNECT THESE TO SERVICE STUBS FROM THE BUILDING.
- 31. ALL CONDITIONS ON THIS PLAN SHALL REMAIN IN EFFECT IN PERPETUITY PURSUANT TO THE REQUIREMENTS OF THE SITE PLAN REVIEW REGULATIONS.

CONNECT TO GAS MAIN -

318/858/3

ETT 85/2

CONNECT TO WATER MAIN WITH -

TAPPING SLEEVE AND VALVE

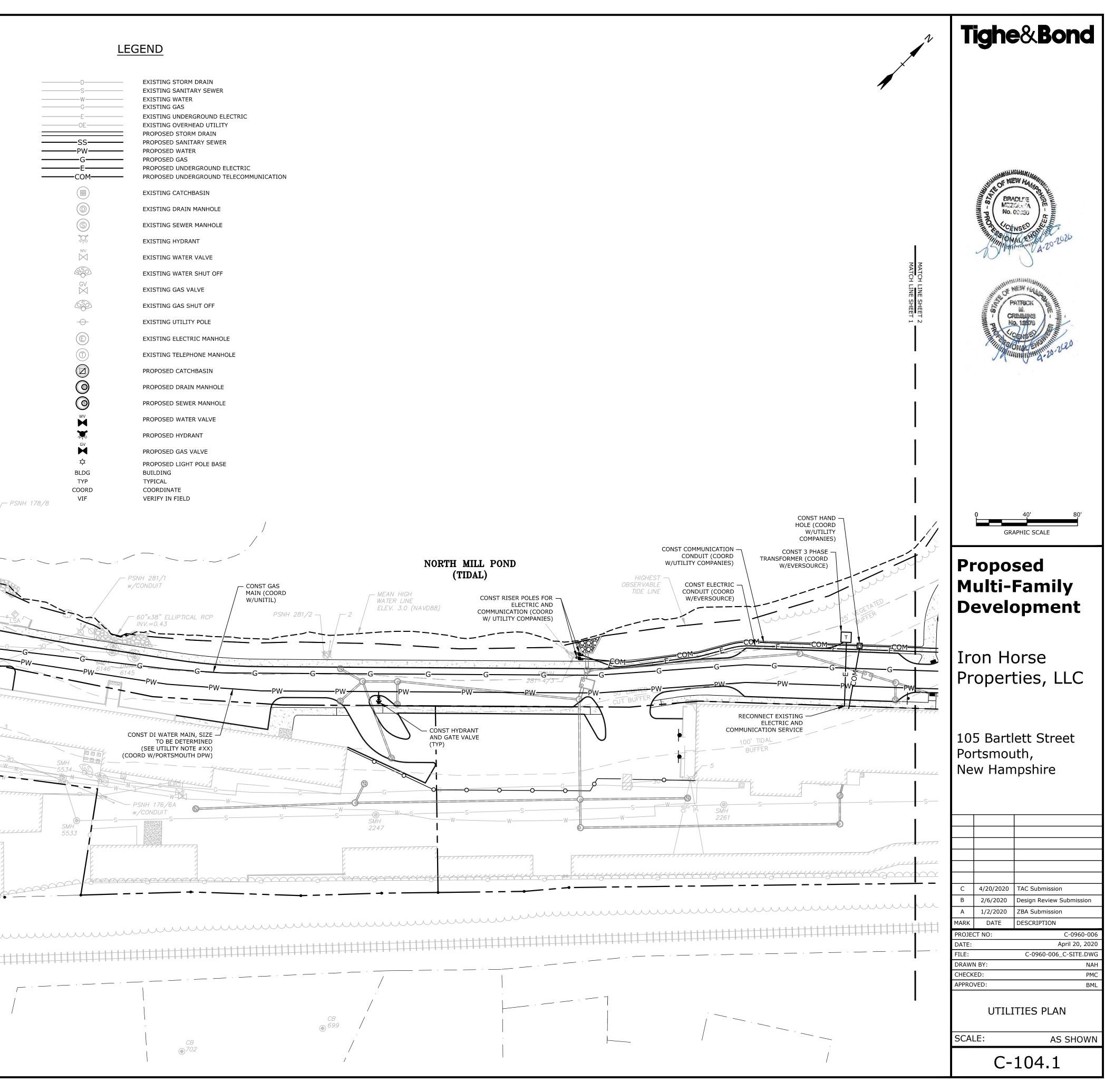
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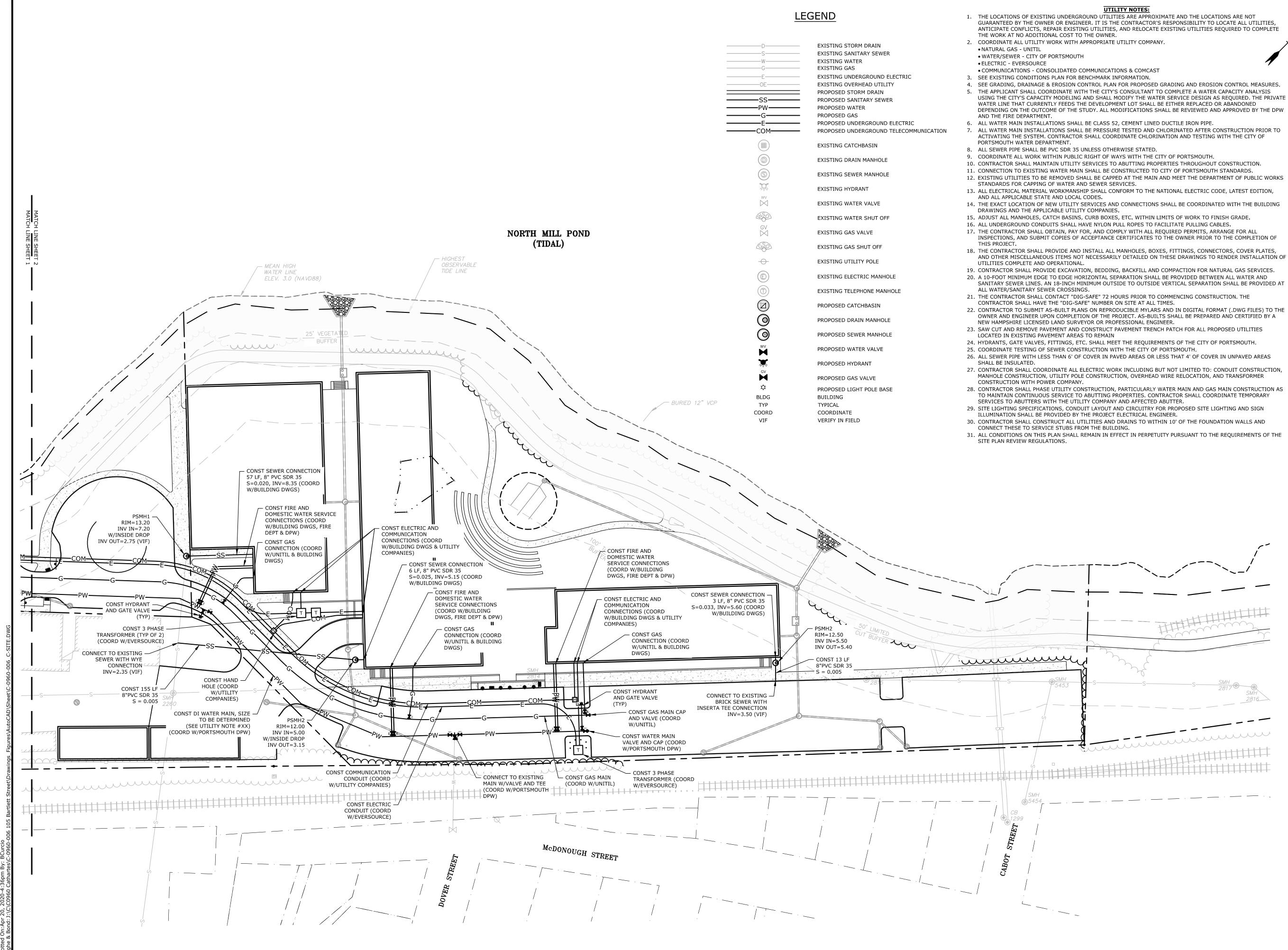
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PSNH 176/6



EXISTING STORM DRAIN EXISTING SANITARY SEWER EXISTING WATER EXISTING GAS EXISTING UNDERGROUND ELECTRIC EXISTING OVERHEAD UTILITY PROPOSED STORM DRAIN PROPOSED SANITARY SEWER PROPOSED WATER PROPOSED GAS PROPOSED UNDERGROUND ELECTRIC PROPOSED UNDERGROUND TELECOMMUNICATION EXISTING CATCHBASIN \bigcirc EXISTING DRAIN MANHOLE S EXISTING SEWER MANHOLE EXISTING HYDRANT \bowtie EXISTING WATER VALVE (NSO) EXISTING WATER SHUT OFF \bowtie EXISTING GAS VALVE (SO) EXISTING GAS SHUT OFF EXISTING UTILITY POLE \rightarrow Ð EXISTING ELECTRIC MANHOLE EXISTING TELEPHONE MANHOLE PROPOSED CATCHBASIN 0 PROPOSED DRAIN MANHOLE 0 PROPOSED SEWER MANHOLE PROPOSED WATER VALVE PROPOSED HYDRANT PROPOSED GAS VALVE ά PROPOSED LIGHT POLE BASE BUILDING BLDG TYP TYPICAL COORD COORDINATE VERIFY IN FIELD VIF



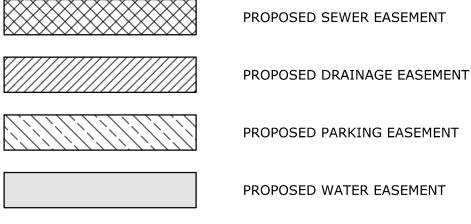


Tighe&Bond ATRIC GRAPHIC SCALE Proposed Multi-Family Development Iron Horse Properties, LLC 105 Bartlett Street Portsmouth, New Hampshire C 4/20/2020 TAC Submission 2/6/2020 Design Review Submission В Α 1/2/2020 ZBA Submission MARK DATE DESCRIPTION PROJECT NO: C-0960-00 April 20, 202 DATE: C-0960-006_C-SITE.DW0 FILE: DRAWN BY: NAI CHECKED: APPROVED: UTILITIES PLAN

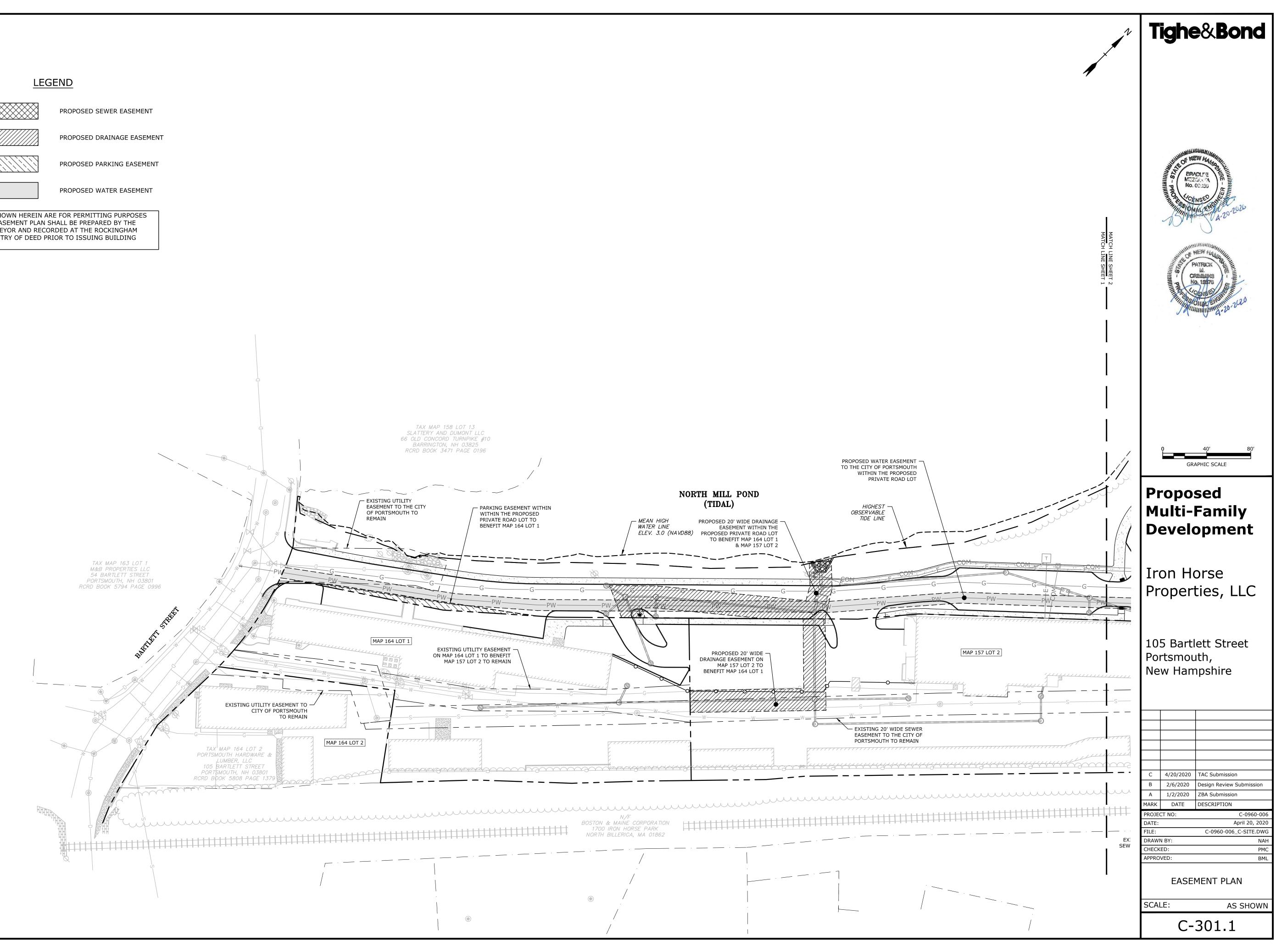
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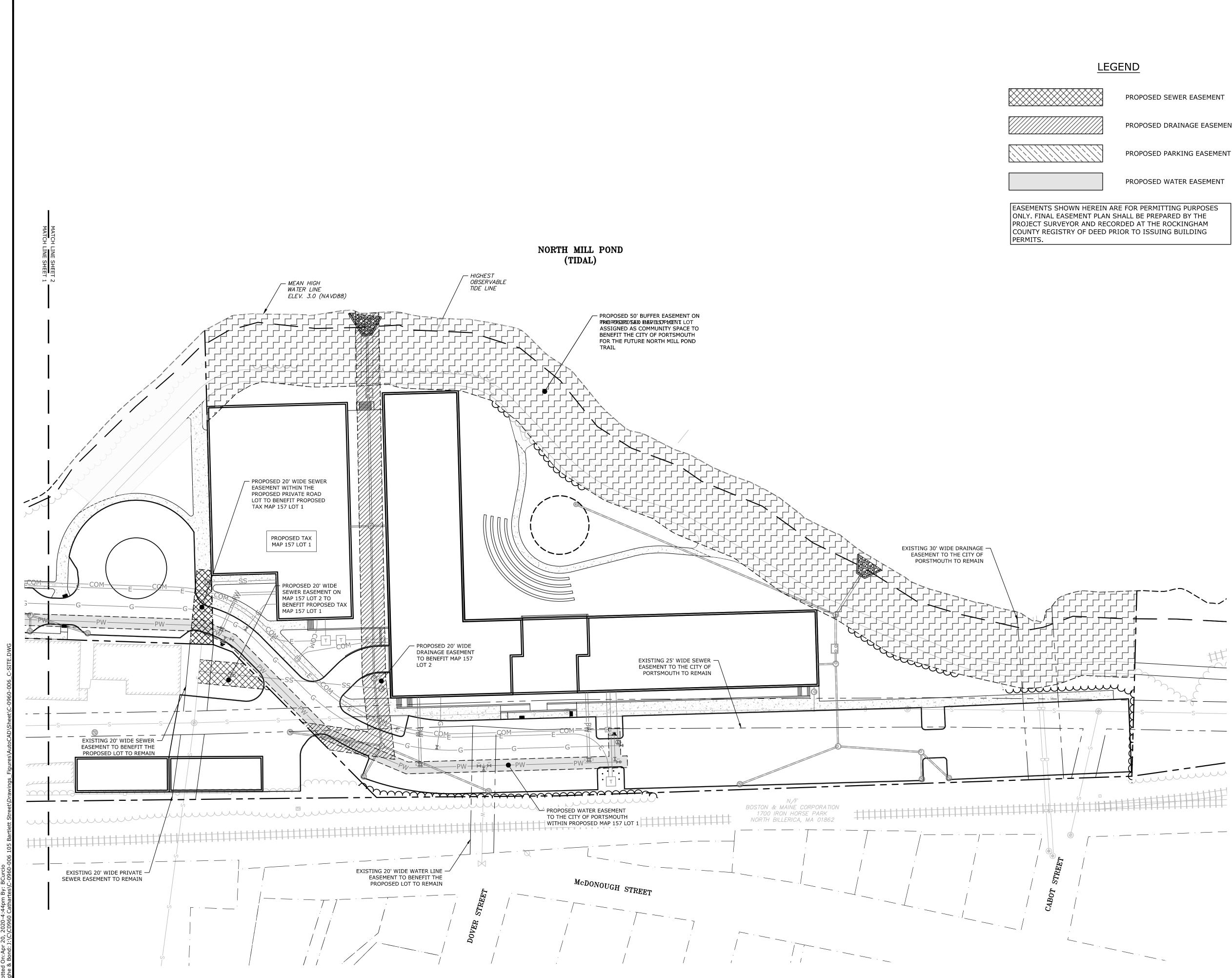
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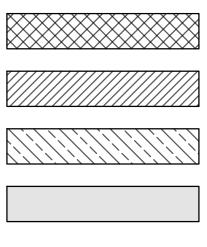


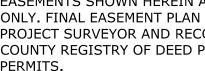
EASEMENTS SHOWN HEREIN ARE FOR PERMITTING PURPOSES ONLY. FINAL EASEMENT PLAN SHALL BE PREPARED BY THE PROJECT SURVEYOR AND RECORDED AT THE ROCKINGHAM COUNTY REGISTRY OF DEED PRIOR TO ISSUING BUILDING PERMITS.

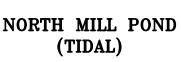




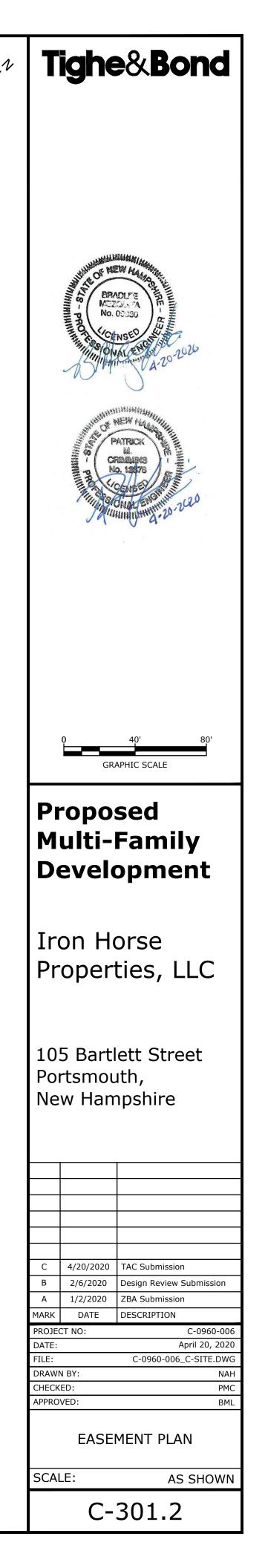








- PROPOSED DRAINAGE EASEMENT



| GENERAL PROJECT INFORMATION | RACKS. THE SITE SHALL BE STABILIZED FOR THE WINTER BY OCTOBER 15. |
|--|---|
| PROJECT APPLICANT:IRON HORSE PROPERTIES, LLCPROJECT MAP/LOT:105 BARTLETT STREETMAP 157 / LOT 1PORTSMOUTH, NH 03801MAP 157 / LOT 2 | DUST CONTROL: 1. THE CONTRACTOR SHALL BE RESPONSIBLE TO CONTROL DUST THROUGHOUT THE CONSTRU |
| PROJECT NAME:PROPOSED MULTI-FAMILY DEVELOPMENTMAP 164 / LOT 1PROJECT ADDRESS:105 BARTLETT STREETMAP 164 / LOT 4-2 | PERIOD. 2. DUST CONTROL METHODS SHALL INCLUDE, BUT BE NOT LIMITED TO SPRINKLING WATER ON |
| PORTSMOUTH, NH 03801 PROJECT LATITUDE/LONGITUDE: 43°-04'-20" N / 70°-46'-15" W | EXPOSED AREAS, COVERING LOADED DUMP TRUCKS LEAVING THE SITE, AND TEMPORARY MULCHING.3. DUST CONTROL MEASURES SHALL BE UTILIZED SO AS TO PREVENT THE MIGRATION OF DUS |
| PROJECT DESCRIPTION THE PROJECT CONSISTS OF CONSTRUCTING TWO (2) MULTI-FAMILY APARTMENT BUILDINGS WITH | FROM THE SITE TO ABUTTING AREAS. <u>STOCKPILES:</u> |
| BASEMENT LEVEL PARKING, ONE (1) MIXED-USE BUILDING WITH FIRST FLOOR OFFICE AND AMENITY SPACE, AS WELL AS UPPER STORY APARTMENTS. DISTURBED AREA | 1. LOCATE STOCKPILES A MINIMUM OF 50 FEET AWAY FROM CATCH BASINS, SWALES, AND CULVERTS. |
| THE TOTAL AREA TO BE DISTURBED IS APPROXIMATELY 6.5 ACRES. | ALL STOCKPILES SHOULD BE SURROUNDED WITH TEMPORARY EROSION CONTROL MEASURE PRIOR TO THE ONSET OF PRECIPITATION. PERIMETER BARRIERS SHOULD BE MAINTAINED AT ALL TIMES, AND ADJUSTED AS NEEDED T |
| SOIL CHARACTERISTICS BASED ON THE SITE SPECIFIC SOIL SURVEY CONDUCTED BY LEONARD LORD, PHD, CSS, CSW ON OCTOBER 29 AND DECEMBER 2, 2019, THE SOILS ON SITE CONSIST OFWITH A HYDROLOGIC SOIL GROUP RATING OF A TO D. | ACCOMMODATE THE DELIVERY AND REMOVAL OF MATERIALS FROM THE STOCKPILE. THE INTEGRITY OF THE BARRIER SHOULD BE INSPECTED AT THE END OF EACH WORKING DAY. PROTECT ALL STOCKPILES FROM STORMWATER RUN-OFF USING TEMPORARY EROSION CONT MEASURES SUCH AS BERMS, SILT SOCK, OR OTHER APPROVED PRACTICE TO PREVENT MIGR |
| NAME OF RECEIVING WATERS THE STORMWATER RUNOFF FROM THE SITE WILL BE DISCHARGED VIA SUBSURFACE DRAINAGE WHICH ULTIMATELY FLOWS TO NORTH MILL POND. | OF MATERIAL BEYOND THE IMMEDIATE CONFINES OF THE STOCKPILES. OFF SITE VEHICLE TRACKING: |
| CONSTRUCTION SEQUENCE OF MAJOR ACTIVITIES: | 1. THE CONTRACTOR SHALL CONSTRUCT STABILIZED CONSTRUCTION ENTRANCE(S) PRIOR TO EXCAVATION ACTIVITIES. |
| CUT AND CLEAR TREES. CONSTRUCT TEMPORARY AND PERMANENT SEDIMENT, EROSION AND DETENTION CONTROL FACILITIES. EROSION, SEDIMENT AND DETENTION MEASURES SHALL BE INSTALLED PRIOR TO | VEGETATION: 1. TEMPORARY GRASS COVER: |
| ANY EARTH MOVING OPERATIONS THAT WILL INFLUENCE STORMWATER RUNOFF SUCH AS: NEW CONSTRUCTION CONTROL OF DUST | A. SEEDBED PREPARATION: a. APPLY FERTILIZER AT THE RATE OF 600 POUNDS PER ACRE OF 10-10-10. APPLY LIMESTONE (EQUIVALENT TO 50 PERCENT CALCIUM PLUS MAGNESIUM OXIDE) AT A OF THREE (3) TONS PER ACRE; |
| NEARNESS OF CONSTRUCTION SITE TO RECEIVING WATERS CONSTRUCTION DURING LATE WINTER AND EARLY SPRING ALL PERMANENT DITCHES, SWALES, DETENTION, RETENTION AND SEDIMENTATION BASINS TO BE | B. SEEDING: a. UTILIZE ANNUAL RYE GRASS AT A RATE OF 40 LBS/ACRE; |
| STABILIZED USING THE VEGETATIVE AND NON-STRUCTURAL BMPS PRIOR TO DIRECTING RUNOFF TO THEM. | WHERE THE SOIL HAS BEEN COMPACTED BY CONSTRUCTION OPERATIONS, LOOSEN TO A DEPTH OF TWO (2) INCHES BEFORE APPLYING FERTILIZER, LIME AND SEED; |
| CLEAR AND DISPOSE OF DEBRIS. CONSTRUCT TEMPORARY CULVERTS AND DIVERSION CHANNELS AS REQUIRED. CRAPE AND CRAVEL DOADWAYC AND DADI/UNC ADEAC. AND DADI/UNC ADEAC. | c. APPLY SEED UNIFORMLY BY HAND, CYCLONE SEEDER, OR HYDROSEEDER (SLURRY INCLUDING SEED AND FERTILIZER). HYDROSEEDINGS, WHICH INCLUDE MULCH, MA LEFT ON SOIL SURFACE. SEEDING RATES MUST BE INCREASED 10% WHEN |
| GRADE AND GRAVEL ROADWAYS AND PARKING AREAS - ALL ROADS AND PARKING AREA SHALL BE STABILIZED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE. BEGIN PERMANENT AND TEMPORARY SEEDING AND MULCHING. ALL CUT AND FILL SLOPES SHALL | HYDROSEEDING; C. MAINTENANCE: |
| BE SEEDED AND MULCHED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE. 8. DAILY, OR AS REQUIRED, CONSTRUCT TEMPORARY BERMS, DRAINS, DITCHES, PERIMETER | a. TEMPORARY SEEDING SHALL BE PERIODICALLY INSPECTED. AT A MINIMUM, 95% OF SOIL SURFACE SHOULD BE COVERED BY VEGETATION. IF ANY EVIDENCE OF EROS OR SEDIMENTATION IS APPARENT, REPAIRS SHALL BE MADE AND OTHER TEMPORAR |
| EROSION CONTROL MEASURES, SEDIMENT TRAPS, ETC., MULCH AND SEED AS REQUIRED. 9. SEDIMENT TRAPS AND/OR BASINS SHALL BE USED AS NECESSARY TO CONTAIN RUNOFF UNTIL SOILS ARE STABILIZED. | MEASURES USED IN THE INTERIM (MULCH, FILTER BARRIERS, CHECK DAMS, ETC.). 2. VEGETATIVE PRACTICE: |
| 10. FINISH PAVING ALL ROADWAYS AND PARKING LOTS. 11. INSPECT AND MAINTAIN ALL EROSION AND SEDIMENT CONTROL MEASURES. | A. FOR PERMANENT MEASURES AND PLANTINGS: a. LIMESTONE SHALL BE THOROUGHLY INCORPORATED INTO THE LOAM LAYER AT A RA THREE (3) TONS PER ACRE IN ORDER TO PROVIDE A PH VALUE OF 5.5 TO 6.5; |
| COMPLETE PERMANENT SEEDING AND LANDSCAPING. REMOVE TRAPPED SEDIMENTS FROM COLLECTOR DEVICES AS APPROPRIATE AND THEN REMOVE TEMPORARY EROSION CONTROL MEASURES. | b. FERTILIZER SHALL BE SPREAD ON THE TOP LAYER OF LOAM AND WORKED INTO THE SURFACE. FERTILIZER APPLICATION RATE SHALL BE 800 POUNDS PER ACRE OF 10-2 |
| SPECIAL CONSTRUCTION NOTES: 1. THE CONSTRUCTION SEQUENCE MUST LIMIT THE DURATION AND AREA OF DISTURBANCE. | FERTILIZER; c. SOIL CONDITIONERS AND FERTILIZER SHALL BE APPLIED AT THE RECOMMENDED R/ AND SHALL BE THOROUGHLY WORKED INTO THE LOAM. LOAM SHALL BE RAKED UN |
| THE PROJECT IS TO BE MANAGED IN A MANNER THAT MEETS THE REQUIREMENTS AND INTENT OF RSA 430:53 AND CHAPTER AGR 3800 RELATIVE TO INVASIVE SPECIES. | THE SURFACE IS FINELY PULVERIZED, SMOOTH AND EVEN, AND THEN COMPACTED EVEN SURFACE CONFORMING TO THE REQUIRED LINES AND GRADES WITH APPROV |
| EROSION CONTROL NOTES: 1. ALL EROSION CONTROL MEASURES AND PRACTICES SHALL CONFORM TO THE "NEW HAMPSHIRE | ROLLERS WEIGHING BETWEEN 4-1/2 POUNDS AND 5-1/2 POUNDS PER INCH OF WID d. SEED SHALL BE SOWN AT THE RATE SHOWN BELOW. SOWING SHALL BE DONE ON A CALM, DRY DAY, PREFERABLY BY MACHINE, BUT IF BY HAND, ONLY BY EXPERIENCE |
| STORMWATER MANUAL VOLUME 3: EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION" PREPARED BY THE NHDES. 2. PRIOR TO ANY WORK OR SOIL DISTURBANCE, CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR | WORKMEN. IMMEDIATELY BEFORE SEEDING, THE SOIL SHALL BE LIGHTLY RAKED. O HALF THE SEED SHALL BE SOWN IN ONE DIRECTION AND THE OTHER HALF AT RIGH |
| EROSION CONTROL MEASURES AS REQUIRED IN THE PROJECT MANUAL. CONTRACTOR SHALL INSTALL TEMPORARY EROSION CONTROL BARRIERS, INCLUDING HAY BALES, | ANGLES TO THE ORIGINAL DIRECTION. IT SHALL BE LIGHTLY RAKED INTO THE SOIL DEPTH NOT OVER 1/4 INCH AND ROLLED WITH A HAND ROLLER WEIGHING NOT OVE POUNDS PER LINEAR FOOT OF WIDTH; |
| SILT FENCES, MULCH BERMS, SILT SACKS AND SILT SOCKS AS SHOWN IN THESE DRAWINGS AS THE FIRST ORDER OF WORK. 4. SILT SACK INLET PROTECTION SHALL BE INSTALLED IN ALL EXISTING AND PROPOSED CATCH | e. HAY MULCH SHALL BE APPLIED IMMEDIATELY AFTER SEEDING AS INDICATED ABOVE f. THE SURFACE SHALL BE WATERED AND KEPT MOIST WITH A FINE SPRAY AS REQUIP |
| BASIN INLETS WITHIN THE WORK LIMITS AND BE MAINTAINED FOR THE DURATION OF THE PROJECT. | WITHOUT WASHING AWAY THE SOIL, UNTIL THE GRASS IS WELL ESTABLISHED. AN AREAS WHICH ARE NOT SATISFACTORILY COVERED WITH GRASS SHALL BE RESEED AND ALL NOXIOUS WEEDS REMOVED; |
| PERIMETER CONTROLS INCLUDING SILT FENCES, MULCH BERM, SILT SOCK, AND/OR HAY BALE BARRIERS SHALL BE MAINTAINED FOR THE DURATION OF THE PROJECT UNTIL NON-PAVED AREAS HAVE BEEN STABILIZED. | g. THE CONTRACTOR SHALL PROTECT AND MAINTAIN THE SEEDED AREAS UNTIL ACCE h. A GRASS SEED MIXTURE CONTAINING THE FOLLOWING SEED REQUIREMENTS SHAL |
| 6. THE CONTRACTOR SHALL REMOVE AND PROPERLY DISPOSE OF ALL TEMPORARY EROSION CONTROL DEVICES UPON COMPLETION OF CONSTRUCTION. | APPLIED AT THE INDICATED RATE: <u>SEED MIX APPLICATION RATE</u> TALL FESCUE 20 LBS/ACRE |
| ALL DISTURBED AREAS NOT OTHERWISE BEING TREATED SHALL RECEIVE 6" LOAM, SEED AND FERTILIZER. INSPECT ALL INLET PROTECTION AND PERIMETER CONTROLS WEEKLY AND AFTER EACH RAIN | CREEPING RED FESCUE 20 LBS/ACRE BIRDSFOOT TREFOIL 8 LBS/ACRE |
| STORM OF 0.25 INCH OR GREATER. REPAIR/MODIFY PROTECTION AS NECESSARY TO MAXIMIZE EFFICIENCY OF FILTER. REPLACE ALL FILTERS WHEN SEDIMENT IS 1/3 THE FILTER HEIGHT. | IN NO CASE SHALL THE WEED CONTENT EXCEED ONE (1) PERCENT BY WEIGHT. ALL SHALL COMPLY WITH STATE AND FEDERAL SEED LAWS. SEEDING SHALL BE DONE N LATER THAN SEPTEMBER 15. IN NO CASE SHALL SEEDING TAKE PLACE OVER SNOW. |
| 9. CONSTRUCT EROSION CONTROL BLANKETS ON ALL SLOPES STEEPER THAN 3:1. STABILIZATION: | DORMANT SEEDING (SEPTEMBER 15 TO FIRST SNOWFALL): A. FOLLOW PERMANENT MEASURES SLOPE, LIME, FERTILIZER AND GRADING REQUIREMENT |
| AN AREA SHALL BE CONSIDERED STABLE WHEN ONE OF THE FOLLOWING HAS OCCURRED: A. BASE COURSE GRAVELS HAVE BEEN INSTALLED IN AREAS TO BE PAVED; B. A MINIMUM OF 85% VEGETATED GROWTH HAS BEEN ESTABLISHED; | APPLY SEED MIXTURE AT TWICE THE INDICATED RATE. APPLY MULCH AS INDICATED FO PERMANENT MEASURES. |
| C. A MINIMUM OF 3" OF NON-EROSIVE MATERIAL SUCH AS STONE OR RIPRAP HAS BEEN INSTALLED; | CONCRETE WASHOUT AREA: 1. THE FOLLOWING ARE THE ONLY NON-STORMWATER DISCHARGES ALLOWED. ALL OTHER NON-STORMWATER DISCHARGES ARE PROHIBITED ON SITE: |
| D. EROSION CONTROL BLANKETS HAVE BEEN PROPERLY INSTALLED.; E. IN AREAS TO BE PAVED, "STABLE" MEANS THAT BASE COURSE GRAVELS MEETING THE REQUIREMENTS OF NHDOT STANDARD FOR ROAD AND BRIDGE CONSTRUCTION, 2016, ITEM | A. THE CONCRETE DELIVERY TRUCKS SHALL, WHENEVER POSSIBLE, USE WASHOUT FACILI AT THEIR OWN PLANT OR DISPATCH FACILITY; |
| 304.2 HAVE BEEN INSTALLED. 2. WINTER STABILIZATION PRACTICES: | B. IF IT IS NECESSARY, SITE CONTRACTOR SHALL DESIGNATE SPECIFIC WASHOUT AREAS DESIGN FACILITIES TO HANDLE ANTICIPATED WASHOUT WATER; |
| A. ALL PROPOSED VEGETATED AREAS THAT DO NOT EXHIBIT A MINIMUM OF 85 PERCENT VEGETATIVE GROWTH BY OCTOBER 15, OR WHICH ARE DISTURBED AFTER OCTOBER 15, | C. CONTRACTOR SHALL LOCATE WASHOUT AREAS AT LEAST 150 FEET AWAY FROM STORM DRAINS, SWALES AND SURFACE WATERS OR DELINEATED WETLANDS; D. INSPECT WASHOUT FACILITIES DAILY TO DETECT LEAKS OR TEARS AND TO IDENTIFY W |
| SHALL BE STABILIZED BY SEEDING AND INSTALLING EROSION CONTROL BLANKETS ON SLOPES GREATER THAN 3:1, AND SEEDING AND PLACING 3 TO 4 TONS OF MULCH PER ACRE, SECURED WITH ANCHORED NETTING, ELSEWHERE. THE INSTALLATION OF EROSION CONTROL | MATERIALS NEED TO BE REMOVED. ALLOWABLE NON-STORMWATER DISCHARGES: |
| BLANKETS OR MULCH AND NETTING SHALL NOT OCCUR OVER ACCUMULATED SNOW OR ON FROZEN GROUND AND SHALL BE COMPLETED IN ADVANCE OF THAW OR SPRING MELT | FIRE-FIGHTING ACTIVITIES; FIRE HYDRANT FLUSHING; |
| EVENTS; B. ALL DITCHES OR SWALES WHICH DO NOT EXHIBIT A MINIMUM OF 85 PERCENT VEGETATIVE GROWTH BY OCTOBER 15, OR WHICH ARE DISTURBED AFTER OCTOBER 15, SHALL BE | WATERS USED TO WASH VEHICLES WHERE DETERGENTS ARE NOT USED; WATER USED TO CONTROL DUST; POTABLE WATER INCLUDING UNCONTAMINATED WATER LINE FLUSHING; |
| STABILIZED TEMPORARILY WITH STONE OR EROSION CONTROL BLANKETS APPROPRIATE FOR THE DESIGN FLOW CONDITIONS; | ROUTINE EXTERNAL BUILDING WASH DOWN WHERE DETERGENTS ARE NOT USED; PAVEMENT WASH WATERS WHERE DETERGENTS ARE NOT USED; |
| C. AFTER OCTOBER 15, INCOMPLETE ROAD OR PARKING SURFACES, WHERE WORK HAS STOPPED FOR THE WINTER SEASON, SHALL BE PROTECTED WITH A MINIMUM OF 3 INCHES OF CRUSHED GRAVEL PER NHDOT ITEM 304.3, OR IF CONSTRUCTION IS TO CONTINUE THROUGH THE | UNCONTAMINATED AIR CONDITIONING/COMPRESSOR CONDENSATION; UNCONTAMINATED GROUND WATER OR SPRING WATER; EQUINDATION OR EQUING DRAINS WHICH ARE UNCONTAMINATED; |
| WINTER SEASON BE CLEARED OF ANY ACCUMULATED SNOW AFTER EACH STORM EVENT; 3. STABILIZATION SHALL BE INITIATED ON ALL LOAM STOCKPILES, AND DISTURBED AREAS, WHERE | FOUNDATION OR FOOTING DRAINS WHICH ARE UNCONTAMINATED; UNCONTAMINATED EXCAVATION DEWATERING; LANDSCAPE IRRIGATION. |
| CONSTRUCTION ACTIVITY SHALL NOT OCCUR FOR MORE THAN TWENTY-ONE (21) CALENDAR DAYS BY THE FOURTEENTH (14TH) DAY AFTER CONSTRUCTION ACTIVITY HAS PERMANENTLY OR | WASTE DISPOSAL: 1. WASTE MATERIAL: |
| TEMPORARILY CEASED IN THAT AREA. STABILIZATION MEASURES TO BE USED INCLUDE: A. TEMPORARY SEEDING; B. MULCHING. | A. ALL WASTE MATERIALS SHALL BE COLLECTED AND STORED IN SECURELY LIDDED RECEPTACLES. ALL TRASH AND CONSTRUCTION DEBRIS FROM THE SITE SHALL BE DEPO |
| ALL AREAS SHALL BE STABILIZED WITHIN 45 DAYS OF INITIAL DISTURBANCE. WHEN CONSTRUCTION ACTIVITY PERMANENTLY OR TEMPORARILY CEASES WITHIN 100 FEET OF | IN A DUMPSTER; B. NO CONSTRUCTION WASTE MATERIALS SHALL BE BURIED ON SITE; C. ALL PERSONNEL SHALL BE INSTRUCTED REGARDING THE CORRECT PROCEDURE FOR WA |
| NEARBY SURFACE WATERS OR DELINEATED WETLANDS, THE AREA SHALL BE STABILIZED WITHIN SEVEN (7) DAYS OR PRIOR TO A RAIN EVENT. ONCE CONSTRUCTION ACTIVITY CEASES PERMANENTLY IN AN THESE AREAS, SILT FENCES, MULCH BERMS, HAY BALE BARRIERS AND ANY | DISPOSAL BY THE SUPERINTENDENT. 2. HAZARDOUS WASTE: |
| | A. ALL HAZARDOUS WASTE MATERIALS SHALL BE DISPOSED OF IN THE MANNER SPECIFIED |
| EARTH/DIKES SHALL BE REMOVED ONCE PERMANENT MEASURES ARE ESTABLISHED. 6. DURING CONSTRUCTION, RUNOFF WILL BE DIVERTED AROUND THE SITE WITH EARTH DIKES, | LOCAL OR STATE REGULATION OR BY THE MANUFACTURER; |
| • | |

R THE WINTER BY OCTOBER 15.

- TO CONTROL DUST THROUGHOUT THE CONSTRUCTION
- , BUT BE NOT LIMITED TO SPRINKLING WATER ON
- ZED SO AS TO PREVENT THE MIGRATION OF DUST
- ET AWAY FROM CATCH BASINS, SWALES, AND
- WITH TEMPORARY EROSION CONTROL MEASURES
- INED AT ALL TIMES, AND ADJUSTED AS NEEDED TO AL OF MATERIALS FROM THE STOCKPILE. THE ISPECTED AT THE END OF EACH WORKING DAY. ATER RUN-OFF USING TEMPORARY EROSION CONTROL R OTHER APPROVED PRACTICE TO PREVENT MIGRATION VFINES OF THE STOCKPILES.
- BILIZED CONSTRUCTION ENTRANCE(S) PRIOR TO ANY
- 600 POUNDS PER ACRE OF 10-10-10. APPLY ERCENT CALCIUM PLUS MAGNESIUM OXIDE) AT A RATE
- RATE OF 40 LBS/ACRE;
- ACTED BY CONSTRUCTION OPERATIONS, LOOSEN SOIL EFORE APPLYING FERTILIZER, LIME AND SEED; CYCLONE SEEDER, OR HYDROSEEDER (SLURRY). HYDROSEEDINGS, WHICH INCLUDE MULCH, MAY BE RATES MUST BE INCREASED 10% WHEN
- ERIODICALLY INSPECTED. AT A MINIMUM, 95% OF THE ERED BY VEGETATION. IF ANY EVIDENCE OF EROSION REPAIRS SHALL BE MADE AND OTHER TEMPORARY (MULCH, FILTER BARRIERS, CHECK DAMS, ETC.).
- TINGS: Y INCORPORATED INTO THE LOAM LAYER AT A RATE OF ER TO PROVIDE A PH VALUE OF 5.5 TO 6.5; THE TOP LAYER OF LOAM AND WORKED INTO THE N RATE SHALL BE 800 POUNDS PER ACRE OF 10-20-20
- ZER SHALL BE APPLIED AT THE RECOMMENDED RATES KED INTO THE LOAM. LOAM SHALL BE RAKED UNTIL ED, SMOOTH AND EVEN, AND THEN COMPACTED TO AN HE REQUIRED LINES AND GRADES WITH APPROVED /2 POUNDS AND 5-1/2 POUNDS PER INCH OF WIDTH; E SHOWN BELOW. SOWING SHALL BE DONE ON A
- MACHINE, BUT IF BY HAND, ONLY BY EXPERIENCED SEEDING, THE SOIL SHALL BE LIGHTLY RAKED. ONE N ONE DIRECTION AND THE OTHER HALF AT RIGHT ION. IT SHALL BE LIGHTLY RAKED INTO THE SOIL TO A
- OLLED WITH A HAND ROLLER WEIGHING NOT OVER 100 MEDIATELY AFTER SEEDING AS INDICATED ABOVE;
- AND KEPT MOIST WITH A FINE SPRAY AS REQUIRED, IL, UNTIL THE GRASS IS WELL ESTABLISHED. ANY TORILY COVERED WITH GRASS SHALL BE RESEEDED, ED:
- AND MAINTAIN THE SEEDED AREAS UNTIL ACCEPTED; ING THE FOLLOWING SEED REQUIREMENTS SHALL BE
- ATION RATE
- LBS/ACRE
- LBS/ACRE LBS/ACRE
- ITENT EXCEED ONE (1) PERCENT BY WEIGHT. ALL SEED EDERAL SEED LAWS. SEEDING SHALL BE DONE NO CASE SHALL SEEDING TAKE PLACE OVER SNOW. ST SNOWFALL):
- E, LIME, FERTILIZER AND GRADING REQUIREMENTS. NDICATED RATE. APPLY MULCH AS INDICATED FOR
- MWATER DISCHARGES ALLOWED. ALL OTHER
- HIBITED ON SITE:
- ALL, WHENEVER POSSIBLE, USE WASHOUT FACILITIES ACILITY;
- R SHALL DESIGNATE SPECIFIC WASHOUT AREAS AND IPATED WASHOUT WATER;
- T AREAS AT LEAST 150 FEET AWAY FROM STORM
- RS OR DELINEATED WETLANDS; TO DETECT LEAKS OR TEARS AND TO IDENTIFY WHEN

<u> GES:</u>

- DETERGENTS ARE NOT USED;
- ATED WATER LINE FLUSHING;
- N WHERE DETERGENTS ARE NOT USED; ENTS ARE NOT USED;
- MPRESSOR CONDENSATION
- RING WATER;
- ARE UNCONTAMINATED; ING
- ECTED AND STORED IN SECURELY LIDDED RUCTION DEBRIS FROM THE SITE SHALL BE DEPOSITED
- S SHALL BE BURIED ON SITE;
- D REGARDING THE CORRECT PROCEDURE FOR WASTE
- HALL BE DISPOSED OF IN THE MANNER SPECIFIED BY HE MANUFACTURER ED IN THESE PRACTICES BY THE SUPERINTENDENT.
- CTED FROM THE PORTABLE UNITS A MINIMUM OF ONCE ASTE MANAGEMENT CONTRACTOR.

SPILL PREVENTION:

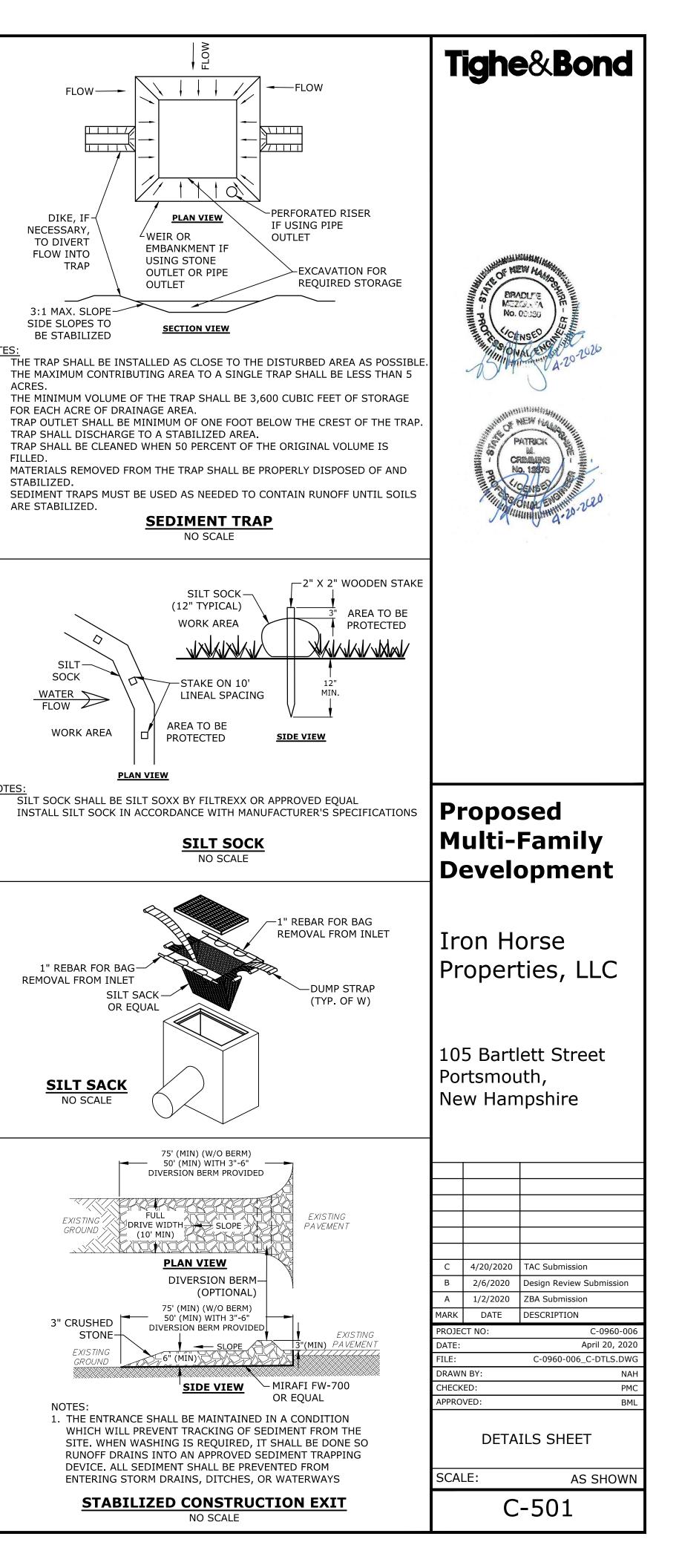
- CONTRACTOR SHALL BE FAMILIAR WITH SPILL PREVENTION MEASURES REQUIRED BY LOCAL, STATE AND FEDERAL AGENCIES. AT A MINIMUM, CONTRACTOR SHALL FOLLOW THE BEST MANAGEMENT SPILL PREVENTION PRACTICES OUTLINED BELOW.
- 2. THE FOLLOWING ARE THE MATERIAL MANAGEMENT PRACTICES THAT SHALL BE USED TO REDUCE THE RISK OF SPILLS OR OTHER ACCIDENTAL EXPOSURE OF MATERIALS AND SUBSTANCES DURING CONSTRUCTION TO STORMWATER RUNOFF:
- A. GOOD HOUSEKEEPING THE FOLLOWING GOOD HOUSEKEEPING PRACTICE SHALL BE FOLLOWED ON SITE DURING CONSTRUCTION:
- ONLY SUFFICIENT AMOUNTS OF PRODUCTS TO DO THE JOB SHALL BE STORED ON SITE; b. ALL REGULATED MATERIALS STORED ON SITE SHALL BE STORED IN A NEAT, ORDERLY MANNER IN THEIR PROPER (ORIGINAL IF POSSIBLE) CONTAINERS AND, IF POSSIBLE, UNDER A ROOF OR OTHER ENCLOSURE, ON AN IMPERVIOUS SURFACE;
- c. MANUFACTURER'S RECOMMENDATIONS FOR PROPER USE AND DISPOSAL SHALL BE FOLLOWED;
- d. THE SITE SUPERINTENDENT SHALL INSPECT DAILY TO ENSURE PROPER USE AND DISPOSAL OF MATERIALS;
- e. SUBSTANCES SHALL NOT BE MIXED WITH ONE ANOTHER UNLESS RECOMMENDED BY THE MANUFACTURER; WHENEVER POSSIBLE ALL OF A PRODUCT SHALL BE USED UP BEFORE DISPOSING OF THE
- CONTAINER. g. THE TRAINING OF ON-SITE EMPLOYEES AND THE ON-SITE POSTING OF RELEASE
- RESPONSE INFORMATION DESCRIBING WHAT TO DO IN THE EVENT OF A SPILL OF REGULATED SUBSTANCES.
- B. HAZARDOUS PRODUCTS THE FOLLOWING PRACTICES SHALL BE USED TO REDUCE THE RISKS ASSOCIATED WITH HAZARDOUS MATERIALS:
- a. PRODUCTS SHALL BE KEPT IN THEIR ORIGINAL CONTAINERS UNLESS THEY ARE NOT RESEALABLE; b. ORIGINAL LABELS AND MATERIAL SAFETY DATA SHALL BE RETAINED FOR IMPORTANT
- PRODUCT INFORMATION: c. SURPLUS PRODUCT THAT MUST BE DISPOSED OF SHALL BE DISCARDED ACCORDING TO
- THE MANUFACTURER'S RECOMMENDED METHODS OF DISPOSAL C. PRODUCT SPECIFIC PRACTICES - THE FOLLOWING PRODUCT SPECIFIC PRACTICES SHALL BE
- FOLLOWED ON SITE: a. PETROLEUM PRODUCTS:
- i. ALL ON SITE VEHICLES SHALL BE MONITORED FOR LEAKS AND RECEIVE REGULAR PREVENTIVE MAINTENANCE TO REDUCE LEAKAGE;
- ii. PETROLEUM PRODUCTS SHALL BE STORED IN TIGHTLY SEALED CONTAINERS WHICH ARE CLEARLY LABELED. ANY ASPHALT BASED SUBSTANCES USED ON SITE SHALL BE APPLIED ACCORDING TO THE MANUFACTURER'S RECOMMENDATIONS.
- iii. SECURE FUEL STORAGE AREAS AGAINST UNAUTHORIZED ENTRY;
- iv. INSPECT FUEL STORAGE AREAS WEEKLY;
- v. WHEREVER POSSIBLE, KEEP REGULATED CONTAINERS THAT ARE STORED OUTSIDE MORE THAN 50 FEET FROM SURFACE WATER AND STORM DRAINS, 75 FEET FROM PRIVATE WELLS, AND 400 FEET FROM PUBLIC WELLS;
- vi. COVER REGULATED CONTAINERS IN OUTSIDE STORAGE AREAS;
- vii. SECONDARY CONTAINMENT IS REQUIRED FOR CONTAINERS CONTAINING REGULATED SUBSTANCES STORED OUTSIDE, EXCEPT FOR ON PREMISE USE HEATING FUEL TANKS, OR ABOVEGROUND OR UNDERGROUND STORAGE TANKS OTHERWISE REGULATED. viii. THE FUEL HANDLING REQUIREMENTS SHALL INCLUDE:
 - (1) EXCEPT WHEN IN USE, KEEP CONTAINERS CONTAINING REGULATED SUBSTANCES CLOSED AND SEALED;
 - (2) PLACE DRIP PANS UNDER SPIGOTS, VALVES, AND PUMPS; (3) HAVE SPILL CONTROL AND CONTAINMENT EQUIPMENT READILY AVAILABLE IN
 - ALL WORK AREAS;
 - (4) USE FUNNELS AND DRIP PANS WHEN TRANSFERRING REGULATED SUBSTANCES; (5) PERFORM TRANSFERS OF REGULATED SUBSTANCES OVER AN IMPERVIOUS SURFACE.
- ix. FUELING AND MAINTENANCE OF EXCAVATION, EARTHMOVING AND OTHER
- CONSTRUCTION RELATED EQUIPMENT SHALL COMPLY WITH THE REGULATIONS OF THE NEW HAMPSHIRE DEPARTMENT OF ENVIRONMENTAL SERVICES THESE REQUIREMENTS ARE SUMMARIZED IN WD-DWGB-22-6 BEST MANAGEMENT PRACTICES FOR FUELING AND MAINTENANCE OF EXCAVATION AND EARTHMOVING EQUIPMENT, OR ITS SUCCESSOR DOCUMENT HTTPS://WWW.DES.NH.GOV/ORGANIZATION/COMMISSIONER/PIP/FACTSHEETS/DWGB/DOCUMENTS/DWGB-22-6.PDF
- FERTILIZERS FERTILIZERS USED SHALL BE APPLIED ONLY IN THE MINIMUM AMOUNTS DIRECTED BY
- THE SPECIFICATIONS; ii. ONCE APPLIED FERTILIZER SHALL BE WORKED INTO THE SOIL TO LIMIT EXPOSURE TO
- STORMWATER: iii. STORAGE SHALL BE IN A COVERED SHED OR ENCLOSED TRAILERS. THE CONTENTS OF ANY PARTIALLY USED BAGS OF FERTILIZER SHALL BE TRANSFERRED TO A SEALABLE PLASTIC BIN TO AVOID SPILLS.
- c. PAINTS:
- i. ALL CONTAINERS SHALL BE TIGHTLY SEALED AND STORED WHEN NOT REQUIRED FOR USE;
- EXCESS PAINT SHALL NOT BE DISCHARGED TO THE STORM SEWER SYSTEM;
- iii. EXCESS PAINT SHALL BE DISPOSED OF PROPERLY ACCORDING TO MANUFACTURER'S INSTRUCTIONS OR STATE AND LOCAL REGULATIONS. D. SPILL CONTROL PRACTICES - IN ADDITION TO GOOD HOUSEKEEPING AND MATERIAL
- MANAGEMENT PRACTICES DISCUSSED IN THE PREVIOUS SECTION, THE FOLLOWING PRACTICES SHALL BE FOLLOWED FOR SPILL PREVENTION AND CLEANUP: a. MANUFACTURER'S RECOMMENDED METHODS FOR SPILL CLEANUP SHALL BE CLEARLY
- POSTED AND SITE PERSONNEL SHALL BE MADE AWARE OF THE PROCEDURES AND THE LOCATION OF THE INFORMATION AND CLEANUP SUPPLIES; b. MATERIALS AND EQUIPMENT NECESSARY FOR SPILL CLEANUP SHALL BE KEPT IN THE
- MATERIAL STORAGE AREA ON SITE. EQUIPMENT AND MATERIALS SHALL INCLUDE BUT NOT BE LIMITED TO BROOMS, DUSTPANS, MOPS, RAGS, GLOVES, GOGGLES, KITTY LITTER, SAND, SAWDUST AND PLASTIC OR METAL TRASH CONTAINERS SPECIFICALLY FOR THIS PURPOSE;
- c. ALL SPILLS SHALL BE CLEANED UP IMMEDIATELY AFTER DISCOVERY;
- d. THE SPILL AREA SHALL BE KEPT WELL VENTILATED AND PERSONNEL SHALL WEAR APPROPRIATE PROTECTIVE CLOTHING TO PREVENT INJURY FROM CONTACT WITH A HAZARDOUS SUBSTANCE;
- e. SPILLS OF TOXIC OR HAZARDOUS MATERIAL SHALL BE REPORTED TO THE APPROPRIATE LOCAL, STATE OR FEDERAL AGENCIES AS REQUIRED;
- f. THE SITE SUPERINTENDENT RESPONSIBLE FOR DAY-TO-DAY SITE OPERATIONS SHALL BE THE SPILL PREVENTION AND CLEANUP COORDINATOR. E. VEHICLE FUELING AND MAINTENANCE PRACTICE:
- a. CONTRACTOR SHALL MAKE AN EFFORT TO PERFORM EQUIPMENT/VEHICLE FUELING AND MAINTENANCE AT AN OFF-SITE FACILITY;
- b. CONTRACTOR SHALL PROVIDE AN ON-SITE FUELING AND MAINTENANCE AREA THAT IS CLEAN AND DRY;
- c. IF POSSIBLE THE CONTRACTOR SHALL KEEP AREA COVERED;
- d. CONTRACTOR SHALL KEEP A SPILL KIT AT THE FUELING AND MAINTENANCE AREA;
- e. CONTRACTOR SHALL REGULARLY INSPECT VEHICLES FOR LEAKS AND DAMAGE; CONTRACTOR SHALL USE DRIP PANS, DRIP CLOTHS, OR ABSORBENT PADS WHEN

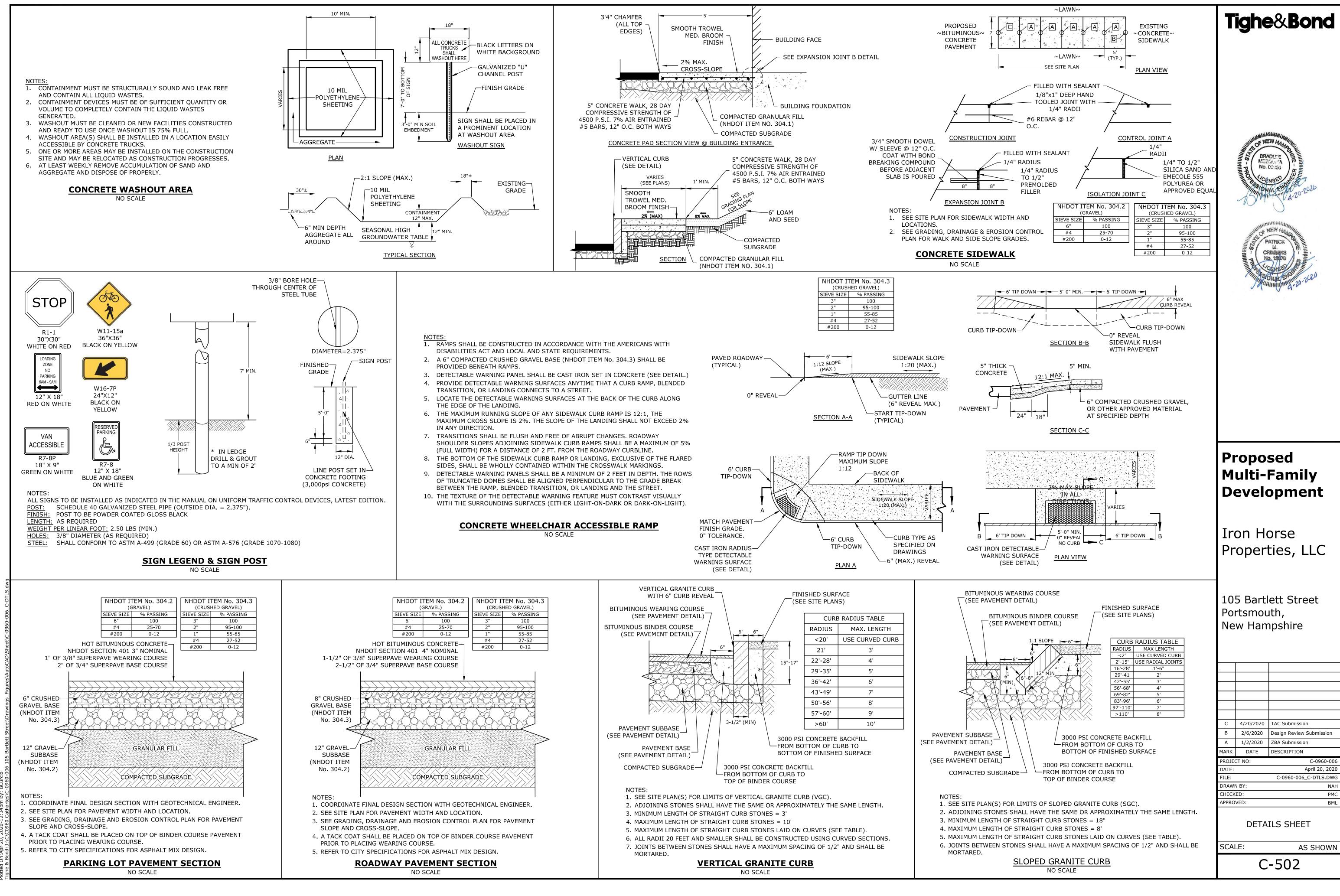
EROSION CONTROL OBSERVATIONS AND MAINTENANCE PRACTICES

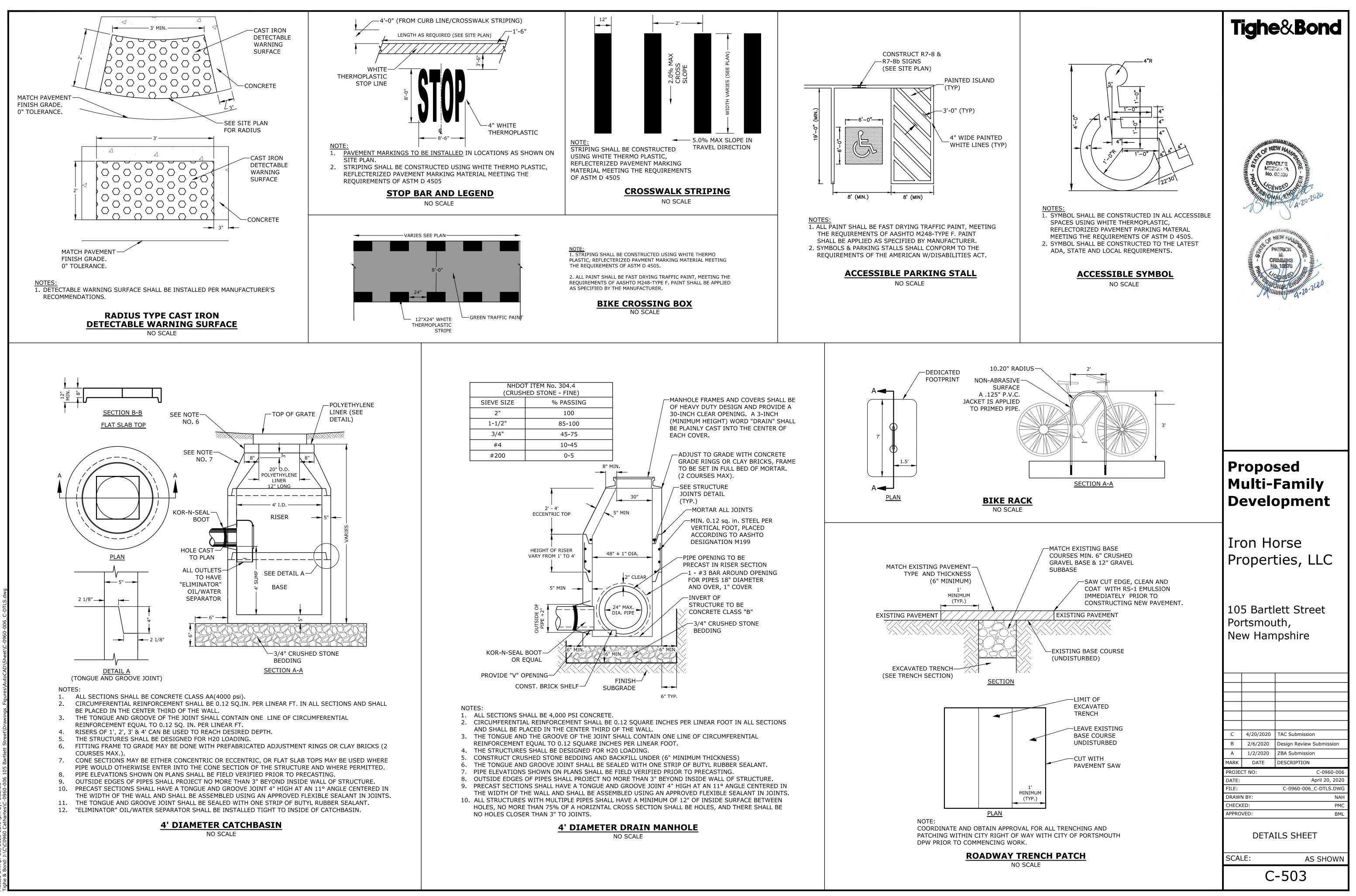
REPLACING SPENT FLUID.

THIS PROJECT EXCEEDS ONE (1) ACRE OF DISTURBANCE AND THUS REQUIRES A SWPPP. THE SWPPP SHALL BE PREPARED BY THE CONTRACTOR. THE CONTRACTOR SHALL BE FAMILIAR WITH THE SWPPP AND KEEP AN UPDATED COPY OF THE SWPPP ONSITE AT ALL TIMES.

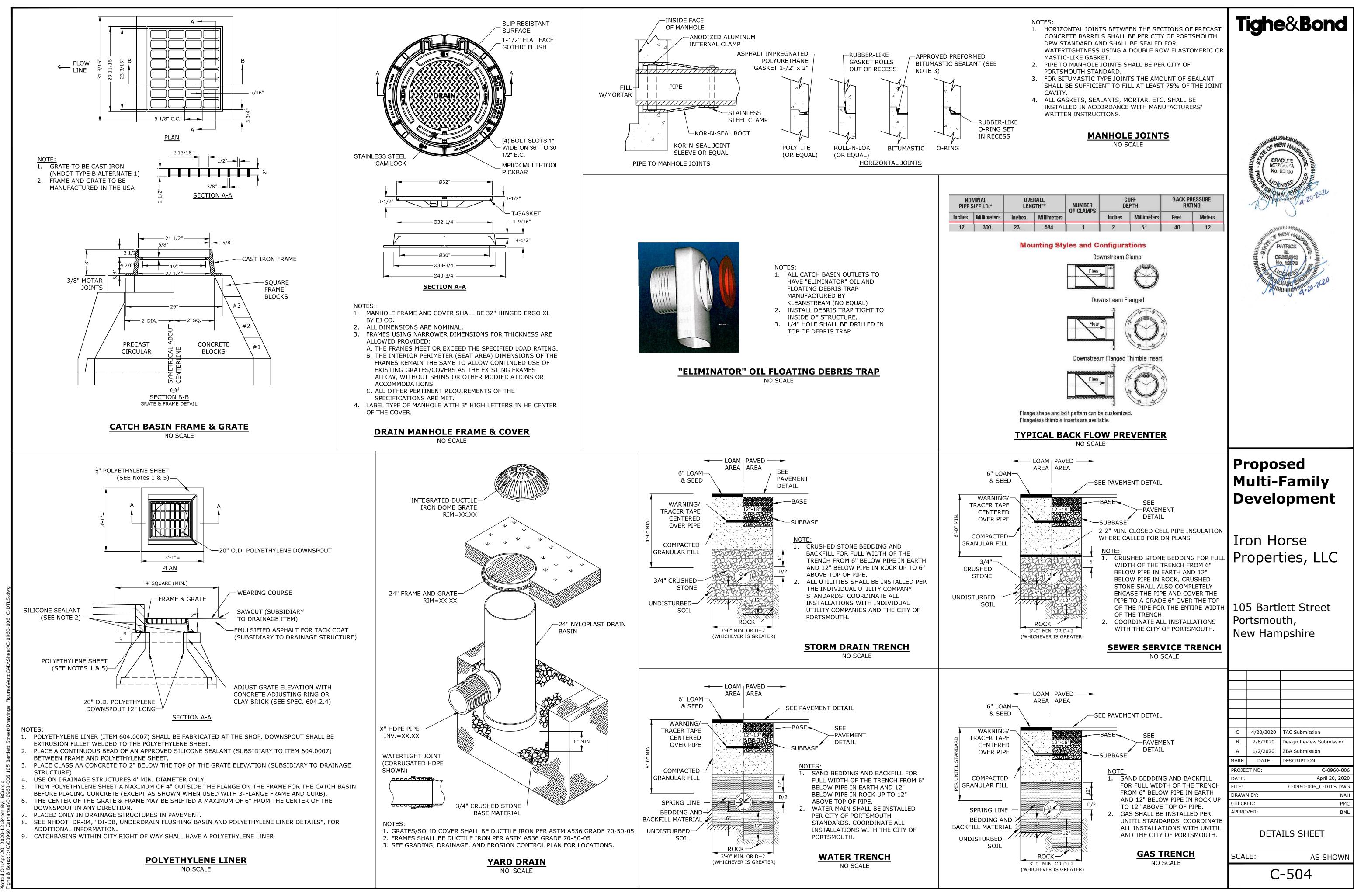
- 2. THE FOLLOWING REPRESENTS THE GENERAL OBSERVATION AND REPORTING PRACTICES THAT SHALL BE FOLLOWED AS PART OF THIS PROJECT: A. OBSERVATIONS OF THE PROJECT FOR COMPLIANCE WITH THE SWPPP SHALL BE MADE BY THE
 - CONTRACTOR AT LEAST ONCE A WEEK OR WITHIN 24 HOURS OF A STORM 0.25 INCHES OR GREATER;
- B. AN OBSERVATION REPORT SHALL BE MADE AFTER EACH OBSERVATION AND DISTRIBUTED TO THE ENGINEER, THE OWNER, AND THE CONTRACTOR;
- C. A REPRESENTATIVE OF THE SITE CONTRACTOR, SHALL BE RESPONSIBLE FOR MAINTENANCE AND REPAIR ACTIVITIES;
- D. IF A REPAIR IS NECESSARY, IT SHALL BE INITIATED WITHIN 24 HOURS OF REPORT.



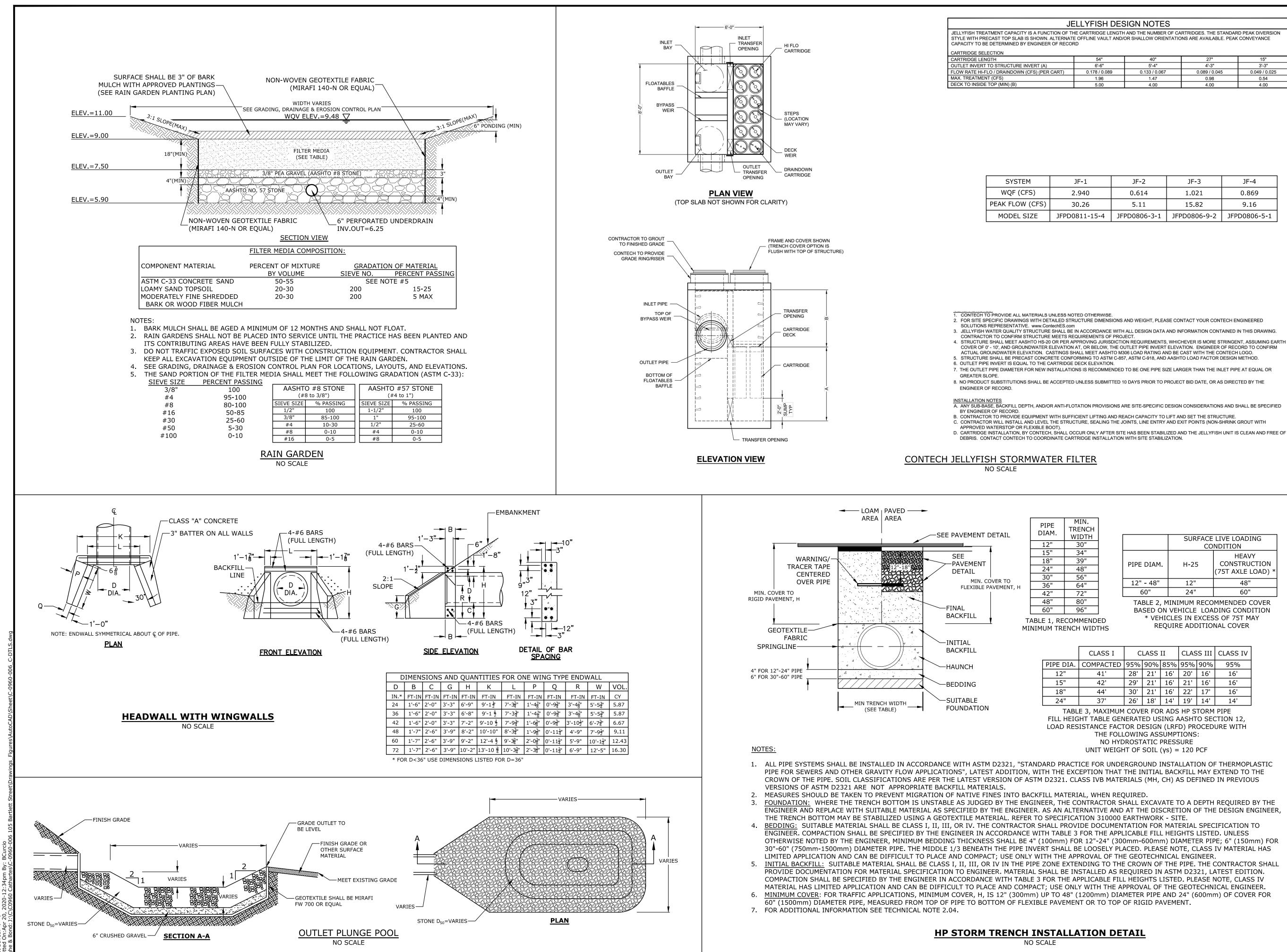




5aved: 4/20/2020 ed On:Apr 20, 2020-12:34pm By: BCurcio • & Rond: 1-0:C/C0060 Cathartee/C-0060-006 105 Bartlett Street\Drawings_Figuree\AutoCAD\Sheet\C-0060-006_C-DT



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JELLYFISH DESIGN NOTES

JELLYFISH TREATMENT CAPACITY IS A FUNCTION OF THE CARTRIDGE LENGTH AND THE NUMBER OF CARTRIDGES. THE STANDARD PEAK DIVERSION

Tighe&Bond

PATRIC

Proposed

Iron Horse

Multi-Family Development

Properties, LLC

105 Bartlett Street

4/20/2020 TAC Submission

1/2/2020

MARK DATE DESCRIPTION

2/6/2020 Design Review Submission

ZBA Submission

DETAILS SHEET

C-505

C-0960-00

NAF

PM

BM

April 20, 202

AS SHOWN

C-0960-006_C-DTLS.DW0

В

Α

DATE:

FILE:

PROJECT NO:

DRAWN BY:

CHECKED:

APPROVED:

SCALE:

New Hampshire

Portsmouth,

| 54" | 40" | 27" | 15" |
|---------------|---------------|---------------|---------------|
| 6'-6" | 5'-4" | 4'-3" | 3'-3" |
| 0.178 / 0.089 | 0.133 / 0.067 | 0.089 / 0.045 | 0.049 / 0.025 |
| 1.96 | 1.47 | 0.98 | 0.54 |
| 5.00 | 4 00 | 4 00 | 4 00 |

| F-1 | JF-2 | JF-3 | JF-4 |
|----------|--------------|--------------|--------------|
| .940 | 0.614 | 1.021 | 0.869 |
| 0.26 | 5.11 | 15.82 | 9.16 |
| 811-15-4 | JFPD0806-3-1 | JFPD0806-9-2 | JFPD0806-5-1 |

| F-1 | JF-2 | JF-3 | JF-4 |
|------|-------|-------|-------|
| .940 | 0.614 | 1.021 | 0.869 |
| 0.26 | 5.11 | 15.82 | 9.16 |
| _ | | | |

| F-1 | JF-2 | JF-3 | JF-4 |
|------|-------|-------|-------|
| 940 | 0.614 | 1.021 | 0.869 |
|).26 | 5.11 | 15.82 | 9.16 |

| 940 | 0.614 | 1.021 | 0.869 |
|---------|--------------|--------------|--------------|
| .26 | 5.11 | 15.82 | 9.16 |
| 11-15-4 | JFPD0806-3-1 | JFPD0806-9-2 | JFPD0806-5-1 |
| | | | |

COVER OF 0' - 10', AND GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM

PIPE DIAM.

12" - 48"

60"

CLASS II

THE FOLLOWING ASSUMPTIONS:

NO HYDROSTATIC PRESSURE

UNIT WEIGHT OF SOIL (γ s) = 120 PCF

28' 21' 16' 20' 16'

29' 21' 16' 21' 16'

30' | 21' | 16' | 22' | 17' |

26' 18' 14' 19' 14'

CLASS I

41'

44'

42'

37'

3. JELLYFISH WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING. 4. STRUCTURE SHALL MEET AASHTO HS-20 OR PER APPROVING JURISDICTION REQUIREMENTS, WHICHEVER IS MORE STRINGENT, ASSUMING EARTH

SURFACE LIVE LOADING

CONDITION

H-25

12"

24"

TABLE 2, MINIMUM RECOMMENDED COVER

BASED ON VEHICLE LOADING CONDITION

* VEHICLES IN EXCESS OF 75T MAY

REQUIRE ADDITIONAL COVER

CLASS III CLASS IV

95%

16'

16'

16'

14'

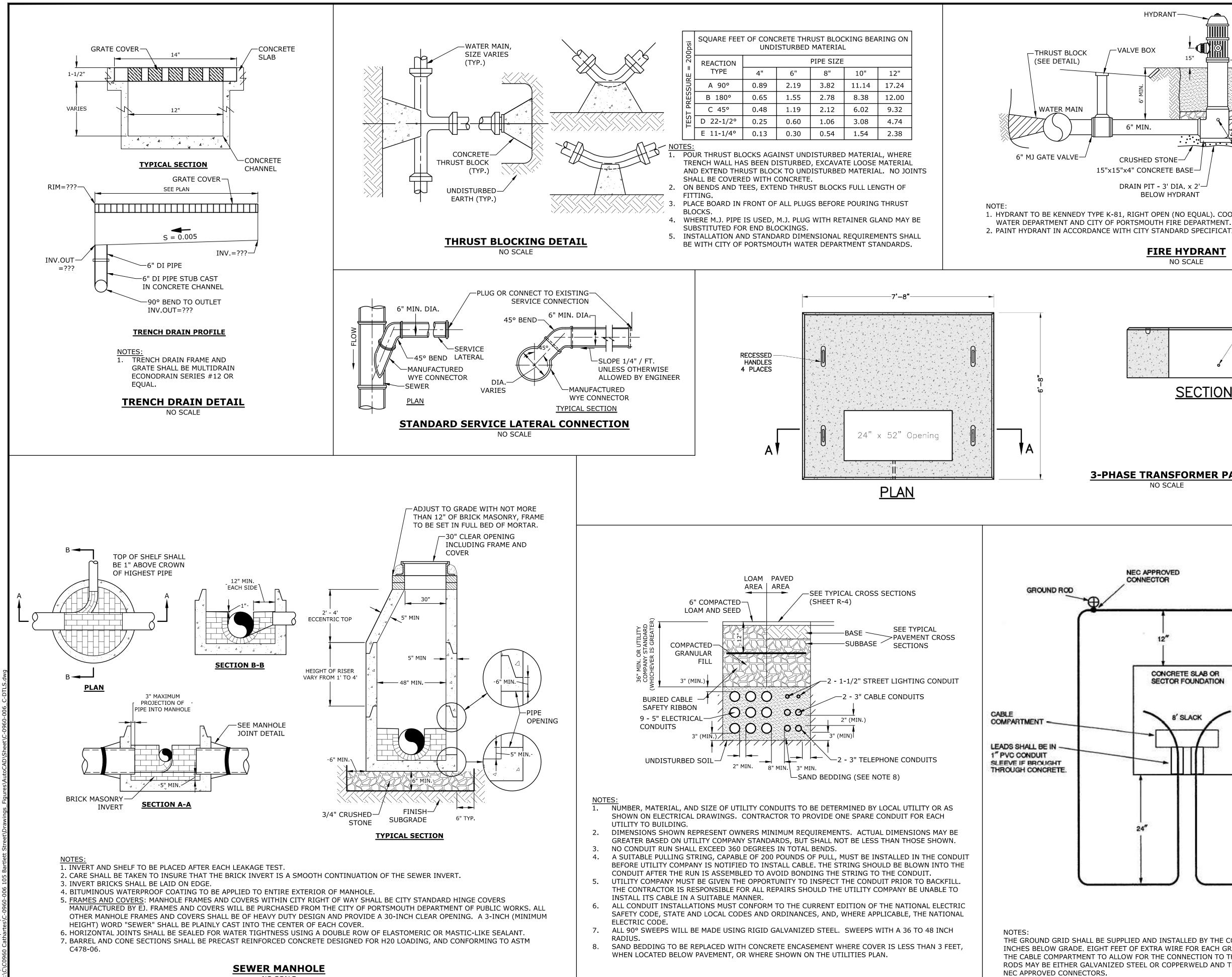
HEAVY

CONSTRUCTION

(75T AXLE LOAD)

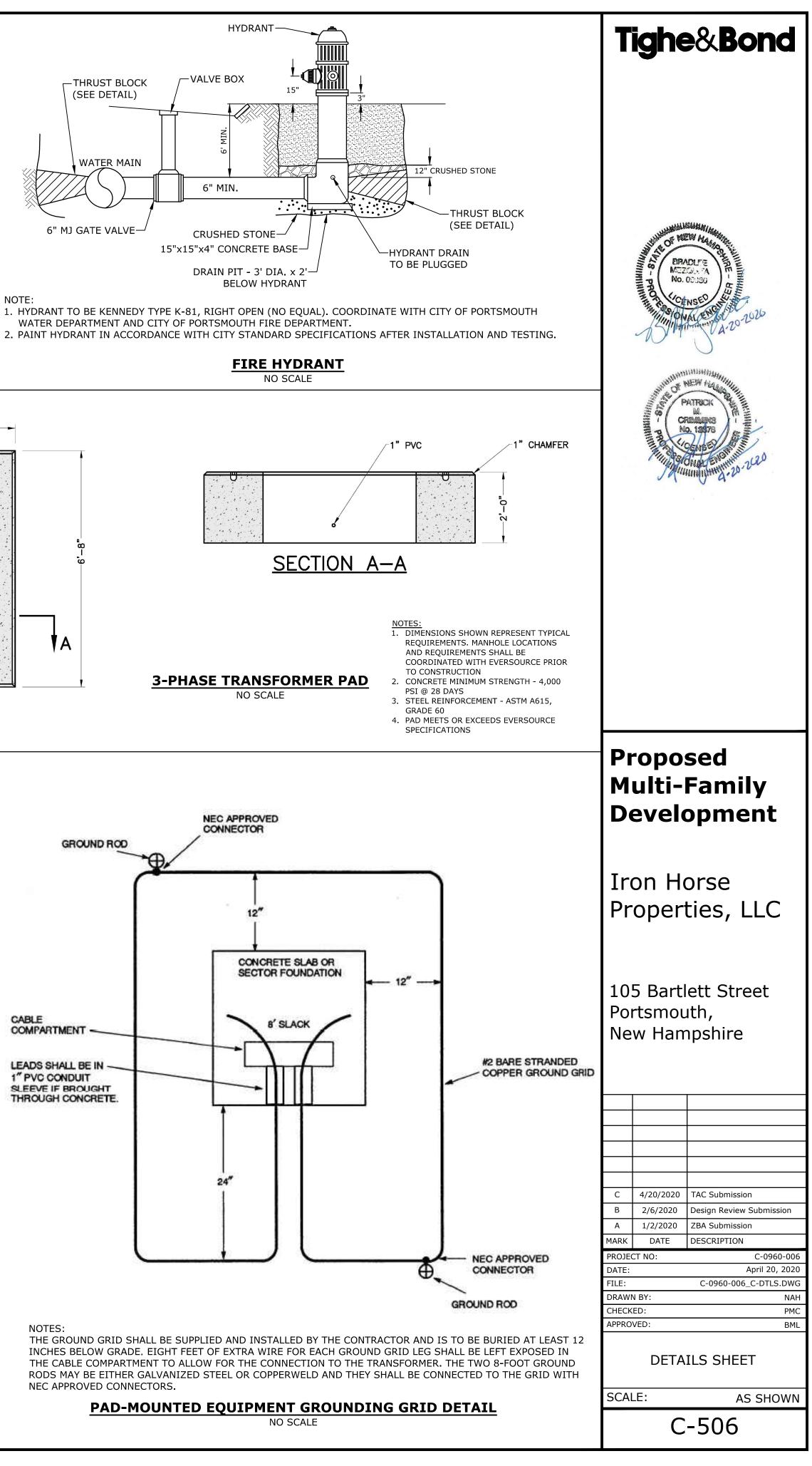
48"

60"

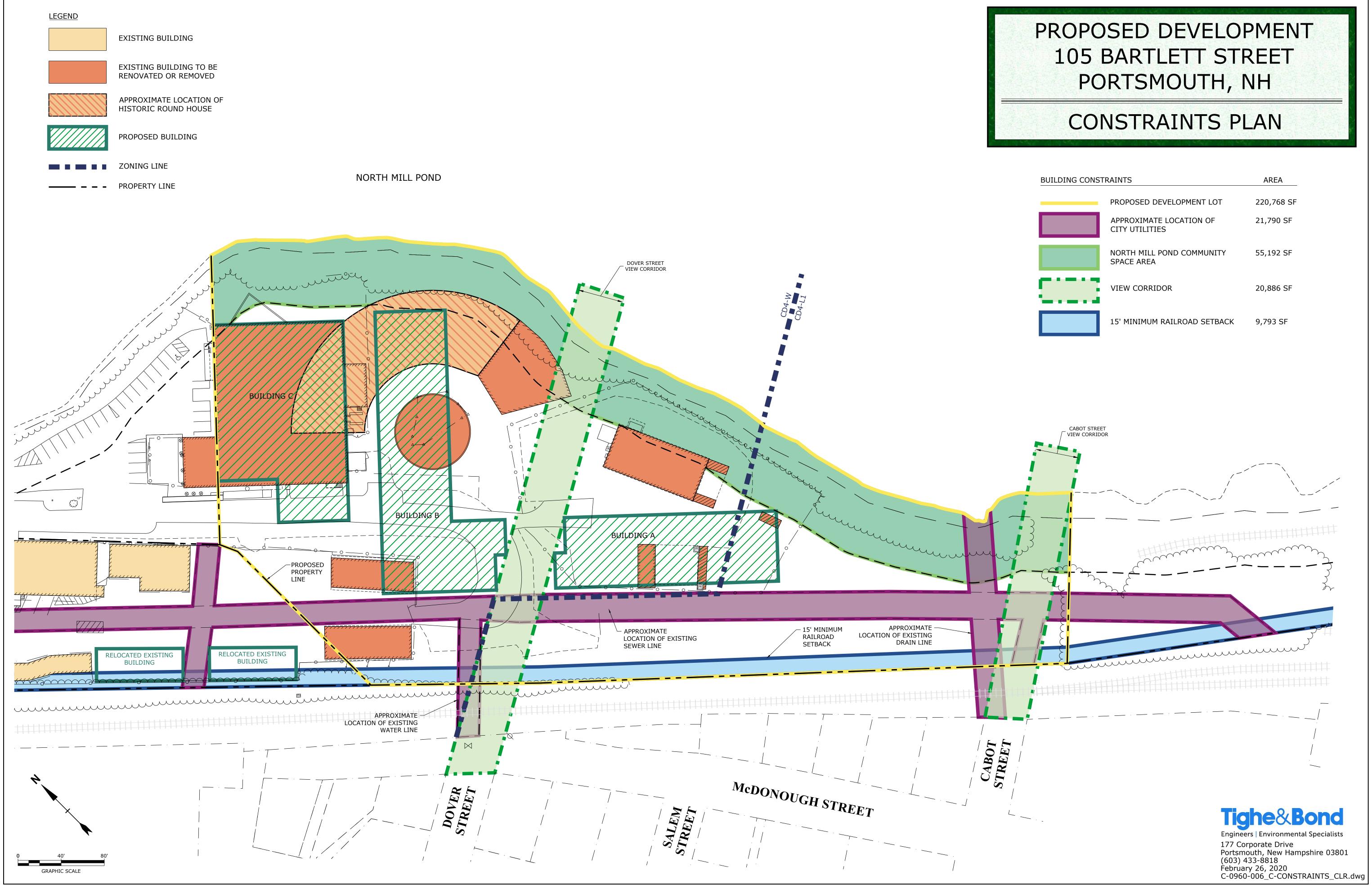


NO SCALE

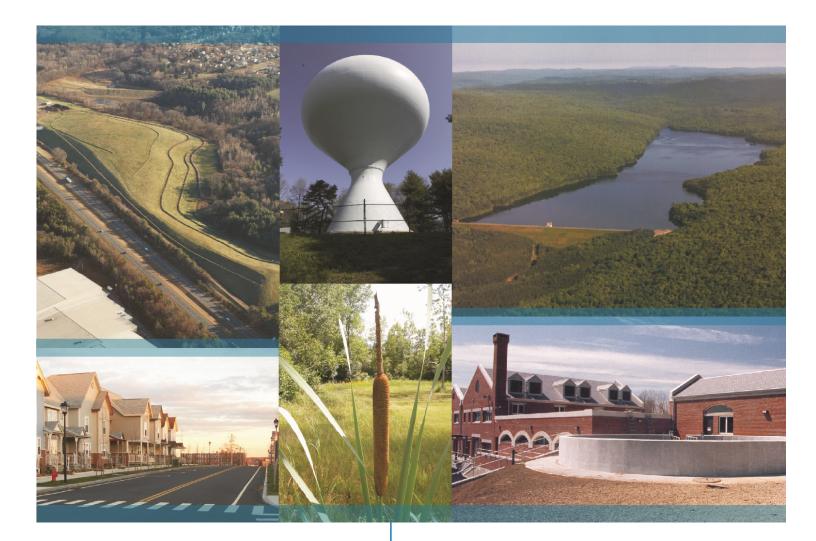
ELECTRICAL AND COMMUNICATION CONDUIT NO SCALE







| BUILDING CONS | AREA | |
|---------------|---|------------|
| | PROPOSED DEVELOPMENT LOT | 220,768 SF |
| | APPROXIMATE LOCATION OF CITY UTILITIES | 21,790 SF |
| | NORTH MILL POND COMMUNITY SPACE AREA | 55,192 SF |
| 21213 | VIEW CORRIDOR | 20,886 SF |
| | 15' MINIMUM RAILROAD SETBACK | 9,793 SF |







Proposed Multi-Family Development 105 Bartlett Street Portsmouth, NH

Drainage Analysis

Prepared For:

Iron Horse Properties LLC

April 20, 2020

Section 1 Project Description

The Bartlett Street property is comprised of a five (5) parcels that are bounded by Bartlett Street to the west and south, North Mill Pond to the north, and the Boston and Maine (B&M) railroad to the east. The existing parcels are listed below.

| Tax Map/Lot No. | Area (ac) |
|-----------------|-----------|
| 157 / 1 | 1.42 |
| 157 / 2 | 2.34 |
| 164 / 1 | 1.19 |
| 164 / 4-2 | 5.73 |
| R.O.W. | 1.60 |

Lot 157-1 is currently occupied by a 20,000-square-foot, 2-story, concrete block/steel frame building and associated parking lot. Lot 157-2 contains another 20,000-square foot wood frame structure with a variety of outbuildings, such as sheds, to support an impervious lumber yard. Lot 164-1 also contains an approximately 20,000-square foot wood frame structure that hosts a variety of smaller businesses and associated parking. Lot 164-4-2 is currently occupied by a former B&M railroad turntable, a dilapidated former brick roundhouse, and an approximately 3,600-square-foot machine shop. Several shipping containers are also present throughout the lot. The northern end of the lot is comprised mostly of abandoned rail lines that run down the shoreline between North Mill Pond and the active railroad past the northern limits of the project site

The proposed project includes the demolition and relocation of the structures on Lots 157-1, 157-2, and 164-4-2 and construction of two (2) multi-family apartment buildings with basement level parking, as well as one (1) mixed used building with first floor office spaces, amenity space, and upper story apartments In order to accommodate the footprints of the new structure and site improvements, the listed lots are proposed to be revised into five (5) new lots. The project will include associated site improvements that consist of a private road with cul-de-sac, parking, utilities, stormwater management and treatment, landscaping, lighting, and a greenway park. Additionally, the land from North Mill Pond's mean high water (MHW) line to the 50ft buffer will be deeded to the City of Portsmouth and designated as community space for the City's North Mill Pond Trail project. The City will be responsible for the future design and permitting of the North Mill Pond Trail project and any of its associated work within the 50ft buffer.

The pre-development and post-development watershed areas have been analyzed at two points of analysis. While the points of analysis remained unchanged, their contributing sub-catchment areas varied between pre-development and post-development conditions. These adjustments were made to reflect the differences in drainage patterns between the existing and proposed conditions. The overall area analyzed as part of this drainage analysis was held constant. For reference, PA-1 assesses flows that discharge directly to North Mill Pond via overland flow or various outlets, and PA-2 assesses flows that discharge to the 36" brick sewer which runs through the development area. In the post-development condition, however, all stormwater flows have been separated from the brick sewer and are discharged to North Mill Pond after being treated. PA-2 would have no stormwater flows in the post-development condition, therefore removing the need to assess it.

Furthermore, since North Mill Pond is a tidal water, NHDES does not require peak runoff control requirements to be met (Env-Wq 1507.06 (d)). For this reason, a comparison of peak runoff rates for the various storm events has not been provided.

1.1 Calculation Methods

The design storms analyzed in this study are the 2-year, 10-year, 25-year and 50-year 24-hour duration storm events. The stormwater modeling system, HydroCAD 10.0 was utilized to predict the peak runoff rates from these storm events. The peak discharge rates were determined by analyzing Type III 24-hour storm events. The rainfall data for these storm events was obtained from the data published by the Northeast Regional Climate Center at Cornell University, with an additional 15% added factor of safety as required by Env-Wq 1503.08(I).

The time of concentration was computed using the TR-55 Method, which provides a means of determining the time for an entire watershed to contribute runoff to a specific location via sheet flows, shallow concentrated flow and channel flow. Runoff curve numbers were calculated by estimating the coverage areas and then summing the curve number for the coverage area as a percent of the entire watershed.

References:

- 1. HydroCAD Stormwater Modeling System, by HydroCAD Software Solutions LLC, Chocorua, New Hampshire.
- New Hampshire Stormwater Management Manual, Volume 2, Post-Construction Best Management Practices Selection and Design, December 2008.
- "Extreme Precipitation in New York & New England." Extreme Precipitation in New York & New England by Northeast Regional Climate Center (NRCC), 26 June 2012.

Section 2 Pre-Development Conditions

In order to analyze the pre-development condition, the site has been divided into two (2) watershed areas modeled at two (2) points of analysis. These points of analysis and watersheds are depicted on the plans entitled "Pre-Development Watershed Plan", Sheets C-801.1 and C-801.2.

Each of the points of analysis and their contributing watershed areas are described below:

Point of Analysis (PA-1)

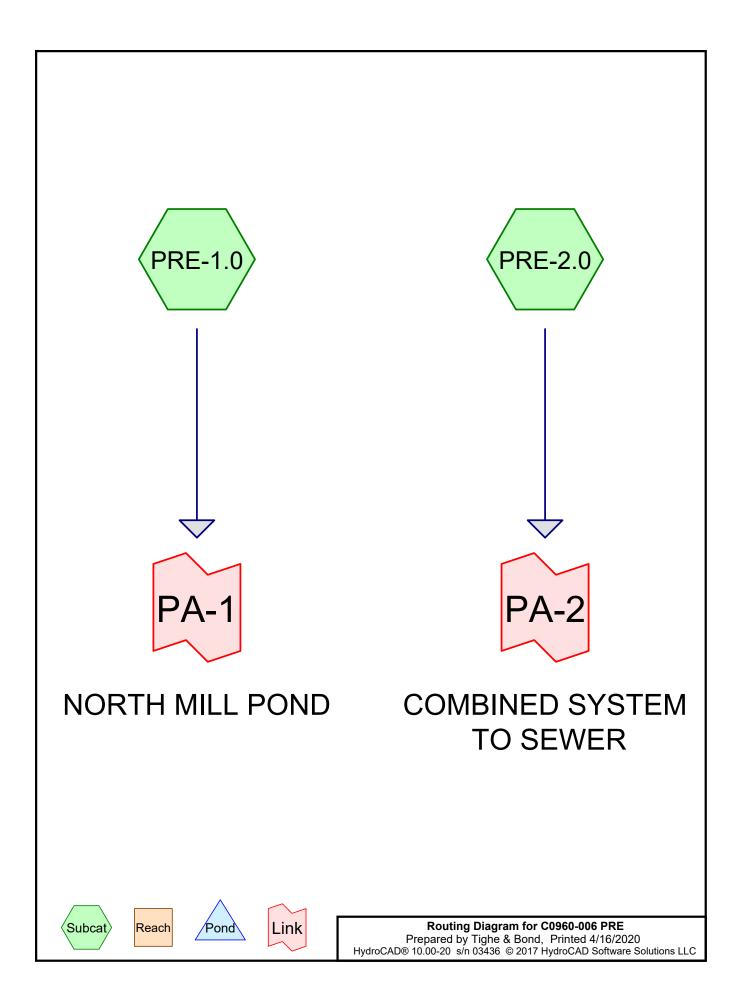
Pre-development Watershed 1.0 (PRE 1.0) is comprised of mostly impervious surfaces from paved parking and structures, as well as some disturbed forested areas to the northeast that surround a dilapidated roundhouse, former railroad turntable, and a machine shop. Banks along the shoreline of North Mill Pond consist of lawn, various species associated with disturbed sites, and rubble. Runoff from this watershed area travels via overland flow or underground drainage system to discharge into North Mill Pond. The runoff is currently untreated before discharge.

Point of Analysis (PA-2)

Pre-development Watershed 2.0 (PRE-2.0) is comprised mostly of impervious surfaces in the form of paved parking, roadway, and structures. Additional runoff comes from a grassy wooded strip that slopes down from the railroad to the east of the watershed. Runoff from this watershed drains to various catch basins that tie into an existing brick sewer.

2.2.1 Pre-Development Calculations

2.2.2 Pre-Development Watershed Plans



Area Listing (all nodes)

| Area | CN | Description |
|---------|----|--|
| (acres) | | (subcatchment-numbers) |
| 0.532 | 39 | >75% Grass cover, Good, HSG A (PRE-1.0) |
| 0.847 | 61 | >75% Grass cover, Good, HSG B (PRE-1.0, PRE-2.0) |
| 0.716 | 74 | >75% Grass cover, Good, HSG C (PRE-1.0, PRE-2.0) |
| 0.106 | 80 | >75% Grass cover, Good, HSG D (PRE-1.0) |
| 0.279 | 96 | Gravel surface (PRE-1.0, PRE-2.0) |
| 4.304 | 98 | Paved parking (PRE-1.0, PRE-2.0) |
| 1.689 | 98 | Roofs (PRE-1.0, PRE-2.0) |
| 0.550 | 30 | Woods, Good, HSG A (PRE-1.0) |
| 0.952 | 55 | Woods, Good, HSG B (PRE-1.0, PRE-2.0) |
| 0.908 | 70 | Woods, Good, HSG C (PRE-1.0, PRE-2.0) |
| 0.294 | 77 | Woods, Good, HSG D (PRE-1.0) |
| 11.177 | 81 | TOTAL AREA |

Soil Listing (all nodes)

| Area | Soil | Subcatchment |
|---------|-------|------------------|
| (acres) | Group | Numbers |
| 1.082 | HSG A | PRE-1.0 |
| 1.799 | HSG B | PRE-1.0, PRE-2.0 |
| 1.625 | HSG C | PRE-1.0, PRE-2.0 |
| 0.400 | HSG D | PRE-1.0 |
| 6.272 | Other | PRE-1.0, PRE-2.0 |
| 11.177 | | TOTAL AREA |

| C0960-006 PRE | Type III 24-hr 2-YR Rainfall=3.69" |
|--|---|
| Prepared by Tighe & Bond | Printed 4/16/2020 |
| HydroCAD® 10.00-20 s/n 03436 © 2017 Hy | vdroCAD Software Solutions LLC Page 4 |
| Runoff by SCS | .00-24.00 hrs, dt=0.05 hrs, 481 points TR-20 method, UH=SCS, Weighted-CN Ind method - Pond routing by Dyn-Stor-Ind method |
| SubcatchmentPRE-1.0: | Runoff Area=294,600 sf 45.84% Impervious Runoff Depth>1.57" Flow Length=461' Tc=12.7 min CN=77 Runoff=9.77 cfs 0.884 af |
| SubcatchmentPRE-2.0: | Runoff Area=192,280 sf 65.54% Impervious Runoff Depth>2.27" Flow Length=248' Tc=5.0 min CN=86 Runoff=11.70 cfs 0.833 af |
| Link PA-1: NORTH MILL POND | Inflow=9.77 cfs 0.884 af Primary=9.77 cfs 0.884 af |

Link PA-2: COMBINED SYSTEM TO SEWER

Inflow=11.70 cfs 0.833 af Primary=11.70 cfs 0.833 af

Total Runoff Area = 11.177 acRunoff Volume = 1.718 afAverage Runoff Depth = 1.84"46.38% Pervious = 5.184 ac53.62% Impervious = 5.993 ac

| C0960-006 PRE Prepared by Tighe & Bond HydroCAD® 10.00-20 s/n 03436 © 2017 H | | Type III 24-hr 10-YR Rainfall=5. Printed 4/16/20 _C Pag |)20 |
|---|---|---|-----|
| Runoff by SCS | 0.00-24.00 hrs, dt=0.05 hrs, 4 TR-20 method, UH=SCS, W -Ind method - Pond routing | /eighted-CN | |
| SubcatchmentPRE-1.0: | | 5.84% Impervious Runoff Depth>3.1 hin CN=77 Runoff=19.76 cfs 1.761 | |
| SubcatchmentPRE-2.0: | | 5.54% Impervious Runoff Depth>4.0 hin CN=86 Runoff=20.56 cfs 1.482 | |
| Link PA-1: NORTH MILL POND | | Inflow=19.76 cfs 1.761 | af |
| | | Primary=19.76 cfs 1.761 | af |
| Link PA-2: COMBINED SYSTEM TO SE | WER | Inflow=20.56 cfs 1.482 Primary=20.56 cfs 1.482 | |
| | | | |

Total Runoff Area = 11.177 acRunoff Volume = 3.243 afAverage Runoff Depth = 3.48"46.38% Pervious = 5.184 ac53.62% Impervious = 5.993 ac

Summary for Subcatchment PRE-1.0:

Runoff = 19.76 cfs @ 12.18 hrs, Volume= 1.761 af, Depth> 3.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=5.60"

| | Ar | ea (sf) | CN | Description | | | | |
|----|-----|---------|---------|-------------------------------|-------------|---------------------------------|--|--|
| | | 23,176 | 39 | >75% Grass cover, Good, HSG A | | | | |
| | : | 23,971 | 30 | Woods, Good, HSG A | | | | |
| | | 17,052 | 61 | >75% Grass cover, Good, HSG B | | | | |
| | | 12,344 | 55 | Woods, Go | od, HSG B | | | |
| * | 4 | 43,613 | 98 | Roofs | | | | |
| | | 18,324 | 74 | >75% Gras | s cover, Go | bod, HSG C | | |
| * | 9 | 91,423 | 98 | Paved park | ing | | | |
| | | 39,137 | 70 | Woods, Go | od, HSG C | | | |
| * | | 8,152 | 96 | Gravel surfa | ace | | | |
| | | 4,614 | 80 | >75% Gras | s cover, Go | bod, HSG D | | |
| | | 12,794 | 77 | Woods, Go | od, HSG D | | | |
| | 2 | 94,600 | 77 | Weighted A | verage | | | |
| | 1 | 59,564 | | 54.16% Pei | rvious Area | l | | |
| | 1 | 35,036 | | 45.84% Imp | pervious Ar | ea | | |
| | | | | | | | | |
| | Тс | Length | Slope | Velocity | Capacity | Description | | |
| (m | in) | (feet) | (ft/ft) | (ft/sec) | (cfs) | | | |
| (| 6.9 | 100 | 0.0400 | 0.24 | | Sheet Flow, | | |
| | | | | | | Grass: Short n= 0.150 P2= 3.69" | | |
| Ę | 5.8 | 361 | 0.0219 | 1.04 | | Shallow Concentrated Flow, | | |
| | | | | | | Short Grass Pasture Kv= 7.0 fps | | |
| 12 | 2.7 | 461 | Total | | | · | | |

Summary for Subcatchment PRE-2.0:

Runoff = 20.56 cfs @ 12.07 hrs, Volume= 1.482 af, Depth> 4.03"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=5.60"

| | Area (sf) | CN | Description |
|---|-----------|----|-------------------------------|
| | 19,840 | 61 | >75% Grass cover, Good, HSG B |
| | 29,125 | 55 | Woods, Good, HSG B |
| * | 29,959 | 98 | Roofs |
| | 12,871 | 74 | >75% Grass cover, Good, HSG C |
| * | 96,061 | 98 | Paved parking |
| | 432 | 70 | Woods, Good, HSG C |
| * | 3,992 | 96 | Gravel surface |
| | 192,280 | 86 | Weighted Average |
| | 66,260 | | 34.46% Pervious Area |
| | 126,020 | | 65.54% Impervious Area |

C0960-006 PRE

 Type III 24-hr
 10-YR Rainfall=5.60"

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 Page 7

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| | Tc (min) | Length | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|---|-------------|--------|------------------|----------------------|-------------------|---------------------------------|
| _ | | (feet) | (11/11) | (it/sec) | (CIS) | |
| | 2.1 | 48 | 0.1875 | 0.39 | | Sheet Flow, |
| | | | | | | Grass: Short n= 0.150 P2= 3.69" |
| | 1.6 | 200 | 0.0100 | 2.03 | | Shallow Concentrated Flow, |
| | | | | | | Paved Kv= 20.3 fps |
| _ | 3.7 | 248 | Total, I | ncreased t | o minimum | Tc = 5.0 min |

Summary for Link PA-1: NORTH MILL POND

| Inflow Area | a = | 6.763 ac, 45.84% Impervious, Inflow Depth > 3.13" for 10-YR eve | nt |
|-------------|-----|---|--------|
| Inflow | = | 19.76 cfs @ 12.18 hrs, Volume= 1.761 af | |
| Primary | = | 19.76 cfs @ 12.18 hrs, Volume= 1.761 af, Atten= 0%, Lag= 0 | .0 min |

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link PA-2: COMBINED SYSTEM TO SEWER

| Inflow Area = | | 4.414 ac, 65.54% Impervious, | Inflow Depth > 4.03" for 10-YR event |
|---------------|---|--------------------------------|--------------------------------------|
| Inflow | = | 20.56 cfs @ 12.07 hrs, Volume= | = 1.482 af |
| Primary | = | 20.56 cfs @ 12.07 hrs, Volume= | = 1.482 af, Atten= 0%, Lag= 0.0 min |

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

| C0960-006 PRE | Type III 24-hr 25-YR Rainfall=7.10" |
|---------------------------------------|---|
| Prepared by Tighe & Bond | Printed 4/16/2020 |
| HydroCAD® 10.00-20 s/n 03436 © 2017 H | lydroCAD Software Solutions LLC Page 8 |
| Runoff by SCS | 0.00-24.00 hrs, dt=0.05 hrs, 481 points 5 TR-20 method, UH=SCS, Weighted-CN -Ind method - Pond routing by Dyn-Stor-Ind method |
| SubcatchmentPRE-1.0: | Runoff Area=294,600 sf 45.84% Impervious Runoff Depth>4.45" Flow Length=461' Tc=12.7 min CN=77 Runoff=28.19 cfs 2.506 af |

Runoff Area=192,280 sf 65.54% Impervious Runoff Depth>5.46" Flow Length=248' Tc=5.0 min CN=86 Runoff=27.48 cfs 2.008 af

Link PA-1: NORTH MILL POND

SubcatchmentPRE-2.0:

Inflow=28.19 cfs 2.506 af Primary=28.19 cfs 2.506 af

Link PA-2: COMBINED SYSTEM TO SEWER

Inflow=27.48 cfs 2.008 af Primary=27.48 cfs 2.008 af

Total Runoff Area = 11.177 ac Runoff Volume = 4.514 af Average Runoff Depth = 4.85" 46.38% Pervious = 5.184 ac 53.62% Impervious = 5.993 ac

| C0960-006 PRE Prepared by Tighe & Bond <u>HydroCAD® 10.00-20 s/n 03436 © 2017 H</u> | | 50-YR Rainfall=8.50" Printed 4/16/2020 Page 9 |
|--|---|---|
| Runoff by SCS | 0.00-24.00 hrs, dt=0.05 hrs, 481 points TR-20 method, UH=SCS, Weighted-CN -Ind method - Pond routing by Dyn-Stor-Ir | id method |
| SubcatchmentPRE-1.0: | Runoff Area=294,600 sf 45.84% Impervio Flow Length=461' Tc=12.7 min CN=77 Ru | |
| SubcatchmentPRE-2.0: | Runoff Area=192,280 sf 65.54% Impervio Flow Length=248' Tc=5.0 min CN=86 Ru | • |
| Link PA-1: NORTH MILL POND | | nflow=36.08 cfs 3.226 af |
| | Pri | mary=36.08 cfs 3.226 af |
| Link PA-2: COMBINED SYSTEM TO SE | | nflow=33.90 cfs 2.506 af |
| | Pri | mary=33.90 cfs 2.506 af |

Total Runoff Area = 11.177 acRunoff Volume = 5.732 afAverage Runoff Depth = 6.15"46.38% Pervious = 5.184 ac53.62% Impervious = 5.993 ac

<u>LEGEND</u>



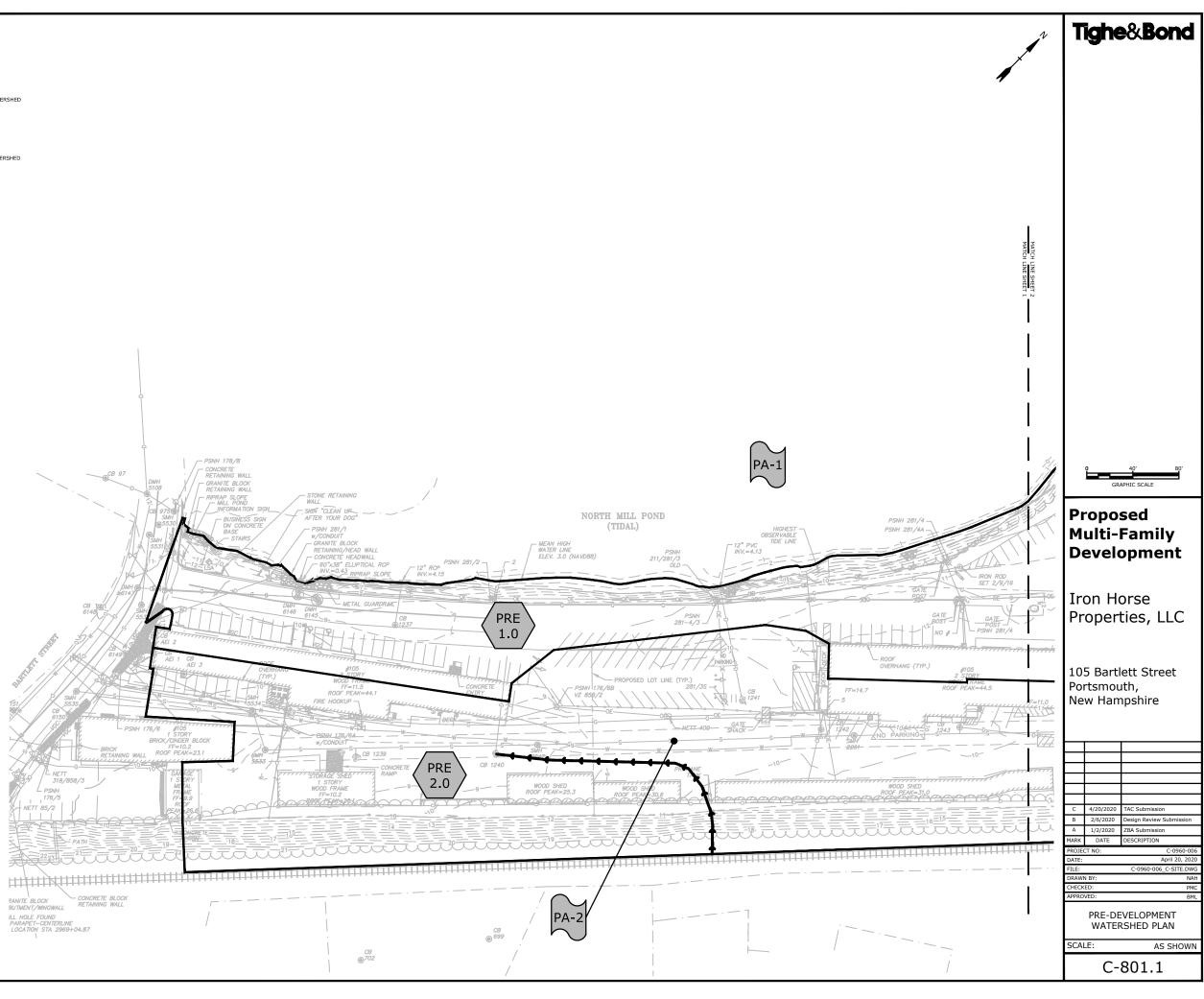


1.0

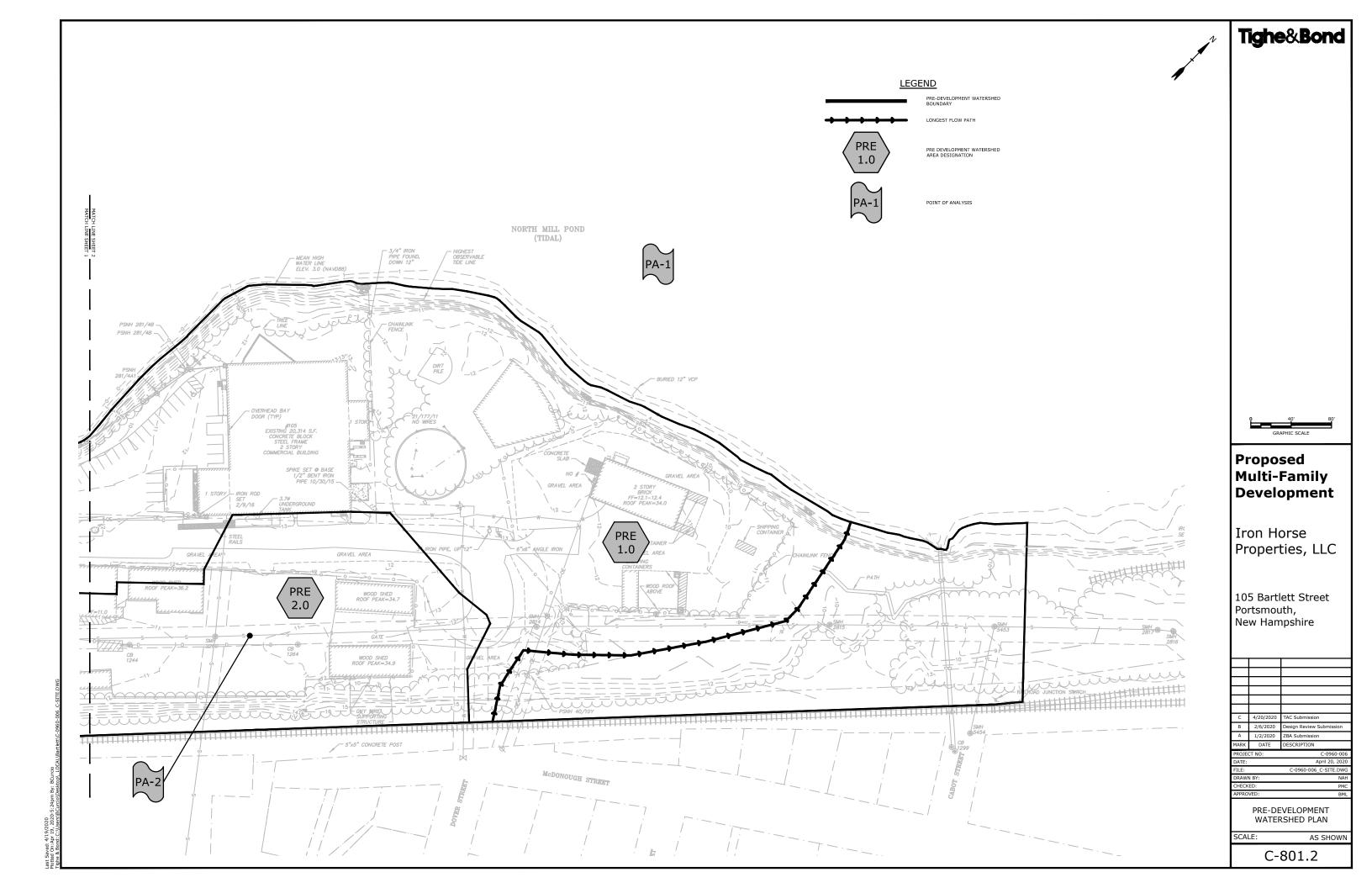
PA-1

PRE DEVELOPMENT WATERSHED AREA DESIGNATION

POINT OF ANALYSIS



Last Saved: 4/19/2020 Plotted On:Apr. 19, 2020 Date: 8 Donator 11, 2020 5:22pm By: BCurdo



Post-development Watershed 3.1 (POST-3.1) is comprised of proposed parking, pavement, and forested grassy cover from the slope up to the railroad. Runoff from this watershed area travels via overland flow to deep-sump catch basins that tie into the drainage system described in POST-3.0, where it is treated (by JF-3) and exits via an outfall at North Mill Pond (PA-1).

Post-development Watershed 4.0 (POST-4.0) collects the roof runoff from the third proposed building, paved parking, and some forested grassy slopes parallel to the existing railroad. Runoff from this watershed area travels via overland flow or roof leader to deepsump catch basins and a Contech Jellyfish Stormwater Filter (JF-4). Flows exiting the Jellyfish Filter discharge to North Mill Pond (PA-1). Similar to Post-Development Watershed 1.0 (POST-1.0), the pipe network is protected by a backflow preventer within the outlet invert of a manhole structure at the most downstream location. A concrete winged headwall and plunge pool provide erosion control and bank stability to the outfall.

Post-development Watershed 5.0 (POST-5.0) collects the runoff from the modified roadway and parking areas at the entrance of the site. Runoff from this watershed area travels via overland flow to deep-sump catch basins that tie into a structure (PDMH3) just upstream of the Jellyfish Filter described in POST 1.0 (JF-1). Runoff is treated and discharged through manhole structure equipped with a backflow preventer before exiting to North Mill Pond (PA-1). A concrete winged headwall and plunge pool provide erosion control and bank stability to the outfall.

Post-development Watershed 5.1 (POST-5.1) is comprised mostly of grassy and slightly forested areas along the shoreline of North Mill Pond. Runoff from this watershed simply sheets toward and discharges into North Mill Pond, as in the existing condition. There are no proposed impervious surfaces that are within this watershed area that would require treatment.

Post-development Watershed 5.0 (POST-5.2) collects the runoff from the proposed greenway park area behind the proposed buildings. A circular rain garden (RG-1), paired with an outlet control structure, has been designed to provide treatment for the runoff from this park area. The outlet control structure directs flows to a manhole structure downstream that combines flows with those of POST-4.0 before exiting to North Mill Pond.

Point of Analysis (PA-2)

All runoff previously draining to the existing brick sewer (PA-2) is proposed to be diverted to underground drainage that flows to North Mill Pond, effectively eliminating all flows to this point of analysis from this site.

3.3.1 Post-Development Calculations

3.3.2 Post-Development Watershed Plans

Section 3 Post-Development Conditions

The post-development condition was analyzed by dividing the watersheds into eight (8) watershed areas. Stormwater runoff from these sub-catchment areas flow via subsurface drainage systems prior to discharging to North Mill Pond. Unlike in the pre-development condition, flows from these sub-catchment areas are modeled at only one point of analysis at North Mill Pond (PA-1). As previously described, a large portion of the site has been designed to reroute runoff to North Mill Pond instead of the existing brick sewer. This moderately increases the watershed area flowing to PA-1 compared to the pre-development condition but eliminates the site's contribution to a combined sewer system. As per Env-Wq 1507.06 (d), the resulting increased peak flows from a larger effective watershed area are not of concern since North Mill Pond is a tidal water and the peak runoff control requirements do not apply. For this reason, a comparison of peak runoff rates for the various storm events has not been provided.

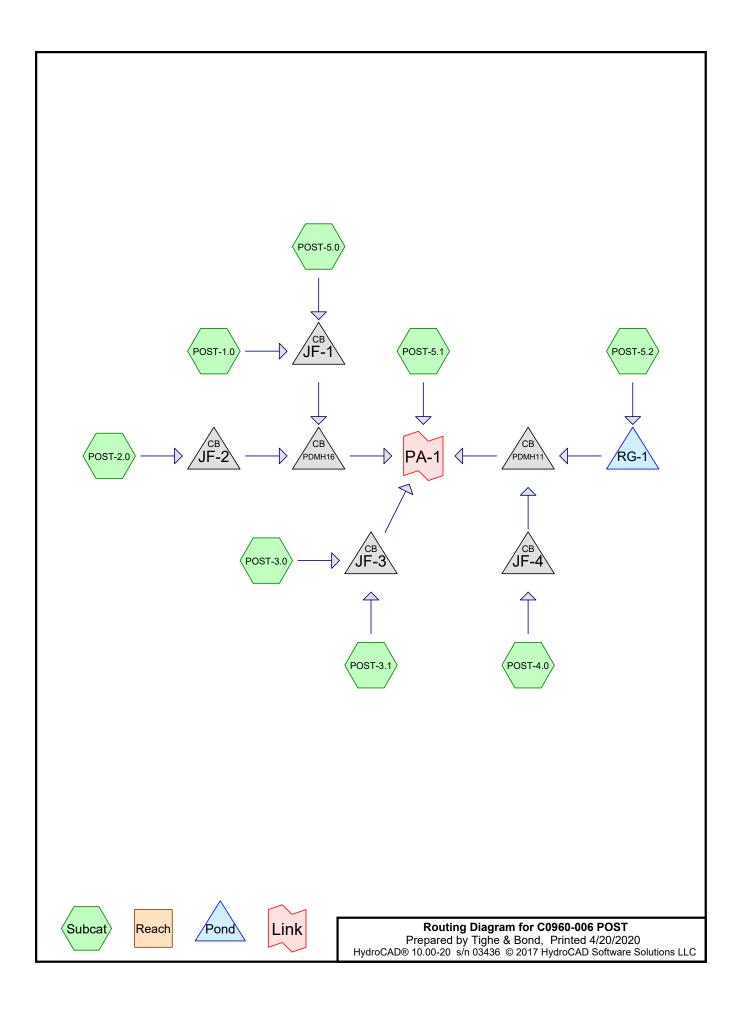
The point of analysis (PA-1) and its sub-catchment areas are depicted on the plans entitled "Post-Development Watershed Plan," Sheets C-802.1 and C-802.2. Each of the points of analysis and their contributing watershed areas are described below:

Point of Analysis (PA-1)

Post-development Watershed 1.0 (POST-1.0) is comprised of mostly existing impervious areas on the south end of the site behind some of the existing structures. Runoff from this watershed area travels via overland flow to the modified subsurface drainage system, where it is pretreated by deep-sump catch basins and treated by a Contech Jellyfish Stormwater Filter (JF-1). The Jellyfish units proposed are Contech's Peak Diversion model, which include an internal bypass that route flows greater than the design Water Quality Flow past the internal treatment system. Flows exiting the Jellyfish Filter discharge to a manhole structure outfitted with a backflow preventer within the outlet invert to protect the drainage system from tidal backflow and flooding. Flows then directly discharge to North Mill Pond (PA-1). through an outfall protected by a concrete winged headwall and plunge pool.

Post-development Watershed 2.0 (POST-2.0) is comprised mostly of asphalt roadway and concrete sidewalks in the center of the site. Runoff from this watershed area travels via overland flow to deep-sump catch basins and a Contech Jellyfish Stormwater Filter (JF-2). Flows exiting the Jellyfish Filter tie into a manhole structure that combines the flows with those of POST-1.0 before similarly exiting to North Mill Pond.

Post-development Watershed 3.0 (POST-3.0) primarily collects the roof runoff from two (2) of the proposed buildings, as well as some additional impervious cover below. Runoff from this watershed area travels via roof leaders or a trench drain to a Contech Jellyfish Stormwater Filter (JF-3). Flows exiting the Jellyfish Filter discharge to North Mill Pond (PA-1). Similar to Post-Development Watershed 1.0 (POST-1.0), the pipe network is protected by a backflow preventer within the outlet invert of a manhole structure at the most downstream location. A concrete winged headwall and plunge pool provide erosion control and bank stability to the outfall.



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Area Listing (all nodes)

| A | Area | CN | Description |
|-----|------|----|--|
| (ac | res) | | (subcatchment-numbers) |
| 0. | .517 | 39 | >75% Grass cover, Good, HSG A (POST-5.1, POST-5.2) |
| 0. | .769 | 61 | >75% Grass cover, Good, HSG B (POST-1.0, POST-3.1, POST-4.0, POST-5.1) |
| 1. | .623 | 74 | >75% Grass cover, Good, HSG C (POST-1.0, POST-2.0, POST-3.0, POST-3.1, |
| | | | POST-4.0, POST-5.0, POST-5.1, POST-5.2) |
| 0. | .108 | 80 | >75% Grass cover, Good, HSG D (POST-5.1) |
| 4. | .520 | 98 | Paved parking (POST-1.0, POST-2.0, POST-3.0, POST-3.1, POST-4.0, |
| | | | POST-5.0, POST-5.1, POST-5.2) |
| 2. | .174 | 98 | Roofs (POST-1.0, POST-2.0, POST-3.0, POST-3.1, POST-4.0, POST-5.0) |
| 0. | .479 | 30 | Woods, Good, HSG A (POST-5.1) |
| 0. | .667 | 55 | Woods, Good, HSG B (POST-1.0, POST-3.1, POST-4.0, POST-5.1) |
| 0. | .028 | 70 | Woods, Good, HSG C (POST-5.1, POST-5.2) |
| 0. | .292 | 77 | Woods, Good, HSG D (POST-5.1) |
| 11 | .177 | 83 | TOTAL AREA |

Soil Listing (all nodes)

| A | rea | Soil | Subcatchment |
|------|------|-------|---|
| (acı | res) | Group | Numbers |
| 0.9 | 997 | HSG A | POST-5.1, POST-5.2 |
| 1.4 | 436 | HSG B | POST-1.0, POST-3.1, POST-4.0, POST-5.1 |
| 1. | 651 | HSG C | POST-1.0, POST-2.0, POST-3.0, POST-3.1, POST-4.0, POST-5.0, POST-5.1, |
| | | | POST-5.2 |
| 0.4 | 400 | HSG D | POST-5.1 |
| 6. | 693 | Other | POST-1.0, POST-2.0, POST-3.0, POST-3.1, POST-4.0, POST-5.0, POST-5.1, |
| | | | POST-5.2 |
| 11. | 177 | | TOTAL AREA |

| C0960-006 POST | Τγμ |
|--|-----------------|
| Prepared by Tighe & Bond | |
| HydroCAD® 10.00-20 s/n 03436 © 2017 HydroCAD Softwar | e Solutions LLC |

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

| SubcatchmentPOST-1.0: | Runoff Area=129,041 sf 64.77% Impervious Runoff Depth>2.10" Tc=5.0 min CN=84 Runoff=7.30 cfs 0.518 af |
|-----------------------|---|
| SubcatchmentPOST-2.0: | Runoff Area=31,757 sf 85.86% Impervious Runoff Depth>3.12" Tc=5.0 min CN=95 Runoff=2.52 cfs 0.190 af |
| SubcatchmentPOST-3.0: | Runoff Area=49,909 sf 94.93% Impervious Runoff Depth>3.34" Tc=5.0 min CN=97 Runoff=4.09 cfs 0.319 af |
| SubcatchmentPOST-3.1: | Runoff Area=50,861 sf 65.22% Impervious Runoff Depth>2.27" Tc=5.0 min CN=86 Runoff=3.10 cfs 0.220 af |
| SubcatchmentPOST-4.0: | Runoff Area=57,392 sf 79.63% Impervious Runoff Depth>2.72" Tc=5.0 min CN=91 Runoff=4.13 cfs 0.299 af |
| SubcatchmentPOST-5.0: | Runoff Area=42,205 sf 92.18% Impervious Runoff Depth>3.23" Tc=5.0 min CN=96 Runoff=3.41 cfs 0.261 af |
| SubcatchmentPOST-5.1: | Runoff Area=93,820 sf 9.88% Impervious Runoff Depth>0.57" Tc=5.0 min CN=59 Runoff=1.02 cfs 0.103 af |
| SubcatchmentPOST-5.2: | Runoff Area=31,895 sf 19.69% Impervious Runoff Depth>1.71" Tc=5.0 min CN=79 Runoff=1.47 cfs 0.105 af |
| Pond JF-1: | Peak Elev=5.88' Inflow=10.74 cfs 0.779 af 24.0" Round Culvert n=0.013 L=4.0' S=0.0125 '/' Outflow=10.74 cfs 0.779 af |
| Pond JF-2: | Peak Elev=6.28' Inflow=2.52 cfs 0.190 af 15.0" Round Culvert n=0.013 L=50.0' S=0.0040 '/' Outflow=2.52 cfs 0.190 af |
| Pond JF-3: | Peak Elev=5.59' Inflow=7.20 cfs 0.540 af 18.0" Round Culvert n=0.013 L=55.0' S=0.0045 '/' Outflow=7.20 cfs 0.540 af |
| Pond JF-4: | Peak Elev=6.23' Inflow=4.13 cfs 0.299 af 15.0" Round Culvert n=0.013 L=23.0' S=0.0065 '/' Outflow=4.13 cfs 0.299 af |
| Pond PDMH11: | Peak Elev=5.81' Inflow=4.39 cfs 0.386 af 18.0" Round Culvert n=0.013 L=37.0' S=0.0054 '/' Outflow=4.39 cfs 0.386 af |
| Pond PDMH16: | Peak Elev=5.46' Inflow=13.25 cfs 0.969 af 24.0" Round Culvert n=0.013 L=11.0' S=0.0045 '/' Outflow=13.25 cfs 0.969 af |
| Pond RG-1: | Peak Elev=9.84' Storage=1,822 cf Inflow=1.47 cfs 0.105 af Outflow=0.26 cfs 0.088 af |
| Link PA-1: | Inflow=25.61 cfs 1.997 af Primary=25.61 cfs 1.997 af |

Total Runoff Area = 11.177 ac Runoff Volume = 2.014 af Average Runoff Depth = 2.16" 40.12% Pervious = 4.484 ac 59.88% Impervious = 6.693 ac

| C0960-006 POST | Туре |
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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

| SubcatchmentPOST-1.0: | Runoff Area=129,041 sf 64.77% Impervious Runoff Depth>3.82" Tc=5.0 min CN=84 Runoff=13.20 cfs 0.943 af |
|-----------------------|--|
| SubcatchmentPOST-2.0: | Runoff Area=31,757 sf 85.86% Impervious Runoff Depth>5.01" Tc=5.0 min CN=95 Runoff=3.93 cfs 0.304 af |
| SubcatchmentPOST-3.0: | Runoff Area=49,909 sf 94.93% Impervious Runoff Depth>5.24" Tc=5.0 min CN=97 Runoff=6.29 cfs 0.501 af |
| SubcatchmentPOST-3.1: | Runoff Area=50,861 sf 65.22% Impervious Runoff Depth>4.03" Tc=5.0 min CN=86 Runoff=5.44 cfs 0.392 af |
| SubcatchmentPOST-4.0: | Runoff Area=57,392 sf 79.63% Impervious Runoff Depth>4.56" Tc=5.0 min CN=91 Runoff=6.74 cfs 0.501 af |
| SubcatchmentPOST-5.0: | Runoff Area=42,205 sf 92.18% Impervious Runoff Depth>5.13" Tc=5.0 min CN=96 Runoff=5.28 cfs 0.414 af |
| SubcatchmentPOST-5.1: | Runoff Area=93,820 sf 9.88% Impervious Runoff Depth>1.59" Tc=5.0 min CN=59 Runoff=3.72 cfs 0.285 af |
| SubcatchmentPOST-5.2: | Runoff Area=31,895 sf 19.69% Impervious Runoff Depth>3.32" Tc=5.0 min CN=79 Runoff=2.85 cfs 0.203 af |
| Pond JF-1: | Peak Elev=7.83' Inflow=18.47 cfs 1.357 af 24.0" Round Culvert n=0.013 L=4.0' S=0.0125 '/' Outflow=18.47 cfs 1.357 af |
| Pond JF-2: | Peak Elev=6.86' Inflow=3.93 cfs 0.304 af 15.0" Round Culvert n=0.013 L=50.0' S=0.0040 '/' Outflow=3.93 cfs 0.304 af |
| Pond JF-3: | Peak Elev=6.80' Inflow=11.73 cfs 0.893 af 18.0" Round Culvert n=0.013 L=55.0' S=0.0045 '/' Outflow=11.73 cfs 0.893 af |
| Pond JF-4: | Peak Elev=7.43' Inflow=6.74 cfs 0.501 af 15.0" Round Culvert n=0.013 L=23.0' S=0.0065 '/' Outflow=6.74 cfs 0.501 af |
| Pond PDMH11: | Peak Elev=6.27' Inflow=7.00 cfs 0.687 af 18.0" Round Culvert n=0.013 L=37.0' S=0.0054 '/' Outflow=7.00 cfs 0.687 af |
| Pond PDMH16: | Peak Elev=6.55' Inflow=22.40 cfs 1.662 af 24.0" Round Culvert n=0.013 L=11.0' S=0.0045 '/' Outflow=22.40 cfs 1.662 af |
| Pond RG-1: | Peak Elev=10.43' Storage=2,763 cf Inflow=2.85 cfs 0.203 af Outflow=1.64 cfs 0.186 af |
| Link PA-1: | Inflow=44.77 cfs 3.526 af Primary=44.77 cfs 3.526 af |

Total Runoff Area = 11.177 ac Runoff Volume = 3.543 af Average Runoff Depth = 3.80" 40.12% Pervious = 4.484 ac 59.88% Impervious = 6.693 ac

Summary for Subcatchment POST-1.0:

Runoff = 13.20 cfs @ 12.07 hrs, Volume= 0.943 af, Depth> 3.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=5.60"

| | Area (sf) | CN | Description | | | | |
|---|--------------|------|-------------------------------|----------|---------------|--|--|
| | 15,556 | 61 | >75% Grass cover, Good, HSG B | | | | |
| | 24,181 | 55 | Woods, Good, HSG B | | | | |
| * | 27,528 | 98 | Roofs | | | | |
| | 5,719 | 74 | >75% Grass cover, Good, HSG C | | | | |
| * | 56,057 | 98 | Paved parking | | | | |
| | 129,041 | 84 | 84 Weighted Average | | | | |
| | 45,456 | | 35.23% Pervious Area | | | | |
| | 83,585 | | 64.77% Impervious Area | | | | |
| | | | | | | | |
| | Tc Length | | | Capacity | Description | | |
| | (min) (feet) | (ft/ | ft) (ft/sec) | (cfs) | | | |
| | 5.0 | | | | Direct Entry, | | |
| | | | | | | | |

Summary for Subcatchment POST-2.0:

| Runoff | = | 3.93 cfs @ | 12.07 hrs, | Volume= | 0.304 af, Depth> 5.01" |
|--------|---|------------|------------|---------|------------------------|
|--------|---|------------|------------|---------|------------------------|

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=5.60"

| | Area | (sf) | CN [| Description | | | | |
|---|---------|-------|---------|-------------------------------|----------|---------------|--|--|
| * | 6, | 843 | 98 F | Roofs | | | | |
| | 4,4 | 491 | 74 > | >75% Grass cover, Good, HSG C | | | | |
| * | 20,4 | 423 | 98 F | Paved parking | | | | |
| | | 0 | 70 \ | Woods, Good, HSG C | | | | |
| | 31, | 757 | 95 \ | Weighted Average | | | | |
| | 4,4 | 491 | | 14.14% Pervious Area | | | | |
| | 27, | 266 | 8 | 85.86% Impervious Area | | | | |
| | | | | | | | | |
| | | ngth | Slope | Velocity | Capacity | | | |
| | (min) (| feet) | (ft/ft) | (ft/sec) | (cfs) | | | |
| | 5.0 | | | | | Direct Entry, | | |

Summary for Subcatchment POST-3.0:

Runoff = 6.29 cfs @ 12.07 hrs, Volume= 0.501 af, Depth> 5.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=5.60"

C0960-006 POST

 Type III 24-hr
 10-YR Rainfall=5.60"

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| | Area (sf) | CN | Description | | | | | | |
|------|-----------|-------|------------------------|-------------------------------|---------------|--|--|--|--|
| | 0 | 61 | >75% Gras | s cover, Go | ood, HSG B | | | | |
| | 0 | 55 | Woods, Go | od, HSG B | | | | | |
| * | 39,803 | 98 | Roofs | | | | | | |
| | 2,532 | 74 | >75% Gras | >75% Grass cover, Good, HSG C | | | | | |
| * | 7,574 | 98 | Paved park | ing | | | | | |
| | 0 | 70 | Woods, Good, HSG C | | | | | | |
| | 49,909 | 97 | Weighted A | verage | | | | | |
| | 2,532 | | 5.07% Perv | ious Area | | | | | |
| | 47,377 | | 94.93% Impervious Area | | | | | | |
| | | | | | | | | | |
| Т | c Length | Slop | e Velocity | Capacity | Description | | | | |
| (min |) (feet) | (ft/f | t) (ft/sec) | (cfs) | | | | | |
| 5.0 |) | | | | Direct Entry, | | | | |

Summary for Subcatchment POST-3.1:

| Runoff | = | 5.44 cfs @ | 12.07 hrs, | Volume= | 0.392 af, D | 0epth> 4.03" |
|--------|---|------------|------------|---------|-------------|--------------|
|--------|---|------------|------------|---------|-------------|--------------|

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=5.60"

| | A | rea (sf) | CN | Description | | | | | |
|---|------------------------------------|----------|-------------|------------------------|-------------|-------------|--|--|--|
| | | 6,684 | 61 | >75% Gras | s cover, Go | bod, HSG B | | | |
| | | 4,601 | 55 | Woods, Go | od, HSG B | | | | |
| * | | 2,400 | 98 | Roofs | | | | | |
| | | 6,403 | 74 | >75% Gras | s cover, Go | bod, HSG C | | | |
| * | | 30,773 | 98 | Paved park | ling | | | | |
| | | 0 | 70 | Woods, Good, HSG C | | | | | |
| | | 50,861 | 86 | Weighted A | verage | | | | |
| | | 17,688 | | 34.78% Pervious Area | | | | | |
| | | 33,173 | | 65.22% Impervious Area | | | | | |
| | т. | 1 | <u>Olan</u> | - \/_l;tr. | 0 | Description | | | |
| | Tc | Length | Slop | , | | Description | | | |
| | (min) | (feet) | (ft/f | t) (ft/sec) | (cfs) | | | | |
| | 5.0 Direct Entry, | | | | | | | | |
| | Summary for Subcatchment POST-4.0: | | | | | | | | |

Runoff = 6.74 cfs @ 12.07 hrs, Volume= 0.501 af, Depth> 4.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=5.60"

C0960-006 POST

 Type III 24-hr
 10-YR Rainfall=5.60"

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| a (sf) (| CN | Description | | | | |
|----------|---|---|--|--|--|--|
| 1,166 | 61 | >75% Gras | s cover, Go | lood, HSG B | | |
| 159 | 55 | Woods, Goo | od, HSG B | 3 | | |
| 3,090 | 98 | Roofs | | | | |
| 363 | 74 | >75% Gras | s cover, Go | ood, HSG C | | |
| 2,614 | 98 | Paved park | ing | | | |
| 7,392 | 91 | Weighted A | verage | | | |
| 1,688 | | 20.37% Per | vious Area | а | | |
| 5,704 | | 79.63% Impervious Area | | | | |
| | | | | | | |
| _ength | | | Capacity | | | |
| (feet) | (ft/ft | (ft/sec) | (cfs) | | | |
| | | | | Direct Entry, | | |
| | 1,166 159 3,090 <u>363</u> 2,614 7,392 1,688 5,704 _ength | 1,166 61 159 55 3,090 98 363 74 <u>2,614 98</u> 7,392 91 1,688 5,704 Slope | 1,166 61 >75% Grass 159 55 Woods, Goods, Goods 3,090 98 Roofs 363 74 >75% Grass 2,614 98 Paved parki 7,392 91 Weighted A 1,688 20.37% Per 5,704 79.63% Imp _ength Slope Velocity | 1,166 61 >75% Grass cover, G 159 55 Woods, Good, HSG E 3,090 98 Roofs 363 74 >75% Grass cover, G 2,614 98 Paved parking 7,392 91 Weighted Average 1,688 20.37% Pervious Area 5,704 79.63% Impervious A _ength Slope Velocity | | |

Summary for Subcatchment POST-5.0:

Runoff = 5.28 cfs @ 12.07 hrs, Volume= 0.414 af, Depth> 5.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=5.60"

| _ | A | rea (sf) | CN | Description | | | | | |
|---|-------------|---------------------------|----------------|--|-------------------|---------------|--|--|--|
| * | | 5,020 | 98 | Roofs | | | | | |
| | | 3,301 | 74 | >75% Gras | s cover, Go | bod, HSG C | | | |
| * | | 33,884 | 98 | Paved park | Paved parking | | | | |
| | | 42,205 3,301 38,904 | 96 | Weighted A 7.82% Perv 92.18% Imp | vious Area | ea | | | |
| | Tc (min) | Length (feet) | Slop (ft/fl | , | Capacity (cfs) | Description | | | |
| _ | 5.0 | | | | | Direct Entry, | | | |

Summary for Subcatchment POST-5.1:

Runoff = 3.72 cfs @ 12.09 hrs, Volume= 0.285 af, Depth> 1.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=5.60"

C0960-006 POST

 Type III 24-hr
 10-YR Rainfall=5.60"

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| A | rea (sf) | CN | Description | | | | | | |
|-------|----------|-------|-----------------------|--------------------|---------------|--|--|--|--|
| | 22,473 | 39 | >75% Gras | s cover, Go | ood, HSG A | | | | |
| | 20,870 | 30 | Woods, Go | od, HSG A | | | | | |
| | 87 | 61 | >75% Gras | s cover, Go | ood, HSG B | | | | |
| | 135 | 55 | Woods, Go | Woods, Good, HSG B | | | | | |
| | 22,413 | 74 | >75% Gras | s cover, Go | ood, HSG C | | | | |
| * | 9,273 | 98 | Paved park | ing | | | | | |
| | 1,161 | 70 | Woods, Good, HSG C | | | | | | |
| | 4,696 | 80 | >75% Gras | s cover, Go | ood, HSG D | | | | |
| | 12,712 | 77 | Woods, Good, HSG D | | | | | | |
| | 93,820 | 59 | Weighted A | verage | | | | | |
| | 84,547 | | 90.12% Per | vious Area | l | | | | |
| | 9,273 | | 9.88% Impervious Area | | | | | | |
| - | | | | 0 | | | | | |
| Tc | Length | Slop | • | Capacity | Description | | | | |
| (min) | (feet) | (ft/f | t) (ft/sec) | (cfs) | | | | | |
| 5.0 | | | | | Direct Entry, | | | | |

Summary for Subcatchment POST-5.2:

| Runoff | = | 2.85 cfs @ | 12.08 hrs, Volume= | 0.203 af, Depth> 3.32" |
|--------|---|------------|--------------------|------------------------|
|--------|---|------------|--------------------|------------------------|

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=5.60"

| | A | rea (sf) | CN | Description | | | | | |
|---|-------|----------|---------|------------------------|-------------|---------------|--|--|--|
| | | 67 | 39 | >75% Gras | s cover, Go | ood, HSG A | | | |
| | | 25,479 | 74 | >75% Gras | s cover, Go | ood, HSG C | | | |
| * | | 6,279 | 98 | Paved parking | | | | | |
| | | 70 | 70 | Woods, Good, HSG C | | | | | |
| | | 31,895 | 79 | Weighted A | verage | | | | |
| | | 25,616 | | 80.31% Pervious Area | | | | | |
| | | 6,279 | | 19.69% Impervious Area | | | | | |
| | | | | | | | | | |
| | Тс | Length | Slope | e Velocity | Capacity | Description | | | |
| | (min) | (feet) | (ft/ft) | (ft/sec) | (cfs) | | | | |
| | 5.0 | | | | | Direct Entry, | | | |
| | | | | | | • · | | | |

Summary for Pond JF-1:

| Inflow Area = | 3.931 ac, 71.53% Impervious, Inflow | Depth > 4.14" for 10-YR event |
|---------------|-------------------------------------|-----------------------------------|
| Inflow = | 18.47 cfs @ 12.07 hrs, Volume= | 1.357 af |
| Outflow = | 18.47 cfs @ 12.07 hrs, Volume= | 1.357 af, Atten= 0%, Lag= 0.0 min |
| Primary = | 18.47 cfs @ 12.07 hrs, Volume= | 1.357 af |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 7.83' @ 12.10 hrs Flood Elev= 10.80' C0960-006 POST Type III 24-hr 10-YR Rainfall=5.60" Prepared by Tighe & Bond Printed 4/20/2020 HydroCAD® 10.00-20 s/n 03436 © 2017 HydroCAD Software Solutions LLC Page 12 Device Routing Invert Outlet Devices Primary #1 3.45' **24.0" Round Culvert** L= 4.0' Ke= 0.500 Inlet / Outlet Invert= 3.45' / 3.40' S= 0.0125 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf Summary for Pond JF-2:

| Inflow Area = | 0.729 ac, 85.86% Impervious, Inflow D | epth > 5.01" for 10-YR event |
|---------------|---------------------------------------|-----------------------------------|
| Inflow = | 3.93 cfs @ 12.07 hrs, Volume= | 0.304 af |
| Outflow = | 3.93 cfs @ 12.07 hrs, Volume= | 0.304 af, Atten= 0%, Lag= 0.0 min |
| Primary = | 3.93 cfs @ 12.07 hrs, Volume= | 0.304 af |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 6.86' @ 12.10 hrs Flood Elev= 10.25'

| Device | Routing | Invert | Outlet Devices |
|--------|---------|--------|--|
| #1 | Primary | 5.30' | 15.0" Round Culvert L= 50.0' Ke= 0.500 |
| | | | Inlet / Outlet Invert= 5.30' / 5.10' S= 0.0040 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf |

Primary OutFlow Max=2.89 cfs @ 12.07 hrs HW=6.71' TW=6.45' (Dynamic Tailwater) -1=Culvert (Outlet Controls 2.89 cfs @ 2.62 fps)

Summary for Pond JF-3:

| Inflow Area | = | 2.313 ac, 79.93% Impervious, Inflow Depth > 4.63" for 10-YR e | event |
|-------------|---|---|-----------|
| Inflow | = | 11.73 cfs @ 12.07 hrs, Volume= 0.893 af | |
| Outflow | = | 11.73 cfs @ 12.07 hrs, Volume= 0.893 af, Atten= 0%, Lag | = 0.0 min |
| Primary | = | 11.73 cfs @ 12.07 hrs, Volume= 0.893 af | |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 6.80' @ 12.07 hrs Flood Elev= 13.00'

| Device | Routing | Invert | Outlet Devices | |
|--------|---------|--------|--|--|
| #1 | Primary | 3.85' | 18.0" Round Culvert L= 55.0' Ke= 0.500 | |
| | | | Inlet / Outlet Invert= 3.85' / 3.60' S= 0.0045 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf | |

Primary OutFlow Max=11.29 cfs @ 12.07 hrs HW=6.69' TW=0.00' (Dynamic Tailwater) -1=Culvert (Barrel Controls 11.29 cfs @ 6.39 fps)

Summary for Pond JF-4:

| Inflow A Inflow Outflow Primary | = = | 6.74 cfs @ 12 6.74 cfs @ 12 | 63% Impervious, Inflow Depth > 4.56" for 10-YR event 2.07 hrs, Volume= 0.501 af 2.07 hrs, Volume= 0.501 af, Atten= 0%, Lag= 0.0 min 2.07 hrs, Volume= 0.501 af |
|--|---------|--------------------------------|---|
| Peak Ele | | @ 12.09 hrs | Time Span= 0.00-24.00 hrs, dt= 0.05 hrs |
| Device | Routing | Invert | Outlet Devices |
| #1 | Primary | 4.85' | 15.0" Round Culvert L= 23.0' Ke= 0.500 Inlet / Outlet Invert= 4.85' / 4.70' S= 0.0065 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf |

Primary OutFlow Max=5.99 cfs @ 12.07 hrs HW=7.25' TW=6.22' (Dynamic Tailwater) -1=Culvert (Inlet Controls 5.99 cfs @ 4.88 fps)

Summary for Pond PDMH11:

| Inflow Area = | 2.050 ac, 58.22% Impervious, Inflow D | epth > 4.02" for 10-YR event |
|---------------|---------------------------------------|-----------------------------------|
| Inflow = | 7.00 cfs @ 12.07 hrs, Volume= | 0.687 af |
| Outflow = | 7.00 cfs @ 12.07 hrs, Volume= | 0.687 af, Atten= 0%, Lag= 0.0 min |
| Primary = | 7.00 cfs @ 12.07 hrs, Volume= | 0.687 af |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 6.27' @ 12.07 hrs Flood Elev= 10.90'

| Device | Routing | Invert | Outlet Devices |
|--------|---------|--------|---|
| #1 | Primary | 4.60' | 18.0" Round Culvert L= 37.0' Ke= 0.500 Inlet / Outlet Invert= 4.60' / 4.40' S= 0.0054 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf |

Primary OutFlow Max=6.75 cfs @ 12.07 hrs HW=6.22' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 6.75 cfs @ 4.39 fps)

Summary for Pond PDMH16:

| Inflow Are | a = | 4.660 ac, 73.77% Impervious, Inflow Depth > 4.28" for 10-YR event |
|------------|-----|---|
| Inflow | = | 22.40 cfs @ 12.07 hrs, Volume= 1.662 af |
| Outflow | = | 22.40 cfs @ 12.07 hrs, Volume= 1.662 af, Atten= 0%, Lag= 0.0 min |
| Primary | = | 22.40 cfs @ 12.07 hrs, Volume= 1.662 af |
| - | | - |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 6.55' @ 12.07 hrs Flood Elev= 10.50'

| Device | Routing | Invert | Outlet Devices |
|--------|---------|--------|--|
| #1 | Primary | 3.30' | 24.0" Round Culvert L= 11.0' Ke= 0.500 |

Inlet / Outlet Invert= 3.30' / 3.25' S= 0.0045 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Primary OutFlow Max=21.57 cfs @ 12.07 hrs HW=6.45' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 21.57 cfs @ 6.87 fps)

Summary for Pond RG-1:

| Inflow Area = | 0.732 ac, 19.69% Impervious, Inflow | Depth > 3.32" for 10-YR event | |
|---------------|-------------------------------------|-----------------------------------|----|
| Inflow = | 2.85 cfs @ 12.08 hrs, Volume= | 0.203 af | |
| Outflow = | 1.64 cfs @ 12.21 hrs, Volume= | 0.186 af, Atten= 42%, Lag= 8.3 mi | in |
| Primary = | 1.64 cfs @ 12.21 hrs, Volume= | 0.186 af | |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 10.43' @ 12.22 hrs Surf.Area= 1,712 sf Storage= 2,763 cf Flood Elev= 11.00' Surf.Area= 1,964 sf Storage= 3,807 cf

Plug-Flow detention time= 106.1 min calculated for 0.185 af (91% of inflow) Center-of-Mass det. time= 64.7 min (882.9 - 818.2)

| Volume | Inve | rt Ava | il.Stora | ge Storage Desc | ription | |
|----------|----------|-----------|----------------|-----------------------------------|-------------------|---------------------------------|
| #1 | 6.25 | 5' | 3,807 | cf Custom Stag | e Data (Prismatio | c) Listed below (Recalc) |
| | | | \/aida | In a Starra | Curra Starra | |
| Elevatio | | Surf.Area | Voids | | Cum.Store | |
| (fee | et) | (sq-ft) | (%) | (cubic-feet) | (cubic-feet) | |
| 6.2 | 25 | 1,134 | 0.0 | 0 | 0 | |
| 7.5 | 50 | 1,134 | 40.0 | 567 | 567 | |
| 9.0 | 00 | 1,134 | 10.0 | 170 | 737 | |
| 10.0 | 00 | 1,521 | 100.0 | 1,328 | 2,065 | |
| 11.0 | 00 | 1,964 | 100.0 | 1,743 | 3,807 | |
| | | | | | | |
| Device | Routing | In | vert (| Dutlet Devices | | |
| #1 | Primary | 6 | 5.10' * | 2.0" Round Culv | ert | |
| | - | | l | _= 238.0' CPP, sq | uare edge headw | all. Ke= 0.500 |
| | | | | | | 0.0046 '/' Cc= 0.900 |
| | | | | | | erior, Flow Area= 0.79 sf |
| #0 | Davias 1 | 10 | | 1- 0.013 Condgate | | |
| #2 | Device 1 | IC IC | - | | | - 0.000 |
| | | | | Limited to weir flow at low heads | | |
| #3 | Device 1 | - | | 5.0" Vert. UD C= | | |
| #4 | Device 3 | ç |).00' (|).26 cfs Exfiltratio | n when above 9. | 00' |
| | | | | | | |

Primary OutFlow Max=1.58 cfs @ 12.21 hrs HW=10.43' TW=5.89' (Dynamic Tailwater)

-1=Culvert (Passes 1.58 cfs of 4.43 cfs potential flow)

2=Orifice/Grate (Weir Controls 1.32 cfs @ 1.37 fps)

-3=UD (Passes 0.26 cfs of 1.87 cfs potential flow)

4=Exfiltration (Exfiltration Controls 0.26 cfs)

Summary for Link PA-1:

| Inflow Are | a = | 11.177 ac, 59.88% Impervious, Inflow Depth > 3.79" for 10-YR event |
|------------|-----|--|
| Inflow | = | 44.77 cfs @ 12.07 hrs, Volume= 3.526 af |
| Primary | = | 44.77 cfs @ 12.07 hrs, Volume= 3.526 af, Atten= 0%, Lag= 0.0 min |

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

| C0960-006 POST | Туре |
|--|-------------|
| Prepared by Tighe & Bond | |
| HydroCAD® 10.00-20 s/n 03436 © 2017 HydroCAD Software So | lutions LLC |

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

| SubcatchmentPOST-1.0: | Runoff Area=129,041 sf 64.77% Impervious Runoff Depth>5.23" Tc=5.0 min CN=84 Runoff=17.85 cfs 1.292 af |
|-----------------------|--|
| SubcatchmentPOST-2.0: | Runoff Area=31,757 sf 85.86% Impervious Runoff Depth>6.50" Tc=5.0 min CN=95 Runoff=5.04 cfs 0.395 af |
| SubcatchmentPOST-3.0: | Runoff Area=49,909 sf 94.93% Impervious Runoff Depth>6.74" Tc=5.0 min CN=97 Runoff=8.00 cfs 0.643 af |
| SubcatchmentPOST-3.1: | Runoff Area=50,861 sf 65.22% Impervious Runoff Depth>5.46" Tc=5.0 min CN=86 Runoff=7.27 cfs 0.531 af |
| SubcatchmentPOST-4.0: | Runoff Area=57,392 sf 79.63% Impervious Runoff Depth>6.03" Tc=5.0 min CN=91 Runoff=8.77 cfs 0.663 af |
| SubcatchmentPOST-5.0: | Runoff Area=42,205 sf 92.18% Impervious Runoff Depth>6.62" Tc=5.0 min CN=96 Runoff=6.73 cfs 0.535 af |
| SubcatchmentPOST-5.1: | Runoff Area=93,820 sf 9.88% Impervious Runoff Depth>2.57" Tc=5.0 min CN=59 Runoff=6.32 cfs 0.462 af |
| SubcatchmentPOST-5.2: | Runoff Area=31,895 sf 19.69% Impervious Runoff Depth>4.67" Tc=5.0 min CN=79 Runoff=4.00 cfs 0.285 af |
| Pond JF-1: | Peak Elev=10.32' Inflow=24.58 cfs 1.826 af 24.0" Round Culvert n=0.013 L=4.0' S=0.0125 '/' Outflow=24.58 cfs 1.826 af |
| Pond JF-2: | Peak Elev=8.64' Inflow=5.04 cfs 0.395 af 15.0" Round Culvert n=0.013 L=50.0' S=0.0040 '/' Outflow=5.04 cfs 0.395 af |
| Pond JF-3: | Peak Elev=7.98' Inflow=15.27 cfs 1.175 af 18.0" Round Culvert n=0.013 L=55.0' S=0.0045 '/' Outflow=15.27 cfs 1.175 af |
| Pond JF-4: | Peak Elev=8.86' Inflow=8.77 cfs 0.663 af 15.0" Round Culvert n=0.013 L=23.0' S=0.0065 '/' Outflow=8.77 cfs 0.663 af |
| Pond PDMH11: | Peak Elev=7.34' Inflow=11.53 cfs 0.931 af 18.0" Round Culvert n=0.013 L=37.0' S=0.0054 '/' Outflow=11.53 cfs 0.931 af |
| Pond PDMH16: | Peak Elev=8.07' Inflow=29.61 cfs 2.221 af 24.0" Round Culvert n=0.013 L=11.0' S=0.0045 '/' Outflow=29.61 cfs 2.221 af |
| Pond RG-1: | Peak Elev=10.56' Storage=2,990 cf Inflow=4.00 cfs 0.285 af Outflow=3.38 cfs 0.268 af |
| Link PA-1: | Inflow=61.87 cfs 4.788 af Primary=61.87 cfs 4.788 af |

Total Runoff Area = 11.177 ac Runoff Volume = 4.805 af Average Runoff Depth = 5.16" 40.12% Pervious = 4.484 ac 59.88% Impervious = 6.693 ac

| C0960-006 POST | Туре |
|---|------|
| Prepared by Tighe & Bond | |
| HydroCAD® 10.00-20 s/n 03436 © 2017 HydroCAD Software Solutions | LLC |

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

| SubcatchmentPOST-1.0: | Runoff Area=129,041 sf 64.77% Impervious Runoff Depth>6.57" Tc=5.0 min CN=84 Runoff=22.17 cfs 1.623 af |
|-----------------------|--|
| SubcatchmentPOST-2.0: | Runoff Area=31,757 sf 85.86% Impervious Runoff Depth>7.90" Tc=5.0 min CN=95 Runoff=6.06 cfs 0.480 af |
| SubcatchmentPOST-3.0: | Runoff Area=49,909 sf 94.93% Impervious Runoff Depth>8.14" Tc=5.0 min CN=97 Runoff=9.60 cfs 0.777 af |
| SubcatchmentPOST-3.1: | Runoff Area=50,861 sf 65.22% Impervious Runoff Depth>6.81" Tc=5.0 min CN=86 Runoff=8.97 cfs 0.663 af |
| SubcatchmentPOST-4.0: | Runoff Area=57,392 sf 79.63% Impervious Runoff Depth>7.41" Tc=5.0 min CN=91 Runoff=10.66 cfs 0.814 af |
| SubcatchmentPOST-5.0: | Runoff Area=42,205 sf 92.18% Impervious Runoff Depth>8.02" Tc=5.0 min CN=96 Runoff=8.09 cfs 0.647 af |
| SubcatchmentPOST-5.1: | Runoff Area=93,820 sf 9.88% Impervious Runoff Depth>3.59" Tc=5.0 min CN=59 Runoff=8.98 cfs 0.645 af |
| SubcatchmentPOST-5.2: | Runoff Area=31,895 sf 19.69% Impervious Runoff Depth>5.97" Tc=5.0 min CN=79 Runoff=5.08 cfs 0.364 af |
| Pond JF-1: | Peak Elev=13.39' Inflow=30.26 cfs 2.270 af 24.0" Round Culvert n=0.013 L=4.0' S=0.0125 '/' Outflow=30.26 cfs 2.270 af |
| Pond JF-2: | Peak Elev=10.83' Inflow=6.06 cfs 0.480 af 15.0" Round Culvert n=0.013 L=50.0' S=0.0040 '/' Outflow=6.06 cfs 0.480 af |
| Pond JF-3: | Peak Elev=9.35' Inflow=18.57 cfs 1.440 af 18.0" Round Culvert n=0.013 L=55.0' S=0.0045 '/' Outflow=18.57 cfs 1.440 af |
| Pond JF-4: | Peak Elev=11.02' Inflow=10.66 cfs 0.814 af 15.0" Round Culvert n=0.013 L=23.0' S=0.0065 '/' Outflow=10.66 cfs 0.814 af |
| Pond PDMH11: | Peak Elev=8.18' Inflow=14.52 cfs 1.161 af 18.0" Round Culvert n=0.013 L=37.0' S=0.0054 '/' Outflow=14.52 cfs 1.161 af |
| Pond PDMH16: | Peak Elev=10.01' Inflow=36.32 cfs 2.749 af 24.0" Round Culvert n=0.013 L=11.0' S=0.0045 '/' Outflow=36.32 cfs 2.749 af |
| Pond RG-1: | Peak Elev=10.79' Storage=3,397 cf Inflow=5.08 cfs 0.364 af Outflow=3.96 cfs 0.347 af |
| Link PA-1: | Inflow=78.36 cfs 5.996 af Primary=78.36 cfs 5.996 af |

Total Runoff Area = 11.177 ac Runoff Volume = 6.013 af Average Runoff Depth = 6.46" 40.12% Pervious = 4.484 ac 59.88% Impervious = 6.693 ac

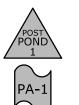
LEGEND

POST-DEVELOPMENT WATERSHED BOUNDARY

LONGEST FLOW PATH

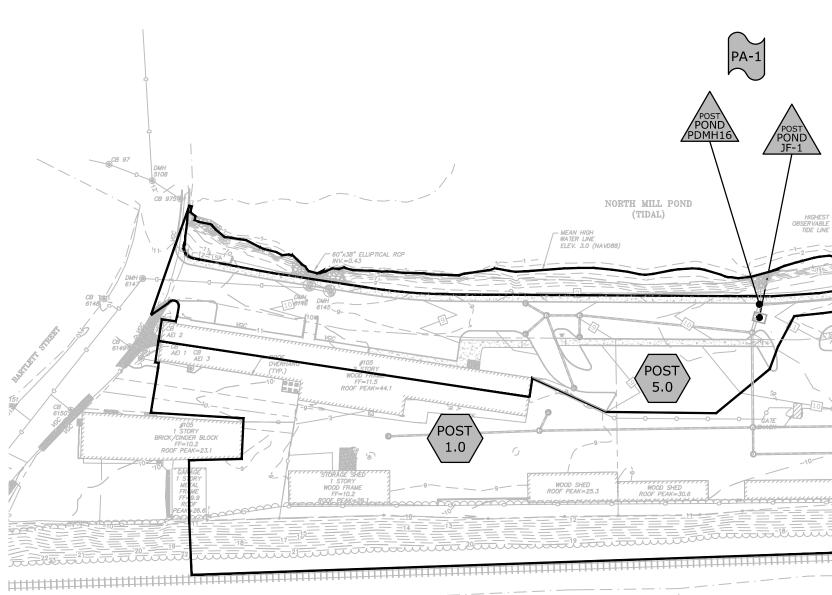


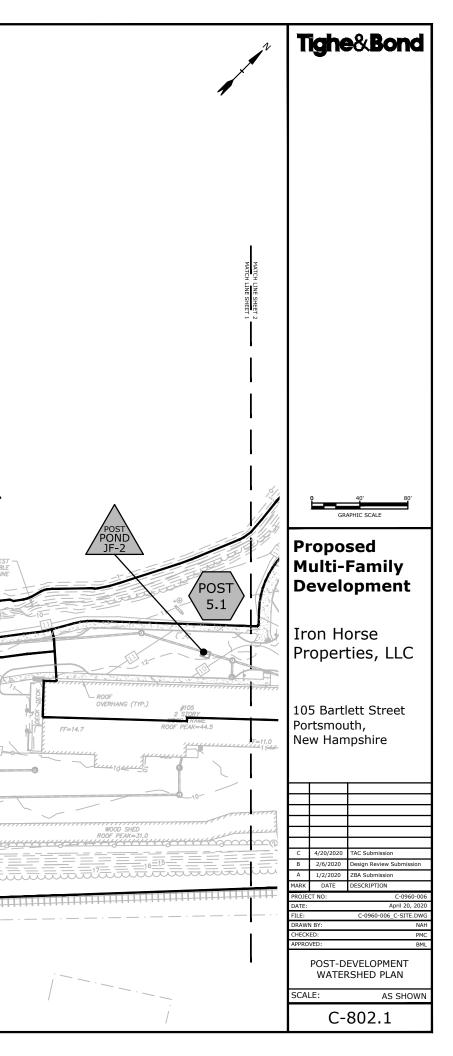
PRE DEVELOPMENT WATERSHED AREA DESIGNATION

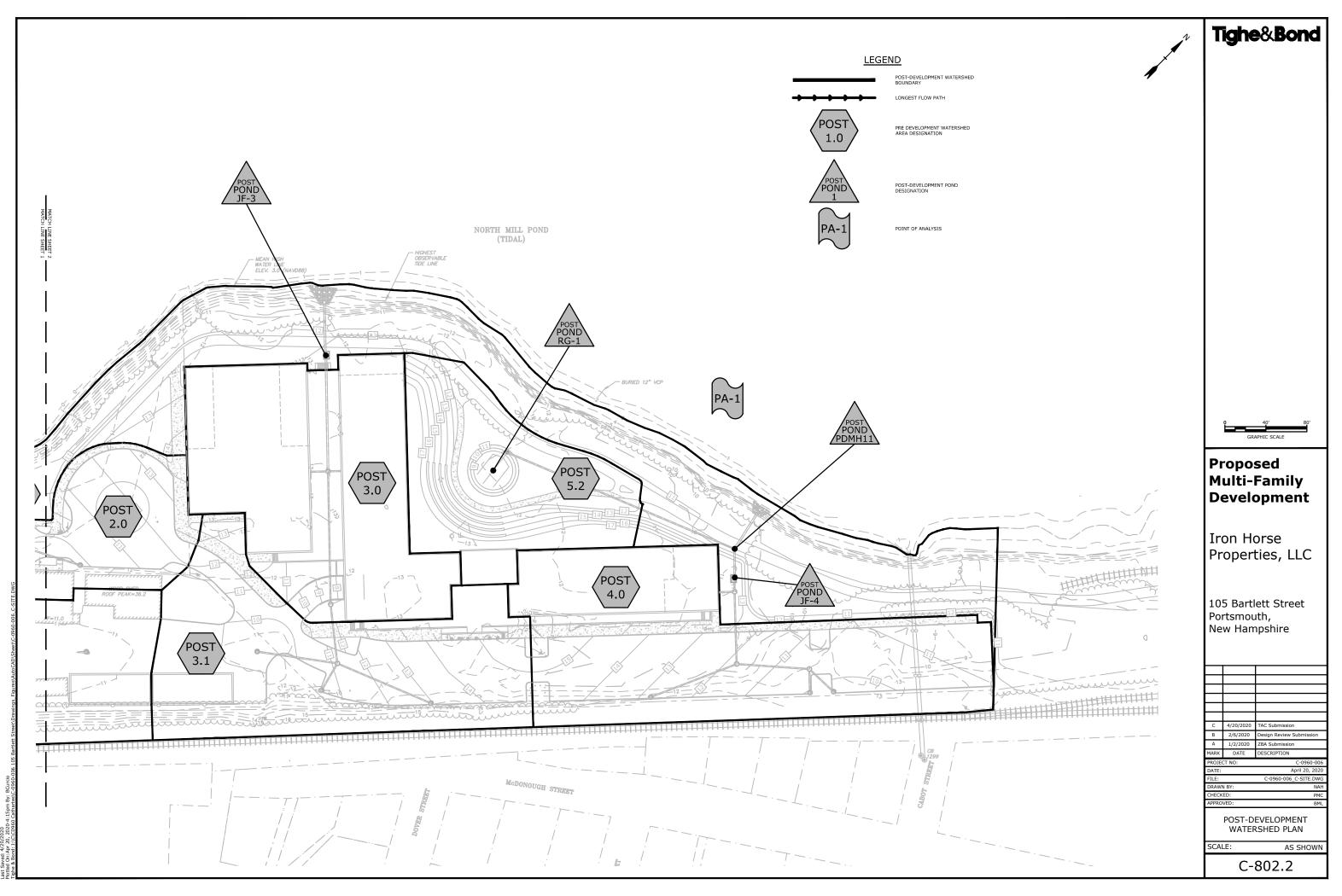


POST-DEVELOPMENT POND DESIGNATION

POINT OF ANALYSIS







₩ 5

Section 4 Stormwater Treatment

The stormwater management system has been designed to provide stormwater treatment as required by the City of Portsmouth Site Review Regulations and NHDES AoT Regulations (Env-Wq 1500).

4.4.1 Pre-Treatment Methods for Protecting Water Quality

Pre-treatment for the stormwater filtration systems consist of deep sump catch basins.

4.4.2 Treatment Methods for Protecting Water Quality.

The runoff from existing and proposed impervious areas will be treated by various Contech Jellyfish stormwater filtration systems. These Jellyfish systems are sized to treat the Water Quality Flows of their respective subcatchment areas. Each system is outfitted with an internal bypass that diverts peak flows away from treatment. The BMP worksheet for these treatment practices have been included in Section 5 of this report.

Additionally, a rain garden within the proposed greenway park is included to treat runoff from the surrounding area. The rain garden has been designed and sized to contain the 50-year storm without overtopping, as well as treat a volume of runoff greater than the WQV. Due to poor infiltration rates of the surrounding soils, the bottom of the rain garden is proposed to be lined and outfitted with underdrains to convey treated runoff to the system's outlet structure. The BMP worksheet for this treatment practice has been included in Section 5 of this report, as well.

Section 5 BMP Worksheets and Sizing Memos



This worksheet may be useful when designing a BMP <u>that does not fit into one of the specific worksheets</u> <u>already provided (i.e. for a technology which is not a stormwater wetland, infiltration practice, etc.)</u>

Water Quality Volume (WQV)

| 4.26 ac | A = Area draining to the practice |
|---------------|---|
| 2.96 ac | A_{I} = Impervious area draining to the practice |
| 0.69 decimal | I = percent impervious area draining to the practice, in decimal form |
| 0.68 unitless | Rv = Runoff coefficient = 0.05 + (0.9 x I) |
| 2.88 ac-in | WQV= 1" x Rv x A |
| 10,444 cf | WQV conversion (ac-in x 43,560 sf/ac x 1ft/12") |

Water Quality Flow (WQF)

| 1 | • 1 | |
|-------|-------------------------|--|
| 1 | inches | P = amount of rainfall. For WQF in NH, $P = 1$ ". |
| 0.68 | inches | Q = water quality depth. $Q = WQV/A$ |
| 97 | unitless | CN = unit peak discharge curve number. CN = 1000/(10+5P+10Q-10*[Q2 + 1.25*Q*P]0.5) |
| 0.4 | inches | S = potential maximum retention. S = $(1000/CN)$ - 10 |
| 0.070 | inches | Ia = initial abstraction. Ia = $0.2S$ |
| 5.0 | minutes | $T_c = Time of Concentration$ |
| 655.0 | cfs/mi ² /in | qu is the unit peak discharge. Obtain this value from TR-55 exhibits 4-II and 4-III |
| 2.944 | cfs | WQF = $q_u x$ WQV. Conversion: to convert "cfs/mi ² /in * ac-in" to "cfs" multiply by $1mi^2/640ac$ |

Designer's Notes: POST 1.0 & 5.0 combined

PEAK FLOW = 30.26 CFS (50 year)



| CONTECH Stormwater Solutions Inc. Engineer Date Prepared: | JBS 4/17/2020 |
|--|---|
| Site Information | |
| Project Name Project State Project City | 105 Bartlett Street (Post 1&5) NH Portsmouth |
| Total Drainage Area, Ad Post Development Impervious Area, Ai Pervious Area, Ap % Impervious Runoff Coefficient, Rc | 4.26 ac 2.96 ac 1.30 ac 69% 0.68 |
| Mass Loading Calculations Mean Annual Rainfall, P Agency Required % Removal Percent Runoff Capture Mean Annual Runoff, Vt Event Mean Concentration of Pollutant, EMC Annual Mass Load, M total | 51 in 80% 90% 479357 ft ³ 75 mg/l 2243.06 lbs |
| Filter System Filtration Brand Cartridge Length | Jelly Fish 54 in |
| Jelly Fish Sizing Mass to be Captured by System Water Quality Flow | 1794.45 lbs 2.94 cfs |
| Method to Use | FLOW BASED |

| Summary | | |
|---------|---------------------|---------------|
| | Treatment Flow Rate | 3.03 cfs |
| Flow | Required Size | JFPD0811-15-4 |



This worksheet may be useful when designing a BMP <u>that does not fit into one of the specific worksheets</u> <u>already provided (i.e. for a technology which is not a stormwater wetland, infiltration practice, etc.)</u>

Water Quality Volume (WQV)

| 0.73 ac | A = Area draining to the practice |
|---------------|---|
| 0.63 ac | A_{I} = Impervious area draining to the practice |
| 0.86 decimal | I = percent impervious area draining to the practice, in decimal form |
| 0.82 unitless | Rv = Runoff coefficient = 0.05 + (0.9 x I) |
| 0.60 ac-in | WQV= 1" x Rv x A |
| 2,177 cf | WQV conversion (ac-in x 43,560 sf/ac x 1ft/12") |

Water Quality Flow (WQF)

| 1 | inches | P = amount of rainfall. For WQF in NH, $P = 1$ ". |
|-------|-------------------------|--|
| 0.82 | inches | Q = water quality depth. $Q = WQV/A$ |
| 98 | unitless | CN = unit peak discharge curve number. CN = 1000/(10+5P+10Q-10*[Q2 + 1.25*Q*P]0.5) |
| 0.2 | inches | S = potential maximum retention. S = $(1000/CN)$ - 10 |
| 0.034 | inches | Ia = initial abstraction. Ia = $0.2S$ |
| 5.0 | minutes | $T_c = Time of Concentration$ |
| 655.0 | cfs/mi ² /in | qu is the unit peak discharge. Obtain this value from TR-55 exhibits 4-II and 4-III |
| 0.614 | cfs | WQF = $q_u x$ WQV. Conversion: to convert "cfs/mi ² /in * ac-in" to "cfs" multiply by $1 mi^2/640 ac$ |

Designer's Notes: POST 2.0

PEAK FLOW = 5.11 CFS



| CONTECH Stormwater Solutions Inc. Enginee Date Prepared: | JBS 3/17/2020 |
|---|-------------------------------|
| Site Information | |
| Project Name | 105 Bartlett Street (Post 2) |
| Project State | NH |
| Project City | Portsmouth |
| Total Drainage Area, Ad | 0.73 ac |
| Post Development Impervious Area, Ai | 0.63 ac |
| Pervious Area, Ap | 0.10 ac |
| % Impervious | 86% |
| Runoff Coefficient, Rc | 0.83 |
| Mass Loading Calculations | |
| Mean Annual Rainfall, P | <mark>51</mark> in |
| Agency Required % Removal | 80% |
| Percent Runoff Capture | 90% |
| Mean Annual Runoff, Vt | 100553 ft ³ |
| Event Mean Concentration of Pollutant, EMC | 75 mg/l |
| Annual Mass Load, M total | 470.52 lbs |
| Filter System | |
| Filtration Brand | Jelly Fish |
| Cartridge Length | 54 in |
| Jelly Fish Sizing | |
| Mass to be Captured by System | 376.42 lbs |
| Water Quality Flow | 0.61 cfs |
| Method to Use | FLOW BASED |

| Summary | | |
|---------|---------------------|--------------|
| | Treatment Flow Rate | 0.62 cfs |
| Flow | Required Size | JFPD0806-3-1 |



This worksheet may be useful when designing a BMP <u>that does not fit into one of the specific worksheets</u> <u>already provided (i.e. for a technology which is not a stormwater wetland, infiltration practice, etc.)</u>

Water Quality Volume (WQV)

| 2.26 ac | A = Area draining to the practice | |
|---------------|---|--|
| 1.79 ac | A_{I} = Impervious area draining to the practice | |
| 0.79 decimal | I = percent impervious area draining to the practice, in decimal form | |
| 0.76 unitless | Rv = Runoff coefficient = 0.05 + (0.9 x I) | |
| 1.72 ac-in | WQV= 1" x Rv x A | |
| 6,258 cf | 6,258 cf WQV conversion (ac-in x 43,560 sf/ac x 1ft/12") | |

Water Quality Flow (WQF)

| | , i i i i i i i i i i i i i i i i i i i | |
|-------|---|--|
| 1 | inches | P = amount of rainfall. For WQF in NH, $P = 1$ ". |
| 0.76 | inches | Q = water quality depth. $Q = WQV/A$ |
| 98 | unitless | CN = unit peak discharge curve number. CN = 1000/(10+5P+10Q-10*[Q2 + 1.25*Q*P]0.5) |
| 0.2 | inches | S = potential maximum retention. S = $(1000/CN)$ - 10 |
| 0.047 | inches | Ia = initial abstraction. Ia = $0.2S$ |
| 5.0 | minutes | $T_c = Time of Concentration$ |
| 655.0 | cfs/mi ² /in | qu is the unit peak discharge. Obtain this value from TR-55 exhibits 4-II and 4-III |
| 1.764 | cfs | WQF = $q_u x$ WQV. Conversion: to convert "cfs/mi ² /in * ac-in" to "cfs" multiply by $1mi^2/640ac$ |

Designer's Notes: POST 3.0, POST 3.1

PEAK FLOW =15.82 CFS



| CONTECH Stormwater Solutions Inc. Engineer Date Prepared: | JBS 3/17/2020 |
|--|--|
| Site Information | |
| Project Name Project State Project City | 105 Bartlett Street (Post 3) NH Portsmouth |
| Total Drainage Area, Ad | 2.26 ac |
| Post Development Impervious Area, Ai | 1.79 ac |
| Pervious Area, Ap | 0.47 ac |
| % Impervious | 79% |
| Runoff Coefficient, Rc | 0.76 |
| Mass Loading Calculations | |
| Mean Annual Rainfall, P | <mark>51</mark> in |
| Agency Required % Removal | 80% |
| Percent Runoff Capture | 90% |
| Mean Annual Runoff, Vt | 287248 ft ³ |
| Event Mean Concentration of Pollutant, EMC | <mark>75</mark> mg/l |
| Annual Mass Load, M total | 1344.12 lbs |
| Filter System | |
| Filtration Brand | Jelly Fish |
| Cartridge Length | 54 in |
| Jelly Fish Sizing | |
| Mass to be Captured by System | 1075.30 lbs |
| Water Quality Flow | 1.76 cfs |
| Method to Use | FLOW BASED |

| | | Summary |
|------|---------------------|--------------|
| Flow | Treatment Flow Rate | 1.78 cfs |
| | Required Size | JFPD0806-9-2 |



This worksheet may be useful when designing a BMP <u>that does not fit into one of the specific worksheets</u> <u>already provided (i.e. for a technology which is not a stormwater wetland, infiltration practice, etc.)</u>

Water Quality Volume (WQV)

| 1.30 ac | A = Area draining to the practice | |
|---------------|---|--|
| 1.04 ac | 1.04 ac $A_I =$ Impervious area draining to the practice | |
| 0.80 decimal | I = percent impervious area draining to the practice, in decimal form | |
| 0.77 unitless | Rv = Runoff coefficient = 0.05 + (0.9 x I) | |
| 1.00 ac-in | WQV=1" x Rv x A | |
| 3,621 cf | 3,621 cf WQV conversion (ac-in x 43,560 sf/ac x 1ft/12") | |

Water Quality Flow (WQF)

| 1 | · 1 | Designed of the Call Designed Designe |
|----------|-------------------------|--|
| <u> </u> | inches | P = amount of rainfall. For WQF in NH, $P = 1$ ". |
| 0.77 | inches | Q = water quality depth. $Q = WQV/A$ |
| 98 | unitless | CN = unit peak discharge curve number. CN = 1000/(10+5P+10Q-10*[Q2 + 1.25*Q*P]0.5) |
| 0.2 | inches | S = potential maximum retention. S = $(1000/CN)$ - 10 |
| 0.046 | inches | Ia = initial abstraction. Ia = $0.2S$ |
| 5.0 | minutes | $T_c = Time of Concentration$ |
| 655.0 | cfs/mi ² /in | qu is the unit peak discharge. Obtain this value from TR-55 exhibits 4-II and 4-III |
| 1.021 | cfs | WQF = $q_u x$ WQV. Conversion: to convert "cfs/mi ² /in * ac-in" to "cfs" multiply by $1 mi^2/640 ac$ |

Designer's Notes: POST 4.0

PEAK FLOW = 9.16 CFS



| CONTECH Stormwater Solutions Inc. Engineer Date Prepared: | JBS 3/17/2020 |
|--|--|
| Site Information | |
| Project Name Project State Project City | 105 Bartlett Street (Post 4) NH Portsmouth |
| Total Drainage Area, Ad | 1.30 ac |
| Post Development Impervious Area, Ai | 1.04 ac |
| Pervious Area, Ap | 0.26 ac |
| % Impervious | 80% |
| Runoff Coefficient, Rc | 0.77 |
| Mass Loading Calculations | |
| Mean Annual Rainfall, P | 51 in |
| Agency Required % Removal | 80% |
| Percent Runoff Capture | 90% |
| Mean Annual Runoff, Vt | 166784 ft ³ |
| Event Mean Concentration of Pollutant, EMC | <mark>75</mark> mg/l |
| Annual Mass Load, M total | 780.43 lbs |
| Filter System | |
| Filtration Brand | Jelly Fish |
| Cartridge Length | <mark>54</mark> in |
| Jelly Fish Sizing | |
| Mass to be Captured by System | 624.35 lbs |
| Water Quality Flow | 1.02 cfs |
| Method to Use | FLOW BASED |

| | | Summary |
|------|---------------------|--------------|
| Flow | Treatment Flow Rate | 1.07 cfs |
| | Required Size | JFPD0806-5-2 |



Type/Node Name:

FILTRATION PRACTICE DESIGN CRITERIA (Env-Wq 1508.07)

RG-1

Enter the type of filtration practice (e.g., bioretention system) and the node name in the drainage analysis, if applicable

| Yes | Have you reviewed the restrictions on unlined systems outlined in Env-W | V_{a} 1508 07(a)? |
|------------------------|---|-----------------------------|
| 0.73 ac | A = Area draining to the practice | (q 1508.07(a): |
| | | |
| 0.14 ac | A_{I} = Impervious area draining to the practice | |
| 0.19 decimal | I = percent impervious area draining to the practice, in decimal form $I = P_{1}$ | |
| 0.22 unitless | Rv = Runoff coefficient = 0.05 + (0.9 x I) | |
| 0.16 ac-in | WQV=1" x Rv x A | |
| <u>590</u> cf | WQV conversion (ac-in x 43,560 sf/ac x 1ft/12") | |
| <u>147</u> cf | 25% x WQV (check calc for sediment forebay volume) | |
| 442 cf | 75% x WQV (check calc for surface sand filter volume) | |
| Clean | Method of Pretreatment? (not required for clean or roof runoff) | |
| cf | V_{SED} = sediment forebay volume, if used for pretreatment | $\leftarrow \geq 25\% WQV$ |
| 1,134 sf | A_{SA} = surface area of the practice | |
| - iph | $K_{sat_{DESIGN}} = design infiltration rate^{1}$ | |
| Yes Yes/No | If Ksat (prior to factor of safety) is < 0.50 iph, has an underdrain been | provided? |
| - hours | $T_{DRAIN} = drain time = V / (A_{SA} * I_{DESIGN})$ | ← <u><</u> 72-hrs |
| 7.50 feet | E_{FC} = elevation of the bottom of the filter course material ² | |
| 6.25 feet | E_{UD} = invert elevation of the underdrain (UD), if applicable | |
| 17.00 feet | E_{SHWT} = elevation of SHWT (if none found, enter the lowest elevation | n of the test pit) |
| feet | E_{ROCK} = elevation of bedrock (if none found, enter the lowest elevation | n of the test pit) |
| 1.25 feet | $D_{FC to UD}$ = depth to UD from the bottom of the filter course | ← ≥ 1' |
| 7.50 feet | $D_{FC \text{ to } ROCK}$ = depth to bedrock from the bottom of the filter course | ← ≥ 1' |
| (9.50) feet | $D_{FC to SHWT}$ = depth to SHWT from the bottom of the filter course | ← ≥ 1' |
| 10.65 ft | Peak elevation of the 50-year storm event (infiltration can be used in a | analysis) |
| 11.00 ft | Elevation of the top of the practice | , , |
| YES | 50 peak elevation \leq Elevation of the top of the practice | ← yes |
| If a surface sand filt | er or underground sand filter is proposed: | |
| YES ac | Drainage Area check. | ← < 10 ac |
| cf | $V = volume of storage^{3}$ (attach a stage-storage table) | $\leftarrow \geq 75\%$ WQV |
| | | ← 18", or 24" if |
| inches | D_{FC} = filter course thickness | within GPA |
| Sheet | Note what sheet in the plan set contains the filter course specification | |
| Yes/No | Access grate provided? | ← yes |
| | | ÷ |

| If a bioretention area | is proposed: | |
|------------------------|--|-----------------------------|
| YES ac | Drainage Area no larger than 5 ac? | ← yes |
| 1,892 cf | V = volume of storage ³ (attach a stage-storage table) | $\leftarrow \geq WQV$ |
| 18.0 inches | D_{FC} = filter course thickness | ← 18", or 24" if within GPA |
| Sheet C-506 | Note what sheet in the plan set contains the filter course specification | |
| 3.0 :1 | Pond side slopes | ← <u>>3</u> :1 |
| Sheet | Note what sheet in the plan set contains the planting plans and surface | cover |
| If porous pavement is | s proposed: | |
| | Type of pavement proposed (concrete? Asphalt? Pavers? Etc) | |
| acres | A_{SA} = surface area of the pervious pavement | |
| :1 | ratio of the contributing area to the pervious surface area | ← ≤ 5:1 |
| inches | D_{FC} = filter course thickness | ← 12", or 18" if within GPA |
| Sheet | Note what sheet in the plan set contains the filter course spec. | ← 304.1 sand |

1. Rate of the limiting layer (either the filter course or the underlying soil). Ksat_{design} includes factor of safey. See Env-Wq 1504.14 for guidance on determining the infiltration rate.

2. See lines 34, 40 and 48 for required depths of filter media.

3. Volume without depending on infiltration. The volume includes the storage above the filter (but below the invert of the outlet stucture, if any), the filter media voids, and the pretreatment area. The storage above the filter media shall not include the volume above the outlet structure, if any.

Designer's Notes:

2019

Section 6 Long-Term Operation & Maintenance Plan

It is the intent of this Operation and Maintenance Plan to identify the areas of this site that need special attention and consideration, as well as implementing a plan to assure routine maintenance. By identifying the areas of concern as well as implementing a frequent and routine maintenance schedule the site will maintain a high-quality stormwater runoff.

6.1 Contact/Responsible Party

Iron Horse Properties, LLC 105 Bartlett Street Portsmouth, NH 03801

(Note: The contact information for the Contact/Responsible Party shall be kept current. If ownership changes, the Operation and Maintenance Plan must be transferred to the new party.)

6.2 Maintenance Items

Maintenance of the following items shall be recorded:

- Litter/Debris Removal
- Landscaping
- Catchbasin Cleaning
- Pavement Sweeping
- Contech Jellyfish Filtration System

The following maintenance items and schedule represent the minimum action required. Periodic site inspections shall be conducted, and all measures must be maintained in effective operating condition. The following items shall be observed during site inspection and maintenance:

- Inspect vegetated areas, particularly slopes and embankments for areas of erosion. Replant and restore as necessary
- Inspect catch basins for sediment buildup
- Inspect site for trash and debris

6.3 Overall Site Operation & Maintenance Schedule

| Overall Site Operation and Maintenance Schedule | | | | | |
|---|--|------------------------|--|--|--|
| Maintenance Item | Frequency of Maintenance | Operation | | | |
| Litter/Debris Removal - Trash and debris to be removed including long the full length of the stream. | Weekly | Management Company | | | |
| Pavement Sweeping - Sweep impervious areas to remove sand and litter. | Annually | Parking Lot Sweeper | | | |
| Catch Basin (CB) Cleaning - CB to be cleaned of solids and oils. | Annually | Vacuum Truck | | | |
| Landscaping - Landscaped islands to be maintained and mulched. | Maintained as required and mulched each Spring | Management Company | | | |

| Rain Garden Inspection/Maintenance Requirements | | | | | |
|--|--|--|--|--|--|
| Inspection/ Maintenance | Frequency | Action | | | |
| Monitor to ensure that Rain Gardens function effectively after storms | Two (2) times annually and after any rainfall event exceeding 2.5" in a 24-hr period | Trash and debris to be removed Any required maintenance shall be addressed | | | |
| Inspect Vegetation | Annually | Inspect the condition of all Rain Garden vegetation Prune back overgrowth Replace dead vegetation Remove any invasive species | | | |
| Inspect Drawdown Time - The system shall drawdown within 48- hours following a rainfall event. | Annually | - Assess the condition of the facility to determine measures required to restore the filtration function, including but not limited to removal of accumulated sediments or reconstruction of the filter. | | | |

| Contech Jellyfish Filter System Inspection/Maintenance Requirements | | | | | |
|--|---|--|--|--|--|
| Inspection/ | Frequency | Action | | | |
| Maintenance | | | | | |
| Inspect vault for sediment build up, static water, plugged media and bypass condition | One (1) time annually and after any rainfall event exceeding 2.5" in a 24-hr period | Maintenance required for any of the following: >4" of sediment on the vault floor >1/4" of sediment on top of the cartridge .4" of static water above the cartridge bottom more than 24 hours after a rain event If pore space between media is absent. If vault is in bypass condition during an average rainfall event. | | | |
| Replace Cartridges | As required by inspection, 1–5 years. | Remove filter cartridges per manufacturer methods. Vacuum sediment from vault. Install new cartridges per manufacturer methods | | | |

6.3.1 Disposal Requirements

Disposal of debris, trash, sediment and other waste material should be done at suitable disposal/recycling sites and in compliance with all applicable local, state and federal waste regulations.

6.3.2 Snow & Ice Management for Standard Asphalt and Walkways

Snow storage areas shall be located such that no direct untreated discharges are possible to receiving waters from the storage site (snow storage areas have been shown on the Site Plan). The property manager will be responsible for timely snow removal from all private sidewalks, driveways, and parking areas. Snow removal will be hauled off-site and legally disposed of when snowbanks exceed 6 feet in height. Salt storage areas shall be covered or located such that no direct untreated discharges are possible to receiving waters from the storage site. Salt and sand shall be used to the minimum extent practical (refer to the attached for de-icing application rate guideline from the New Hampshire Stormwater Management Manual, Volume 2,).

6.4 Chloride Management Plan

Winter Operational Guidelines

The following Chloride Management Plan is for the 105 Bartlett Street, Multi-Family, Mixed Use Development in Portsmouth, New Hampshire. The Plan includes operational guidelines including: winter operator certification requirements, weather monitoring, equipment calibration requirements, mechanical removal, and salt usage evaluation and monitoring. Due to the evolving nature of chloride management efforts, the Chlorides Management Plan will be reviewed annually, in advance of the winter season, to reflect the current management standards.

6.4.1 Background Information

The 105 Bartlett Street, Multi-Family, Mixed Use Development is located along the North Mill Pond in Portsmouth, New Hampshire.

6.4.2 Operational Guidelines – Chloride Management

All private contractors engaged at the development site for the purposes of winter operational snow removal and surface maintenance, are responsible for assisting in meeting compliance for the following protocols. Private contractors are expected to minimize the effects of the use of de-icing, anti-icing and pretreatment materials by adhering to the strict guidelines outlined below.

The winter operational de-icing, anti-icing and pretreatment materials will adhere to the following protocols:

6.4.2.1 Winter Operator Certification Requirements

All private contractors engaged at the premises for the purpose of winter operational snow removal and surface maintenance must be current UNHT2 Green SnowPro Certified operators or equivalent and will use only pre-approved methods for spreading abrasives on private roadways and parking lots. All private contractors engaged at the premises for the purpose of winter operational snow removal and surface maintenance shall provide to the property management two copies of the annual UNHT2 Green SnowPro certificate or equivalent for each operator utilized on the premises. The annual UNHT2 Green SnowPro certificate or equivalent for each operator will be available on file in the Facilities Management office and be present in the vehicle/carrier at all times.

6.4.2.2 Improved Weather Monitoring

The property manager will coordinate weather information for use by winter maintenance contractors. This information in conjunction with site specific air/ground surface temperature monitoring will ensure that private contractors engaged at the premises for the purpose of winter operational snow removal and surface maintenance will make more informed decisions as to when and to what extent de-icing, anti-icing and pretreatment materials are applied to private roadways, sidewalks, and parking lots.

6.4.2.3 Equipment Calibration Requirements

All equipment utilized on the premises for the purpose of winter operational snow removal and surface maintenance will conform to the following calibration requirements.

6.4.2.3.1 Annual Calibration Requirements

All private contractors engaged at the premises for the purpose of winter

operational snow removal and surface maintenance shall provide two copies of the annual calibration report for each piece of equipment utilized on the premises. Each calibration report shall include the vehicle/carrier VIN number and the serial numbers for each component including, but not limited to, spreader control units, salt aggregate spreader equipment, brining/prewetting equipment, ground speed orientation unit, and air/ground surface temperature monitor. Annual calibration reports will be available on file in the Facilities Management office and be present in the vehicle/carrier at all times.

Prior to each use, each vehicle/carrier operator will perform a systems check to verify that unit settings remain within the guidelines established by the Management Team in order to accurately dispense material. All private contractors engaged at the premises for the purpose of winter operational snow removal and surface maintenance will be subject to spot inspections by members of the Property Management Team to ensure that each vehicle/carrier is operating in a manner consistent with the guidelines set herein or State and Municipal regulations. All units will be recalibrated, and the updated calibration reports will be provided each time repairs or maintenance procedures affect the hydraulic system of the vehicle/carrier.

6.4.2.4 Increased Mechanical Removal Capabilities

All private contractors engaged at the premises will endeavor to use mechanical removal means on a more frequent basis for roadways, parking lots and sidewalks. Dedicating more manpower and equipment to increase snow removal frequencies prevents the buildup of snow and the corresponding need for deicing, anti-icing and pretreatment materials. Shortened maintenance routes, with shorter service intervals, will be used to stay ahead of snowfall. Minimized snow and ice packing will reduce the need for abrasives, salt aggregates, and/or brining solution to restore surfaces back to bare surface states after winter precipitation events.

After storm events the management team will be responsible for having the streets swept to recapture un-melted de-icing materials, when practical.

6.4.3 Salt Usage Evaluation and Monitoring

All private contractors engaged at the premises for the purpose of winter operational snow removal and surface maintenance shall provide two copies of a storm report, which includes detailed information regarding treatment areas and the use of de-icing, antiicing and pretreatment materials applied for the removal of snow and surface maintenance on the premises. The property manager will maintain copies of Summary Documents, including copies of the Storm Reports, operator certifications, equipment used for roadway and sidewalk winter maintenance, calibration reports and amount of de-icing materials used.

6.4.4 Summary

The above-described methodologies are incorporated into the Operational Manual and

are to be used to qualify and retain all private contractors engaged at the 105 Bartlett Street premises for the purpose of winter operational snow removal and surface maintenance. This section of the Manual, is intended to be an adaptive management document that is modified as required based on experience gained from past practices and technological advancements that reflect chloride BMP standards. All employees directly involved with winter operational activities are required to review this document and the current standard Best Management Practices published by the UNH Technology Transfer (T2) program annually. All employees directly involved with winter operational activities, and all private contractors engaged at the premises for the purposes of winter operational snow removal and surface maintenance, must be current UNHT2 Green SnowPro Certified operators or equivalent and undergo the necessary requirements to maintain this certification annually.

Deicing Application Rate Guidelines

24' of pavement (typcial two-lane road)

These rates are not fixed values, but rather the middle of a range to be selected and adjusted by an agency according to its local conditions and experience.

| | | | Pounds per two-lane mile | | | |
|---|-----------------------------|---|---|---|--------------------|--|
| Pavement Temp. (°F) and Trend (↑↓) | Weather Condition | Maintenance Actions | Salt Prewetted / Pretreated with Salt Brine | Salt Prewetted / Pretreated with Other Blends | Dry Salt* | Winter Sand (abrasives) |
| >30° ↑ | Snow | Plow, treat intersections only | 80 | 70 | 100* | Not recommended |
| >30" 1 | Freezing Rain | Apply Chemical | 80 - 160 | 70 - 140 | 100 - 200* | Not recommended |
| 30° ↓ | Snow | Plow and apply chemical | 80 - 160 | 70 - 140 | 100 - 200* | Not recommended |
| - UC | Freezing Rain | Apply Chemical | 150 - 200 | 130 - 180 | 180 - 240* | Not recommended |
| 25°-30° ↑ | Snow | Plow and apply chemical | 120 - 160 | 100 - 140 | 150 - 200* | Not recommended |
| 23 - 30 | Freezing Rain | Apply Chemical | 150 - 200 | 130 - 180 | 180 - 240* | Not recommended |
| 25°-30° ↓ | Snow | Plow and apply chemical | 120 - 160 | 100 - 140 | 150 - 200* | Not recommended |
| 23 - 30 4 | Freezing Rain | Apply Chemical | 160 - 240 | 140 - 210 | 200 - 300* | 400 |
| 20°-25°↑ | Snow or Freezing Rain | Plow and apply chemical | 160 - 240 | 140 - 210 | 200 - 300* | 400 |
| 20°-25° ↓ | Snow | Plow and apply chemical | 200 - 280 | 175 - 250 | 250 - 350* | Not recommended |
| 20-25 🗸 | Freezing Rain | Apply Chemical | 240 - 320 | 210 - 280 | 300 - 400* | 400 |
| 15°-20° ↑ | Snow | Plow and apply chemical | 200 - 280 | 175 - 250 | 250 - 350* | Not recommended |
| | Freezing Rain | Apply Chemical | 240 - 320 | 210 - 280 | 300 - 400* | 400 |
| 15°-20° ↓ | Snow or Freezing Rain | Plow and apply chemical | 240 - 320 | 210 - 280 | 300 - 400* | 500 for freezing rain |
| 0°-15° ↑↓ | Snow | Plow, treat with blends, sand hazardous areas | Not recommended | 300 - 400 | Not recommended | 500 - 750 spot treatment as needed |
| < 0° | Snow | Plow, treat with blends, sand hazardous areas | Not recommended | 400 - 600** | Not recommended | 500 - 750 spot treatment as needed |

* Dry salt is not recommended. It is likely to blow off the road before it melts ice.

** A blend of 6 - 8 gal/ton MgCl₂ or CaCl₂ added to NaCl can melt ice as low as -10°.

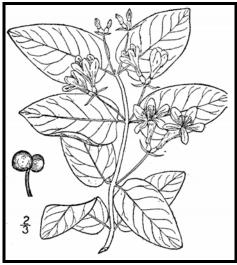
| Anti-icing Route Data Form | | | | | |
|----------------------------|-------------------------|-------------------|-----------|-----|--|
| Truck Station: | | | | | |
| | | | | | |
| Date: | | | | | |
| Air Temperature | Pavement Temperature | Relative Humidity | Dew Point | Sky | |
| Reason for applying: | | | | | |
| Route: | | | | | |
| Chemical: | | | | | |
| Application Time: | | | | | |
| Application Amount: | | | | | |
| Observation (first da | y): | | | | |
| Observation (after ev | vent): | | | | |
| Observation (before | next application): | | | | |
| Name: | | | | | |

6.5 Invasive Species

With respect to a particular ecosystem, any species, including its seeds, eggs, spores, or other biological material capable of propagating that species, that is not native to that ecosystem is classified as an invasive species. Refer to the following fact sheet prepared by the University of New Hampshire Cooperative Extension entitled Methods for Disposing Non-Native Invasive Plants for recommended methods to dispose of invasive plant species.

UNIVERSITY of NEW HAMPSHIRE Methods for Disposing COOPERATIVE EXTENSION Non-Native Invasive Plants

Prepared by the Invasives Species Outreach Group, volunteers interested in helping people control invasive plants. Assistance provided by the Piscataquog Land Conservancy and the NH Invasives Species Committee. Edited by Karen Bennett, Extension Forestry Professor and Specialist.



Tatarian honeysuckleLonicera tataricaUSDA-NRCS PLANTS Database / Britton, N.L., andA. Brown. 1913. An illustrated flora of the northernUnited States, Canada and the British Possessions.Vol. 3: 282.

Non-native invasive plants crowd out natives in natural and managed landscapes. They cost taxpayers billions of dollars each year from lost agricultural and forest crops, decreased biodiversity, impacts to natural resources and the environment, and the cost to control and eradicate them.

Invasive plants grow well even in less than desirable conditions such as sandy soils along roadsides, shaded wooded areas, and in wetlands. In ideal conditions, they grow and spread even faster. There are many ways to remove these nonnative invasives, but once removed, care is needed to dispose the removed plant material so the plants don't grow where disposed.

Knowing how a particular plant reproduces indicates its method of spread and helps determine

the appropriate disposal method. Most are spread by seed and are dispersed by wind, water, animals, or people. Some reproduce by vegetative means from pieces of stems or roots forming new plants. Others spread through both seed and vegetative means.

Because movement and disposal of viable plant parts is restricted (see NH Regulations), viable invasive parts can't be brought to most transfer stations in the state. Check with your transfer station to see if there is an approved, designated area for invasives disposal. This fact sheet gives recommendations for rendering plant parts nonviable.

Control of invasives is beyond the scope of this fact sheet. For information about control visit <u>www.nhinvasives.org</u> or contact your UNH Cooperative Extension office.

New Hampshire Regulations

Prohibited invasive species shall only be disposed of in a manner that renders them nonliving and nonviable. (Agr. 3802.04)

No person shall collect, transport, import, export, move, buy, sell, distribute, propagate or transplant any living and viable portion of any plant species, which includes all of their cultivars and varieties, listed in Table 3800.1 of the New Hampshire prohibited invasive species list. (Agr 3802.01)

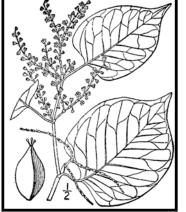
How and When to Dispose of Invasives?

To prevent seed from spreading remove invasive plants before seeds are set (produced). Some plants continue to grow, flower and set seed even after pulling or cutting. Seeds can remain viable in the ground for many years. If the plant has flowers or seeds, place the flowers and seeds in a heavy plastic bag "head first" at the weeding site and transport to the disposal site. The following are general descriptions of disposal methods. See the chart for recommendations by species.

Burning: Large woody branches and trunks can be used as firewood or burned in piles. For outside burning, a written fire permit from the local forest fire warden is required unless the ground is covered in snow. Brush larger than 5 inches in diameter can't be burned. Invasive plants with easily airborne seeds like black swallow-wort with mature seed pods (indicated by their brown color) shouldn't be burned as the seeds may disperse by the hot air created by the fire.

Bagging (solarization): Use this technique with softertissue plants. Use heavy black or clear plastic bags (contractor grade), making sure that no parts of the plants poke through. Allow the bags to sit in the sun for several weeks and on dark pavement for the best effect.

Tarping and Drying: Pile material on a sheet of plastic



Japanese knotweed Polygonum cuspidatum USDA-NRCS PLANTS Database / Britton, N.L., and A. Brown. 1913. An illustrated flora of the northern United States, Canada and the British Possessions. Vol. 1: 676.

and cover with a tarp, fastening the tarp to the ground and monitoring it for escapes. Let the material dry for several weeks, or until it is clearly nonviable.

Chipping: Use this method for woody plants that don't reproduce vegetatively.

Burying: This is risky, but can be done with watchful diligence. Lay thick plastic in a deep pit before placing the cut up plant material in the hole. Place the material away from the edge of the plastic before covering it with more heavy plastic. Eliminate as much air as possible and toss in soil to weight down the material in the pit. Note that the top of the buried material should be at least three feet underground. Japanese knotweed should be at least 5 feet underground!

Drowning: Fill a large barrel with water and place soft-tissue plants in the water. Check after a few weeks and look for rotted plant material (roots, stems, leaves, flowers). Well-rotted plant material may be composted. A word of caution- seeds may still be viable after using this method. Do this before seeds are set. This method isn't used often. Be prepared for an awful stink!

Composting: Invasive plants can take root in compost. Don't compost any invasives unless you know there is no viable (living) plant material left. Use one of the above techniques (bagging, tarping, drying, chipping, or drowning) to render the plants nonviable before composting. Closely examine the plant before composting and avoid composting seeds.

Be diligent looking for seedlings for years in areas where removal and disposal took place.

Suggested Disposal Methods for Non-Native Invasive Plants

This table provides information concerning the disposal of removed invasive plant material. If the infestation is treated with herbicide and left in place, these guidelines don't apply. Don't bring invasives to a local transfer station, unless there is a designated area for their disposal, or they have been rendered non-viable. This listing includes wetland and upland plants from the New Hampshire Prohibited Invasive Species List. The disposal of aquatic plants isn't addressed.

| Woody Plants | Method of Reproducing | Methods of Disposal | | |
|--|-----------------------------------|--|--|--|
| Norway maple (Acer platanoides) European barberry (Berberis vulgaris) Japanese barberry (Berberis thunbergii) autumn olive (Elaeagnus umbellata) burning bush (Euonymus alatus) Morrow's honeysuckle (Lonicera morrowii) Tatarian honeysuckle (Lonicera tatarica) showy bush honeysuckle (Lonicera x bella) common buckthorn (Rhamnus cathartica) glossy buckthorn (Frangula alnus) | Fruit and Seeds | Prior to fruit/seed ripening Seedlings and small plants Pull or cut and leave on site with roots exposed. No special care needed. Larger plants Use as firewood. Make a brush pile. Chip. Burn. After fruit/seed is ripe Don't remove from site. Burn. Make a covered brush pile. Chip once all fruit has dropped from branches. Leave resulting chips on site and monitor. | | |
| oriental bittersweet (<i>Celastrus orbiculatus</i>) multiflora rose (<i>Rosa multiflora</i>) | Fruits, Seeds, Plant Fragments | Prior to fruit/seed ripening Seedlings and small plants Pull or cut and leave on site with roots exposed. No special care needed. Larger plants Make a brush pile. Burn. After fruit/seed is ripe Don't remove from site. Burn. Make a covered brush pile. Chip – only after material has fully dried (1 year) and all fruit has dropped from branches. Leave resulting chips on site and monitor. | | |

| Non-Woody Plants | Method of Reproducing | Methods of Disposal | | |
|---|--|---|--|--|
| <pre>garlic mustard (Alliaria petiolata) spotted knapweed (Centaurea maculosa) • Sap of related knapweed can cause skin irritation and tumors. Wear gloves when handling. black swallow-wort (Cynanchum nigrum) • May cause skin rash. Wear gloves and long sleeves when handling. pale swallow-wort (Cynanchum rossicum) giant hogweed (Heracleum mantegazzianum) • Can cause major skin rash. Wear gloves and long sleeves when handling. dame's rocket (Hesperis matronalis) perennial pepperweed (Lepidium latifolium) purple loosestrife (Lythrum salicaria) Japanese stilt grass (Microstegium vimineum) mile-a-minute weed (Polygonum perfoliatum)</pre> | Fruits and Seeds | Prior to flowering Depends on scale of infestation Small infestation Pull or cut plant and leave on site with roots exposed. Large infestation Pull or cut plant and pile. (You can pile onto or cover with plastic sheeting). Monitor. Remove any re-sprouting material. During and following flowering Do nothing until the following year or remove flowering heads and bag and let rot. Small infestation Pull or cut plant and leave on site with roots exposed. Large infestation Pull or cut plant and pile remaining material. Unity of the plastic or cover with plastic sheeting). Monitor. Remove any re-sprouting material. | | |
| common reed (<i>Phragmites australis</i>) Japanese knotweed (<i>Polygonum cuspidatum</i>) Bohemian knotweed (<i>Polygonum x bohemicum</i>) | Fruits, Seeds, Plant Fragments Primary means of spread in these species is by plant parts. Although all care should be given to preventing the dispersal of seed during control activities, the presence of seed doesn't materially influence disposal activities. | Small infestation Bag all plant material and let rot. Never pile and use resulting material as compost. Burn. Large infestation Remove material to unsuitable habitat (dry, hot and sunny or dry and shaded location) and scatter or pile. Monitor and remove any sprouting material. Pile, let dry, and burn. | | |

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Managing Invasive Plants Methods of Control by Christopher Mattrick

They're out there. The problem of invasive plants is as close as your own backyard.

Maybe a favorite dogwood tree is struggling in the clutches of an Oriental bittersweet vine. Clawlike canes of multiflora rose are scratching at the side of your house. That handsome burning bush you planted few years ago has become a whole clump in practically no time ... but what happened to the azalea that used to grow right next to it?

If you think controlling or managing invasive plants on your property is a daunting task, you're not alone. Though this topic is getting lots of attention from federal, state, and local government agencies, as well as the media, the basic question for most homeowners is simply, "How do I get rid of the invasive plants in my own landscape?" Fortunately, the best place to begin to tackle this complex issue is in our own backyards and on local conservation lands. We hope the information provided here will help you take back your yard. We won't kid you—there's some work involved, but the payoff in beauty, wildlife habitat, and peace of mind makes it all worthwhile.

PLAN OF ATTACK

Three broad categories cover most invasive plant control: mechanical, chemical, and biological. Mechanical control means physically removing plants from the environment



Spraying chemicals to control invasive plants.

through cutting or pulling. Chemical control uses herbicides to kill plants and inhibit regrowth. Techniques and chemicals used will vary depending on the species. Biological controls use plant diseases or insect predators, typically from the targeted species' home range. Several techniques may be effective in controlling a single species, but there is usually one preferred method—the one that is most resource efficient with minimal impact on non-target species and the environment.

MECHANICAL CONTROL METHODS

Mechanical treatments are usually the first ones to look at when evaluating an invasive plant removal project. These procedures do not require special licensing or introduce chemicals into the environment. They do require permits in some situations, such as wetland zones. [See sidebar on page 23.] Mechanical removal is highly labor intensive and creates a significant amount of site disturbance, which can lead to rapid reinvasion if not handled properly.

Pulling and digging

Many herbaceous plants and some woody species (up to about one inch in diameter), if present in limited quantities, can be pulled out or dug up. It's important to remove as much of the root system as possible; even a small portion can restart the infestation. Pull plants by hand or use a digging fork, as shovels can shear off portions of the root

system, allowing for regrowth. To remove larger woody stems (up to about three inches in diameter), use a Weed Wrench[™], Root Jack, or Root Talon. These tools, available from several manufacturers, are designed to remove the aboveground portion of the plant as well as the entire root system. It's easiest to undertake this type of control in the spring or early summer when soils are moist and plants come out more easily.



Using tools to remove woody stems.





Volunteers hand pulling invasive plants.

Suffocation

Try suffocating small seedlings and herbaceous plants. Place double or triple layers of thick UV-stabilized plastic sheeting, either clear or black (personally I like clear), over the infestation and secure the plastic with stakes or weights. Make sure the plastic extends at least five feet past the edge of infestation on all sides. Leave the plastic in place for at least two years. This technique will kill everything beneath the plastic—invasive and non-invasive plants alike. Once the plastic is removed, sow a cover crop such as annual rye to prevent new invasions.

Cutting or mowing

This technique is best suited for locations you can visit and treat often. To be effective, you will need to mow or cut infested areas three or four times a year for up to five years. The goal is to interrupt the plant's ability to photosynthesize by removing as much leafy material as possible. Cut the plants at ground level and remove all resulting debris from the site. With this treatment, the infestation may actually appear to get worse at first, so you will need to be as persistent as the invasive plants themselves. Each time you cut the plants back, the root system gets slightly larger, but must also rely on its energy reserves to push up new growth. Eventually, you will exhaust these reserves and the plants will die. This may take many years, so you have to remain committed to this process once you start; otherwise the treatment can backfire, making the problem worse.

CHEMICAL CONTROL METHODS

Herbicides are among the most effective and resource-efficient tools to treat invasive species. Most of the commonly known invasive plants can be treated using only two herbicides—glyphosate (the active ingredient in Roundup™ and RodeoTM) and triclopyr (the active ingredient in Brush-B-Gone[™] and Garlon[™]). Glyphosate is non-selective, meaning it kills everything it contacts. Triclopyr is selective and does not injure monocots (grasses, orchids, lilies, etc.). Please read labels and follow directions precisely for both environmental and personal safety. These are relatively benign herbicides, but improperly used they can still cause both short- and long-term health and environmental problems. Special aquatic formulations are required when working in wetland zones. You are required to have a stateissued pesticide applicator license when applying these chemicals on land you do not own. To learn more about the pesticide regulations in your state, visit or call your state's pesticide control division, usually part of the state's Department of Agriculture. In wetland areas, additional permits are usually required by the Wetlands Protection Act. [See sidebar on page 23.]

Foliar applications

When problems are on a small scale, this type of treatment is usually applied with a backpack sprayer or even a small handheld spray bottle. It is an excellent way to treat large monocultures of herbaceous plants, or to spot-treat individual plants that are difficult to remove mechanically, such as goutweed, swallowwort, or purple loosestrife. It is also an effective treatment for some woody species, such as Japanese barberry, multiflora rose, Japanese honeysuckle, and Oriental bittersweet that grow in dense masses or large numbers over many acres. The herbicide mixture should contain no more than five percent of the active ingredient, but it is important to follow the instructions on the product label. This treatment is most effective when the plants are actively growing, ideally when they are flowering or beginning to form fruit. It has been shown that plants are often more susceptible to this type of treatment if the existing stems are cut off and the regrowth is treated. This is especially true for Japanese knotweed. The target plants should be thoroughly wetted with the herbicide on a day when there is no rain in the forecast for the next 24 to 48 hours.

Cut stem treatments

There are several different types of cut stem treatments, but here we will review only the one most commonly used. All treatments of this type require a higher concentration of the active ingredient than is used in foliar applications. A 25 to 35 percent solution of the active ingredient should be used for cut stem treatments, but read and follow all label instructions. In most cases, the appropriate herbicide is glyphosate, except for Oriental bittersweet, on which triclopyr should be used. This treatment can be used on all woody stems, as well as phragmites and Japanese knotweed.

For woody stems, treatments are most effective when applied in the late summer and autumn—between late August and November. Stems should be cut close to the ground, but not so close that you will lose track of them. Apply herbicide directly to the cut surface as soon as possible after cutting. Delaying the application will reduce the effectiveness of the treatment. The herbicide can be applied with a sponge, paintbrush, or spray bottle.



For phragmites and Japanese knotweed, treatment is the same, but the timing and equipment are different. Plants should be treated anytime from mid-July through September, but the hottest, most humid days of the summer are best

Cut stem treatment tools.

for this method. Cut the stems halfway between two leaf nodes at a comfortable height. Inject (or squirt) herbicide into the exposed hollow stem. All stems in an infestation should be treated. A wash bottle is the most effective application tool, but you can also use an eyedropper, spray bottle, or one of the recently developed high-tech injection systems.

It is helpful to mix a dye in with the herbicide solution. The dye will stain the treated surface and mark the areas that have been treated, preventing unnecessary reapplication. You can buy a specially formulated herbicide dye, or use food coloring or laundry dye.

There is not enough space in this article to describe all the possible ways to control invasive plants. You can find other treatments, along with more details on the above-described methods, and species-specific recommendations on The Nature Conservancy Web site (tncweeds.ucdavis.edu). An upcoming posting on the Invasive Plant Atlas of New England (www.ipane.org) and the New England Wild Flower Society (www.newfs.org) Web sites will also provide further details.



Hollow stem injection tools.

Biological controls-still on the horizon

Biological controls are moving into the forefront of control methodology, but currently the only widely available and applied biocontrol relates to purple loosestrife. More information on purple loosestrife and other biological control projects can be found at www.invasiveplants.net.

DISPOSAL OF INVASIVE PLANTS

Proper disposal of removed invasive plant material is critical to the control process. Leftover plant material can cause new infestations or reinfest the existing project area. There are many appropriate ways to dispose of invasive plant debris. I've listed them here in order of preference.

- **1. Burn it**—Make a brush pile and burn the material following local safety regulations and restrictions, or haul it to your town's landfill and place it in their burn pile.
- **2. Pile it**—Make a pile of the woody debris. This technique will provide shelter for wildlife as well.
- **3.** Compost it—Place all your herbaceous invasive plant debris in a pile and process as compost. Watch the pile closely for resprouts and remove as necessary. Do not use the resulting compost in your garden. The pile is for invasive plants only.



Injecting herbicide into the hollow stem of phragmites.

4. Dry it/cook it—Place woody debris out on your driveway or any asphalt surface and let it dry out for a month. Place herbaceous material in a doubled-up black trash bag and let it cook in the sun for one month. At the end of the month, the material should be non-viable and you can dump it or dispose of it with the trash. The method assumes there is no viable seed mixed in with the removed material.

Care should be taken in the disposal of all invasive plants, but several species need extra attention. These are the ones that have the ability to sprout vigorously from plant fragments and should ideally be burned or dried prior to disposal: Oriental bittersweet, multiflora rose, Japanese honeysuckle, phragmites, and Japanese knotweed. Christopher Mattrick is the former Senior Conservation Programs Manager for New England Wild Flower Society, where he managed conservation volunteer and invasive and rare plant management programs. Today, Chris and his family work and play in the White Mountains of New Hampshire, where he is the Forest Botanist and Invasive Species Coordinator for the White Mountain National Forest.



Controlling Invasive Plants in Wetlands

Special concerns; special precautions

Control of invasive plants in or around wetlands or bodies of water requires a unique set of considerations. Removal projects in wetland zones can be legal and effective if handled appropriately. In many cases, herbicides may be the least disruptive tools with which to remove invasive plants. You will need a state-issued pesticide license to apply herbicide on someone else's property, but all projects in wetland or aquatic systems fall under the jurisdiction of the Wetlands Protection Act and therefore require a permit. *Yes, even hand-pulling that colony of glossy buckthorn plants from your own swampland requires a permit.* Getting a permit for legal removal is fairly painless if you plan your project carefully.

1. Investigate and understand the required permits and learn how to obtain them. The entity charged with the enforcement of the Wetlands Protection Act varies from state to state. For more information in your state, contact:

ME: Department of Environmental Protection www.state.me.us/dep/blwq/docstand/nrpapage.htm

NH: Department of Environmental Services www.des.state.nh.us/wetlands/

VT: Department of Environmental Conservation www.anr.state.vt.us/dec/waterq/permits/htm/ pm_cud.htm

MA: Consult your local town conservation commission

RI: Department of Environmental Management www.dem.ri.gov/programs/benviron/water/ permits/fresh/index.htm

CT: Consult your local town Inland Wetland and Conservation Commission

- 2. Consult an individual or organization with experience in this area. Firsthand experience in conducting projects in wetland zones and navigating the permitting process is priceless. Most states have wetland scientist societies whose members are experienced in working in wetlands and navigating the regulations affecting them. A simple Web search will reveal the contact point for these societies. Additionally, most environmental consulting firms and some nonprofit organizations have skills in this area.
- **3.** Develop a well-written and thorough project plan. You are more likely to be successful in obtaining a permit for your project if you submit a project plan along with your permit application. The plan should include the reasons for the project, your objectives in completing the project, how you plan to reach those objectives, and how you will monitor the outcome.
- **4.** Ensure that the herbicides you plan to use are approved for aquatic use. Experts consider most herbicides harmful to water quality or aquatic organisms, but rate some formulations as safe for aquatic use. Do the research and select an approved herbicide, and then closely follow the instructions on the label.
- **5.** If you are unsure—research, study, and most of all, ask for help. Follow the rules. The damage caused to aquatic systems by the use of an inappropriate herbicide or the misapplication of an appropriate herbicide not only damages the environment, but also may reduce public support for safe, well-planned projects.

6.6 Annual Updates and Log Requirements

The Owner and/or Contact/Responsible Party shall review this Operation and Maintenance Plan once per year for its effectiveness and adjust the plan and deed as necessary.

A log of all preventative and corrective measures for the stormwater system shall be kept on-site and be made available upon request by any public entity with administrative, health environmental or safety authority over the site including NHDES.

| Stormwater Management Report | | | | | | | |
|------------------------------|-----------------------|------------|--|---|---------------------------------|-----------------|--|
| Project Name | | 105 Bartle | 105 Bartlett Street | | | | |
| BMP Description | Date of Inspection | Inspector | BMP Installed and Operating Properly? | Cleaning / Corrective Action Needed | Date of Cleaning / Repair | Performed By | |
| | | | □Yes □No | | | | |
| | | | □Yes □No | | | | |
| | | | □Yes □No | | | | |
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